

Corrib Causeway Phase 1, Dyke Road





Infrastructure Report

Galway City Council

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1. Introduction

1.1 Project Background

AECOM Ireland Ltd (AECOM) has prepared the Infrastructure Report for the development of the Corrib Causeway Phase 1 lands at Dyke Road, Terryland, Galway City (Refer to **Figure 1-1 & Figure 1-2**). The Dyke Road site is located on the edge of Galway City Centre, Galway.

The Infrastructure Report has been prepared to accompany the planning application for a new residential development on the site.

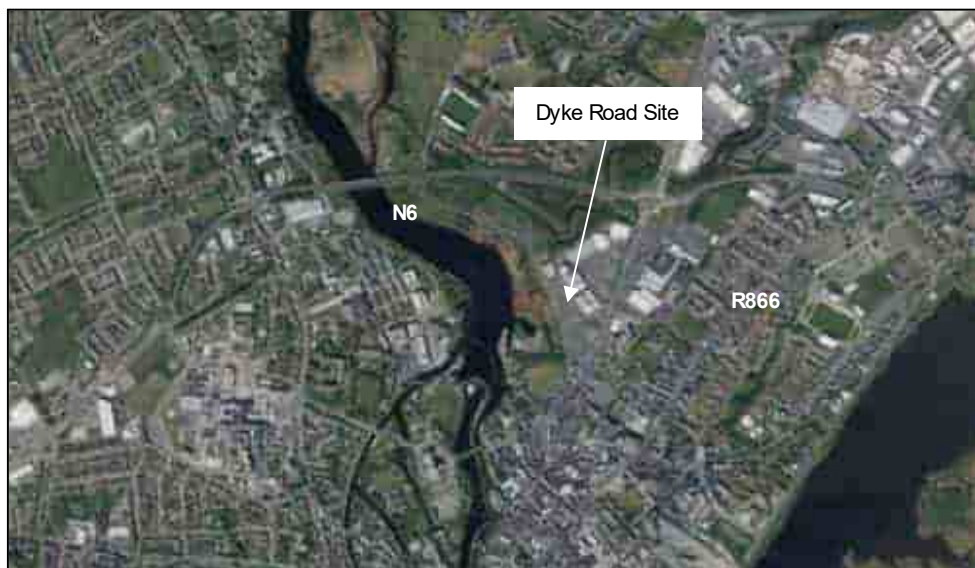


Figure 1-1: Site Location Map



Figure 1-2: Site Location – Phase 1, Tóchar na Coiribe Vision, LDA & MOLA Architecture 2023

The Dyke Road site forms part of a strategic brownfield landbank located on the edge of Galway City Centre which has been identified for comprehensive redevelopment by the Galway Development Plan 2023-2029. The proposed development of the site will be on a phased basis as indicated in **Figure 1-3**. This report pertains to Phase 1 lands only.

The Phase 1 lands are currently used as a public carpark. In the Tóchar na Coiribe Vision the Emerging design proposal explains that “there is also potential to redevelop the south of the car park for civic, commercial, and cultural uses (phase 2). If the Black Box theatre is relocated, there is potential to develop an additional residential block overlooking Terryland Forest Park (Phase 3). In the meantime, the existing uses comprising the Black Box theatre and car parking unaffected by Phase 1 will remain operational as normal, **Figure 1-4**.” (LDA 2024).



Figure 1-3: Project Phasing, Stage 1 Masterplan, MOLA Architecture 2023



Figure 1-4: Emerging Design Proposal, Tóchar na Coiribe Vision, LDA and MOLA 2024

1.2 Existing Development

The existing development consists of a tarmacadam car park which spans over phases 1 & 2 respectively. The most northern part of the existing landholding is home to the Black Box Theatre. The topography of the entire site naturally falls from south to north.

A topographic survey undertaken by Apex Surveys in October 2023 of the overall landholding indicates that ground levels on the site range from 3.84m at the northern end of the site to 7.12m in the southern portion of the site. There is a small retaining wall in the southern portion of the site where the car park levels step up from about 6.0m to around 7.0m. The ground levels on the phase 1 lands typically range from 4.8m to 5.9m with the level in the centre of the site typically being around 5.3m.

It must be noted that the entire site is of hardstanding area. Refer to **Figure 1-5** below.



Figure 1-5: Existing Development

1.3 Proposed Development

The proposed development will consist of the construction of a new residential development of 219 no. apartment units and a childcare facility (approx. 241 sq m) in the form of 1 no. new residential block (5 - 9 storeys over lower ground floor level) with associated car parking, bicycle parking, public and communal open spaces, and all ancillary works on a site area of 1.144 ha.

The proposed development will provide for:

- a) 219 no. residential apartment units (109 no. 1-bedroom units, 100 no. 2-bedroom units and 10 no. 3-bedroom units) each with an associated private open space area in the form of a balcony/terrace.
- b) A raised pedestrian boardwalk along the western elevation of the proposed building.
- c) Open Space (approx. 2,778 sq m) is proposed in the form of (a) public open space (approx. 1,183 sq m) to the west of the proposed building fronting on to Dyke Road accommodating outdoor seating, planting, a sunken garden and pedestrian pathways and connections; and (b) communal open space (approx. 1,605 sq m) to the east of the

proposed building in the form of a courtyard including outdoor seating, planting, a children's play area and outdoor sports equipment.

- d) A childcare facility (approx. 241 sq m) at ground floor level with dedicated external play area (approx. 61 sqm) at surface level.
- e) A total of 33 no. new car parking spaces at surface level to serve the proposed residential development (including 2 no. accessible spaces). In addition, 2 no. set down / drop off spaces are proposed to serve the childcare facility.
- f) A total of 465 no. bicycle parking spaces to include 330 no. standard residential spaces, 100 no. visitor spaces, 25 no. cargo bicycle spaces and 10 no. bicycle parking spaces dedicated for the childcare facility staff, all at surface / lower ground floor level.
- g) Vehicular access to serve the development is proposed via Dyke Road at 2 no. new locations along the western site boundary (to the north west and south west of the main development site). Pedestrian and Cyclist access is also proposed throughout the site via Dyke Road and a new pedestrian crossing is also delivered at Dyke Road. The proposed development will extinguish the existing pedestrian connection between Galway Retail Park and the subject site as part of wider proposals for local improvements to permeability.
- h) The removal of 389 no. existing car parking spaces (311 no. from Car Park 1 and 78 no. from Car Park 2) is proposed to provide for the new development. An overall total of 165 no. existing car parking spaces will be maintained in Car Park 2.
- i) The extinguishment of the main existing vehicular entrance serving Car Park 1 and Car Park 2 at Dyke Road with provision made for a new vehicular access point (to the south of the main development site) to facilitate continued access to existing Car Park 2 and the remaining car parking spaces (165 no.).
- j) The removal of existing bring bank facilities including 2 no. clothing banks and 8 no. bottle banks from Dyke Road.
- k) 2 no. telecommunications lattice towers (overall height 6.45 m and 7.67 m) affixed to the rooftop supporting 9 no. 2m 2G/3G/4G antennas; 9 no. 0.8m 5G antennas; 6 no. 0.3m microwave transmission links; together with all associated telecommunications equipment and cabinets. The proposed overall building height including the telecommunications towers is approx. 38.18 m (+43.18 AOD).

The development will also provide for all associated site development works, infrastructure, excavation and clearance works including decommissioning the existing Black Box Theatre waste water pumping station, provision for a new pumping station complete with below ground emergency storage, all boundary treatment/retaining walls, public lighting, internal roads and pathways, ESB substations, switch rooms, water tank rooms, cleaner store and WC, meter rooms, facilities management office, parcel store, comms rooms, plant room, generator room / associated plant space, bin storage, bicycle stores, hard and soft landscaping, play equipment, below ground attenuation tanks, nature based SUDs features, green roofs, roof plant, new and replacement site services and connections for foul drainage, surface water drainage and water supply.

This planning application is accompanied by an Environmental Impact Assessment Report and Natura Impact Statement.

1.4 Galway City Development Plan (2023 - 2029)

In the preparation of this Infrastructure Report, AECOM has considered the Galway City Council Development Plan (2023 - 2029). Chapter 9, which focuses on Environment and Infrastructure, is particularly relevant to the production of this report.

1.4.1 Galway City Development Plan – Flood Risk Assessment Policy

Policy 9.1 of the Galway City Development Plan sets out the following objectives regarding Flood Risk:

1. *Support, in co-operation with the OPW, the implementation of EU Flood Risk Directive (2007/60/EC), the Flood Risk Regulations (SI No, 122 of 2010) and the DECLG and OPW Guidelines for Planning Authorities, the Planning System and Flood Risk Assessment Management (2009), updated/superseding legislation or departmental guidelines and have regard to the findings and relevant identified actions of the Corrib Catchment Flood Risk Management (CFRAM) Study.*
2. *Support and facilitate the implementation of the Coirib go Cósta Galway City Flood Relief Scheme in conjunction with the OPW to support a climate resilient city, protect against flooding and minimise the impact of future climate events. Support in general the associated mitigation and adaptation measures in order to prevent flooding and coastal erosion, subject to appropriate environmental, visual, built heritage and other relevant considerations.*
3. *Ensure the recommendations of the Strategic Flood Risk Assessment (SFRA) for the Galway City Development Plan 2023-2029 are taken into consideration in the assessment of developments in identified areas of flood risk and require site specific Flood Risk Assessment (FRA) and associated design and construction measures appropriate to the scale and nature of the development and the risks arising, in all areas of identified flood risk including on sites where a only small proportion of the site is at risk of flooding and adopt a sequential approach in accordance with the Planning System and Flood Risk Management Guidelines for Planning Authorities (2009).*
4. *Protect and promote sustainable management and uses of water bodies and watercourses from inappropriate development, including rivers, streams, associated undeveloped riparian strips, wetlands, and natural floodplains.*
5. *Ensure flood risk is incorporated into the preparation of any future local area plans, framework plans and masterplans in the city.*
6. *Ensure any proposed measure designed to alleviate flooding/coastal erosion is subject to Appropriate Assessment in accordance with Article 6 of the EU Habitats Directive, where appropriate.*
7. *Continue to protect the coastal area and the foreshore and avoid inappropriate development in areas at risk of coastal erosion and/or would cause and escalate coastal erosion in adjoining areas.*
8. *Protect and maintain, where feasible, undeveloped riparian zones and natural floodplains along the River Corrib and its tributaries.*

1.4.2 Galway City Development Plan – Water Services Policy

Policy 9.3 of the Galway City Development Plan sets out the following objectives regarding Water Services:

1. *Work in close liaison with Irish Water in the operation of water and wastewater facilities in the city and the upgrade and expansion of the network and the delivery of strategic projects such as the Terryland Water Treatment Plant Intake Works.*
2. *Support the delivery of the objectives of the Irish Water Water Services Strategic Plan (2015) and implementation of the Irish Water Capital Investment Plan 2020 - 2024.*
3. *Work in conjunction with Irish Water to ensure the provision and maintenance, of a high quality and efficient water supply capable of meeting existing and future needs of the city and support any ongoing water mains rehabilitation and water conservation projects.*
4. *Encourage all significant water users to use best practices in water conservation and continue to promote water conservation measures in the design of all new development in the city, such as rainwater harvesting and re-use of grey water, in liaison with Irish Water.*
5. *Support and liaise with Irish Water in the provision of a sustainable and effective wastewater drainage collection and treatment system capable of meeting the existing and future needs of domestic, commercial, and industrial users in the city and MASP area.*
6. *Support the Irish Water ongoing watermain rehabilitation and water leak reduction programme in order to conserve the city's water supply.*
7. *Support the decommissioning of existing individual effluent treatment systems which include septic tanks at locations which include Ballyloughane, where there is a feasible option to connect to the public sewer network. Galway City Council will collaborate with Irish Water in this regard.*
8. *Support the development and implementation of Drinking Water Safety Plans by Irish Water, which seek to protect human health by identifying, assessing, and managing risks to water quality and quantity; taking a holistic approach from source to tap.*
9. *Support the promotion of effective management of trade in discharges to sewers by Irish Water in order to maximise the capacity of existing sewer networks and minimise detrimental impacts on sewage treatment works.*

1.4.3 Galway City Development Plan – Sustainable Urban Drainage Systems (SuDS) Policy

Policy 9.4 of the Galway City Development Plan sets out the following objectives regarding SuDs:

1. *Ensure the use of Sustainable Urban Drainage Systems (SuDS) and sustainable surface water drainage management, wherever practical in the design of development to enable surface water run-off to be managed as near to its source as possible and achieve wider benefits such as sustainable development, water quality, biodiversity local amenity and climate adaptation.*
2. *Promote the use of green infrastructure e.g. green roofs, green walls, bioswales, planting and green spaces for surface water retention purposes as an integrated part of SUDS and to deliver all the ancillary benefits.*

1.5 Terryland River

As per the Galway City – County Geological Site Report (Hennessy et al., 2020. Geological Survey Ireland), the Terryland River exhibits a unique flow pattern; It can either flow out of the River Corrib and disappear underground, acting as a natural sink, or rise and flow toward the Corrib, facilitated by two estavelles akin to springs. The river's general groundwater flow direction originates from the Ballindooley Lough area, flowing southward through an underground conduit system. Within the karst depression, the Eastern and Western estavelles play a crucial role. Although these estavelles connect to Galway Bay or Lough Atalia, the precise discharge locations into these bodies of water remain unidentified. The Terryland River (European Code IE_WE_30T010500) is subject to various sensitivities, including hydro morphology pressures, urban run-off pressures, and being considered a River waterbody risk ('At risk') under the Water Framework Directive.

1.6 Pre-Planning Consultations with Galway City Council

There have been a number of preplanning consultations with GCC and the pre-planning design was shared with GCC in March 2024. All comments raised with respect to the pre-planning design have been addressed in the developed planning stage design through a collaborative and iterative design process.

AECOM presented the surface water strategy during a Teams meeting on 18.06.24, attended by the following GCC representatives Frank Clancy, Padraic MacGoillabhride and Peter Staunton. AECOM outlined the current conditions, the constraints, and the surface water strategy. The strategy which proposes limiting surface water run-off from the site to 25 l/s, by providing two (2) shallow Reinforcement Concrete (RC) attenuation tanks and SuDS measures including Green Roofs, exfiltration permeable paving and lengths of raingarden / swale (also exfiltration systems) was agreed.

1.7 Summary

This report outlines the proposed wastewater drainage, surface water drainage, and water supply network to serve the development. Additionally, it discusses the site constraints that were taken into account during the preliminary design phase.

1.7.1 Surface Water

Current conditions:

The carpark site is nearly 100% impermeable and currently unattenuated flows discharge to the Terryland Stream via an existing drainage network.

Proposal:

It is proposed to restrict the surface water run-off from the site to 25 l/s, by providing two (2) shallow Reinforcement Concrete (RC) attenuation tanks. It is also proposed to incorporate SuDS measures such as an area of Intensive Green Roof, exfiltration permeable paving and lengths of raingarden / swales. The ground conditions are unsuitable for discharge of surface water to ground. Hence, it is simply proposed to attenuate the run-off.

It is also proposed to provide a new separate gravity surface water drainage network to serve the development. The new network will discharge into the existing 525mm Ø concrete pipe and continues northwards to the discharge point into the Terryland Stream.

Further, it is proposed to divert the surface water sewer which serves the Retail development on the Headford Road around the new building and then reconnect it to the 525mm Ø concrete pipe. It is also proposed to relay circa ±155m of the GCC 525mm Ø concrete trunk surface water pipe that runs through the site.

Refer to **Section 2.2** for further details.

1.7.2 Wastewater

Current conditions:

Based on the record drawings from Uisce Éireann and a utility survey, the existing Black Box Theatre is serviced with a foul sewer that runs south to a pumping station ("Black Box WWPS") located close to the current main entrance to the Dyke Road car park. From there, with a chamber invert level of 2.3m, the sewage is pumped further south along Dyke Road until it joins a combined sewer network on Headford Road at a level of 6.8m.

Proposal:

It is proposed to relay the gravity foul sewer serving the Black Box Theatre and install a new gravity sewer network to serve the development. The existing wastewater pumping station (WWPS) that serves the Black Box Theatre is to be decommissioned and a new WWPS constructed.

The new WWPS has been positioned based on the flood extents within the site and to maximize the separation from buildings. An emergency tank with 24-hour storage capacity at DWF has been provided to serve Phase 1 development and the Black Box Theatre.

The existing 150mm rising main serving the existing WWPS is to be retained and reused. Uisce Éireann have confirmed that a 20m upgrade of a 150mm diameter sewer from Dyke Road to Wood Quay will be required. These works will be undertaken by UÉ.

Refer to **Section 3.2** for further details.

1.7.3 Water Supply

Current Conditions:

A 9" cast-iron watermain runs along Dyke Road. From this watermain, a water connection feeds the Black Box theatre and the Headford Road shopping centre. A 300mm asbestos-cement watermain runs in Headford Road and Bóthar Na Dige Road, while a shorter section of 100mm uPVC water distribution main runs along a short section of Headford Road.

Proposal:

It is proposed to take a connection off the 250mm asbestos watermain on the Dyke Road. The new watermain will pass through the phase 1 lands and loop around 3 sides (south, east, and north) of the development. Refer to **Section 4.2** for further details.

1.7.4 Flood Risk Management

A Stage 3 Site-Specific Flood Risk Assessment has been prepared to accompany this application which addresses the requirements of the Galway City Development Plan 2023 – 2029, the Regional Spatial and Economic Strategy 2020 - 2032 and “The Planning System and Flood Risk Management Guidelines for Planning Authorities”.

2. Surface Water Drainage

2.1 Existing Surface Water Drainage

Based on the record drawings supplied by Uisce Éireann and a utility survey of the site, there are surface water drainage pipes within the proposed Dyke Road development site, collecting surface water from the site and areas to the south of the site (at an invert level of 5.8m), and discharging unattenuated flows into the Terryland wetlands area, north of the existing Black Box Theatre.

The main surface water pipe running south to north along the western boundary of the site is a 450mm Ø concrete pipe. The pipe starts at an invert level of 5.8m on Bóthar Na Dige Road and falls to an invert level of 3.73m around the middle of the subject site, where it increases in size to a 525mm Ø concrete pipe and continues northwards to the discharge point. There is also a surface water pipe running through the site which serves the Retail development on the Headford Road to the east of the proposed development which discharges into this surface water pipe.

Based on the information shown on the record mapping, the existing network runs in a northerly direction to an existing drain within the green area which further discharges to the Terryland Stream. GCC have confirmed the surface water drainage discharges into the Terryland Stream. The approx. bed level of the anticipated discharge point is ~2.9m. The utility records and utility survey are included in **Appendix D**. Refer to **Figure 2-1** for the layout of surface water infrastructure in the vicinity of the site.



Figure 2-1: Existing Surface Water Infrastructure, UÉ 2024

2.2 Proposed Surface Water Drainage

It is proposed to install a new separate gravity surface water drainage network to serve the development which will discharge to the existing GCC 525mm Ø concrete pipe network which runs from south – north along the western boundary of the site and ultimately discharges to the Terryland Stream. The discharge flow will be limited to 25 l/s by providing two (2) shallow Reinforced Concrete (RC) attenuation tanks with a hydrobrake installed at each outfall manhole. It is also proposed to incorporate SuDS measures such as an area of Intensive Green Roof, exfiltration permeable paving and lengths of raingarden / swales.

Separate connections to the existing surface water network will be provided on the downstream end of each of the attenuation tanks.

Further, it is proposed to divert the surface water sewer which serves the retail development on the Headford Road around the new building and then reconnect it to the 525mm Ø concrete pipe. It is also proposed to relay circa ±155m of the GCC 525mm Ø concrete trunk surface water pipe that runs through the site.

Preliminary investigations undertaken by GCC in 2025 indicate sections of the pipe north of the Phase 1 site (on Phase 3 lands) may require repairs. GCC has confirmed it will carry out any repairs (as may be required) to ensure the Phase 1 development can connect to the existing surface water infrastructure.

Treatment of Run-Off

The quality of the surface water run-off will be much improved from the current situation with the inclusion of the green roofs, exfiltration permeable paving and lengths of raingarden / swales. In addition to these measures, it is proposed to provide a Class I By-Pass hydrocarbon separator upstream of the main development attenuation tanks to remove any hydrocarbons suspended in the site run-off prior to connection to the 525mm Ø concrete pipe.

2.2.1 Design Criteria

The design of residential developments would normally be based on Section 11.3 of the Department of Environment, Heritage and Local Government Recommendations for Site Development Works for Residential Areas and the requirements of Galway City Council.

The following design standards and guidelines have been followed in the design of the proposed surface water drainage network for the site:

- BS EN 752 – Drains and sewer system outside buildings.
- Greater Dublin Strategic Drainage Study (GDSDS) Volume 2 – New Developments.
- BS EN 858-2 – Separator System for Light Liquids (e.g. oil and petrol).
- Pipe network has been designed to ensure no surcharging during a 1 in 5-year return period rainfall event.
- No pipe flooding during a 1 in 100-year return period rainfall event.
- Surface water storage sized based on a 1 in 30-year return period rainfall event.
- An additional 20% has been allowed for climate change in relation to rainfall intensities.
- The following design criteria have been used in the design of the proposed surface water drainage network:

- Carrier pipe network – 1.0m/s to 3.0m/s.
- Colebrook White roughness value of 0.6mm for all pipework.
- Time of entry: 4 minutes.
- Return Period: 5 years.
- Met Eireann rainfall data for site.
- $M5/60 = 17.2 \text{ mm}$.
- Ratio $r = 0.3$.

The following drawings illustrate the proposed surface water drainage network to serve the site:

- 60710277-ACM-XX-XX-DR-CE-00-0500,
- 60710277-ACM-XX-XX-DR-CE-00-0501, and
- 60710277-ACM-XX-XX-DR-CE-00-0502.

Please refer to **Appendix B** for the proposed surface water network calculations.

2.3 Surface Water Attenuation

‘The SuDS Manual’ published by CIRIA Document No. C753, was utilised for the management of surface water run-off within the development. This document promotes the use of a variety of alternative measures in the design of sustainable drainage systems, which take into account quantity, quality, and amenity. SuDS encourages the use of soft systems that replicate as far as possible, the natural treatment of surface water and attenuation storage to manage surface water run-off.

Given the existing site is 100% impermeable, the introduction of SuDS measures to manage surface water run-off will result in an improvement in the quantity & quality of run-off discharged from the site. It is proposed to restrict the rate of run-off discharged from the proposed development to 25 l/s. During a 1 in 5-year return period rainfall event, the rate of run-off discharge from the existing site/ car park is approximately 130l/s, resulting in a reduction of ~80% in the proposed scenario.

Several engineering challenges exist with regards the introduction of surface water attenuation.

Ground Conditions;

The ground conditions encountered during the ground investigation are summarised below with reference to in situ and laboratory test results. The full details of the strata encountered during the ground investigation are provided in the exploratory hole logs included in the appendices of the ground investigation report compiled by Ground Investigation Ireland (GII) Limited.

The sequence of strata encountered were generally consistent across the site and generally comprised;

- Surfacing,
- Made Ground,
- Organic Deposits,
- Soft Cohesive Deposits,

- Cohesive Deposits,
- Granular Deposits,
- Bedrock.

While the Ground Investigation (GI) confirmed that the investigation did not come across any karst features, the possibility of karst features was identified in a Geophysical Survey (GS) undertaken by Minerex Geophysics Limited.

The ground conditions are extremely poor, resulting from a combination of high-water table, and very soft silts and peat to depths of circa 6m. Further, soakaway testing took place and confirmed the water level dropped too slowly to allow calculation of 'f', the soil infiltration rate. Therefore, the ground conditions preclude the use of infiltration systems.

Refer to **Appendix E** for the Ground Investigation and Geophysical Survey reports.

Existing Networks;

The invert level of the existing surface water drainage and bed depth of the Terryland Stream are both relatively shallow compared with the subject site. The limited cover precludes the use of crate systems or hydro chamber systems to provide attenuation storage of run-off.

Considering these engineering constraints and as noted above, it is proposed to limit the surface water run-off from the site to 25 l/s by providing two (2) shallow Reinforced Concrete (RC) attenuation tanks with a hydrobrake installed at each outfall manhole. It is also proposed to incorporate SuDS measures such as an area of Intensive Green Roof, exfiltration permeable paving and lengths of raingarden / swales.

2.3.1 Attenuation Storage

The estimated surface water storage was calculated using the "*hr Wallingford Storm Water Storage Estimation Online Tool*". **Table 2-1** below illustrates the design criteria used to complete the calculation.

The total storage required has been sized based on the volume required (**204m³**) to store run-off from the development site during a 1 in 100 – year return period rainfall event. A 20% increase in rainfall intensities as a result of the impacts of climate changes has also been incorporated in this design.

Table 2-1: hr Wallingford Storm Water Storage Estimation Online Tool Design Criteria

Design Criteria

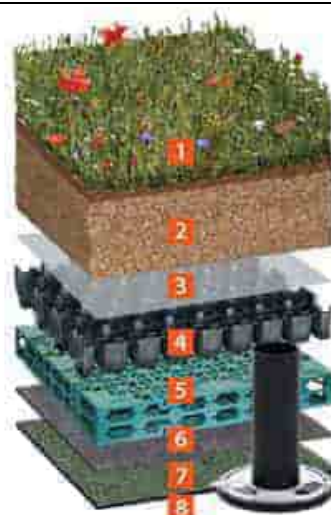
Methodology	IH124
Q _{BAR} Estimation Method	Specify Q _{BAR} Manually
Q _{BAR} for total Site Area	25 l/s
SOIL Type	4
SPR	0.47
Climate Change Allowance Factor	1.2
Interception Rainfall Depth	5mm
Minimum Flow Rate	2 l/s

SAAR	1281
M5-60 Rainfall Depth	17mm
'r' Ratio M5-60/M5-2 day	0.3

The proposed intensive green/ blue roof substructure can be seen in **Table 2-2** below. It was agreed that the Intensive Green Roof will be utilized for this development as the available roof space of 1799m² will yield the required storage of 131.2 m³. The remaining 72.8 m³ will be stored in the two (2) RC Attenuation Tanks.

Table 2-2: Proposed Intensive Green/Blue Roof Substructure

Green Roof	Blue Roof
Roof Area Required	Roof Area Required
Intensive green roof with 300 mm deep substrate	100 mm deep substrate for planting and 100 mm deep attenuation cell
BauderGREEN intensive landscape system	BauderBLUE STORMCell System



2.3.2 Rainwater Harvesting

Rainwater harvesting has been considered by the AECOM design team; the concept was further discussed with GCC. It is advised that the small roof footprint to water demand ratio didn't lend itself to rainwater harvesting. RWH is much more effective for developments with a large roof footprint and small demand.

3. Wastewater Drainage

3.1 Existing Wastewater Drainage

Based on the record drawings from Uisce Éireann and a utility survey, the existing Black Box Theatre is serviced with a foul sewer that runs south to a pumping station (“Black Box WWPS”) located close to the current main entrance to the Dyke Road car park. From there, with a chamber invert level of 2.3m, the sewage is pumped further south along Dyke Road until it joins a combined sewer network on Headford Road at a level of 6.8m. The Headford Road retail park to the east of the site has a foul gravity sewer running from north to south to a pumping station.

The utility records and utility survey are included in **Appendix D**.

Refer to **Figure 3-1** for the layout of foul sewer infrastructure in the vicinity of the site.



Figure 3-1: Existing Foul Water Infrastructure, UÉ 2024

3.1.1 Uisce Éireann Pre-Connection Enquiry

Pre-Connection Enquiries were submitted to Uisce Éireann on 16th of February 2024 for;

- Phase 1 only,
- Phases 1 & 2, and

- Phases 1 to 3.

Confirmation of Feasibility (CoF) letters for all 3 phases of development have been received and the CoF for Phase 1 is discussed in section 3.2.2 and is included in Appendix A.

Uisce Éireann have confirmed that the existing Dyke Road waste water pumping station was designed to cater only for the Black Box Theatre and that it doesn't have capacity to cater for any additional flows.

3.1.2 CCTV Survey

A CCTV survey of the existing wastewater drainage network within the proposed site was carried out to confirm the pipe routes and confirm the condition of the existing network. The CCTV was issued to UÉ as part of the wastewater diversion application.

3.2 Proposed Wastewater Drainage

Based on the Confirmation of Feasibility and the contents of such noted in **Section 3.2.2**. It is proposed to relay the gravity foul sewer serving the Black Box Theatre and install a new gravity sewer network to serve the development. The existing wastewater pumping station (WWPS) that serves the Black Box Theatre is to be decommissioned and a new WWPS constructed.

The new WWPS has been positioned based on the flood extents within the site and to maximize the separation from buildings. An emergency tank with 24-hour storage capacity at DWF has been provided to serve Phase 1 development and the Black Box Theatre.

The existing 150mm rising main serving the existing WWPS is to be retained and reused. Uisce Éireann have confirmed that a 20m upgrade of a 150mm diameter sewer from Dyke Road to Wood Quay will be required. These works will be undertaken by UÉ.

3.2.1 Foul Pumping Station and Emergency Storage

The foul pumping station has been positioned based on the flood extents within the site. The pumping station is located so that it is above the 1 in 100-year return period event water level and as far away from all buildings as possible. In addition, the above ground elements (kiosk and control room) are located above the 1:200-year return period. IW/UÉ's minimum separation distance to be provided between pumping stations and habitable buildings is 15m which can be achieved within the site.

An emergency tank with 24-hour storage capacity at DWF has been provided to serve Phase 1 development and the Black Box Theatre.

The existing rising main is to be utilised along Dyke Road, and thus a new 25m section is to be laid running from the new WWPS to the existing rising main.

3.2.2 Confirmation of Feasibility

The Confirmation of Feasibility (CoF) was received on the 23rd of May 2024. The wastewater connection is confirmed to be feasible subject to upgrades. This letter noted the following in relation to the wastewater drainage network:

Please note, the existing Black Box pumping station would require upgrading to cater for the wastewater loading and to provide adequate emergency storage for the proposed development. Alternatively, a new Pump Station could be constructed by the customer within their own site, the new pump station (WWPS) should be designed to cater for the proposed development wastewater loading. The existing Black Box pumping station shall be decommissioned with this option, and the existing flows diverted to the new WWPS. The customer will be responsible for designing the new WWPS required for the proposed development and the diverted flows that currently discharge to the existing Black Box Pumping station.

A 20m Approx. foul sewer network upgrade from 150mm diameter to 225mm diameter minimum is required to cater the proposed development at the start of Dyke Road and Wood quay. Any such network upgrade would have to be funded by the customer.

The developer should investigate the separation of storm water and foul on their development lands and that any existing storm water which is entering into the Uisce Éireann (UE) combined system will need to be eliminated. There should be no storm water discharge to the UE network.

Please note that according to our records there is an existing sewer running through this site (see drawing below). Any structures or works over or in close proximity to Uisce Éireann infrastructure that will inhibit access for maintenance or endanger structural or functional integrity of the infrastructure are not allowed. The layout of the development must ensure that this pipe is protected, and adequate separation distances are provided between Uisce Éireann infrastructure and any structures on site. Alternatively, you may enter into a diversion agreement with Uisce Éireann and divert the pipe to accommodate your development. If you wish to proceed with this option, please contact Uisce Éireann at Diversions@water.ie and submit detailed design drawings before submitting your planning application. It will be necessary to provide a wayleave over this pipe to the benefit of Uisce Éireann and ensure that it is accessible for maintenance.

Confirmation of Feasibility for the diversion of the wastewater pipework serving the Black Box Theatre and decommissioning of the existing Dyke Road wastewater pumping station has also been received and is included in Appendix A.

3.2.3 Statement of Design Acceptance

A Statement of Design Acceptance (SoDA) has been issued by UÉ. Refer to Appendix A for details.

3.2.4 Wastewater loading

Three (3) separate PCE applications were submitted to UÉ to determine if there is capacity within the existing Uisce Éireann wastewater infrastructure to cater for the increased load associated within the increase in residents.

3.2.4.1 Phase 1

Phase 1: The Dry Weather Flow (DWF) has been calculated as 1.132 l/s while the peak daily flow (taken as 6 times the Dry Weather Flow) has been calculated as 6.800 l/s. Refer to **Table 3-1** below.

Table 3-1: Phase 1 Wastewater Loading

Source	Unit	Quantity	Flow (l/day/unit or l/s/ha)	Daily (l/day)	DWF (m ³ /day)	DWF (l/s)	6xDWF (l/s)	Avg Day / Peak Wk demand (l/s)	Peak Wk Demand (l/s)
Residential Apartments	Pers	646	150	96900	96.9	1.12	6.73	1.40	2.94
Creche	Pers	20	50	1000	1.00	0.012	0.069	0.014	0.030
Total				97900	97.9	1.132	6.800	1.414	2.970

3.2.4.2 Phase 2

Phase 2: The Dry Weather Flow (DWF) has been calculated as 1.208 l/s while the peak daily flow (taken as 6 times the Dry Weather Flow) has been calculated as 7.248 l/s. Refer to **Table 3-2** below.

Table 3-2: Phase 2 Wastewater Loading

Source	Unit	Quantity	Flow (l/day/unit or l/s/ha)	Daily (l/day)	DWF (m ³ /day)	DWF (l/s)	6xDWF (l/s)	Avg Day / Peak Wk demand (l/s)	Peak Wk Demand (l/s)
Commercial Area @ 0.6 l/s/ha: Community Room (1989 m³)	Ha	0.199	0.6	10310.96	10.31	0.119	0.72	0.149	0.313
Hotel	Pers	360	250	90000	90.00	1.042	6.25	1.302	2.734
Hotel FTE	Pers	60	60	3600	3.60	0.042	0.25	0.052	0.109
Retail GIA	Employees	15	30	454.50	0.455	0.005	0.03	0.007	0.014
Total				105462.98	105.46	1.208	7.248	1.510	3.171

3.2.4.3 Phase 3

Phase 3: The Dry Weather Flow (DWF) has been calculated as 0.33 l/s while the peak daily flow (taken as 6 times the Dry Weather Flow) has been calculated as 1.98 l/s. Refer to **Table 3-3** below.

Table 3-3: Phase 3 Wastewater Loading

Source	Unit	Quantity	Flow (l/day/unit or l/s/ha)	Daily (l/day)	DWF (m ³ /day)	DWF (l/s)	6xDWF (l/s)	Avg Day / Peak Wk demand (l/s)	Peak Wk Demand (l/s)
Residential Apartments	Pers	190	150	28500	28.5	0.33	1.98	0.41	0.87

The proposed wastewater drainage network has been designed to achieve self-cleansing velocities of 0.75 m/s, as set out in Section 3.6 of the Code of Practice. The proposed network has been designed to convey between 2.5 and 6 times the DWF.

3.2.5 Wastewater Network Design

The following design standards and guidelines have been followed in the design of the proposed wastewater drainage network for the site:

- Uisce Éireann Code of Practice for Wastewater Infrastructure (IW-CDS-5030-03),
- BS EN 752 – Drains and sewer systems outside buildings,
- Sewers for Adoption, 6th Edition,
- Micro Drainage Software Pipeline Design,
- All pipework will be provided at gradients that will provide velocities in the range of 0.75 m/ sec and 2.5 m/sec, when flowing half full,
- Part H Building Regulations.

The following drawings illustrate the proposed wastewater drainage network and diversions associated with the proposals:

- 60710277-ACM-XX-XX-DR-CE-00-0500,
- 60710277-ACM-XX-XX-DR-CE-00-0501,
- 60710277-ACM-XX-XX-DR-CE-00-0502 and
- 60710277-ACM-XX-XX-DR-CE-00-0503.

Please refer to Appendix C for more information on the wastewater drainage network design calculations.

4. Water Supply

4.1 Existing Water Supply Infrastructure

Utility records were obtained from Uisce Éireann (UÉ) to indicate existing watermains in the vicinity of the site. The location of the watermain was verified with a utility survey.

A 9" cast-iron watermain runs along Dyke Road. From this watermain, a water connection feeds the Black Box theatre and the Headford Road shopping centre. A 300mm asbestos-cement watermain runs in Headford Road and Bóthar Na Dige Road, while a shorter section of 100mm uPVC water distribution main runs along a short section of Headford Road.

The utility records and utility survey are included in **Appendix D**.

Refer to **Figure 4-1** for the layout of potable water infrastructure in the vicinity of the site.



Figure 4-1: Existing Watermain Infrastructure, UÉ 2024

4.1.1 Uisce Éireann Pre-Connection Enquiry

Pre-Connection Enquiries have been submitted to Uisce Éireann for

- Phase 1 only,
- Phases 1 & 2, and
- Phases 1 to 3.

On-site water storage for firefighting may be required by the Galway City Fire Service.

4.2 Proposed Water Supply Network

It is proposed to take a connection off the 250mm asbestos watermain on the Dyke Road. The new watermain will pass through the phase 1 lands and loop around 3 sides (south, east, and north) of the development.

The following drawings illustrate the proposed watermain network within the site:

- 60710277-ACM-XX-XX-DR-CE-00-2700,
- 60710277-ACM-XX-XX-DR-CE-00-2701, and
- 60710277-ACM-XX-XX-DR-CE-00-2702.

The internal water supply network is based on the requirements of the Uisce Éireann Code of Practice for Water Supply and the Technical Guidance Document – Part B of the Building Regulations 2006:

- Hydrants are positioned within 46m of the proposed buildings.
- A bulk water meter has been provided at the connection to the existing Uisce Éireann distribution watermain at the site entrance.
- No new watermain up to and including 150mm in diameter shall be laid within 3m of an existing or proposed structure. See Section 3.5.9 of Water Code of Practice.

Uisce Éireann will be consulted with regard network upgrades to accommodate future demand and consolidate the resilience of the local network.

Firefighting water supplies and fire hydrants will be provided as required in accordance with the Building Regulations and the requirement of Galway City Fire Service.

4.2.1 Confirmation of Feasibility

The Confirmation of Feasibility (CoF) was received on the 23rd of May 2024. The water connection is confirmed to be feasible without infrastructure upgrade by Uisce Éireann (UÉ). This letter noted the following in relation to the water network:

The water main along Dyke Road is the preferred connection point for the proposed development.

4.2.2 Statement of Design Acceptance

A Statement of Design Acceptance (SoDA) has been issued by UÉ. Refer to Appendix A for details.

5. Access and Traffic

The existing access to the development and car park will be decommissioned and a new access to the proposed development will form as part of the proposed works. Please refer to AECOM Drawing **60710277-ACM-XX-XX-DR-CE-00-0002 & 60710277-ACM-XX-XX-DR-CE-00-0003** for the location of the access.

Please refer to the project Traffic and Transport Assessment and Mobility Management Plan reports compiled by PUNCH for further information on site accessibility.

6. Flood Risk Assessment

As part of the preliminary design of the proposed development, AECOM prepared a site-specific Stage 3 Flood Risk Assessment which describes the flood risks to the site and addresses the requirements of Galway City Development Plan (2023– 2029). This FRA has been prepared in accordance with the *Planning System and Flood Risk Management Guidelines*, (OPW, 2009).

Appendix A Uisce Éireann

- 7. Confirmation of Feasibility for Phase 1 Development**
- 8. Statement of Design Compliance (SoDA)**
- 9. Confirmation of Feasibility for Wastewater Diversion**

CONFIRMATION OF FEASIBILITY

Sumana Jain Kelly
Aecom
1st Floor Montrose
Carrigaline Road
Douglas
Cork
T12H90H

11 March 2025

Uisce Éireann
Bosca OP 448
Oifig Sheachadta na
Cathrach Theas
Gathair Chorcaí

Uisce Éireann
PO Box 448
South City
Delivery Office
Cork City

www.water.ie

Our Ref: CDS24001446 Pre-Connection Enquiry
Site at, Dyke Road Car Park, Galway City, Galway

Dear Applicant/Agent,

We have completed the review of the Pre-Connection Enquiry.

Uisce Éireann has reviewed the pre-connection enquiry in relation to a Water & Wastewater connection for a Housing Development of 219no. unit(s) and 1no. Creche unit at Site at, Dyke Road Car Park, Galway City, Galway, (the **Development**).

Based upon the details provided we can advise the following regarding connecting to the networks;

- **Water Connection**
 - Feasible without infrastructure upgrade by Irish Water
 - **Water Treatment Plant**
There is sufficient capacity for the proposed development.
- **Wastewater Connection**
 - Feasible Subject to upgrades
 - **Wastewater Treatment Plant**
There is sufficient capacity for the proposed development.

Water Network

The water main along Dyke Road is the preferred connection point for the proposed development.

Stiúrthóirí / Directors: Tony Keohane (Cathaoirleach / Chairman), Niall Gleeson (POF / CEO), Christopher Banks, Fred Barry, Gerard Britchfield, Liz Joyce, Patricia King, Eileen Maher, Cathy Mannion, Michael Walsh.

Oifig Chláraithe / Registered Office: Teach Colvill, 24-26 Sráid Thalbóid, Baile Átha Cliath 1, D01 NP86 / Colvill House, 24-26 Talbot Street, Dublin, Ireland D01NP86

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Sumana Jain Kelly
1st Floor Montrose
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9 October 2024

**Re: Design Submission for Site at, Dyke Road Car Park, Galway City, Galway (the “Development”)
(the “Design Submission”) / Connection Reference No: CDS24001446**

Dear Sumana Jain Kelly,

Many thanks for your recent Design Submission.

We have reviewed your proposal for the connection(s) at the Development. Based on the information provided, which included the documents outlined in Appendix A to this letter, Uisce Éireann has no objection to your proposals.

This letter does not constitute an offer, in whole or in part, to provide a connection to any Uisce Éireann infrastructure. Before you can connect to our network you must sign a connection agreement with Uisce Éireann. This can be applied for by completing the connection application form at www.water.ie/connections. Uisce Éireann's current charges for water and wastewater connections are set out in the Water Charges Plan as approved by the Commission for Regulation of Utilities (CRU) (https://www.cru.ie/document_group/irish-waters-water-charges-plan-2018/).

You the Customer (including any designers/contractors or other related parties appointed by you) is entirely responsible for the design and construction of all water and/or wastewater infrastructure within the Development which is necessary to facilitate connection(s) from the boundary of the Development to Uisce Éireann's network(s) (the “**Self-Lay Works**”), as reflected in your Design Submission. Acceptance of the Design Submission by Uisce Éireann does not, in any way, render Uisce Éireann liable for any elements of the design and/or construction of the Self-Lay Works.

If you have any further questions, please contact your Uisce Éireann representative:

Name: Kyle Jackson

Email: kyle.jackson@water.ie

Yours sincerely,



Dermot Phelan
Connections Delivery Manager

Stiúrthóirí / Directors: Tony Keohane (Cathaoirleach / Chairman), Niall Gleeson (POF / CEO), Christopher Banks, Fred Barry, Gerard Britchfield, Liz Joyce, Patricia King, Eileen Maher, Cathy Mannion, Michael Walsh.

Oifig Chláraithe / Registered Office: Teach Colvill, 24-26 Sráid Thalbóid, Baile Átha Cliath 1, D01 NP86 / Colvill House, 24-26 Talbot Street, Dublin, Ireland D01NP86

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Appendix A

Document Title & Revision

- [60710277-ACM-XX-XX-DR-CE-00-0500_P3_01_ProposedDrainage]
- [60710277-ACM-XX-XX-DR-CE-00-0501_P3_01_ProposedDrainage]
- [60710277-ACM-XX-XX-DR-CE-00-0502_P3_01_ProposedDrainage]
- [60710277-ACM-XX-XX-DR-CE-00-0503_P3_01_ProsposedWWPS]
- [60710277-ACM-XX-XX-DR-CE-00-0500_P1_01_ ProsposedWWPS]
- [60710277-ACM-XX-XX-DR-CE-00-0501_P3_01_ProposedWater]
- [60710277-ACM-XX-XX-DR-CE-00-0500_P3_01_ProposedWater01]
- [60710277-ACM-XX-XX-DR-CE-00-0501_P3_01_ProposedWater02]

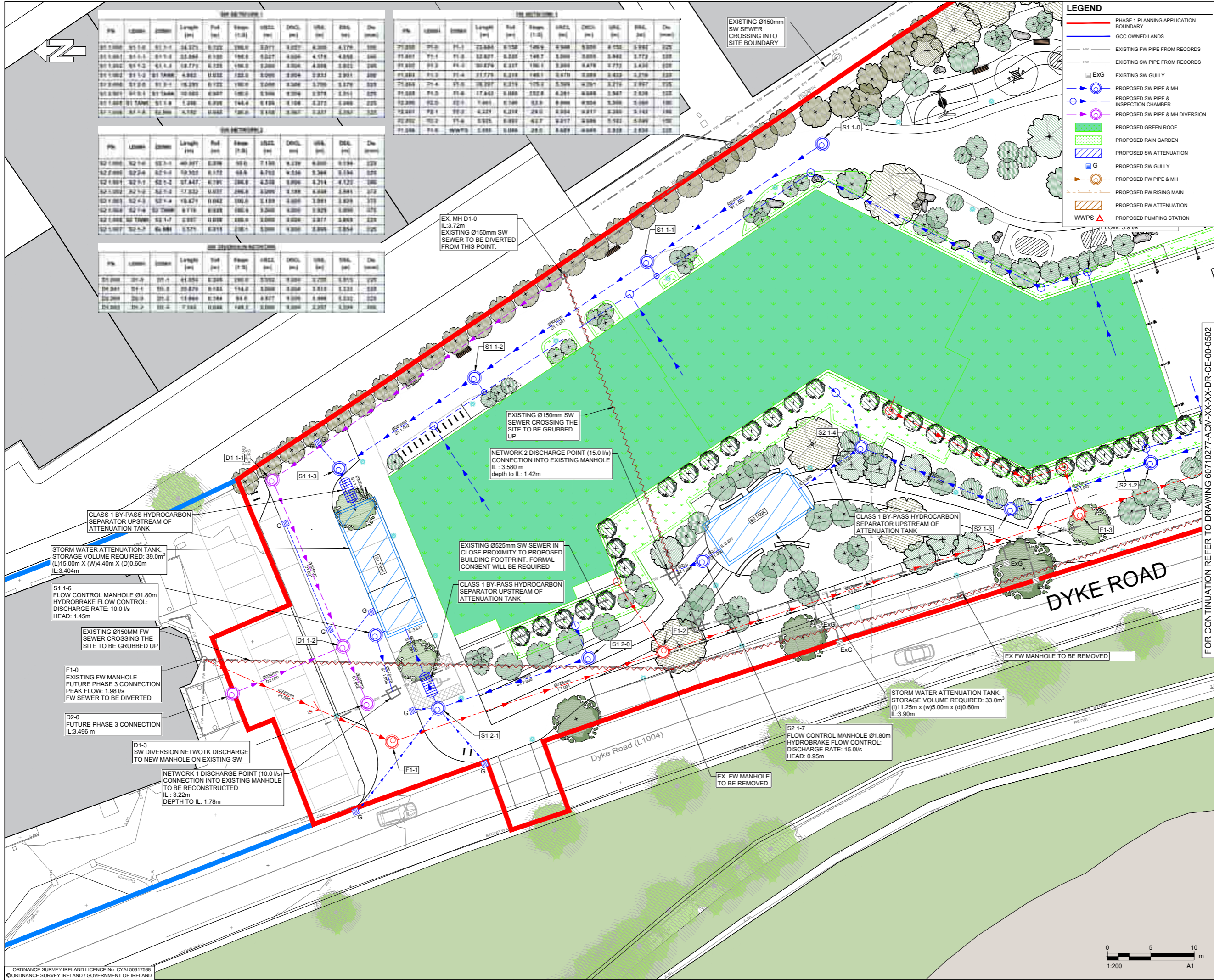
Standard Details/Code of Practice Exemption:

Not applicable

For further information, visit www.water.ie/connections

Notwithstanding any matters listed above, the Customer (including any appointed designers/contractors, etc.) is entirely responsible for the design and construction of the Self-Lay Works. Acceptance of the Design Submission by Uisce Éireann will not, in any way, render Uisce Éireann liable for any elements of the design and/or construction of the Self-Lay Works.





FOR CONTINUATION REFER TO DRAWING 60710277-ACM-XX-XX-DR-CE-00-0502

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AECOM
PROJECT
Phase 1
Corrib Causeway
Dyke Road, Galway

CLIENT
Land Development
Agency (LDA) / GCC

CONSULTANT
AECOM
BLOCK 6, GALWAY TECHNOLOGY PARK
PARKMORE
GALWAY, H91 W30F, IRELAND
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NOTES
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6. THE LOCATION & DEPTH OF SERVICES TO BE CHECKED ON SITE PRIOR TO COMMENCEMENT OF ANY WORKS.
7. MANHOLE COVERS AND FRAMES IN PUBLICLY ACCESSIBLE AREAS SHALL BE HEAVY DUTY CAST IRON, CLASS D400, DOUBLE SEALED AND LOCKABLE TYPE COMPLYING WITH BS EN 124-2:2015.
8. GULLY GRATINGS & FRAMES SHALL COMPLY WITH BS EN 124-2:2015.
9. EXISTING INVERT LEVELS TO BE VERIFIED ON SITE BEFORE COMMENCING CONSTRUCTION.
10. SURFACE WATER & FOUL SEWER PIPES LESS THAN 1.2m BELOW THE ROAD SURFACE OR LESS THAN 0.9m IN NON-TRAFFICKED FOOTPATHS AND LANDSCAPE AREAS (WITH AN ABSOLUTE MINIMUM DEPTH OF COVER ABOVE THE EXTERNAL CROWN OF THE PIPE Ø750mm) SHALL BE PROTECTED FROM DAMAGE BY PROVIDING MINIMUM 150mm THICK CONCRETE C16/20 HAUNCH IN ACCORDANCE WITH IS EN 12620.
11. CCTV SURVEY TO BE CONDUCTED PRIOR TO COMMENCEMENT OF ANY WORKS TO DETERMINE THE CONDITION AND VERIFY LEVELS OF THE EXISTING FOUL AND SURFACE WATER PIPES/ MANHOLES. ANY SUB-STANDARD OR DEFECTIVE ELEMENTS OF THE EXISTING PIPES/MANHOLES TO BE REPORTED AND CORRECTED.
12. ALL SURFACE WATER DRAINAGE DETAILS TO BE IN ACCORDANCE WITH TII SPECIFICATIONS FOR ROAD WORKS.
13. ALL FOUL DRAINAGE TO BE IN ACCORDANCE WITH THE IRISH WATER STANDARD DETAILS AND CODES OF PRACTICE.
14. CONNECTIONS TO THE EXISTING DRAINAGE NETWORK REQUIRE APPROVAL FROM IRISH WATER & GALWAY CITY COUNCIL.
15. REFER TO ARCHITECT DRAWINGS FOR ARCHITECTURAL DETAILS.
16. REFER TO LANDSCAPE ARCHITECT DRAWINGS FOR LANDSCAPING DETAILS.

AECOM
PLANNING
PURPOSE
P3 PLANNING
ISSUE/REVISION

NO	DATE	DESCRIPTION
01	05.07.2024	FOR PLANNING
IR	DATE	DESCRIPTION

PROJECT NUMBER
60710277
SHEET TITLE
PROPOSED DRAINAGE
SHEET 01
SHEET NUMBER
60710277-ACM-XX-XX-DR-CE-00-0501

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ORDNANCE SURVEY IRELAND LICENCE No. CYAL50317588
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PROJECT

Phase 1
Corrib Causeway
Dyke Road, Galway

CLIENT

Land Development
Agency (LDA) / GCC

CONSULTANT

AECOM
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6. THE LOCATION OF SERVICES TO BE CHECKED ON SITE PRIOR TO COMMENCING ANY WORKS.
7. MANHOLE COVERS AND FRAMES IN PUBLICLY OWNED AREAS ARE TO BE CAST IN PLACE TO COMPLY WITH BS EN 124-2-2015.
8. GULLY DRAININGS & SINKS SHALL COMPLY WITH BS EN 124-2-2015.
9. EXISTING INVERT LEVELS TO BE VERIFIED ON SITE PRIOR TO COMMENCEMENT OF WORKS.
10. SURFACE WATER & Foul SEWER PIPES LESS THAN 12m BELOW THE ROAD SURFACE OR LESS THAN 0.9m IN DEPTH SHALL BE PROTECTED BY A MINIMUM OF 150mm (WITH AN ABSOLUTE MINIMUM DEPTH OF COVER ABOVE THE EXTERNAL CROWN OF THE PIPE 0.750m) SHALL BE COVERED FROM THE ROAD SURFACE TO THE INVERT BY 150mm THICK CONCRETE C16/20 HAUNCH IN ACCORDANCE WITH IS EN 12620.
11. ALL EXISTING SERVICES TO BE CONDUCTED PRIOR TO COMMENCEMENT OF ANY WORKS TO DETERMINE THE CONDITION AND VERIFY LEVELS OF THE EXISTING FOUL WATER DRAINAGE. ANY DEFECTIVE ELEMENTS OF THE EXISTING PIPES/MANHOLES TO BE REPORTED AND CORRECTED.
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16. REFER TO LANDSCAPE ARCHITECT DRAWINGS FOR LANDSCAPING DETAILS.

AECOM
PLANNING

PURPOSE

P3 PLANNING

ISSUE/REVISION

01	05.07.2024	FOR PLANNING
I/R	DATE	DESCRIPTION

PROJECT NUMBER

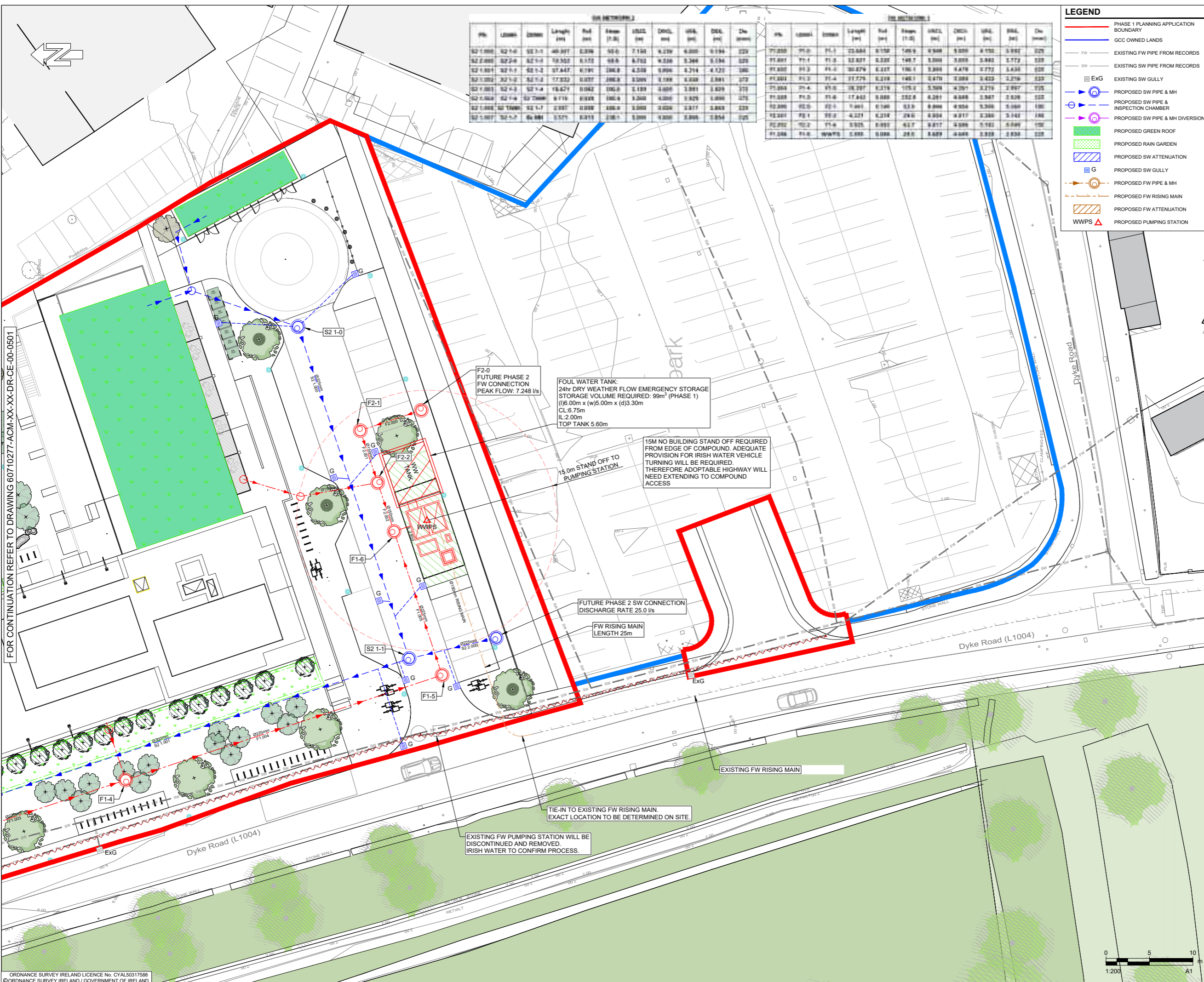
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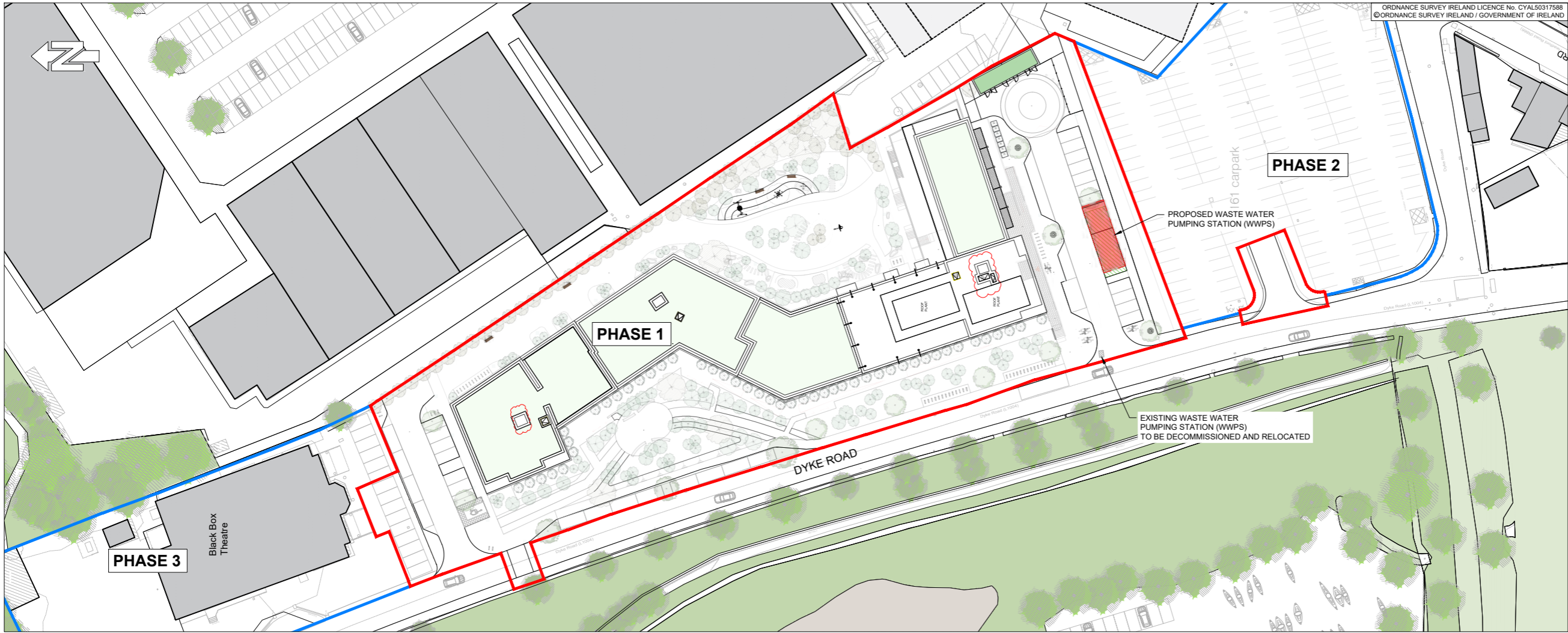
SHEET TITLE

PROPOSED DRAINAGE
SHEET 02

SHEET NUMBER

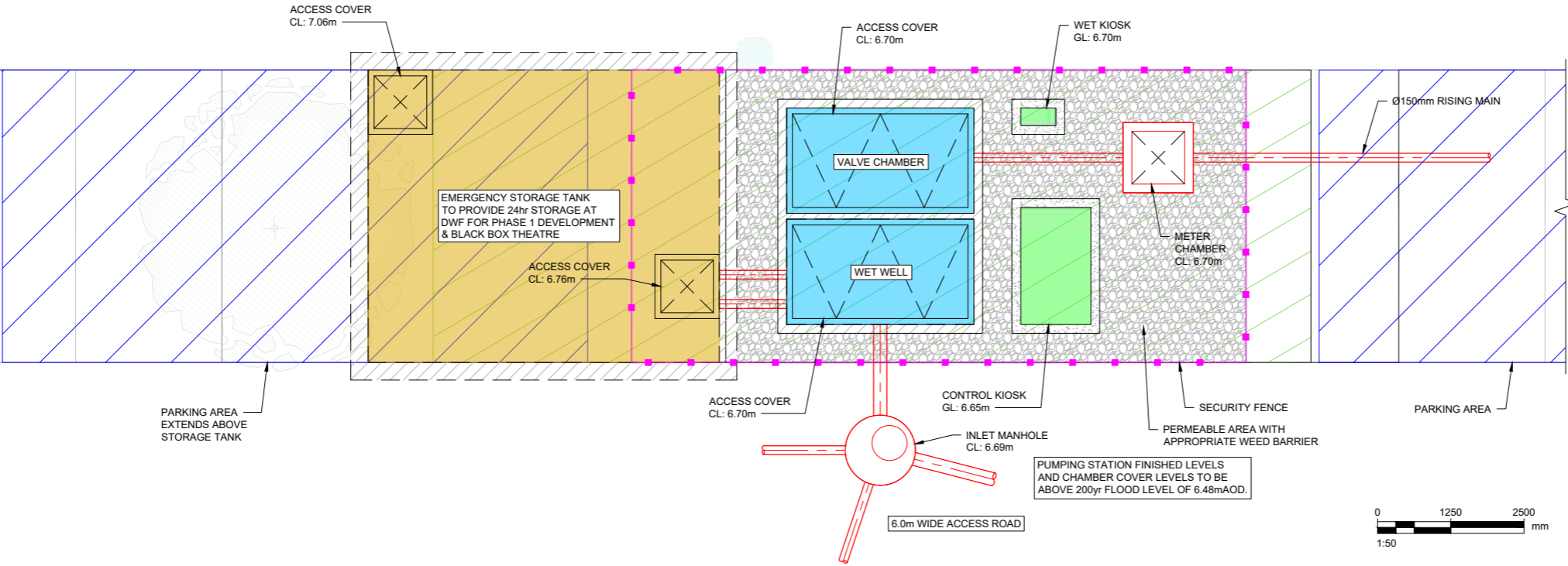
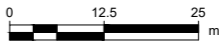
60710277-ACM-XX-XX-DR-CE-00-0502





LEGEND	
	PHASE 1 PROPOSED BOUNDARY
	CLIENT OWNED LAND BOUNDARY
	PROPOSED SECURITY FENCE
	PROPOSED FW PIPE
	PROPOSED PUMPING STATION
	PROPOSED KIOSKS
	PROPOSED STORAGE
	PROPOSED PUBLIC PARKING

LOCATION PLAN
1:500



WWPS DETAILS
1:500



AECOM

PROJECT

Phase 1
Corrib Causeway
Dyke Road, Galway

CLIENT

Land Development
Agency (LDA) / GCC

CONSULTANT

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- DIMENSIONS OF ALL BOUNDARIES AND ADJOINING ROADS TO BE CHECKED ON SITE PRIOR TO COMMENCEMENT OF WORKS.
- DO NOT SCALE. ALL MEASUREMENTS AND COORDINATES TO BE CHECKED ON SITE.
- THE LOCATION & DEPTH OF SERVICES TO BE CHECKED ON SITE PRIOR TO COMMENCING ANY WORKS.
- MANHOLE COVERS AND FRAMES IN PUBLICLY ACCESSIBLE AREAS SHALL BE HEAVY DUTY CAST IRON, CLASS D400, DOUBLE SEALED AND LOCKABLE TYPE COMPLYING WITH BS EN 124-2:2015.
- GULLY GRATINGS & FRAMES SHALL COMPLY WITH BS EN 124-2:2015.
- EXISTING INVERT LEVELS TO BE VERIFIED ON SITE BEFORE COMMENCING CONSTRUCTION.
- SURFACE WATER & FOUL SEWER PIPES LESS THAN 1.2m BELOW THE ROAD SURFACE OR LESS THAN 0.9m IN NON-TRAFFICKED FOOTPATHS AND LANDSCAPE AREAS (WITH AN ABSOLUTE MINIMUM DEPTH OF COVER ABOVE THE EXTERNAL CROWN OF THE PIPE 0.750m) SHALL BE PROTECTED FROM DAMAGE BY PROVIDING MINIMUM 150mm THICK CONCRETE C16/20 HAUNCH IN ACCORDANCE WITH IS EN 12620.
- CCTV SURVEY TO BE CONDUCTED PRIOR TO COMMENCEMENT OF ANY WORKS TO DETERMINE THE CONDITION AND VERIFY LEVELS OF THE EXISTING FOUL AND SURFACE WATER PIPES/ MANHOLES. ANY SUB-STANDARD OR DEFECTIVE ELEMENTS OF THE EXISTING PIPES/MANHOLES TO BE REPORTED AND CORRECTED.
- ALL SURFACE WATER DRAINAGE DETAILS TO BE IN ACCORDANCE WITH TII SPECIFICATIONS FOR ROAD WORKS.
- ALL FOUL DRAINAGE TO BE IN ACCORDANCE WITH THE IRISH WATER STANDARD DETAILS AND CODES OF PRACTICE.
- CONNECTIONS TO THE EXISTING DRAINAGE NETWORK REQUIRE APPROVAL FROM IRISH WATER & GALWAY CITY COUNCIL.
- REFER TO ARCHITECT DRAWINGS FOR ARCHITECTURAL DETAILS.
- REFER TO LANDSCAPE ARCHITECT DRAWINGS FOR LANDSCAPING DETAILS.

AECOM
PLANNING

PURPOSE

P3 PLANNING

ISSUE/REVISION

IR	DATE	DESCRIPTION
01	05.07.2024	FOR PLANNING

PROJECT NUMBER

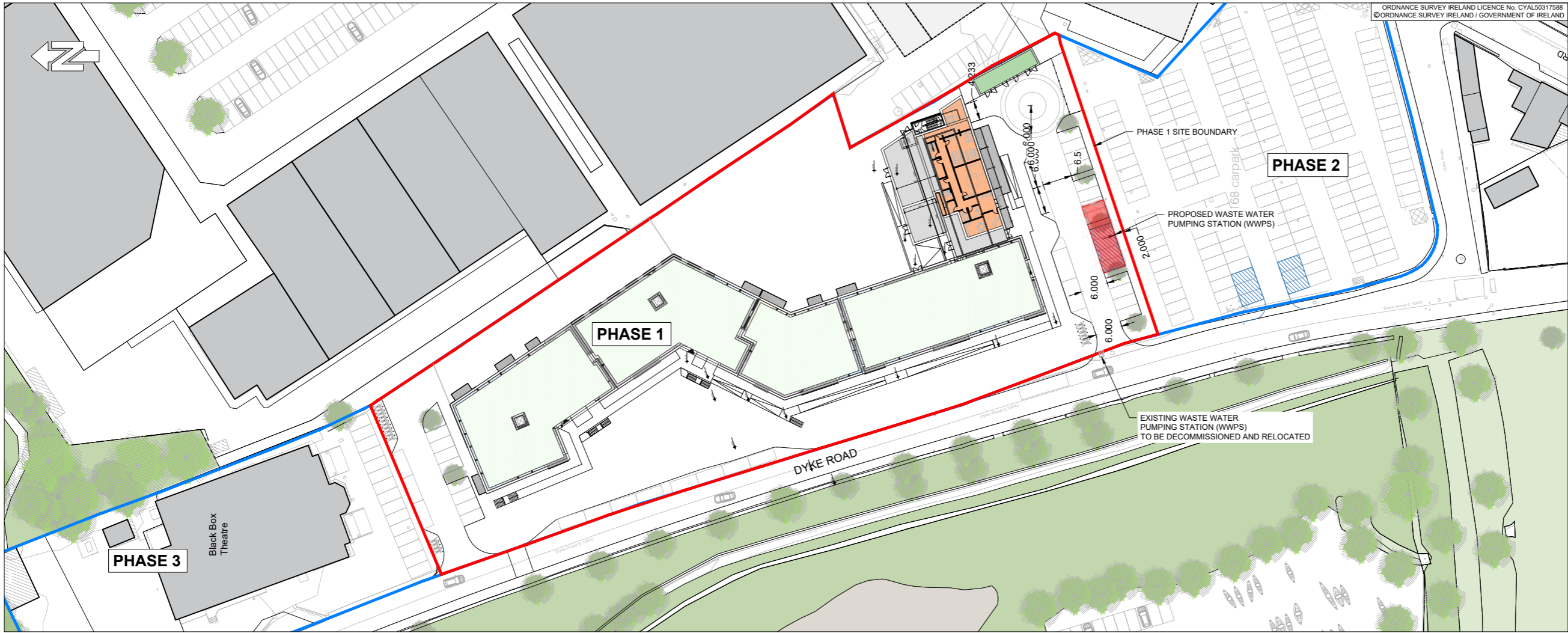
60710277

SHEET TITLE

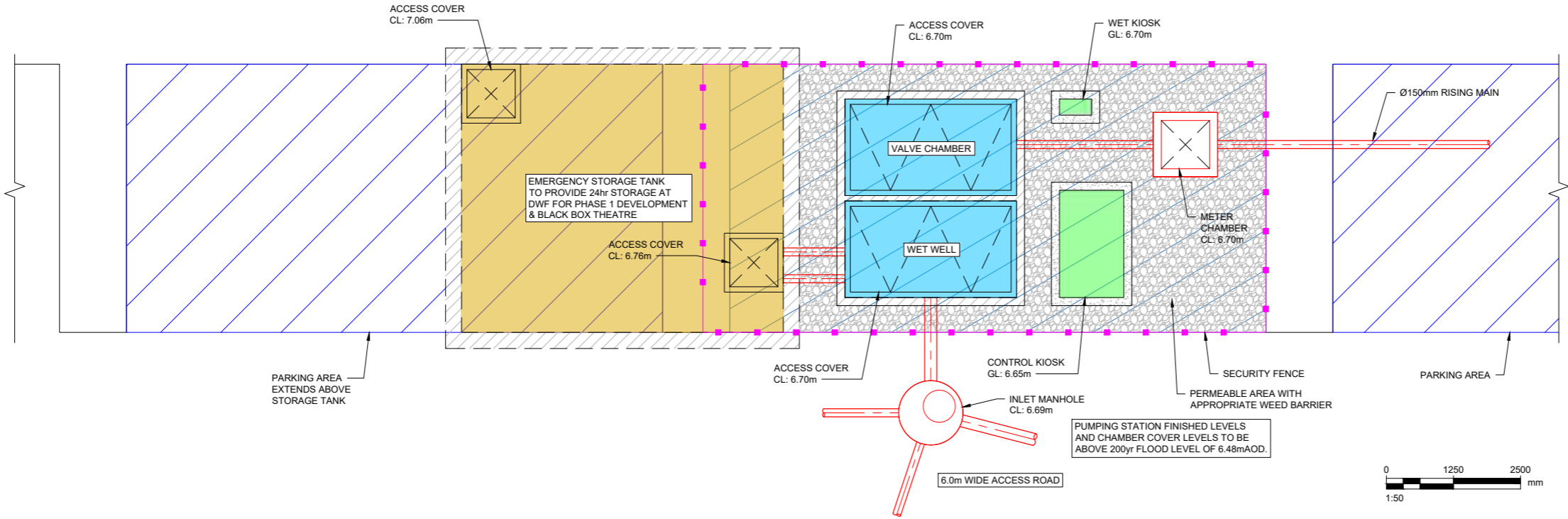
PROPOSED PHASE 1
WASTE WATER PUMPING STATION
(WWPS)

SHEET NUMBER

60710277-ACM-XX-XX-DR-CE-00-0503



LOCATION PLAN
1:500



WWPS DETAILS
1:500

PROJECT

Dyke Road,
Galway

CLIENT

Land Development
Agency

CONSULTANT

AECOM
BLOCK 6, GALWAY TECHNOLOGY PARK
PARKMORE
GALWAY, H91 W30F, IRELAND
www.aecom.com

NOTES

1. THIS DRAWING IS A CONCEPT LAYOUT ONLY. FOR INFORMATION.
2. COORDINATES AND ELEVATIONS IN METRES TO IRISH TRANSVERSE MERCATOR (ITM) AND MALIN HEAD DATUM, UNLESS SPECIFIED OTHERWISE.
3. DIMENSIONS IN MILLIMETRES UNLESS NOTED OTHERWISE.
4. FOR PROPOSED DRAINAGE REFER TO DRAWING 60710277-ACM-XX-XX-DR-CE-00-0501.

LEGEND

- PHASE 1 PROPOSED BOUNDARY
- CLIENT OWNED LAND BOUNDARY
- PROPOSED SECURITY FENCE
- PROPOSED FW PIPE
- PROPOSED PUMPING STATION
- PROPOSED KIOSKS
- PROPOSED STORAGE
- PROPOSED PUBLIC PARKING

PURPOSE

P1 INFORMATION

ISSUE/REVISION

IR	DATE	DESCRIPTION
01	05.06.2024	FIRST ISSUE FOR INFORMATION
IR		

DRAFT COPY

PROJECT NUMBER

60710277

SHEET TITLE

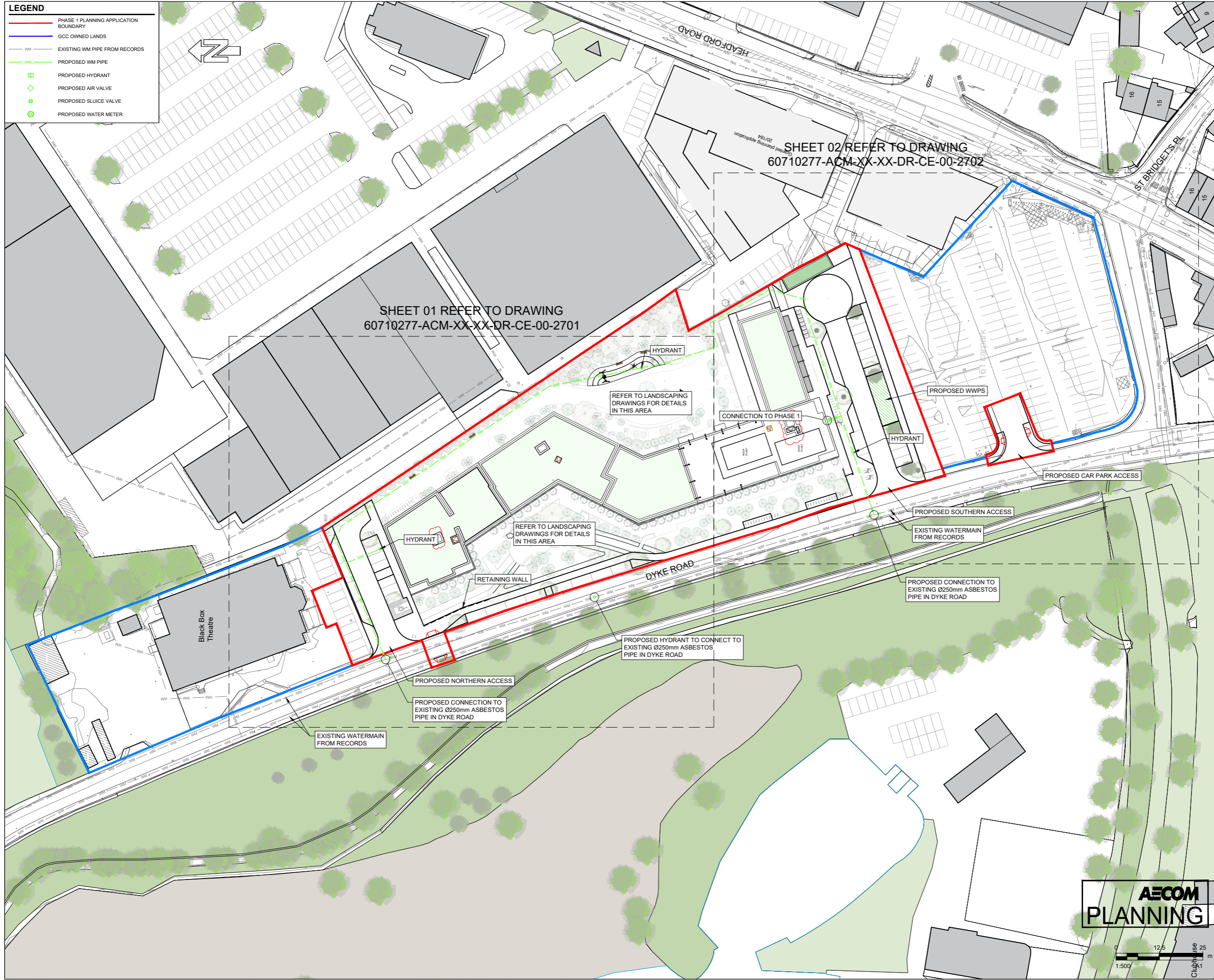
PROPOSED PHASE 1
WASTE WATER PUMPING STATION
(WWPS)

SHEET NUMBER

60710277-ACM-XX-XX-DR-CE-00-0506

File name: C:\USERS\PROPHETT\AECOM\LD\DYKE ROAD GALWAY - GENERAL\900_CAD_GIS\910_CAD\004_CEV1_WIP\02_SHEETS\60710277-ACM-XX-XX-DR-CE-00-2700_P3_01_PROPOSEDWATER.DWG
Last saved by: PROPHETT (2024-07-25) Last Plotted: 2024-07-25

Project Management Initials: _____ Designer: _____ Checked: _____ Approved: _____
ISO A1 594mm x 841mm



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PROJECT

Phase 1
Corrib Causeway
Dyke Road, Galway

CLIENT

Land Development
Agency (LDA) / GCC

CONSULTANT

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7. THE DEVELOPMENT IS TO HAVE A BULK WATER METER (EXACT LOCATION TO BE AGREED WITH UIUCE EIREANN) IN ACCORDANCE WITH UIUCE EIREANN CODE OF PRACTICE SECTION 3.15.4.
8. THE CONNECTION POINT IS TO HAVE A METER INSTALLED IN ACCORDANCE WITH THE UIUCE EIREANN CODE OF PRACTICE FOR WATER INFRASTRUCTURE SECTION 3.15.2.
9. ALL PROPOSED WATERMAIN PIPEWORK TO BE HDPE WITH MINIMUM P250 RATING IN ACCORDANCE WITH UIUCE EIREANN CODE OF PRACTICE FOR WATER INFRASTRUCTURE.
10. THE MINIMUM PIPE DIAMETER PROPOSED IS 150mm.
11. ALL WATERMANS WILL HAVE A MINIMUM COVER OF 900mm. ALL SERVICE PIPES CONNECTING PROPERTIES WILL HAVE MINIMUM COVER OF 750mm.
12. HYDRANTS SHALL BE DOUBLE FLANGED DRILLED TO PN16 AND SHALL COMPLY WITH BS EN 14539, IS EN 1074 PART 6 AND 155 750. REFER TO UIUCE EIREANN CODE OF PRACTICE FOR WATER INFRASTRUCTURE SECTION 3.16.1.
13. SLUICE VALVES HAVE BEEN PROVIDED SO THAT INDIVIDUAL SECTIONS CAN BE ISOLATED & WILL COMPLY TO BS 5163. THE DEPTH OF THE SLUICE VALVE SPINDLE CAP BELOW FINISHED GROUND WILL NOT EXCEED 300mm.
14. ALL WATERMAIN DETAILS TO BE IN ACCORDANCE WITH THE UIUCE EIREANN INFRASTRUCTURE STANDARD DETAILS AND CODE OF PRACTICE FOR WATER INFRASTRUCTURE.
15. ALL WATERMAIN WORKS SHALL BE CARRIED OUT IN ACCORDANCE WITH UIUCE EIREANN STANDARD DETAIL AND THE CODE OF PRACTICE FOR WATER INFRASTRUCTURE DOCUMENT CDS-5030-03.
16. ALL PIPE MATERIALS SHALL BE IN COMPLIANCE WITH SECTION 3.9 OF UIUCE EIREANN CODE OF PRACTICE DOCUMENT CDS-5020-03.
17. INDIVIDUAL WATER SERVICE CONNECTIONS AND BOUNDARY BOXES TO EACH DWELLING SHALL BE IN COMPLIANCE WITH UIUCE EIREANN STANDARD DETAIL STD-W-02.
18. SEPARATION DISTANCES FROM OTHER SERVICES, BOUNDARY WALLS ETC. SHALL BE IN ACCORDANCE WITH UIUCE EIREANN STANDARD DETAIL STD-W-11.
19. SEPARATION DISTANCES FROM TREES, SHRUBS ETC. SHALL BE IN ACCORDANCE WITH UIUCE EIREANN STANDARD DETAIL STD-W-12 & 12A.
20. ON-LINE AND OFF-LINE AIR VALVE DETAILS SHALL BE IN ACCORDANCE WITH UIUCE EIREANN STANDARD DETAIL STD-W-22 & STD-W-23 RESPECTIVELY.
21. SLUICE VALVE DETAILS SHALL BE IN ACCORDANCE WITH UIUCE EIREANN STANDARD DETAIL STD-W-18 & STD-W-19 RESPECTIVELY.
22. ON-LINE AND OFF-LINE HYDRANT DETAILS SHALL BE IN ACCORDANCE WITH UIUCE EIREANN STANDARD DETAIL STD-W-18 & STD-W-19 RESPECTIVELY.
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24. SCOUR VALVE AND CHAMBER DETAILS SHALL BE IN ACCORDANCE WITH UIUCE EIREANN STANDARD DETAIL STD-W-30.

PURPOSE

P3 PLANNING

ISSUE/REVISION

IR	DATE	DESCRIPTION
01	05.07.2024	FOR PLANNING

PROJECT NUMBER

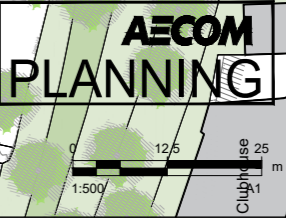
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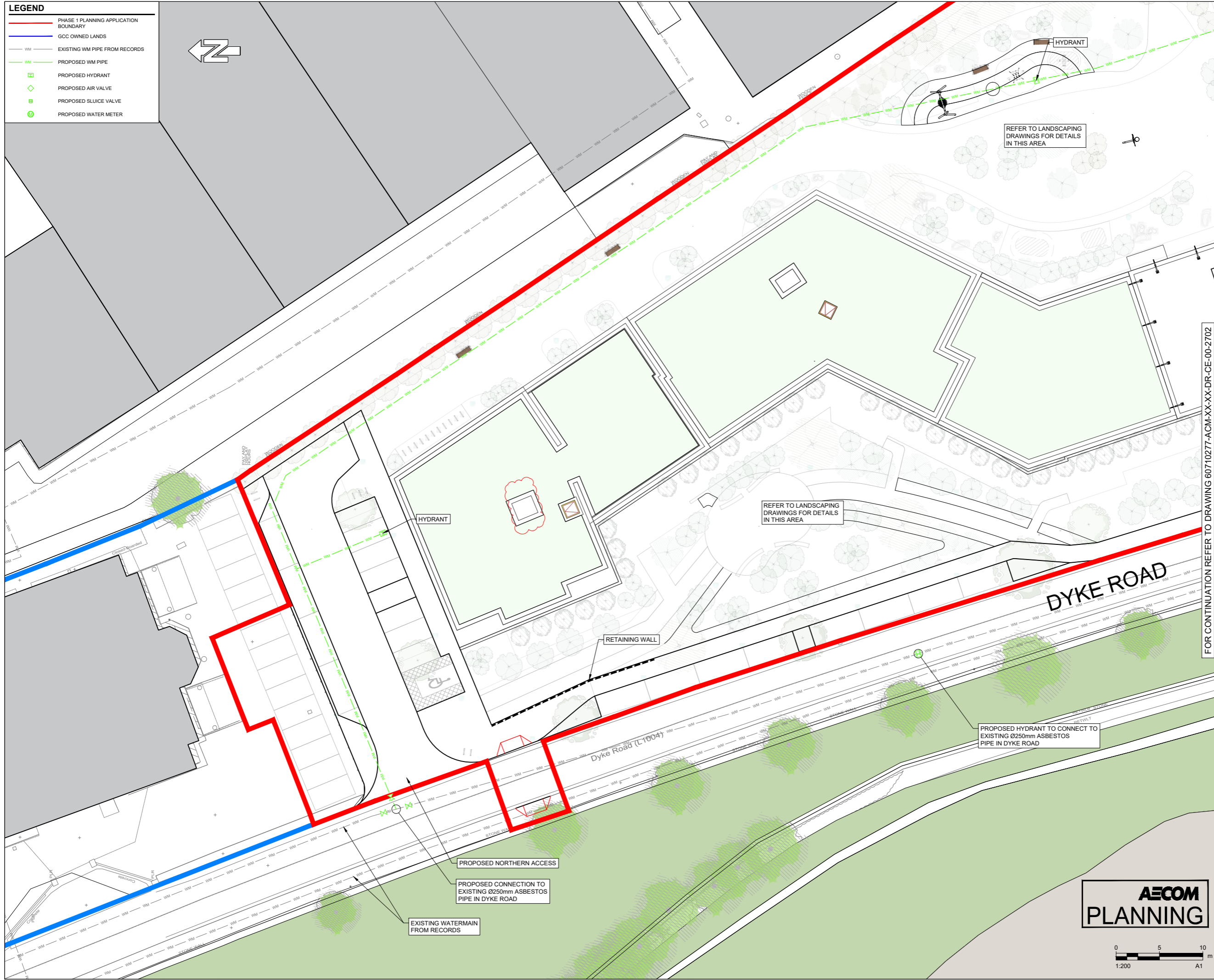
SHEET TITLE

PROPOSED WATERMAIN
KEYPLAN

SHEET NUMBER

60710277-ACM-XX-XX-DR-CE-00-2700





FOR CONTINUATION REFER TO DRAWING 60710277-ACM-XX-XX-DR-CE-00-2702

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PROJECT

Phase 1 Corrib Causeway Dyke Road, Galway

CLIENT

Land Development Agency (LDA) / GCC

CONSULTANT

AECOM
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- SLUICE VALVE DETAILS SHALL BE IN ACCORDANCE WITH UISCÉ EIREANN STANDARD DETAIL STD-W-15.
- ON-LINE AND OFF-LINE HYDRANT DETAILS SHALL BE IN ACCORDANCE WITH UISCÉ EIREANN STANDARD DETAIL STD-W-18 & STD-W-19 RESPECTIVELY.
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- SCOUR VALVE AND CHAMBER DETAILS SHALL BE IN ACCORDANCE WITH UISCÉ EIREANN STANDARD DETAIL STD-W-30.

PURPOSE

P3 PLANNING

ISSUE/REVISION

NO	DATE	DESCRIPTION
01	05.07.2024	FOR PLANNING
IR	DATE	DESCRIPTION

PROJECT NUMBER

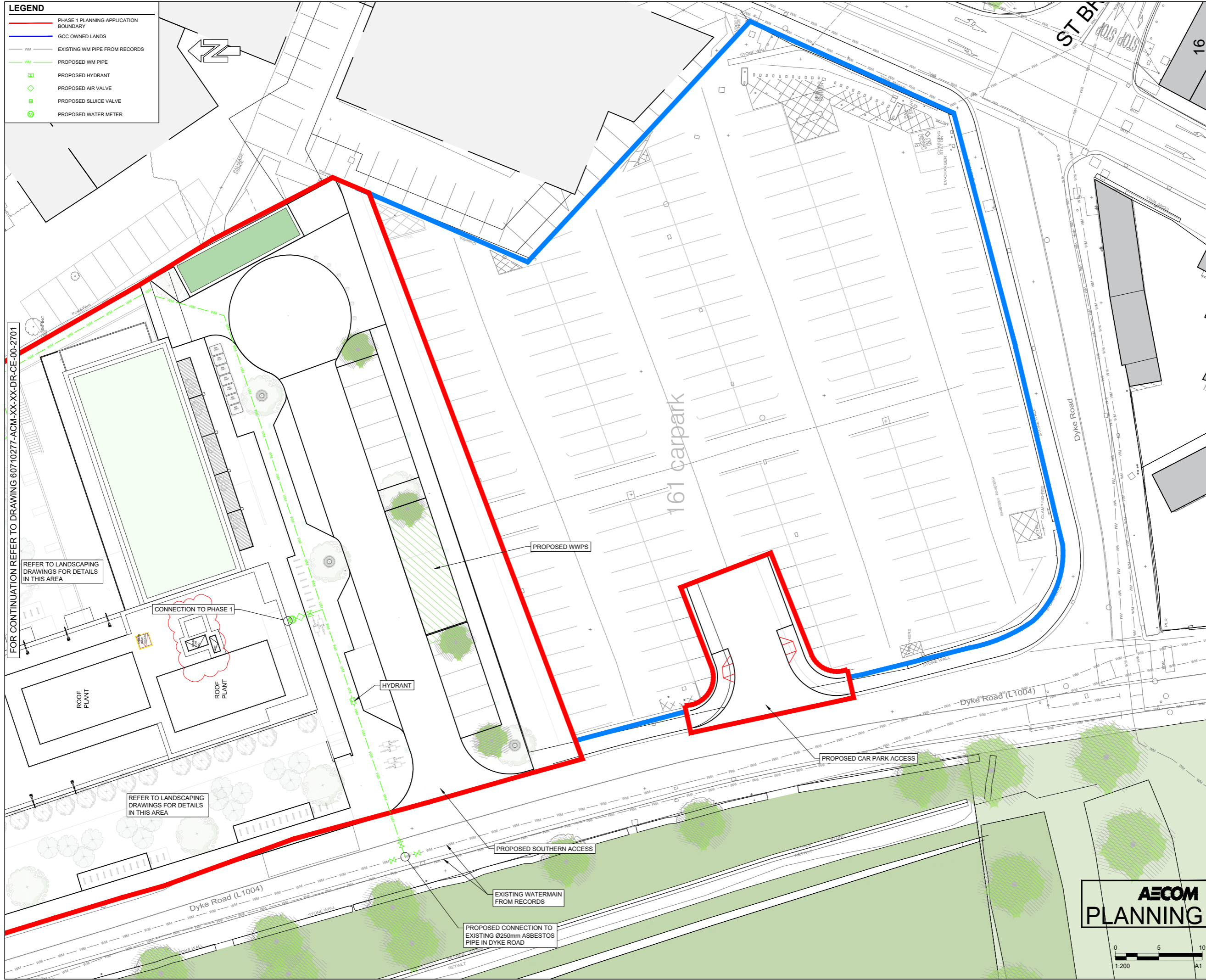
60710277

SHEET TITLE

PROPOSED WATERMAIN
(SHEET 01)

SHEET NUMBER

60710277-ACM-XX-XX-DR-CE-00-2701



LEGEND

- PHASE 1 PLANNING APPLICATION BOUNDARY
- GCC OWNED LANDS
- EXISTING WM PIPE FROM RECORDS
- PROPOSED WM PIPE
- PROPOSED HYDRANT
- PROPOSED AIR VALVE
- PROPOSED SLUICE VALVE
- PROPOSED WATER METER

FOR CONTINUATION REFER TO DRAWING 60710277-ACM-XX-XX-DR-CE-00-2701

REFER TO LANDSCAPING DRAWINGS FOR DETAILS IN THIS AREA

CONNECTION TO PHASE 1

REFER TO LANDSCAPING DRAWINGS FOR DETAILS IN THIS AREA

PROPOSED WWPS

HYDRANT

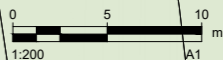
PROPOSED SOUTHERN ACCESS

EXISTING WATERMAIN FROM RECORDS

PROPOSED CONNECTION TO EXISTING Ø250mm ASBESTOS PIPE IN DYKE ROAD

PROPOSED CAR PARK ACCESS

AECOM
PLANNING



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PROJECT

Phase 1
Corrib Causeway
Dyke Road, Galway

CLIENT

Land Development
Agency (LDA) / GCC

CONSULTANT

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PURPOSE

P3 PLANNING

ISSUE/REVISION

IR	DATE	DESCRIPTION
01	05.07.2024	FOR PLANNING

PROJECT NUMBER

60710277

SHEET TITLE

PROPOSED WATERMAIN
(SHEET 02)

SHEET NUMBER

60710277-ACM-XX-XX-DR-CE-00-2702

Wastewater Network

Please note, the existing Black Box pumping station would require upgrading to cater for the wastewater loading and to provide adequate emergency storage for the proposed development.

Alternatively, a new Pump Station could be constructed by the customer within their own site, the new pump station (WWPS) should be designed to cater for the proposed development wastewater loading. The existing Black Box pumping station shall be decommissioned with this option, and the existing flows diverted to the new WWPS. The customer will be responsible for designing the new WWPS required for the proposed development and the diverted flows that currently discharge to the existing Black Box Pumping station.

A 20m Approx. foul sewer network upgrade from 150mm diameter to 225mm diameter minimum is required to cater the proposed development at the start of Dyke Road and Wood quay. Any such network upgrade would have to be funded by the customer.

The developer should investigate the separation of storm water and foul on their development lands and that any existing storm water which is entering into the Uisce Eireann (UE) combined system will need to be eliminated. There should be no storm water discharge to the UE network.

Please note that according to our records there is an existing sewer running through this site (see drawing below). Any structures or works over or in close proximity to Uisce Eireann infrastructure that will inhibit access for maintenance or endanger structural or functional integrity of the infrastructure are not allowed. The layout of the development must ensure that this pipe is protected and adequate separation distances are provided between Uisce Eireann infrastructure and any structures on site. Alternatively you may enter into a diversion agreement with Uisce Eireann and divert the pipe to

accommodate your development. If you wish to proceed with this option please contact Uisce Éireann at Diversions@water.ie and submit detailed design drawings before submitting your planning application. It will be necessary to provide a wayleave over this pipe to the benefit of Uisce Éireann and ensure that it is accessible for maintenance. For more information, please see go to the link below: <https://www.water.ie/connections/developer-services/diversions/>

This letter does not constitute an offer, in whole or in part, to provide a connection to any Uisce Éireann infrastructure. Before the Development can be connected to our network(s) you must submit a connection application and be granted and sign a connection agreement with Uisce Éireann.

As the network capacity changes constantly, this review is only valid at the time of its completion. As soon as planning permission has been granted for the Development, a completed connection application should be submitted. The connection application is available at www.water.ie/connections/get-connected/

Where can you find more information?

- **Section A** - What is important to know?
- **Section B** - Details of Uisce Éireann's Network(s)

This letter is issued to provide information about the current feasibility of the proposed connection(s) to Uisce Éireann's network(s). This is not a connection offer and capacity in Uisce Éireann's network(s) may only be secured by entering into a connection agreement with Uisce Éireann.

For any further information, visit www.water.ie/connections, email newconnections@water.ie or contact 1800 278 278.

Yours sincerely,



Dermot Phelan
Connections Delivery Manager

Section A - What is important to know?

What is important to know?	Why is this important?
Do you need a contract to connect?	<ul style="list-style-type: none"> • Yes, a contract is required to connect. This letter does not constitute a contract or an offer in whole or in part to provide a connection to Uisce Éireann's network(s). • Before the Development can connect to Uisce Éireann's network(s), you must submit a connection application <u>and be granted and sign</u> a connection agreement with Uisce Éireann.
When should I submit a Connection Application?	<ul style="list-style-type: none"> • A connection application should only be submitted after planning permission has been granted.
Where can I find information on connection charges?	<ul style="list-style-type: none"> • Uisce Éireann connection charges can be found at: https://www.water.ie/connections/information/charges/
Who will carry out the connection work?	<ul style="list-style-type: none"> • All works to Uisce Éireann's network(s), including works in the public space, must be carried out by Uisce Éireann*. <p>*Where a Developer has been granted specific permission and has been issued a connection offer for Self-Lay in the Public Road/Area, they may complete the relevant connection works</p>
Fire flow Requirements	<ul style="list-style-type: none"> • The Confirmation of Feasibility does not extend to fire flow requirements for the Development. Fire flow requirements are a matter for the Developer to determine. • What to do? - Contact the relevant Local Fire Authority
Plan for disposal of storm water	<ul style="list-style-type: none"> • The Confirmation of Feasibility does not extend to the management or disposal of storm water or ground waters. • What to do? - Contact the relevant Local Authority to discuss the management or disposal of proposed storm water or ground water discharges.
Where do I find details of Uisce Éireann's network(s)?	<ul style="list-style-type: none"> • Requests for maps showing Uisce Éireann's network(s) can be submitted to: datarequests@water.ie

<p>What are the design requirements for the connection(s)?</p>	<ul style="list-style-type: none"> The design and construction of the Water & Wastewater pipes and related infrastructure to be installed in this Development shall comply with <i>the Uisce Éireann Connections and Developer Services Standard Details and Codes of Practice</i>, available at www.water.ie/connections
<p>Trade Effluent Licensing</p>	<ul style="list-style-type: none"> Any person discharging trade effluent** to a sewer, must have a Trade Effluent Licence issued pursuant to section 16 of the Local Government (Water Pollution) Act, 1977 (as amended). More information and an application form for a Trade Effluent License can be found at the following link: https://www.water.ie/business/trade-effluent/about/ <p>**trade effluent is defined in the Local Government (Water Pollution) Act, 1977 (as amended)</p>

Section B – Details of Uisce Éireann’s Network(s)

The map included below outlines the current Uisce Éireann infrastructure adjacent the Development: To access Uisce Éireann Maps email

datarequests@water.ie



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Note: The information provided on the included maps as to the position of Uisce Éireann’s underground network(s) is provided as a general guide only. The information is based on the best available information provided by each Local Authority in Ireland to Uisce Éireann.

Whilst every care has been taken in respect of the information on Uisce Éireann’s network(s), Uisce Éireann assumes no responsibility for and gives no guarantees, undertakings or warranties concerning the accuracy, completeness or up to date nature of the information provided, nor does it accept any liability whatsoever arising from or out of any errors or omissions. This information

should not be solely relied upon in the event of excavations or any other works being carried out in the vicinity of Uisce Éireann's underground network(s). The onus is on the parties carrying out excavations or any other works to ensure the exact location of Uisce Éireann's underground network(s) is identified prior to excavations or any other works being carried out. Service connection pipes are not generally shown but their presence should be anticipated.

Emma McKendrick,
AECOM,
Block 6,
Galway Technology Park,
Parkmore,
Galway

5 December 2024

Uisce Éireann
Teach Colvill
24-26 Sráid Thalbóid
Baile Átha Cliath 1
D01 NP86
Éire

Uisce Éireann
Colvill House
24-26 Talbot Street
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D01 NP86
Ireland

T: +353 1 89 25000
F: +353 1 89 25001
www.water.ie

Dear Emma,

Re: Diversion Reference DIV24324 Diversion enquiry. Subject to contract | Contract denied

Uisce Éireann has reviewed your enquiry in relation to a diversion of Uisce Éireann's 225mm uPVC sewer as part of the proposed Development at Dyke Road Carpark, Galway as indicated on drawing no. 60710277-ACM-XX-XX-DR-CE-00-0501 Rev02 and 60710277-ACM-XX-XX-DR-CE-00-05 Rev 01.

Based upon the details you have provided with your enquiry and as assessed by Uisce Éireann, we wish to advise you that, subject to valid agreement/s being put in place, the proposed diversion can be facilitated.

A deed of easement will be required over the diverted sewer.

You are advised that this correspondence does not constitute an agreement in whole or in part to provide a diversion or to build near any Uisce Éireann infrastructure and is provided subject to diversion agreement being executed at a later date. You are advised to make contact with the diversions team at diversions@water.ie once planning permission has been granted and prior to any works commencing on site in order to enter into a diversion agreement with Uisce Éireann Water.

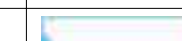
If you have any further questions, please contact Niall Byrne from the diversions team on 087 165 7337 or email niall.byrne@water.ie. For further information, visit www.water.ie/connections.

Yours sincerely,














Dermot Phelan
Connections Delivery Manager

Appendix B Surface Water Drainage Calculations

AECOM		Page 2
Midpoint	60710277 LDA Dyke Road Galway	
Alencon Link	Surafce Water Network 1	
Basingstoke, RG21 7PP	Hydraulic Design	
Date 16/12/2024 15:42	Designed by Thorne Prophet	
File Storm Water Network.MDX	Checked by Emma McKendrick	
Innovyze	Network 2020.1.3	

Manhole Schedules for Storm 1

MH Name	MH CL (m)	MH Depth (m)	MH Connection	MH Diam., L*W (mm)	PN	Pipe Out Invert Level (m)	Diameter (mm)	PN	Pipes In Invert Level (m)	Diameter (mm)	Backdrop (mm)
S1 1-0	5.071	0.771	Open Manhole	1500	S1 1.000	4.300	300				
S1 1-1	5.027	0.849	Open Manhole	1350	S1 1.001	4.178	300	S1 1.000	4.178	300	
S1 1-2	5.000	0.942	Open Manhole	1350	S1 1.002	4.058	300	S1 1.001	4.058	300	
S1 1-3	5.000	1.067	Open Manhole	1350	S1 1.003	3.933	300	S1 1.002	3.933	300	
S1 1-4	5.004	1.600	Open Manhole	1500	S1 1.004	3.404	-1	S1 1.003	3.901	300	
S1 2-0	5.000	1.300	Open Manhole	1200	S1 2.000	3.700	225				
S1 2-1	5.306	1.728	Open Manhole	1200	S1 2.001	3.578	225	S1 2.000	3.578	225	
S1 2-2	5.209	1.698	Open Manhole	1200	S1 2.002	3.511	225	S1 2.001	3.511	225	
S1 1-5	5.186	1.811	Open Manhole	1500	S1 1.005	3.375	225	S1 1.004	3.375	-1	
								S1 2.002	3.496	225	121
S1 1-6	5.108	1.771	Open Manhole	1800	S1 1.006	3.337	225	S1 1.005	3.366	225	29
S1 Ex MH	5.067	1.775	Open Manhole	525		OUTFALL		S1 1.006	3.292	225	

MH Name	Manhole Easting (m)	Manhole Northing (m)	Intersection Easting (m)	Intersection Northing (m)	Manhole Access	Layout (North)
S1 1-0	529879.590	725989.868	529879.590	725989.868	Required	
S1 1-1	529865.991	726010.095	529865.991	726010.095	Required	
S1 1-2	529852.608	726030.003	529852.608	726030.003	Required	
S1 1-3	529842.136	726045.583	529842.136	726045.583	Required	
S1 1-4	529837.643	726043.722	529837.643	726043.722	Required	
S1 2-0	529820.276	726016.828	529820.276	726016.828	Required	
S1 2-1	529814.467	726034.174	529814.467	726034.174	Required	
S1 2-2	529823.777	726038.000	529823.777	726038.000	Required	
S1 1-5	529823.405	726040.206	529823.405	726040.206	Required	
S1 1-6	529822.907	726041.407	529822.907	726041.407	Required	
S1 Ex MH	529816.410	726039.462			No Entry	

AECOM

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Midpoint

Alencon Link

Basingstoke, RG21 7PP

Date 16/12/2024 15:42

File Storm Water Network.MDX

Innovyze

60710277 LDA Dyke Road Galway

Surafce Water Network 1

Hydraulic Design

Designed by Thorne Prophet

Checked by Emma McKendrick

Network 2020.1.3

PIPELINE SCHEDULES for Storm 1

Upstream Manhole

PN	Hyd Sect	Diam (mm)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S1 1.000	o	300	S1 1-0	5.071	4.300	0.471	Open Manhole	1500
S1 1.001	o	300	S1 1-1	5.027	4.178	0.549	Open Manhole	1350
S1 1.002	o	300	S1 1-2	5.000	4.058	0.642	Open Manhole	1350
S1 1.003	o	300	S1 1-3	5.000	3.933	0.767	Open Manhole	1350
S1 1.004	[]	-1	S1 1-4	5.004	3.404	0.800	Open Manhole	1500
S1 2.000	o	225	S1 2-0	5.000	3.700	1.075	Open Manhole	1200
S1 2.001	o	225	S1 2-1	5.306	3.578	1.503	Open Manhole	1200
S1 2.002	o	225	S1 2-2	5.209	3.511	1.473	Open Manhole	1200
S1 1.005	o	225	S1 1-5	5.186	3.375	1.586	Open Manhole	1500
S1 1.006	o	225	S1 1-6	5.108	3.337	1.546	Open Manhole	1800

Downstream Manhole

PN	Length (m)	Slope (1:X)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S1 1.000	24.373	200.0	S1 1-1	5.027	4.178	0.549	Open Manhole	1350
S1 1.001	23.988	199.9	S1 1-2	5.000	4.058	0.642	Open Manhole	1350
S1 1.002	18.773	150.0	S1 1-3	5.000	3.933	0.767	Open Manhole	1350
S1 1.003	4.863	152.0	S1 1-4	5.004	3.901	0.803	Open Manhole	1500
S1 1.004	14.666	500.0	S1 1-5	5.186	3.375	1.011	Open Manhole	1500
S1 2.000	18.293	150.0	S1 2-1	5.306	3.578	1.503	Open Manhole	1200
S1 2.001	10.065	150.0	S1 2-2	5.209	3.511	1.473	Open Manhole	1200
S1 2.002	2.237	150.0	S1 1-5	5.186	3.496	1.465	Open Manhole	1500
S1 1.005	1.300	144.4	S1 1-6	5.108	3.366	1.517	Open Manhole	1800
S1 1.006	6.782	150.0	S1 Ex MH	5.067	3.292	1.550	Open Manhole	525

Free Flowing Outfall Details for Storm 1

Outfall Pipe Number	Outfall Name	C. Level (m)	I. Level (m)	Min I. Level (m)	D,L (mm)	W (mm)
S1 1.006	S1 Ex MH	5.067	3.292	3.220	525	0

Simulation Criteria for Storm 1

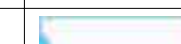
Volumetric Runoff Coeff	0.750	Additional Flow - % of Total Flow	0.000
Areal Reduction Factor	1.000	MADD Factor * 10m³/ha Storage	2.000
Hot Start (mins)	0	Inlet Coefficient	0.800
Hot Start Level (mm)	0	Flow per Person per Day (l/per/day)	0.000
Manhole Headloss Coeff (Global)	0.500	Run Time (mins)	60
Foul Sewage per hectare (l/s)	0.000	Output Interval (mins)	1

Number of Input Hydrographs 0 Number of Offline Controls 0 Number of Time/Area Diagrams 2
Number of Online Controls 1 Number of Storage Structures 1 Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model	FSR	Profile Type	Summer
Return Period (years)	5	Cv (Summer)	0.750
Region	Scotland and Ireland	Cv (Winter)	0.840
M5-60 (mm)	17.100	Storm Duration (mins)	30
Ratio R	0.300		

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AECOM		Page 4
Midpoint	60710277 LDA Dyke Road Galway	
Alencon Link	Surafce Water Network 1	
Basingstoke, RG21 7PP	Hydraulic Design	
Date 16/12/2024 15:42	Designed by Thorne Prophet	
File Storm Water Network.MDX	Checked by Emma McKendrick	
Innovyze	Network 2020.1.3	

Online Controls for Storm 1

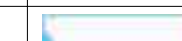
Hydro-Brake® Optimum Manhole: S1 1-6, DS/PN: S1 1.006, Volume (m³): 4.5

Unit Reference	MD-SHE-0139-1000-1450-1000	Sump Available	Yes
Design Head (m)	1.450	Diameter (mm)	139
Design Flow (l/s)	10.0	Invert Level (m)	3.337
Flush-Flo™	Calculated	Minimum Outlet Pipe Diameter (mm)	225
Objective	Minimise upstream storage	Suggested Manhole Diameter (mm)	1200
Application	Surface		

Control Points	Head (m)	Flow (l/s)	Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	1.450	10.0	Kick-Flo®	0.911	8.0
Flush-Flo™	0.430	10.0	Mean Flow over Head Range	-	8.7






The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	5.0	0.800	9.1	2.000	11.6	4.000	16.2	7.000	21.2
0.200	9.1	1.000	8.4	2.200	12.2	4.500	17.1	7.500	21.9
0.300	9.8	1.200	9.1	2.400	12.7	5.000	18.0	8.000	22.6
0.400	10.0	1.400	9.8	2.600	13.2	5.500	18.8	8.500	23.2
0.500	10.0	1.600	10.5	3.000	14.1	6.000	19.6	9.000	23.9
0.600	9.8	1.800	11.1	3.500	15.2	6.500	20.4	9.500	24.5

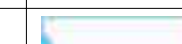
AECOM		Page 2
Midpoint	60710277 LDA Dyke Road Galway	
Alencon Link	Surafce Water Network 2	
Basingstoke, RG21 7PP	Hydraulic Design	
Date 16/12/2024 15:57	Designed by Thorne Prophet	
File Storm Water Network.MDX	Checked by Emma McKendrick	
Innovyze	Network 2020.1.3	

Manhole Schedules for Storm 2

MH Name	MH CL (m)	MH Depth (m)	MH Connection	MH Diam., L*W (mm)	PN	Pipe Out Invert Level (m)	Diameter (mm)	PN	Pipes In Invert Level (m)	Diameter (mm)	Backdrop (mm)
S2 1-0	7.150	1.150	Open Manhole	1200	S2 1.000	6.000	225				
S2 2-0	6.702	1.336	Open Manhole	1200	S2 2.000	5.366	225				
S2 1-1	6.336	2.022	Open Manhole	1200	S2 1.001	4.314	300	S2 1.000	5.194	225	805
								S2 2.000	5.194	225	805
S2 1-2	5.000	0.952	Open Manhole	1350	S2 1.002	4.048	375	S2 1.001	4.123	300	
S2 1-3	5.189	1.198	Open Manhole	1350	S2 1.003	3.991	375	S2 1.002	3.991	375	
S2 1-4	5.000	1.071	Open Manhole	1350	S2 1.004	3.929	375	S2 1.003	3.929	375	
S2 1-5	5.000	1.100	Open Manhole	3000	S2 1.005	3.900	-2	S2 1.004	3.900	375	
S2 1-6	5.000	1.123	Open Manhole	3000	S2 1.006	3.877	225	S2 1.005	3.877	-2	
S2 1-7	5.000	1.131	Open Manhole	1200	S2 1.007	3.869	225	S2 1.006	3.869	225	
S2 Ex MH	5.299	1.480	Open Manhole	600		OUTFALL		S2 1.007	3.819	225	

MH Name	Manhole Easting (m)	Manhole Northing (m)	Intersection Easting (m)	Intersection Northing (m)	Manhole Access	Layout (North)
S2 1-0	529899.110	725910.236	529899.110	725910.236	Required	
S2 2-0	529863.275	725887.424	529863.275	725887.424	Required	
S2 1-1	529860.886	725897.446	529860.886	725897.446	Required	
S2 1-2	529842.822	725951.978	529842.822	725951.978	Required	
S2 1-3	529837.432	725968.135	529837.432	725968.135	Required	
S2 1-4	529844.564	725985.390	529844.564	725985.390	Required	
S2 1-5	529839.705	725992.619	529839.705	725992.619	Required	
S2 1-6	529832.887	726001.592	529832.887	726001.592	Required	
S2 1-7	529831.488	726003.672	529831.488	726003.672	Required	
S2 Ex MH	529824.069	726004.278			No Entry	

AECOM		Page 3																																																																																																																																																																																																																																																
Midpoint Alencon Link Basingstoke, RG21 7PP		60710277 LDA Dyke Road Galway Surafce Water Network 2 Hydraulic Design																																																																																																																																																																																																																																																
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<div>PIPELINE SCHEDULES for Storm 2</div> <div>Upstream Manhole</div> <table><thead><tr><th>PN</th><th>Hyd Sect</th><th>Diam (mm)</th><th>MH Name</th><th>C.Level (m)</th><th>I.Level (m)</th><th>D.Depth (m)</th><th>MH Connection</th><th>MH DIAM., L*W (mm)</th></tr></thead><tbody><tr><td>S2 1.000</td><td>o</td><td>225</td><td>S2 1-0</td><td>7.150</td><td>6.000</td><td>0.925</td><td>Open Manhole</td><td>1200</td></tr><tr><td>S2 2.000</td><td>o</td><td>225</td><td>S2 2-0</td><td>6.702</td><td>5.366</td><td>1.111</td><td>Open Manhole</td><td>1200</td></tr><tr><td>S2 1.001</td><td>o</td><td>300</td><td>S2 1-1</td><td>6.336</td><td>4.314</td><td>1.722</td><td>Open Manhole</td><td>1200</td></tr><tr><td>S2 1.002</td><td>o</td><td>375</td><td>S2 1-2</td><td>5.000</td><td>4.048</td><td>0.577</td><td>Open Manhole</td><td>1350</td></tr><tr><td>S2 1.003</td><td>o</td><td>375</td><td>S2 1-3</td><td>5.189</td><td>3.991</td><td>0.823</td><td>Open Manhole</td><td>1350</td></tr><tr><td>S2 1.004</td><td>o</td><td>375</td><td>S2 1-4</td><td>5.000</td><td>3.929</td><td>0.696</td><td>Open Manhole</td><td>1350</td></tr><tr><td>S2 1.005</td><td>[]</td><td>-2</td><td>S2 1-5</td><td>5.000</td><td>3.900</td><td>0.500</td><td>Open Manhole</td><td>3000</td></tr><tr><td>S2 1.006</td><td>o</td><td>225</td><td>S2 1-6</td><td>5.000</td><td>3.877</td><td>0.898</td><td>Open Manhole</td><td>3000</td></tr><tr><td>S2 1.007</td><td>o</td><td>225</td><td>S2 1-7</td><td>5.000</td><td>3.869</td><td>0.906</td><td>Open Manhole</td><td>1200</td></tr></tbody></table> <div>Downstream Manhole</div> <table><thead><tr><th>PN</th><th>Length (m)</th><th>Slope (1:X)</th><th>MH Name</th><th>C.Level (m)</th><th>I.Level (m)</th><th>D.Depth (m)</th><th>MH Connection</th><th>MH DIAM., L*W (mm)</th></tr></thead><tbody><tr><td>S2 1.000</td><td>40.307</td><td>50.0</td><td>S2 1-1</td><td>6.336</td><td>5.194</td><td>0.917</td><td>Open Manhole</td><td>1200</td></tr><tr><td>S2 2.000</td><td>10.303</td><td>59.9</td><td>S2 1-1</td><td>6.336</td><td>5.194</td><td>0.917</td><td>Open Manhole</td><td>1200</td></tr><tr><td>S2 1.001</td><td>57.447</td><td>300.8</td><td>S2 1-2</td><td>5.000</td><td>4.123</td><td>0.577</td><td>Open Manhole</td><td>1350</td></tr><tr><td>S2 1.002</td><td>17.032</td><td>298.8</td><td>S2 1-3</td><td>5.189</td><td>3.991</td><td>0.823</td><td>Open Manhole</td><td>1350</td></tr><tr><td>S2 1.003</td><td>18.671</td><td>300.0</td><td>S2 1-4</td><td>5.000</td><td>3.929</td><td>0.696</td><td>Open Manhole</td><td>1350</td></tr><tr><td>S2 1.004</td><td>8.710</td><td>300.4</td><td>S2 1-5</td><td>5.000</td><td>3.900</td><td>0.725</td><td>Open Manhole</td><td>3000</td></tr><tr><td>S2 1.005</td><td>11.269</td><td>500.0</td><td>S2 1-6</td><td>5.000</td><td>3.877</td><td>0.523</td><td>Open Manhole</td><td>3000</td></tr><tr><td>S2 1.006</td><td>2.507</td><td>300.0</td><td>S2 1-7</td><td>5.000</td><td>3.869</td><td>0.906</td><td>Open Manhole</td><td>1200</td></tr><tr><td>S2 1.007</td><td>7.444</td><td>150.0</td><td>S2 Ex MH</td><td>5.299</td><td>3.819</td><td>1.255</td><td>Open Manhole</td><td>600</td></tr></tbody></table> <div>Free Flowing Outfall Details for Storm 2</div> <table><thead><tr><th>Outfall Pipe Number</th><th>Outfall Name</th><th>C. Level (m)</th><th>I. Level (m)</th><th>Min I. Level (m)</th><th>D,L (mm)</th><th>W (mm)</th></tr></thead><tbody><tr><td>S2 1.007</td><td>S2 Ex MH</td><td>5.299</td><td>3.819</td><td>0.000</td><td>600</td><td>0</td></tr></tbody></table> <div>Simulation Criteria for Storm 2</div> <table><tbody><tr><td>Volumetric Runoff Coeff</td><td>0.750</td><td>Additional Flow - % of Total Flow</td><td>0.000</td></tr><tr><td>Areal Reduction Factor</td><td>1.000</td><td>MADD Factor * 10m³/ha Storage</td><td>2.000</td></tr><tr><td>Hot Start (mins)</td><td>0</td><td>Inlet Coeffiecient</td><td>0.800</td></tr><tr><td>Hot Start Level (mm)</td><td>0</td><td>Flow per Person per Day (l/per/day)</td><td>0.000</td></tr><tr><td>Manhole Headloss Coeff (Global)</td><td>0.500</td><td>Run Time (mins)</td><td>60</td></tr><tr><td>Foul Sewage per hectare (l/s)</td><td>0.000</td><td>Output Interval (mins)</td><td>1</td></tr></tbody></table> <div>Number of Input Hydrographs 0 Number of Offline Controls 0 Number of Time/Area Diagrams 1 Number of Online Controls 1 Number of Storage Structures 2 Number of Real Time Controls 0</div> <div>Synthetic Rainfall Details</div> <table><tbody><tr><td>Rainfall Model</td><td>FSR</td><td>Profile Type</td><td>Summer</td></tr><tr><td>Return Period (years)</td><td>5</td><td>Cv (Summer)</td><td>0.750</td></tr><tr><td>Region</td><td>Scotland and Ireland</td><td>Cv (Winter)</td><td>0.840</td></tr><tr><td>M5-60 (mm)</td><td>17.100</td><td>Storm Duration (mins)</td><td>30</td></tr><tr><td>Ratio R</td><td>0.300</td><td></td><td></td></tr></tbody></table>			PN	Hyd Sect	Diam (mm)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)	S2 1.000	o	225	S2 1-0	7.150	6.000	0.925	Open Manhole	1200	S2 2.000	o	225	S2 2-0	6.702	5.366	1.111	Open Manhole	1200	S2 1.001	o	300	S2 1-1	6.336	4.314	1.722	Open Manhole	1200	S2 1.002	o	375	S2 1-2	5.000	4.048	0.577	Open Manhole	1350	S2 1.003	o	375	S2 1-3	5.189	3.991	0.823	Open Manhole	1350	S2 1.004	o	375	S2 1-4	5.000	3.929	0.696	Open Manhole	1350	S2 1.005	[]	-2	S2 1-5	5.000	3.900	0.500	Open Manhole	3000	S2 1.006	o	225	S2 1-6	5.000	3.877	0.898	Open Manhole	3000	S2 1.007	o	225	S2 1-7	5.000	3.869	0.906	Open Manhole	1200	PN	Length (m)	Slope (1:X)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)	S2 1.000	40.307	50.0	S2 1-1	6.336	5.194	0.917	Open Manhole	1200	S2 2.000	10.303	59.9	S2 1-1	6.336	5.194	0.917	Open Manhole	1200	S2 1.001	57.447	300.8	S2 1-2	5.000	4.123	0.577	Open Manhole	1350	S2 1.002	17.032	298.8	S2 1-3	5.189	3.991	0.823	Open Manhole	1350	S2 1.003	18.671	300.0	S2 1-4	5.000	3.929	0.696	Open Manhole	1350	S2 1.004	8.710	300.4	S2 1-5	5.000	3.900	0.725	Open Manhole	3000	S2 1.005	11.269	500.0	S2 1-6	5.000	3.877	0.523	Open Manhole	3000	S2 1.006	2.507	300.0	S2 1-7	5.000	3.869	0.906	Open Manhole	1200	S2 1.007	7.444	150.0	S2 Ex MH	5.299	3.819	1.255	Open Manhole	600	Outfall Pipe Number	Outfall Name	C. 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AECOM		Page 4
Midpoint	60710277 LDA Dyke Road Galway	
Alencon Link	Surafce Water Network 2	
Basingstoke, RG21 7PP	Hydraulic Design	
Date 16/12/2024 15:57	Designed by Thorne Prophet	
File Storm Water Network.MDX	Checked by Emma McKendrick	
Innovyze	Network 2020.1.3	

Online Controls for Storm 2

Hydro-Brake® Optimum Manhole: S2 1-7, DS/PN: S2 1.007, Volume (m³): 1.3

Unit Reference	MD-SHE-0176-1500-0950-1500	Sump Available	Yes
Design Head (m)	0.950	Diameter (mm)	176
Design Flow (l/s)	15.0	Invert Level (m)	3.869
Flush-Flo™	Calculated	Minimum Outlet Pipe Diameter (mm)	225
Objective	Minimise upstream storage	Suggested Manhole Diameter (mm)	1200
Application	Surface		

Control Points	Head (m)	Flow (l/s)	Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	0.950	15.0	Kick-Flo®	0.676	12.8
Flush-Flo™	0.313	15.0	Mean Flow over Head Range	-	12.7

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	6.2	0.800	13.8	2.000	21.3	4.000	29.7	7.000	39.0
0.200	14.5	1.000	15.3	2.200	22.3	4.500	31.5	7.500	40.3
0.300	15.0	1.200	16.7	2.400	23.3	5.000	33.1	8.000	41.6
0.400	14.8	1.400	18.0	2.600	24.2	5.500	34.7	8.500	42.8
0.500	14.5	1.600	19.2	3.000	25.9	6.000	36.2	9.000	44.0
0.600	13.9	1.800	20.3	3.500	27.9	6.500	37.6	9.500	45.2

AECOM

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Midpoint

Alencon Link

Basingstoke, RG21 7PP

Date 16/12/2024 15:57

File Storm Water Network.MDX

60710277 LDA Dyke Road Galway

Surafce Water Network 2

Hydraulic Design

Designed by Thorne Prophet

Checked by Emma McKendrick

Innovyze

Network 2020.1.3

Micro Drainage

100 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for Storm 2

Simulation Criteria

Areal Reduction Factor 1.000

Hot Start (mins) 0

Hot Start Level (mm) 0

Manhole Headloss Coeff (Global) 0.500

Foul Sewage per hectare (l/s) 0.000

Additional Flow - % of Total Flow 0.000

MADD Factor * 10m³/ha Storage 2.000

Inlet Coefficient 0.800

Flow per Person per Day (l/per/day) 0.000

Number of Input Hydrographs 0

Number of Offline Controls 0

Number of Time/Area Diagrams 1

Number of Online Controls 1

Number of Storage Structures 2

Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model FSR M5-60 (mm) 17.200

Region Scotland and Ireland

Cv (Summer) 0.750

Ratio R 0.300

Cv (Winter) 0.840

Margin for Flood Risk Warning (mm) 300.0

Analysis Timestep 2.5 Second Increment (Extended)

DTS Status ON

DVD Status OFF

Inertia Status OFF

Profile(s) Summer and Winter

Duration(s) (mins) 15, 30, 60, 120, 180, 240, 360, 480, 600, 720, 960, 1440, 2160, 2880, 4320, 5760, 7200, 8640, 10080

Return Period(s) (years) 1, 30, 100

Climate Change (%) 20, 20, 20

PN

US/MH Name

Storm

Return Period

Climate Change

First (X) Surge

First (Y) Flood

First (Z) Overflow

Overflow Act.

Water Level (m)

Surcharged Depth (m)

Flooded Volume (m³)

Flow / Cap.

S2 1.000

S2 1-0

60 Winter

100

+20%

6.039

-0.186

0.000

0.07

S2 2.000

S2 2-0

15 Summer

100

+20%

5.366

-0.225

0.000

0.00

S2 1.001

S2 1-1

15 Winter

100

+20%

4.436

-0.178

0.000

0.32

S2 1.002

S2 1-2

15 Winter

100

+20%

4.202

-0.221

0.000

0.33

S2 1.003

S2 1-3

15 Winter

100

+20%

4.152

-0.214

0.000

0.32

S2 1.004

S2 1-4

15 Winter

100

+20%

4.110

-0.193

0.000

0.46

S2 1.005

S2 1-5

30 Winter

100

+20%

4.067

-0.433

0.000

0.01

S2 1.006

S2 1-6

30 Winter

100

+20%

4.067

-0.035

0.000

0.52

S2 1.007

S2 1-7

30 Winter

100

+20%

4.062

-0.032

0.000

0.45

PN

US/MH Name

Overflow (l/s)

Half Drain Time (mins)

Pipe Flow (l/s)

Status

Level Exceeded

S2 1.000

S2 1-0

4.9

OK

S2 2.000

S2 2-0

0.0

OK

S2 1.001

S2 1-1

19.6

OK

S2 1.002

S2 1-2

5

30.7

OK

S2 1.003

S2 1-3

30.1

OK

S2 1.004

S2 1-4

9

38.0

OK

S2 1.005

S2 1-5

29.7

OK

S2 1.006

S2 1-6

14.5

OK

S2 1.007

S2 1-7

14.2

OK

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Midpoint

Alencon Link

Basingstoke, RG21 7PP

Date 16/12/2024 16:12

File Storm Water Network.MDX

Innovyze

60710277 LDA Dyke Road Galway

Surafce Water (Diversion)

Hydraulic Design

Designed by Thorne Prophet

Checked by Emma McKendrick

Network 2020.1.3

Micro Drainage

STORM SEWER DESIGN by the Modified Rational Method

Design Criteria for Storm Diversion

Pipe Sizes STANDARD Manhole Sizes STANDARD

FSR Rainfall Model - Scotland and Ireland

Return Period (years)

5

PIMP (%)

100

M5-60 (mm)

17.200

Add Flow / Climate Change (%)

20

Ratio R

0.300

Minimum Backdrop Height (m)

0.200

Maximum Rainfall (mm/hr)

50

Maximum Backdrop Height (m)

1.500

Maximum Time of Concentration (mins)

30

Min Design Depth for Optimisation (m)

1.200

Foul Sewage (l/s/ha)

0.000

Min Vel for Auto Design only (m/s)

1.00

Volumetric Runoff Coeff.

0.750

Min Slope for Optimisation (1:X)

200

Designed with Level Soffits

Network Design Table for Storm Diversion

PN

Length

Fall

Slope

I.Area

T.E.

Base

k

HYD

DIA

Section Type

Auto Design

(m)

(m)

(1:X)

(ha)

(mins)

Flow (l/s)

(mm)

SECT

(mm)

D1.000

17.805

0.079

225.4

0.000

4.00


11.0

0.600

o

225

Pipe/Conduit



D1.001

80.189

0.356

225.0

0.000

0.00


0.0

0.600

o

225

Pipe/Conduit



D1.002

19.124

0.085

225.0

0.000

0.00


0.0

0.600

o

225

Pipe/Conduit



D2.000

13.864

0.164

84.5

0.360

4.00


0.0

0.600

o

225

Pipe/Conduit



D1.003

6.532

0.048

137.5

0.000

0.00


0.0

0.600

o

300

Pipe/Conduit



D3.000

14.067

0.056

250.0

0.000

4.00


11.0

0.600

o

450

Pipe/Conduit



D3.001

59.020

0.295

200.0

0.000

0.00


0.0

0.600

o

450

Pipe/Conduit



D3.002

9.749

0.049

200.0

0.000

0.00


0.0

0.600

o

450

Pipe/Conduit



D3.003

37.209

0.248

150.0

0.000

0.00


0.0

0.600

o

450

Pipe/Conduit



D3.004

35.667

0.224

159.0

0.000

0.00


0.0

0.600

o

600

Pipe/Conduit



Network Results Table

PN

Rain

T.C.

US/IL

E I.Area

E Base

Foul

Add Flow

Vel

Cap

Flow

(mm/hr)

(mins)

(m)

(ha)

Flow (l/s)

(l/s)

(l/s)

(m/s)

(l/s)

(l/s)

D1.000

50.00

4.34

3.852

0.000

11.0

0.0

2.2

0.87

34.5

13.2

D1.001

50.00

5.88

3.773

0.000

11.0

0.0

2.2

0.87

34.5

13.2

D1.002

50.00

6.25

3.417

0.000

11.0

0.0

2.2

0.87

34.5

13.2

D2.000

50.00

4.16

3.496

0.000

25.0

0.0

4.2

1.42

56.6

25.0

D1.003

50.00

6.33

3.257

0.000

36.0

0.0

7.2

1.34

94.6

43.2

D3.000

50.00

4.18

4.242

0.000

11.0

0.0

2.2

1.28

203.8

13.2

D3.001

50.00

4.87

4.186

0.000

11.0

0.0

2.2

1.43

228.1

13.2

D3.002

50.00

4.98

3.891

0.000

11.0

0.0

2.2

1.43

228.1

13.2

D3.003

50.00

5.36

3.842

0.000

11.0

0.0

2.2

1.66

263.6

13.2

D3.004

50.00

4.31

3.444

0.000

15.0

0.0

2.5

1.93

545.3

15.0

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AECOM		Page 3
Midpoint Alencon Link Basingstoke, RG21 7PP		60710277 LDA Dyke Road Galway Surafce Water (Diversion) Hydraulic Design
Date 16/12/2024 16:12 File Storm Water Network.MDX		Designed by Thorne Prophet Checked by Emma McKendrick
Innovyze		Network 2020.1.3

PIPELINE SCHEDULES for Storm Diversion

Upstream Manhole

PN	Hyd Sect	Diam (mm)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM. , L*W (mm)
D1.000	o	225	D1-0	5.330	3.852	1.253	Open Manhole	1200
D1.001	o	225	D1-1	5.114	3.773	1.116	Open Manhole	1200
D1.002	o	225	D1-2	5.000	3.417	1.358	Open Manhole	1200
D2.000	o	225	D2-0	4.977	3.496	1.256	Open Manhole	1200
D1.003	o	300	D1-3	5.000	3.257	1.443	Open Manhole	1200
D3.000	o	450	D3-0	6.330	4.242	1.638	Open Manhole	1350
D3.001	o	450	D3-1	6.278	4.186	1.642	Open Manhole	1350
D3.002	o	450	D3-2	6.001	3.891	1.660	Open Manhole	1350
D3.003	o	450	D3-3	5.886	3.842	1.594	Open Manhole	1350
D3.004	o	600	D3-4	5.131	3.444	1.087	Open Manhole	1500

Downstream Manhole

PN	Length (m)	Slope (1:X)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM. , L*W (mm)
D1.000	17.805	225.4	D1-1	5.114	3.773	1.116	Open Manhole	1200
D1.001	80.189	225.0	D1-2	5.000	3.417	1.358	Open Manhole	1200
D1.002	19.124	225.0	D1-3	5.000	3.332	1.443	Open Manhole	1200
D2.000	13.864	84.5	D1-3	5.000	3.332	1.443	Open Manhole	1200
D1.003	6.532	137.5	D	5.000	3.209	1.491	Open Manhole	525
D3.000	14.067	250.0	D3-1	6.278	4.186	1.642	Open Manhole	1350
D3.001	59.020	200.0	D3-2	6.001	3.891	1.660	Open Manhole	1350
D3.002	9.749	200.0	D3-3	5.886	3.842	1.594	Open Manhole	1350
D3.003	37.209	150.0	D3-4	5.131	3.594	1.087	Open Manhole	1500
D3.004	35.667	159.0	D	5.002	3.220	1.182	Open Manhole	600


Free Flowing Outfall Details for Storm Diversion

Outfall Pipe Number	Outfall Name	C. Level (m)	I. Level (m)	Min I. Level (m)	D,L (mm)	W (mm)
D1.003	D	5.000	3.209	3.209	525	0

Free Flowing Outfall Details for Storm Diversion

Outfall Pipe Number	Outfall Name	C. Level (m)	I. Level (m)	Min I. Level (m)	D,L (mm)	W (mm)
D3.004	D	5.002	3.220	3.220	600	0

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AECOM		Page 4
Midpoint	60710277 LDA Dyke Road Galway	
Alencon Link	Surafce Water (Diversion)	
Basingstoke, RG21 7PP	Hydraulic Design	
Date 16/12/2024 16:12	Designed by Thorne Prophet	
File Storm Water Network.MDX	Checked by Emma McKendrick	
Innovyze	Network 2020.1.3	
<p align="center"><u>Simulation Criteria for Storm Diversion</u></p> <p> Volumetric Runoff Coeff 0.750 Additional Flow - % of Total Flow 0.000 Areal Reduction Factor 1.000 MADD Factor * 10m³/ha Storage 2.000 Hot Start (mins) 0 Inlet Coeffiecient 0.800 Hot Start Level (mm) 0 Flow per Person per Day (l/per/day) 0.000 Manhole Headloss Coeff (Global) 0.500 Run Time (mins) 60 Foul Sewage per hectare (l/s) 0.000 Output Interval (mins) 1 </p> <p> Number of Input Hydrographs 0 Number of Offline Controls 0 Number of Time/Area Diagrams 0 Number of Online Controls 0 Number of Storage Structures 0 Number of Real Time Controls 0 </p> <p align="center"><u>Synthetic Rainfall Details</u></p> <p> Rainfall Model FSR Profile Type Summer Return Period (years) 5 Cv (Summer) 0.750 Region Scotland and Ireland Cv (Winter) 0.840 M5-60 (mm) 17.100 Storm Duration (mins) 30 Ratio R 0.300 </p>		
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AECOM

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Midpoint

Alencon Link

Basingstoke, RG21 7PP

Date 16/12/2024 16:12

File Storm Water Network.MDX

60710277 LDA Dyke Road Galway

Surafce Water (Diversion)

Hydraulic Design

Designed by Thorne Prophet

Checked by Emma McKendrick

Innovyze

Network 2020.1.3

Micro Drainage

100 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for Storm Diversion

Simulation Criteria

Areal Reduction Factor 1.000

Hot Start (mins) 0

Hot Start Level (mm) 0

Manhole Headloss Coeff (Global) 0.500

Foul Sewage per hectare (l/s) 0.000

Additional Flow - % of Total Flow 0.000

MADD Factor * 10m³/ha Storage 2.000

Inlet Coefficient 0.800

Flow per Person per Day (l/per/day) 0.000

Number of Input Hydrographs 0

Number of Offline Controls 0

Number of Online Controls 0

Number of Time/Area Diagrams 0

Number of Storage Structures 0

Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model FSR M5-60 (mm) 17.200

Region Scotland and Ireland

Cv (Summer) 0.750

Ratio R 0.300

Cv (Winter) 0.840

Margin for Flood Risk Warning (mm) 300.0

Analysis Timestep 2.5 Second Increment (Extended)

DTS Status ON

DVD Status OFF

Inertia Status OFF

Profile(s) Summer and Winter

Duration(s) (mins) 15, 30, 60, 120, 180, 240, 360, 480, 600, 720, 960, 1440, 2160, 2880, 4320, 5760, 7200, 8640, 10080

Return Period(s) (years) 1, 30, 100

Climate Change (%) 20, 20, 20

PN	US/MH Name	Storm	Return Period	Climate Change	First (X) Surge	First (Y) Flood	First (Z) Overflow	Overflow Act.	Water Level (m)	Surcharged Depth (m)	Flooded Volume (m³)	Flow / Cap.
D1.000	D1-0	15 Summer	100	+20%					3.945	-0.132	0.000	0.36
D1.001	D1-1	15 Winter	100	+20%					3.875	-0.123	0.000	0.33
D1.002	D1-2	15 Winter	100	+20%	30/15 Summer				3.826	0.184	0.000	0.72
D2.000	D2-0	15 Winter	100	+20%	1/15 Summer	100/15 Summer			4.978	1.257	0.896	2.69
D1.003	D1-3	15 Winter	100	+20%	30/15 Summer				3.802	0.245	0.000	2.27
D3.000	D3-0	15 Summer	100	+20%					4.321	-0.371	0.000	0.07
D3.001	D3-1	15 Summer	100	+20%					4.252	-0.384	0.000	0.05
D3.002	D3-2	15 Summer	100	+20%					3.968	-0.372	0.000	0.07
D3.003	D3-3	15 Summer	100	+20%					3.905	-0.387	0.000	0.05
D3.004	D3-4	15 Summer	100	+20%					3.507	-0.537	0.000	0.02

PN	US/MH Name	Overflow (l/s)	Half Drain Time (mins)	Pipe Flow (l/s)	Status	Level Exceeded
D1.000	D1-0			11.0	OK	
D1.001	D1-1			11.2	OK	
D1.002	D1-2			22.4	SURCHARGED	
D2.000	D2-0			132.6	FLOOD	2
D1.003	D1-3			139.4	SURCHARGED	
D3.000	D3-0			11.0	OK	
D3.001	D3-1			11.0	OK	
D3.002	D3-2			11.0	OK	
D3.003	D3-3			11.1	OK	
D3.004	D3-4			11.0	OK	

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Appendix C Wastewater Drainage Calculations

AECOM

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Midpoint

Alencon Link

Basingstoke, RG21 7PP

Date 16/12/2024 16:16

File Storm Water Network.MDX

Innovyze

60710277 LDA Dyke Road Galway

Foul Network 1

Hydraulic Design

Designed by Thorne Prophet

Checked by Emma McKendrick

Network 2020.1.3


Micro Drainage

Manhole Schedules for Foul

MH Name	MH CL (m)	MH Depth (m)	MH Connection	MH Diam., L*W (mm)	PN	Pipe Out Invert Level (m)	Diameter (mm)	PN	Pipes In Invert Level (m)	Diameter (mm)	Backdrop (mm)
F1-0	4.953	0.803	Open Manhole	1200	F1.000	4.150	225				
F1-1	5.000	1.008	Open Manhole	1200	F1.001	3.992	225	F1.000	3.992	225	
F1-2	5.805	2.033	Open Manhole	1200	F1.002	3.772	225	F1.001	3.772	225	
F1-3	5.478	2.043	Open Manhole	1200	F1.003	3.435	225	F1.002	3.435	225	
F1-4	5.589	2.373	Open Manhole	1200	F1.004	3.216	225	F1.003	3.216	225	
F2-0	6.164	3.083	Open Manhole	1200	F2.000	3.081	225				
F1-5	6.261	3.264	Open Manhole	1200	F1.005	2.997	225	F1.004	2.997	225	
								F2.000	2.997	225	
F3-0	6.986	1.486	Open Manhole	1200	F3.000	5.500	150				
F3-1	6.954	1.594	Open Manhole	1200	F3.001	5.360	150	F3.000	5.360	150	
F3-2	6.817	1.675	Open Manhole	1200	F3.002	5.142	150	F3.001	5.142	150	
F1-6	6.689	3.761	Open Manhole	1200	F1.006	2.928	225	F1.005	2.928	225	
								F3.002	5.049	150	2046
FWWPS	6.689	3.830	Open Manhole	0		OUTFALL		F1.006	2.859	225	

MH Name	Manhole Easting (m)	Manhole Northing (m)	Intersection Easting (m)	Intersection Northing (m)	Manhole Access	Layout (North)
F1-0	529820.352	726061.058	529820.352	726061.058	Required	
F1-1	529810.592	726039.502	529810.592	726039.502	Required	
F1-2	529820.962	726008.251	529820.962	726008.251	Required	
F1-3	529836.889	725960.245	529836.889	725960.245	Required	
F1-4	529846.896	725930.087	529846.896	725930.087	Required	
F2-0	529853.728	725889.749	529853.728	725889.749	Required	
F1-5	529858.987	725893.643	529858.987	725893.643	Required	
F3-0	529889.491	725896.046	529889.491	725896.046	Required	
F3-1	529887.143	725903.065	529887.143	725903.065	Required	
F3-2	529881.148	725901.059	529881.148	725901.059	Required	
F1-6	529875.529	725899.178	529875.529	725899.178	Required	
FWWPS	529876.164	725897.282			No Entry	

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AECOM		Page 3
Midpoint	60710277 LDA Dyke Road Galway	
Alencon Link	Foul Network 1	
Basingstoke, RG21 7PP	Hydraulic Design	
Date 16/12/2024 16:16	Designed by Thorne Prophet	
File Storm Water Network.MDX	Checked by Emma McKendrick	
Innovyze	Network 2020.1.3	

PIPELINE SCHEDULES for Foul

Upstream Manhole

PN	Hyd Sect	Diam (mm)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
F1.000	o	225	F1-0	4.953	4.150	0.578	Open Manhole	1200
F1.001	o	225	F1-1	5.000	3.992	0.783	Open Manhole	1200
F1.002	o	225	F1-2	5.805	3.772	1.808	Open Manhole	1200
F1.003	o	225	F1-3	5.478	3.435	1.818	Open Manhole	1200
F1.004	o	225	F1-4	5.589	3.216	2.148	Open Manhole	1200
F2.000	o	225	F2-0	6.164	3.081	2.858	Open Manhole	1200
F1.005	o	225	F1-5	6.261	2.997	3.039	Open Manhole	1200
F3.000	o	150	F3-0	6.986	5.500	1.336	Open Manhole	1200
F3.001	o	150	F3-1	6.954	5.360	1.444	Open Manhole	1200
F3.002	o	150	F3-2	6.817	5.142	1.525	Open Manhole	1200
F1.006	o	225	F1-6	6.689	2.928	3.536	Open Manhole	1200

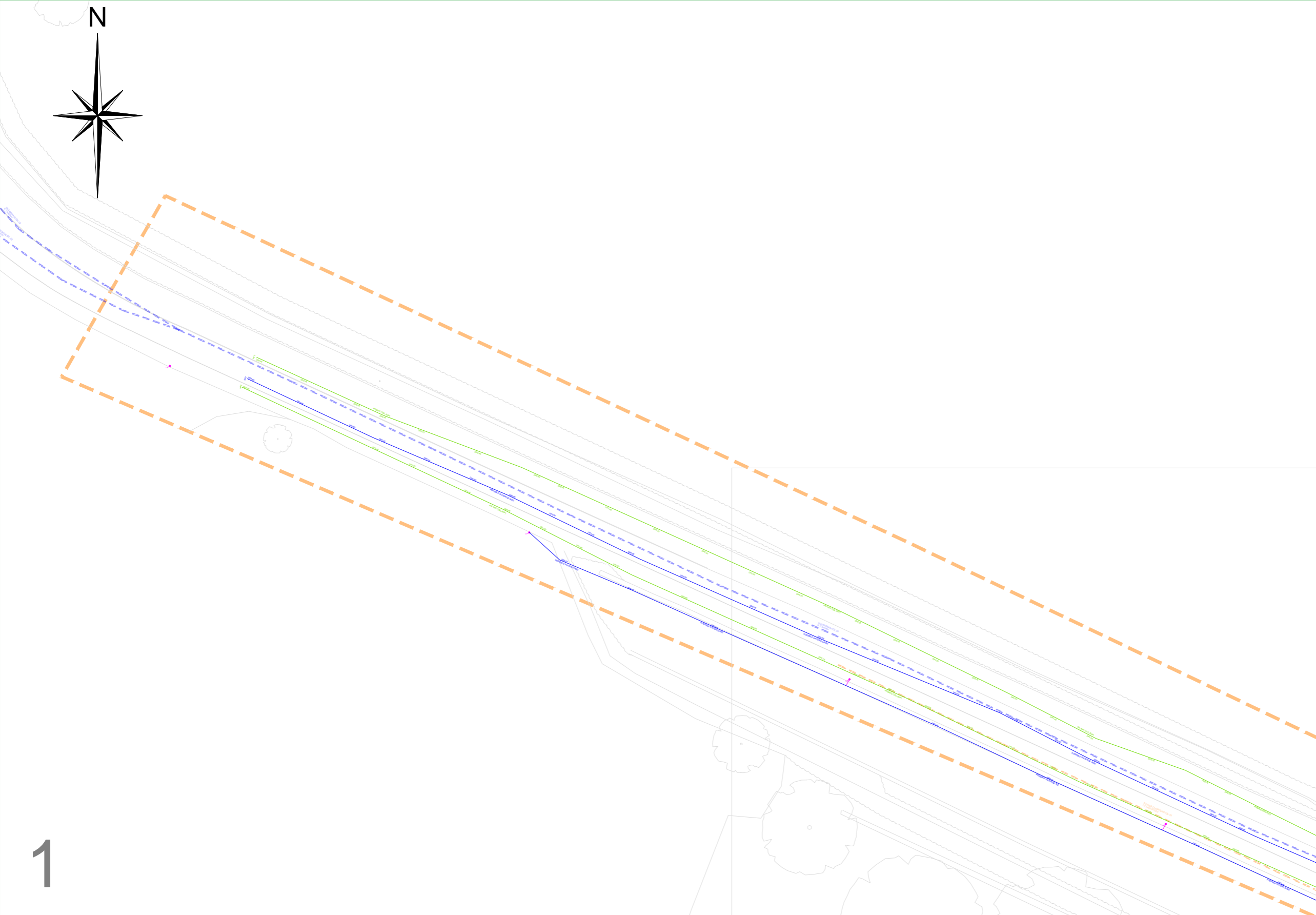
Downstream Manhole

PN	Length (m)	Slope (1:X)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
F1.000	23.663	149.8	F1-1	5.000	3.992	0.783	Open Manhole	1200
F1.001	32.927	149.7	F1-2	5.805	3.772	1.808	Open Manhole	1200
F1.002	50.579	150.1	F1-3	5.478	3.435	1.818	Open Manhole	1200
F1.003	31.775	145.1	F1-4	5.589	3.216	2.148	Open Manhole	1200
F1.004	38.397	175.3	F1-5	6.261	2.997	3.039	Open Manhole	1200
F2.000	6.544	77.9	F1-5	6.261	2.997	3.039	Open Manhole	1200
F1.005	17.443	252.8	F1-6	6.689	2.928	3.536	Open Manhole	1200
F3.000	7.401	52.9	F3-1	6.954	5.360	1.444	Open Manhole	1200
F3.001	6.321	29.0	F3-2	6.817	5.142	1.525	Open Manhole	1200
F3.002	5.925	63.7	F1-6	6.689	5.049	1.490	Open Manhole	1200
F1.006	2.000	29.0	FWWPS	6.689	2.859	3.605	Open Manhole	0

Free Flowing Outfall Details for Foul

Outfall Pipe Number	Outfall Name	C. Level (m)	I. Level (m)	Min I. Level (m)	D,L (mm)	W (mm)
F1.006	FWWPS	6.689	2.859	0.000	0	0

Appendix D Utility Records and Utility Survey



PAS 128: 2014 (Quality of Survey Level Outputs):	
DESKTOP UTILITY RECORDS SEARCH	
QL-D	Drafted from utility records
SITE RECONNAISSANCE	
QL-C	Location Demonstrated by visual reference to street furniture or evidence of previous streetworks, ie - reinstatement scars
DETECTION	
QL-B4	A segment of utility suspected to exist but has not been detected by a geophysical technique
QL-B3	Horizontal location only of the utility detected by one of the geophysical techniques used
QL-B2	Horizontal and vertical location of the utility detected by one of the geophysical techniques used
QL-B1	Horizontal and vertical location of the utility detected by multiple geophysical techniques
VERIFICATION	
QL-A	Horizontal and vertical location of the top and/or bottom of the utility

Apex Surveys Ltd. Disclaimer - Utility Survey

The interpretative nature and the non-intrusive, indirect and non-destructive survey methods must be taken into account when considering the results of the survey. Therefore Apex Surveys, while using appropriate practice to execute, interpret and present the data, gives no guarantees that all underground utilities and underground structures will be located and mapped. Furthermore, Apex Surveys cannot guarantee the accuracy of the utility depths annotated on the survey drawings. Apex Survey shall not be liable for any omissions or inaccuracies in the survey which arise due to the limitations of the service. No liability shall attach to Apex Surveys, in any circumstances, howsoever arising, in respect of any consequential loss or damages suffered by the Client.

The following is a non-exhaustive list of the limitations of utility surveys:

- The Survey aims to map existing utilities subsurface utilities and provide information with respect to pipe size, material type and drainage connectivity. However utility surveying is limited by the following guidelines and it may not be possible to accurately survey, define and locate all services and sub-surface features.
- Depth of Utility: The depth and size of a utility affect the signal response and the degree with which a utility can be located. Due to attenuation of the radar signal with depth, resolution is restricted, hence making identification of utilities more difficult with increasing depth.
- Size of Utility: The smaller the diameter of a utility the more difficult it is to locate. This difficulty increases with depth.
- Ground Conditions: The depth penetration and quality of the data depends on the ground conditions of the site. GPR Surveying works best within high resistivity material. Clay overburden can impair GPR Surveying. Poor data may be a result of areas with high conductivity.
- Utility Congestion: Where different utilities converge together into a service corridor or cross paths it becomes difficult to isolate a specific utility and to map its route. The reflected signal will display a single response to multiple utilities. Therefore multiple utilities may appear to be a single utility. Where similar services run on close proximity, separation may be impossible.
- Signal Jumping: Signal from surrounding services may 'jump' to a highly conductive line masking its true identity.
- Shadowing: (of deeper utilities by shallower objects) Shallow utilities will mask the existence of deeper utilities where they are in close proximity. Also, high reflective materials close to the surface i.e rebar may hide deeper anomalies.
- Surface Obstructions: The GPR system relies on a relatively flat and even surface on which to perform radar passes. If ground obstructions such as vehicles, organic material (long grass, scrub) or undulating ground surface are present then the acquired data will be of lower resolution and in some cases not viable.
- Loss of signal: It is not always possible to trace the entire length of each underground service.
- Connections between manholes: Connections between manhole chambers are assumed to be straight.
- Non-metallic objects: Nonmetallic objects are amongst the most difficult to trace therefore successful tracing of non-metallic pipes/ utilities may be limited.
- Fiber Optic Cables: Fiber optic cables may not be possible to locate except where laid with a built in tracer wire or similar conductor system.
- Defective / flooded manholes or pipework: It may not be possible to establish connections between flooded or defective manholes or pipework.
- Acute bends in pipework: It may not be possible to trace a pipe past an acute bend.

Accuracy estimates:

- Locational accuracy is determined by referring to the manufacturers guidelines for the detector used.
- In ideal conditions the spatial accuracy for the underground utilities may be +/- 5% for Radiodetection and +/- 10% of depth for the GPR to 2.5m deep. However variations within the subsurface, depth below the ground, close proximity of other services and local magnetic, atmospheric or ground conditions, bends, lateral service connections and any of the other limitations listed in this disclaimer may alter this estimated accuracy.
- Plan accuracies of + or - 150mm may be achieved but this figure will depend on the depth of service below ground level. However variations within the subsurface, depth below the ground, close proximity of other services and local magnetic, atmospheric or ground conditions, bends, lateral service connections and any of the other limitations listed in this disclaimer may alter this estimated accuracy.
- DP represents distance from the surface level to the top of the service/ target
- Where technically possible, depth indications will be given. These along with plan positions should be used for guidance only and wherever critical accuracy is required these should be confirmed by the client by undertaking trial excavations or similar.


Record Drawing Information

- Services which have been untraceable are shown from records where possible or available. These lines are annotated as "Taken From Records" or "From Records".
- Existing record information showing underground services is often incomplete and with unknown accuracies therefore it should be regarded as indicative only.
- Where Apex Surveys issue a utility drawing, this should be read in conjunction with all available public or private utility records.
- Apex Surveys endeavor to add relevant Public Utility record information onto the final drawing. However, we would recommend that direct contact is made with the asset owner or statutory undertaker.
- We shall not be held responsible for the accuracy, or otherwise, of the location of a service, as issued by the utility provider and therefore shown as "Taken from Records" on the drawing.

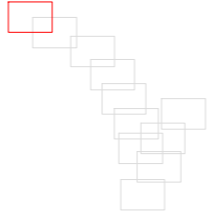
The following have been excluded from the survey:

- Location of individual service feeds to properties or buildings as access would be required into each property to apply direct connections to inlet points and this would significantly increase the scope of works, survey cost and also cause possible disruption to occupants.
- Pot ended or disconnected cables or terminated short lengths of pipe.
- Internal building services.
- Small diameter cables less than 20mm diameter or pipes less than 40mm diameter.
- Above ground services unless specifically requested.
- Lifting manholes which require longer than 10 minutes effort using standard heavy duty apparatus.

All works carried out be Apex Surveys conforms to the guidelines set out by The Survey Association (TSA) and PAS:128 Standard for utility mapping



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info@apexsurveys.ie
00353 1 691 0156

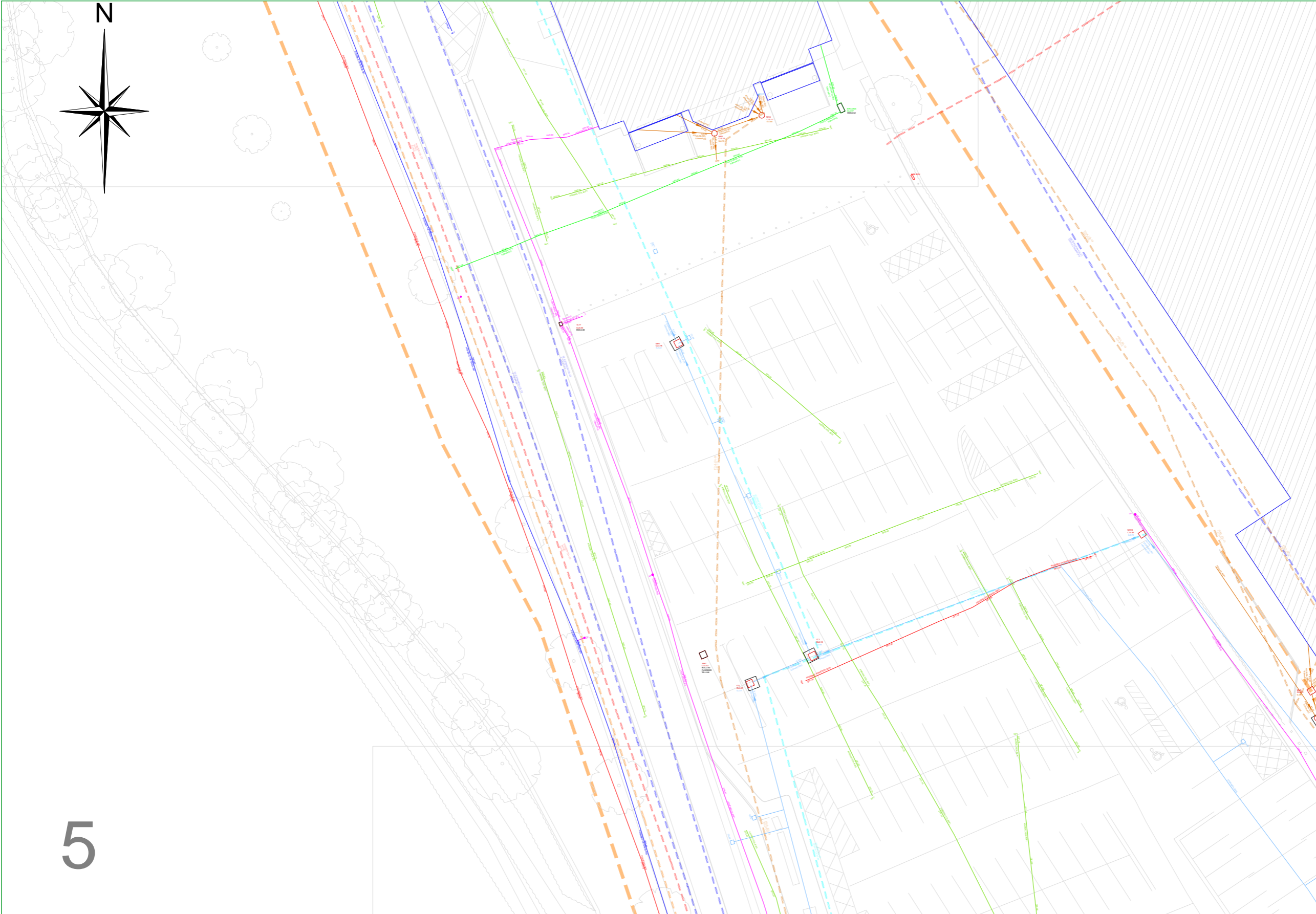
STREET FURNITURE :	
BOLLARDS BUS STOP CRASH BARRIER GATE ELECTRICITY POLE TELEPHONE POLE EARTHING ROD LAMP POST MARKER POST SIGN POST TRAFFIC LIGHT TELEPHONE BOX POST BOX RS-RS ROADSIGN BORE HOLE TRIAL PIT	BD + BS + CB EP + TP + ER + LP + MKR + SIGN TL + TB POST + RS-RS BH + TPIT +
SERVICES :	
AIR VALVE ARMSTRONG JUNCTION CABLE TV IC COVER LEVEL EIRCOM COVER EIRCOM JUNCTION BOX ELECTRIC CABLE PIT ESAT COVER ESB COVER FIRE HYDRANT GAS VALVE GULLY INSPECTION COVER MANHOLE SEPTIC TANK SLUICE VALVE	AV AJ CATV QL EIRCOM EIRCOM BOX ECP ESAT ESB FH GV IC MH SEPTIC SV DP EW NFT O/S
LEVELS :	
BED LEVEL FLOOR LEVEL INVERT LEVEL ROAD LEVEL SOFFIT LEVEL SPOT LEVEL TOP OF WALL LEVEL WATER LEVEL SURVEY CONTROL STATION	+ BED101.50 + FL101.50 + L101.50 + 101.50 + SL101.50 + 101.50 + TOW101.50 + WL101.50
UNDERGROUND LEGEND :	
WATER MAIN GAS MAIN STORM DRAIN POUL SEWER COMBINED SEWER ELECTRIC CABLE ELECTRIC LIGHTING EIRCOM FIBRE OPTIC CABLE BROADBAND CABLE TV TRAFFIC AND SIGNAL CABLE CCTV IRRIGATION PIPE EMPTY DUCT GPR ANOMALY UNKNOWN CABLE O'HEAD ELECTRICITY O'HEAD TELECOM	WATER GAS STORM POUL COMB POWER LIGHTING EIRCOM F.OPTIC BROADBAND TV TRAFFIC CCTV IRRIGATION EMPTY ANOMALY CABLE OR
SHEET LAYOUT :	
	

PLAN PRODUCED BY:		
APEX SURVEYS		
CONTACT INFORMATION:		
Apex Surveys Unit 78 Dunboyne Business Park Dunboyne, Co. Meath, Ireland www.apexsurveys.ie info@apexsurveys.ie 00353 1 691 0156		
CLIENT:		
Aecom		
PROJECT:		
Dyke Road, Galway		
GRID SYSTEM: Irish Transverse Mercator DATUM: Malin Head (OSGM15) NOTES: Drawing Contains Scale Factor		
REVISIONS:		
No.	Date	Description
001	00/00	Original Drawing
002	19/12/23	Anomalies Updated

SCALE :	1/200 A1	DATE :	10/11/2023
DRG No:	5999	DESCRIPTION :	2D Utilities
SHEET:	1 of 11	SURVEYED BY :	K.K.
		PROCESSED BY :	J.P.
		CHECKED BY :	Alan Brady



CHECKED BY : Alan Brady



PAS 128: 2014 (Quality of Survey Level Outputs):	
DESKTOP UTILITY RECORDS SEARCH QL-D	Drafted from utility records
SITE RECONNAISSANCE QL-C	Location Demonstrated by visual reference to street furniture or evidence of previous streetworks, ie - reinstatement scars
DETECTION QL-B4	A segment of utility suspected to exist but has not been detected by a geophysical technique
QL-B3	Horizontal location only of the utility detected by one of the geophysical techniques used
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www.apexsurveys.ie
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STREET FURNITURE :

BOLLARDS

BD +

BUS STOP

BS +

CRASH BARRIER

CB

GATE

EP +

ELECTRICITY POLE

TP +

TELEPHONE POLE

ER +

EARTHING ROD

LP +

LAMP POST

MIKR +

MARKER POST

TL +

SIGN POST

TB

TRAFFIC LIGHT

POST +

TELEPHONE BOX

RS - RS

POST BOX

SH +

ROADSIGN

TPIT +

BORE HOLE

TRIAL PIT

BOTTOM OF CHAMBER

CAST-IRON

C1

CONCRETE

CONC

DIAMETER

DIA

SERVICES :

AIR VALVE

AV

ARMSTRONG JUNCTION

AJ

CABLE TV IC

CATV

COVER LEVEL

QL

EIRCOM COVER

EIRCOM

EIRCOM JUNCTION BOX

EIRCOM BOX

ELECTRICAL CABLE PIT

ECP

ESAT COVER

ESAT

ESB COVER

ESB

ESB JUNCTION BOX

ESB BOX

FIRE HYDRANT

PH

GAS VALVE

GV

GULLY

G

INSPECTION COVER

IC

MANHOLE

MH

SEPTIC TANK

SEPTIC

SLUICE VALVE

SV

DOWNSPIRE

DP

EARTHENWARE

EW

NO FURTHER TRACE

NFT

OFFSITE

O/S

STOPCOCK

ST

SERVICE BOX (UNKNOWN)

BOX

TRAFFIC COVER

TLIC

VENT

WM +

WATER METER

LEVELS :

BED LEVEL

+ BED101.50

FLOOR LEVEL

+ FL101.50

INVERT LEVEL

+ IL101.50

ROAD LEVEL

+ 101.50

SOFFIT LEVEL

+ SL101.50

SPOT LEVEL

+ 101.50

TOP OF WALL LEVEL

+ TOW101.50

WATER LEVEL

+ WL101.50

SURVEY CONTROL STATION

START OF RUN

SOR

UNABLE TO OPEN

UTO

UNABLE TO TRACE

UTT

UNDERGROUND LEGEND :

WATER

WATER

GAS MAIN

GAS

STORM DRAIN

STORM

FULL SEWER

COLL

COMBINED SEWER

COMB

ELECTRIC CABLE

POWER

ELECTRIC LIGHTING

LIGHTING

EIRCOM

EIRCOM

FIBRE OPTIC CABLE

F.OPTIC

BROADBAND

BROADBAND

CABLE TV

TV

TRAFFIC AND SIGNAL CABLE

TRAFFIC

CCTV

CCTV

IRRIGATION PIPE

IRRIGATION

EMPTY DUCT

EMPTY

GPR ANOMALY

ANOMALY

UNKNOWN CABLE

CABLE

O'HEAD ELECTRICITY

OH

O'HEAD TELECOM

OT

SHEET LAYOUT :

PLAN PRODUCED BY:

APEX SURVEYS

Apex Surveys
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Dunboyne, Co. Meath, Ireland
www.apexsurveys.ie
info@apexsurveys.ie
00353 1 691 0156

CONTACT INFORMATION:

CLIENT:

Aecom

GRID SYSTEM:

Irish Transverse Mercator

DATUM:

Malin Head (OSGM15)

NOTES:

Drawing Contains Scale Factor

REVISIONS:

No.	Date	Description
001	00/00	Original Drawing
002	19/12/23	Anomalies Updated

PROJECT:

Dyke Road, Galway

SCALE :

1/200 A1

DATE :

10/11/2023

DRG No:

5999

DESCRIPTION :

2D Utilities

SHEET:

5 of 11

SURVEYED BY :

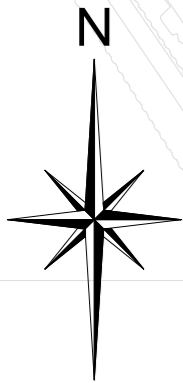
K.K.

PROCESSED BY :

J.P.

CHECKED BY :

Alan Brady



6

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SURVEYS

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STREET FURNITURE :

BOLLARDS	BD +
BUS STOP	BS +
CRASH BARRIER	CB
GATE	EP +
ELECTRICITY POLE	TP +
TELEPHONE POLE	ER +
EARTHING ROD	LP +
LAMP POST	TL +
MARKER POST	MKR +
SIGN POST	SIGN
TRAFFIC LIGHT	TL +
TELEPHONE BOX	TB
POST	POST +
POST BOX	RS - RS
ROADSIGN	BH +
BORE HOLE	TPIT +
TRIAL PIT	
BOTTOM OF CHAMBER	BOC
CAST-IRON	CI
CONCRETE	CONC
DIAMETER	DIA

SERVICES :

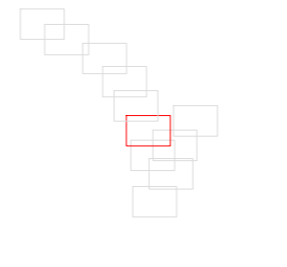
AIR VALVE	AV
ARMSTRONG JUNCTION	AJ
CABLE TV IC	CATV
COVER LEVEL	CL
EIRCOM COVER	EIRCOM
EIRCOM JUNCTION BOX	EIRCOM BOX
ELECTRICAL CABLE PIT	ECP
ESAT COVER	ESAT
ESB COVER	ESB
ESB JUNCTION BOX	ESB BOX
FIRE HYDRANT	FH
GAS VALVE	GV
GULLY	G
INSPECTION COVER	IC
MANHOLE	MH
SEPTIC TANK	SEPTIC
SLUICE VALVE	SV
DOWNPIPE	DP
EARTHENWARE	EW
NO FURTHER TRACE	NFT
OFFSITE	O/S

STOPCOCK	ST
SERVICE BOX (UNKNOWN)	BOX
TRAFFIC COVER	TLIC
VENT	VENT
WATER METER	WM
BED LEVEL	+ BED101.50
FLOOR LEVEL	+ FL101.50
INVERT LEVEL	+ I101.50
ROAD LEVEL	+ 101.50
SOFFIT LEVEL	+ SL101.50
SPOT LEVEL	+ TOW101.50
TOP OF WALL LEVEL	+ TW101.50
WATER LEVEL	+ WL101.50
SURVEY CONTROL STATION	SCS
START OF RUN	SOR
UNABLE TO OPEN	UTO
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UNDERGROUND LEGEND :

WATER MAIN	WATER
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STORM DRAIN	STORM
FOUL SEWER	FOUL
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EIRCOM	EIRCOM
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TRAFFIC AND SIGNAL CABLE	TRAFFIC
CCTV	CCTV
IRRIGATION PIPE	IRRIGATION
EMPTY DUCT	EMPTY
GPR ANOMALY	ANOMALY
UNKNOWN CABLE	CABLE
O'HEAD ELECTRICITY	O'HEAD
O'HEAD TELECOM	TELECOM

SHEET LAYOUT :



PLAN PRODUCED BY:

APEX
SURVEYS

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CLIENT:

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GRID SYSTEM: Irish Transverse Mercator
DATUM: Malin Head (OSGM15)
NOTES: Drawing Contains Scale Factor

REVISIONS:

No.	Date	Description
001	00/00	Original Drawing
002	19/12/23	Anomalies Updated

PROJECT:

Dyke Road, Galway

SCALE : 1/200 A1

DATE : 10/11/2023

DRG No: 5999

DESCRIPTION: 2D Utilities

SURVEYED BY : K.K.

SHEET: 6 of 11

PROCESSED BY : J.P.

CHECKED BY : Alan Brady

PAS 128: 2014 (Quality of Survey Level Outputs):

DESKTOP UTILITY RECORDS SEARCH
QL-D Drafted from utility records

SITE RECONNAISSANCE
QL-C Location Demonstrated by visual reference to street furniture or evidence of previous streetworks, ie - reinstatement scars

DETECTION
QL-B4 A segment of utility suspected to exist but has not been detected by a geophysical technique
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QL-A Horizontal and vertical location of the top and/or bottom of the utility

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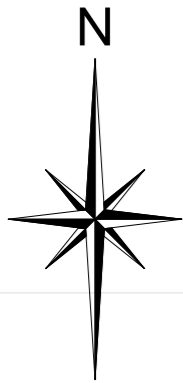
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APEX
SURVEYS

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00353 1 691 0156

STREET FURNITURE :

BOLLARDS	BD +
BUS STOP	BS +
CRASH BARRIER	CB
GATE	
ELECTRICITY POLE	EP +
TELEPHONE POLE	TP +
EARTHING ROD	ER +
LAMP POST	LP +
MARKER POST	MKR +
SIGN POST	SIGN +
TRAFFIC LIGHT	TL +
TELEPHONE BOX	TB
GULLY	
POST BOX	POST +
ROADSIGN	RS - RS
BORE HOLE	BH +
TRIAL PIT	TPIT +
BOTTOM OF CHAMBER	BOC
CAST-IRON	CI
CONCRETE	CONC
DIAMETER	DIA

SERVICES :

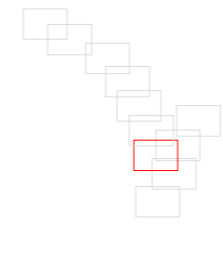
AIR VALVE	AV
ARMSTRONG JUNCTION	AJ
CABLE TV IC	CATV
COVER LEVEL	QL
EIRCOM COVER	EIRCOM
EIRCOM JUNCTION BOX	EIRCOM BOX
ELECTRICAL CABLE PIT	ECP
ESAT COVER	ESAT
ESB COVER	ESB
ESB JUNCTION BOX	ESB BOX
FIRE HYDRANT	FH
GAS VALVE	GV
GULLY	G
INSPECTION COVER	IC
MANHOLE	MH
SEPTIC TANK	SEPTIC
SLUICE VALVE	SV
DOWNPIPE	DP
EARTHENWARE	EW
NO FURTHER TRACE	NFT
OFFSITE	O/S

STOPCOCK	ST
SERVICE BOX (UNKNOWN)	BOX
TRAFFIC COVER	TLIC
VENT	VENT +
WATER METER	WM +
BED LEVEL	+ BED101.50
FLOOR LEVEL	+ FL101.50
INVERT LEVEL	+ I101.50
ROAD LEVEL	+ 101.50
SOFFIT LEVEL	+ SL101.50
SPOT LEVEL	+ 101.50
TOP OF WALL LEVEL	+ TOW101.50
WATER LEVEL	+ WL101.50
SURVEY CONTROL STATION	SCS
START OF RUN	SOR
UNABLE TO OPEN	UTO
UNABLE TO TRACE	UTT

UNDERGROUND LEGEND :

WATER MAIN	WATER
GAS MAIN	GAS
STORM DRAIN	STORM
POUL SEWER	POUL
COMBINED SEWER	COMB
ELECTRIC CABLE	POWER
ELECTRIC LIGHTING	LIGHTING
EIRCOM	EIRCOM
FIBRE OPTIC CABLE	F.OPTIC
BROADBAND	BROADBAND
CABLE TV	TV
TRAFFIC AND SIGNAL CABLE	TRAFFIC
CCTV	CCTV
IRRIGATION PIPE	IRRIGATION
EMPTY DUCT	EMPTY
GPR ANOMALY	ANOMALY
UNKNOWN CABLE	CABLE
O'HEAD ELECTRICITY	OH
O'HEAD TELECOM	OT

SHEET LAYOUT :



PLAN PRODUCED BY:

APEX
SURVEYS

CONTACT INFORMATION:

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00353 1 691 0156

CLIENT:

Aecom

GRID SYSTEM: Irish Transverse Mercator
DATUM: Malin Head (OSGM15)
NOTES: Drawing Contains Scale Factor

REVISIONS:

No.	Date	Description
001	00/00	Original Drawing
002	19/12/23	Anomalies Updated

PROJECT:

Dyke Road, Galway

SCALE : 1/200 A1

DATE : 10/11/2023

DRG No: 5999

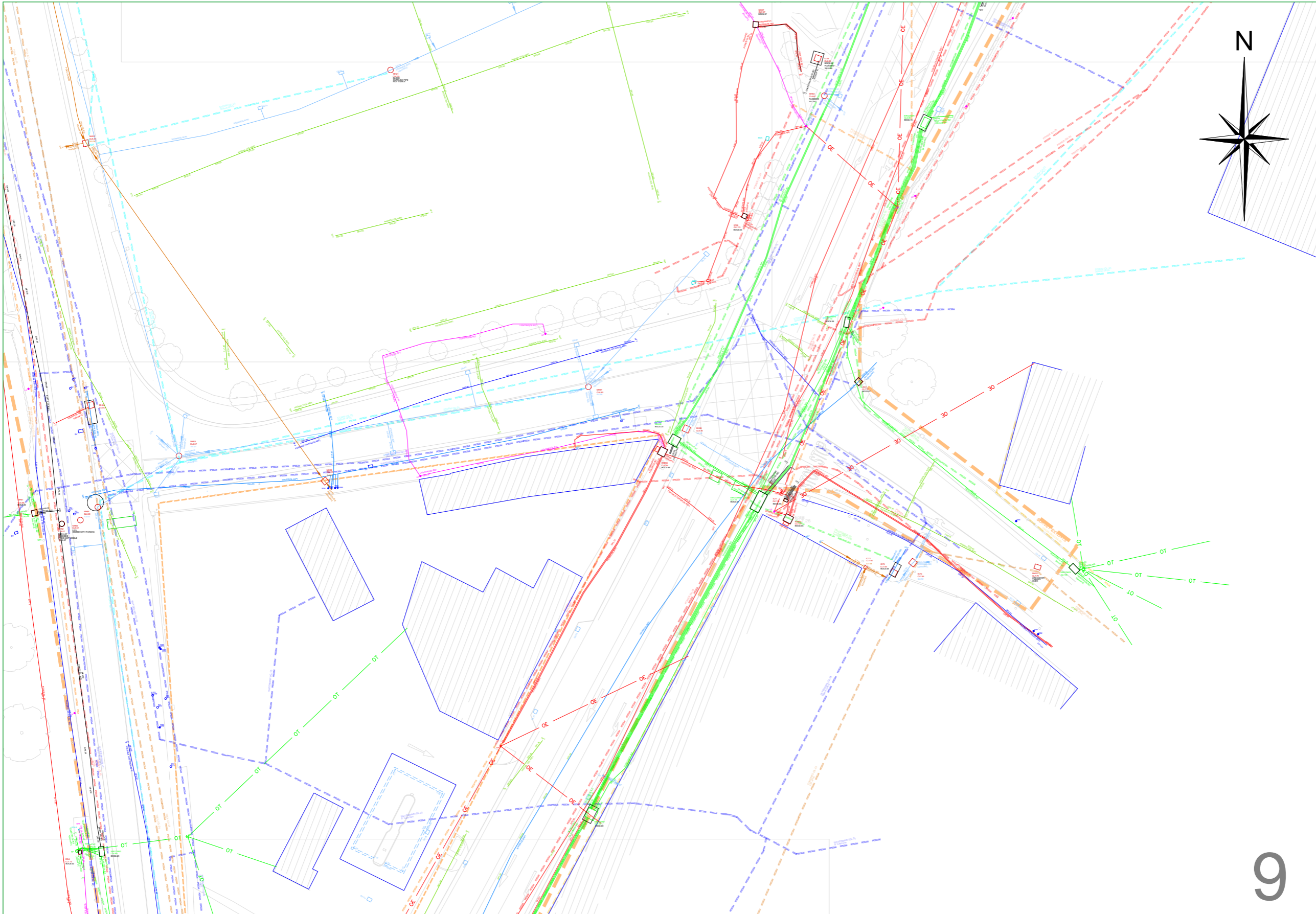
DESCRIPTION : 2D Utilities

SURVEYED BY : K.K.

SHEET: 7 of 11

PROCESSED BY : J.P.

CHECKED BY : Alan Brady



PAS 128: 2014 (Quality of Survey Level Outputs):

DESKTOP UTILITY RECORDS SEARCH
QL-D Drafted from utility records

SITE RECONNAISSANCE
QL-C Location Demonstrated by visual reference to street furniture or evidence of previous streetworks, ie - reinstatement scars

DETECTION
QL-B4 A segment of utility suspected to exist but has not been detected by a geophysical technique
QL-B3 Horizontal location only of the utility detected by one of the geophysical techniques used
QL-B2 Horizontal and vertical location of the utility detected by one of the geophysical techniques used
QL-B1 Horizontal and vertical location of the utility detected by multiple geophysical techniques

VERIFICATION
QL-A Horizontal and vertical location of the top and/or bottom of the utility

Apex Surveys Ltd. Disclaimer - Utility Survey

The interpretative nature and the non-intrusive, indirect and non-destructive survey methods must be taken into account when considering the results of the surveys. Therefore Apex Surveys, while using appropriate practice to execute, interpret and present the data, gives no guarantees that all underground utilities and underground structures will be located and mapped. Furthermore, Apex Surveys cannot guarantee the accuracy of the utility depths annotated on the survey drawings. Apex Survey shall not be liable for any omissions or inaccuracies in the survey which arise due to the limitations of the service. No liability shall attach to Apex Surveys, in any circumstances, howsoever arising, in respect of any consequential loss or damages suffered by the Client.

The following is a non-exhaustive list of the limitations of utility surveys:

- The Survey aims to map existing utilities subsurface utilities and provide information with respect to pipe size, material type and drainage connectivity. However utility surveying is limited by the following guidelines and it may not be possible to accurately survey, define and locate all services and sub-surface features.
- Depth of Utility: The depth and size of a utility affect the signal response and the degree with which a utility can be located. Due to attenuation of the radar signal with depth, resolution is restricted, hence making identification of utilities more difficult with increasing depth.
- Size of Utility: The smaller the diameter of a utility the more difficult it is to locate. This difficulty increases with depth.
- Ground Conditions: The depth penetration and quality of the data depends on the ground conditions of the site. GPR Surveying works best within high resistivity material. Clay overburden can impair GPR Surveying. Poor data may be a result of areas with high conductivity.
- Utility Congestion: Where different utilities converge together into a service corridor or cross paths it becomes difficult to isolate a specific utility and to map its route. The reflected signal will display a single response to multiple utilities. Therefore multiple utilities may appear to be a single utility. Where similar services run on close proximity, separation may be impossible.
- Signal Jumping: Signal from surrounding services may 'jump' to a highly conductive line masking its true identity.
- Shadowing: (of deeper utilities by shallower objects) Shallow utilities will mask the existence of deeper utilities where they are in close proximity. Also, high reflective materials close to the surface i.e rebar may hide deeper anomalies.
- Surface Obstructions: The GPR system relies on a relatively flat and even surface on which to perform radar passes. If ground obstructions such as vehicles, organic material (long grass, scrub) or undulating ground surface are present then the acquired data will be of lower resolution and in some cases not viable.
- Loss of signal: It is not always possible to trace the entire length of each underground service.
- Connections between manholes: Connections between manhole chambers are assumed to be straight.
- Non-metallic objects: Nonmetallic objects are amongst the most difficult to trace therefore successful tracing of non-metallic pipes/ utilities may be limited.
- Fiber Optic Cables: Fiber optic cables may not be possible to locate except where laid with a built in tracer wire or similar conductor system.
- Defective / flooded manholes or pipework: It may not be possible to establish connections between flooded or defective manholes or pipework.
- Acute bends in pipework: It may not be possible to trace a pipe past an acute bend.

Accuracy estimates:

- Locational accuracy is determined by referring to the manufacturers guidelines for the detector used.
- In ideal conditions the spatial accuracies for the underground utilities may be +/- 5% for Radiodetection and +/- 10% of depth for the GPR to 2.5m deep. However variations within the subsurface, depth below the ground, close proximity of other services and local magnetic, atmospheric or ground conditions, bends, lateral service connections and any of the other limitations listed in this disclaimer may alter this estimated accuracy.
- Plan accuracies of + or - 150mm may be achieved but this figure will depend on the depth of service below ground level. However variations within the subsurface, depth below the ground, close proximity of other services and local magnetic, atmospheric or ground conditions, bends, lateral service connections and any of the other limitations listed in this disclaimer may alter this estimated accuracy.
- DP represents distance from the surface level to the top of the service/ target
- Where technically possible, depth indications will be given. These along with plan positions should be used for guidance only and wherever critical accuracy is required these should be confirmed by the client by undertaking trial excavations or similar.

Record Drawing Information

- Services which have been untraceable are shown from records where possible or available. These lines are annotated as "Taken From Records" or "From Records".
- Existing record information showing underground services is often incomplete and with unknown accuracies therefore it should be regarded as indicative only.
- Where Apex Surveys issue a utility drawing, this should be read in conjunction with all available public or private utility records.
- Apex Surveys endeavor to add relevant Public Utility record information onto the final drawing. However, we would recommend that direct contact is made with the asset owner or statutory undertaker.
- We shall not be held responsible for the accuracy, or otherwise, of the location of a service, as issued by the utility provider and therefore shown as "Taken for Records" on the drawing.

The following have been excluded from the survey:

- Location of individual service feeds to properties or buildings as access would be required into each property to apply direct connections to inlet points and this would significantly increase the scope of works, survey cost and also cause possible disruption to occupants.
- Pot ended or disconnected cables or terminated short lengths of pipe.
- Internal building services.
- Small diameter cables less than 20mm diameter or pipes less than 40mm diameter.
- Above ground services unless specifically requested.
- Lifting manholes which require longer than 10 minutes effort using standard heavy duty apparatus.

All works carried out be Apex Surveys conforms to the guidelines set out by The Survey Association (TSA) and PAS:128 Standard for utility mapping

APEX SURVEYS

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info@apexsurveys.ie
00353 1 691 0156

STREET FURNITURE :

BD +
BS +
CB

EP +
TP +
ER +
LP +
MKR +
SIGN +
TL +
TB
POST +
RS - RS
BH +
TPIT +

BOC
C1
CONC
DIA

SERVICES :

AIR VALVE
ARMSTRONG JUNCTION
CABLE TV IC
COVER LEVEL
EIRCOM COVER
EIRCOM JUNCTION BOX
ELECTRICAL CABLE PIT
ESAT COVER
ESB COVER
ESB JUNCTION BOX
FIRE HYDRANT
GAS VALVE
GULLY
INSPECTION COVER
MANHOLE
SEPTIC TANK
SLUICE VALVE

AV
AJ
CATV
QL
EIRCOM
EIRCOM BOX
ECP
ESAT
ESB
ESB BOX
FH
GV
IC
MH
SEPTIC
SV

DP
EW
NFT
O/S

LEVELS :

STOPCOCK
SERVICE BOX (UNKNOWN)
TRAFFIC COVER
VENT
WATER METER

+ BED101.50
+ FL101.50
+ L101.50
+ 101.50
+ SL101.50
+ 101.50
+ TOW101.50
+ WL101.50

SOR
UTO
UTT

UNDERGROUND LEGEND :

WATER MAIN
GAS MAIN
STORM DRAIN
POUL SEWER
COMBINED SEWER
ELECTRIC CABLE
ELECTRIC LIGHTING
EIRCOM
FIBRE OPTIC CABLE
BROADBAND
CABLE TV
TRAFFIC AND SIGNAL CABLE
CCTV
IRRIGATION PIPE
EMPTY DUCT
GPR ANOMALY
UNKNOWN CABLE
O'HEAD ELECTRICITY
O'HEAD TELECOM

WATER
GAS
STORM
POUL
COMB
POWER
LIGHTING
EIRCOM
F.OPTIC
BROADBAND
TV
TRAFFIC
CCTV
IRRIGATION
EMPTY
ANOMALY
CABLE

OT

SHEET LAYOUT :

PLAN PRODUCED BY:

APEX SURVEYS

CONTACT INFORMATION:

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00353 1 691 0156

CLIENT:

Aecom

GRID SYSTEM: Irish Transverse Mercator
DATUM: Malin Head (OSGM15)
NOTES: Drawing Contains Scale Factor

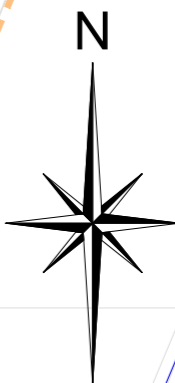
REVISIONS:

No.	Date	Description
001	00/00	Original Drawing
002	19/12/23	Anomalies Updated

PROJECT:

Dyke Road, Galway

SCALE :	1/200 A1	DATE :	10/11/2023
DRG No:	5999	DESCRIPTION :	2D Utilities
		SURVEYED BY :	K.K.
		PROCESSED BY :	J.P.
		CHECKED BY :	Alan Brady



DESKTOP UTILITY RECORDS SEARCH	
QL-D	Queried from utility records
SITE RECONNAISSANCE	
QL-C	Location Demonstrated by visual reference to street furniture or evidence of previous structures, ie - reinstatement scars
DETECTION	
QL-B4	A segment of utility suspected to exist but has not been detected by a geophysical technique
QL-B3	Horizontal location only of the utility detected by one of the geophysical techniques used
QL-B2	Horizontal and vertical location of the utility detected by one of the geophysical techniques used
QL-B1	Horizontal and vertical location of the utility detected by multiple geophysical techniques
VERIFICATION	
QL-A	Horizontal and vertical location of the top and/or bottom of the utility

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- Plan accuracies of + or - 150mm may be achieved but this figure will depend on the depth of service below ground level. However variations within the subsurface, depth below the ground, close proximity of other services and local magnetic, atmospheric or ground conditions, bends, lateral service connections and any of the other limitations listed in this disclaimer may alter this estimated accuracy.
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- Small diameter cables less than 20mm diameter or pipes less than 40mm diameter.
- Above ground services unless specifically requested.
- Lifting manholes which require longer than 10 minutes effort using standard heavy duty apparatus.

PROJECT:

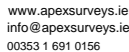
SCALE : 1/200 A

DRG No: 5999

DATE : 10/11/2023



SURVEYED BY : K.K.

PROCESSED BY : J.P.




BOLLARDS	BD +
BUS STOP	BS +
CRASH BARRIER	CB
GATE	
ELECTRICITY POLE	EP +
TELEPHONE POLE	TP +
EARTHING ROD	ER +
LAMP POST	LP +
MARKER POST	MKR +
SIGN POST	SIGN 
TRAFFIC LIGHT	TB
TELEPHONE BOX	POST +
POST	POST BOX
POST BOX	RS - RS 
ROADSIGN	BH +
BORE HOLE	TPIT +
TRIAL PIT	

AIR VALVE	
ARMSTRONG JUNCTION	
CABLE TV IC	
COVER LEVEL	
EIRCOM COVER	
EIRCOM JUNCTION BOX	
ELECTRICAL CABLE PIT	
ESAT COVER	
ESB COVER	
ESB JUNCTION BOX	
FIRE HYDRANT	
GAS VALVE	
GULLY	
INSPECTION COVER	
MANHOLE	
SEPTIC TANK	
SUICIDE VALVE	

STOPCOCK	ST +
SERVICE BOX (UNKNOWN)	BOX 
TRAFFIC COVER	TUC 
VENT	VENT +
WATER METER	WM +

LEVELS :

BED LEVEL	+ BED101.50
FLOOR LEVEL	+ FL101.50
INVERT LEVEL	+ IL101.50
RD LEVEL	+ 101.50
SOFFIT LEVEL	+ SL101.50
SPOT LEVEL	+ 101.50
TOP OF WALL LEVEL	+ TOW101.50
WATER LEVEL	+ WL101.50
SURVEY CONTROL STATION	

WATER MAIN	WATER
GAS MAIN	GAS
STORM DRAIN	STORM
FOUL SEWER	FOUL
COMBINED SEWER	COMB
ELECTRIC CABLE	POWER
ELECTRIC LIGHTING	LIGHTING
EIRCOM	EIRCOM
FIBRE OPTIC CABLE	F OPTIC
BROADBAND	BROADBA
CABLE TV	TV
TRAFFIC AND SIGNAL CABLE	TRAFFIC
CCTV	CCTV
IRRIGATION PIPE	IRRIGATIO
EMPTY DUCT	EMPTY
GPR ANOMALY	ANOMALY
UNKNOWN CABLE	CABLE

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Aecom

GRID SYSTEM: Irish Transverse Mercator
DATUM: Malin Head (OSGM15)
NOTES: Drawing Contains Scale Factor

REVISIONS:

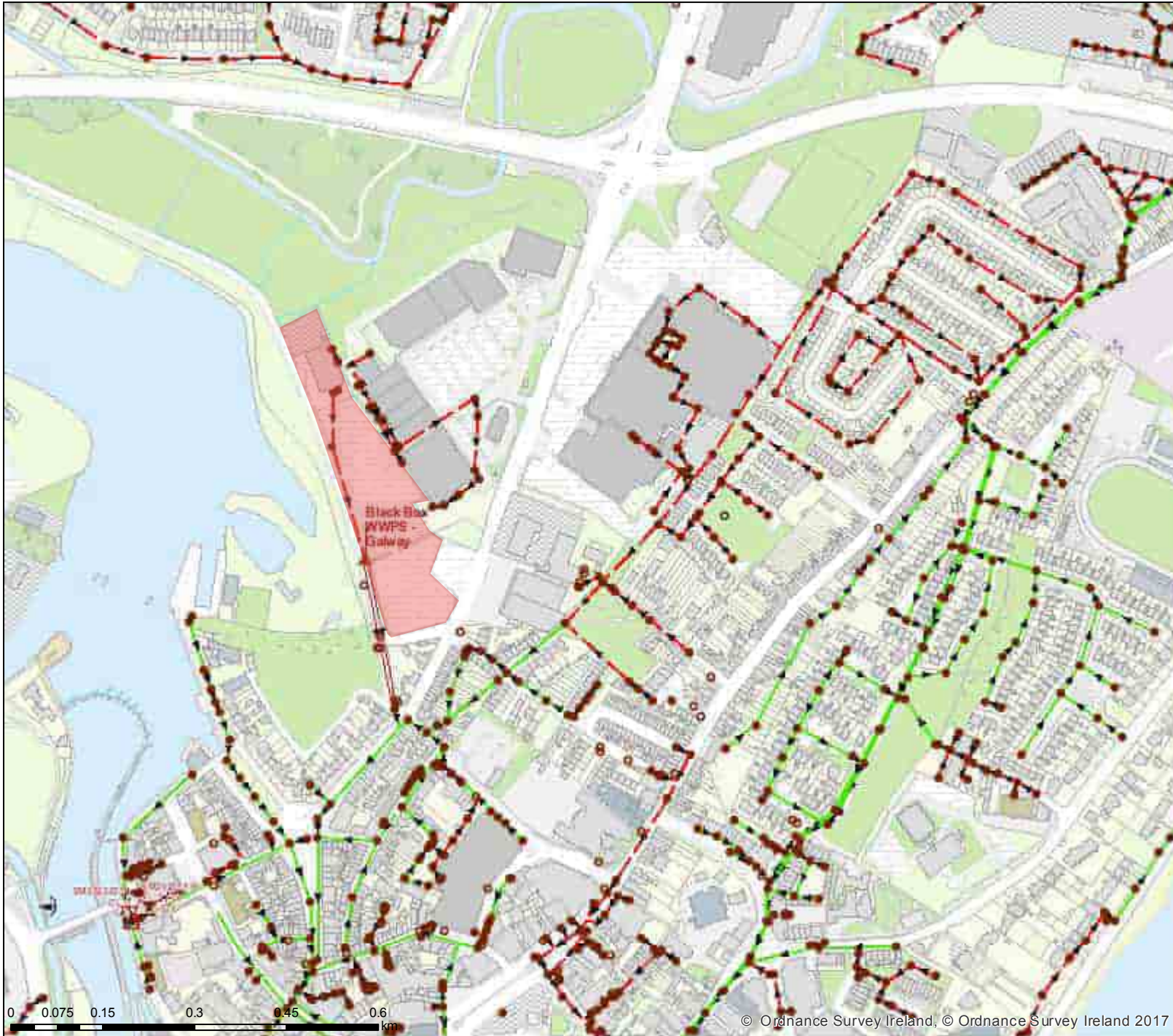
No	Date
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001	00/00
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002	19/12
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Dyke Road - Sewer



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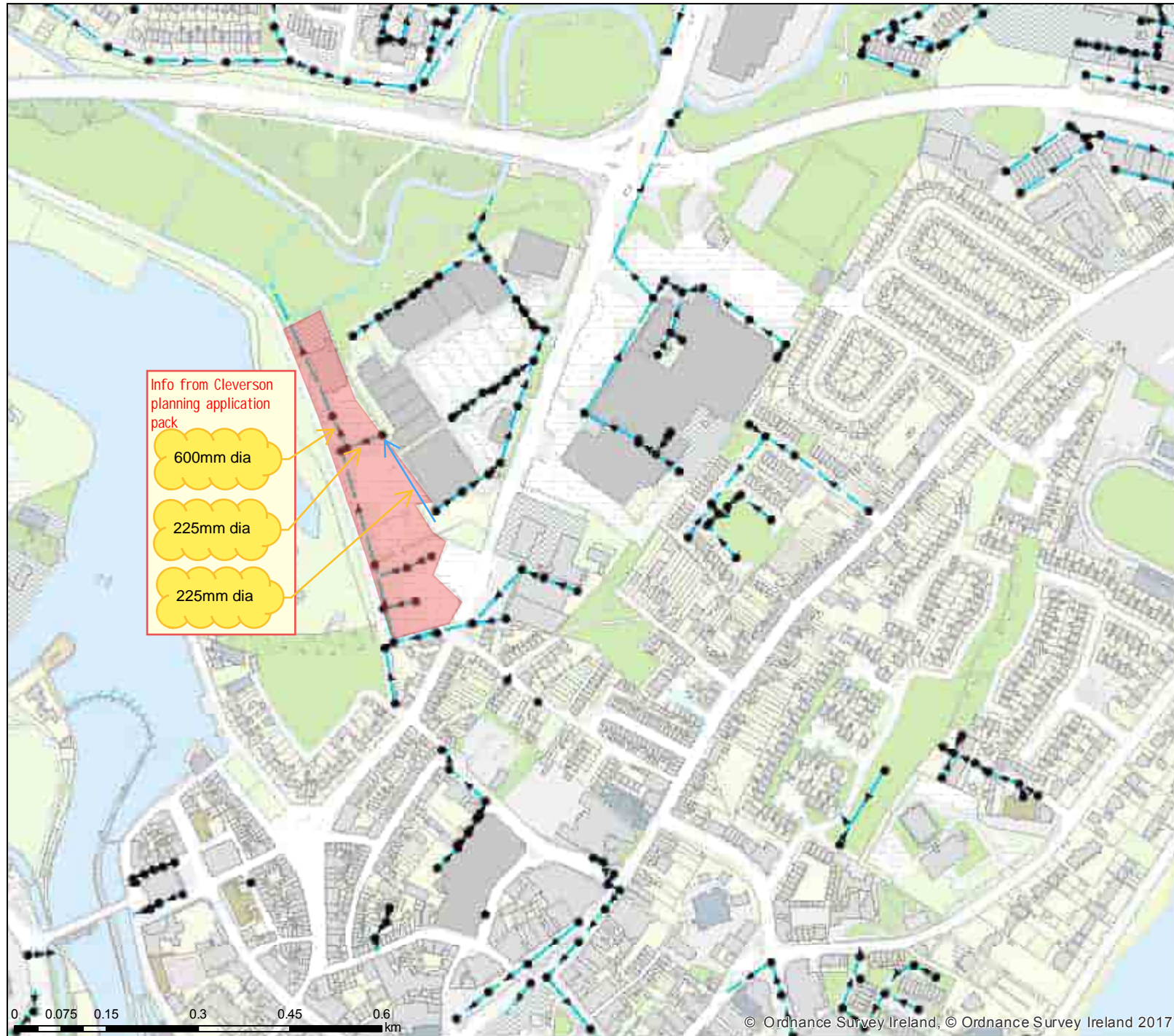
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[illegible]

Dyke Road - Storm





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Legend	Legend	Legend
Water	Water	Water
Gas	Gas	Gas
Electricity	Electricity	Electricity
Other	Other	Other
...

Dyke Road - Water





Print Date: 13/07/2023

Printed by: Irish Water

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Legend	Legend	Legend
<ul style="list-style-type: none"> Water Main Water Service Water Valve Water Manhole Water Pipe Water Fitting Water Connection Water Access Water Entry Water Exit Water Outlet Water Inlet Water Drain Water Overflow Water Spill Water Leak Water Damage Water Repair Water Work Water Test Water Clean Water Flush Water Stop Water Seal Water Plug Water Cap Water Cover Water Box Water Tank Water Reservoir Water Pond Water Lake Water River Water Stream Water Canal Water Ditch Water Trench Water Pit Water Well Water Spring Water Fountain Water Jet Water Shower Water Bath Water Toilet Water Sink Water Dish Water Tub Water Bathtub Water Shower Water Bath Water Toilet Water Sink Water Dish Water Tub Water Bathtub 	<ul style="list-style-type: none"> Electricity Main Electricity Service Electricity Valve Electricity Manhole Electricity Pipe Electricity Fitting Electricity Connection Electricity Access Electricity Entry Electricity Exit Electricity Outlet Electricity Inlet Electricity Drain Electricity Overflow Electricity Spill Electricity Leak Electricity Damage Electricity Repair Electricity Work Electricity Test Electricity Clean Electricity Flush Electricity Stop Electricity Seal Electricity Plug Electricity Cap Electricity Cover Electricity Box Electricity Tank Electricity Reservoir Electricity Pond Electricity Lake Electricity River Electricity Stream Electricity Canal Electricity Ditch Electricity Trench Electricity Pit Electricity Well Electricity Spring Electricity Fountain Electricity Jet Electricity Shower Electricity Bath Electricity Toilet Electricity Sink Electricity Dish Electricity Tub Electricity Bathtub 	<ul style="list-style-type: none"> Gas Main Gas Service Gas Valve Gas Manhole Gas Pipe Gas Fitting Gas Connection Gas Access Gas Entry Gas Exit Gas Outlet Gas Inlet Gas Drain Gas Overflow Gas Spill Gas Leak Gas Damage Gas Repair Gas Work Gas Test Gas Clean Gas Flush Gas Stop Gas Seal Gas Plug Gas Cap Gas Cover Gas Box Gas Tank Gas Reservoir Gas Pond Gas Lake Gas River Gas Stream Gas Canal Gas Ditch Gas Trench Gas Pit Gas Well Gas Spring Gas Fountain Gas Jet Gas Shower Gas Bath Gas Toilet Gas Sink Gas Dish Gas Tub Gas Bathtub

Appendix E Ground Investigation Reports

- E.1 Ground Investigation Report (Ground Investigation Ireland Ltd)**
- E.2 Geophysical Survey (Minerex Geophysics Ltd)**



GROUND INVESTIGATIONS IRELAND
Geotechnical & Environmental

Catherinstown House,
Hazelhatch Road,
Newcastle,
Co. Dublin,
D22 YD52

Tel: 01 601 5175 / 5176
Email: info@gii.ie
Web: www.gii.ie

Ground Investigations Ireland

Dyke Road Galway

Aecom

Ground Investigation Report

July 2024

Directors:

Fergal McNameara (MD), Conor Finlierty, Aisling McDonnell, Barry Sexton, Stephen Keady & Michael Sutton
Ground Investigations Ireland Limited | Registered in Ireland, Company Registration No.: 405720



www.gii.ie



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Geotechnical & Environmental

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Hazelhatch Road,
Newcastle,
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Tel: 01 601 5175 / 5176
Email: info@gil.ie
Web: www.gil.ie

DOCUMENT CONTROL SHEET

Project Title	Dyke Road Galway
Client	Land Development Agency
Engineer	Aecom
Project No	13614-02-24
Document Title	Ground Investigation Report

Rev.	Status	Author(s)	Reviewed By	Approved By	Office of Origin	Issue Date
A	Interim	A Mann	M Sutton	M Sutton	Dublin	24 June 2024
B	Final	A Mann	M Sutton	M Sutton	Dublin	04 July 2024

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APPENDICES

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Appendix 2	Trial Pit Records
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1.0 Preamble

On the instructions of Aecom Engineers, a site investigation was carried out by Ground Investigations Ireland Ltd., between April and June 2024 at the site of the proposed development in Galway City.

2.0 Overview

2.1. Background

It is proposed to construct a new residential and commercial/retail development with associated services, access roads and car parking at the proposed site. The site is currently occupied by a car park and is situated near the centre of Galway City. The proposed construction is envisaged to consist of piled foundations and pavement make up with some local excavations for services and plant

2.2. Purpose and Scope

The purpose of the site investigation was to investigate subsurface conditions utilising a variety of investigative methods in accordance with the project specification. The scope of the work undertaken for this project included the following:

- Visit project site to observe existing conditions
- Carry out 6 No. Trial Pits to a maximum depth of 3.0m BGL
- Carry out 4 No. Slit Trenches to determine existing service details
- Carry out 2 No. Soakaways to determine a soil infiltration value to BRE digest 365
- Carry out 11 No. Cable Percussion boreholes to a maximum depth of 10.10m BGL
- Carry out 7 No. Rotary Core Boreholes to a maximum depth of 20.10m BGL
- Carry out 1 No. Insitu Plate Bearing Tests
- Carry out 4 No. TRL Dynamic Cone Penetrometer Tests
- Installation of 4 No. Groundwater monitoring wells
- Geotechnical & Environmental Laboratory testing
- Report with recommendations

3.0 Subsurface Exploration

3.1. General

During the ground investigation a programme of intrusive investigation specified by the Consulting Engineer was undertaken to determine the sub surface conditions at the proposed site. Regular sampling and in-situ testing was undertaken in the exploratory holes to facilitate the geotechnical descriptions and to enable laboratory testing to be carried out on the soil samples recovered during excavation and drilling.

The procedures used in this site investigation are in accordance with Eurocode 7 Part 2: Ground Investigation and testing (ISEN 1997 – 2:2007) and B.S. 5930:2015.

3.2. Trial Pits

The trial pits were excavated using a 3T tracked excavator at the locations shown in the exploratory hole location plan in Appendix 1. The locations were checked using a CAT scan to minimise the potential for encountering services during the excavation. The trial pits were sampled, logged and photographed by a Geotechnical Engineer/Engineering Geologist prior to backfilling with arisings. Notes were made of any services, inclusions, pit stability, groundwater encountered and the characteristics of the strata encountered and are presented on the trial pit logs which are provided in Appendix 2 of this Report.

3.3. Slit Trenching

The slit trenches were excavated using a 3T tracked excavator at the locations shown in the exploratory hole location plan in Appendix 1. The locations were checked using a CAT scan to minimise the potential for encountering services during the excavation. The soil was slowly stripped using a spotter on the trench to alert the driver if any services were seen, to avoid damage to any underlying services. The slit trenches were sampled, logged and photographed by a Geotechnical Engineer/Engineering Geologist prior to backfilling with arisings. Notes were made of any services, inclusions, pit stability, groundwater encountered and the characteristics of the strata encountered and are presented on the slit trench records which are provided in Appendix 3 of this Report.

3.4. Soakaway Testing

The soakaway testing was carried out in selected trial pits at the locations shown in the exploratory hole location plan in Appendix 1. These pits were carefully excavated and filled with water to assess the infiltration characteristics of the proposed site. The pits were allowed to drain and the drop in water level was recorded over time as required by BRE Digest 365. The pits were logged prior to completing the soakaway test and were backfilled with arising's upon completion. The soakaway test results are provided in Appendix 4 of this Report.

3.5. Cable Percussion Boreholes

The Cable Percussion Boreholes were drilled using a Dando 2000 drilling rig with regular in-situ testing and sampling undertaken to facilitate the production of geotechnical logs and laboratory testing.

The standard method of boring in soil for site investigation is known as the Cable Percussion method. It consists of using a Shell in non cohesive soils and a clay cutter in cohesive soils, both operated on a wire cable. Very hard soils, boulders and other hard obstructions are broken up by chiselling and the fragments removed with the Shell. Where ground conditions made it necessary, the borehole was lined with 200mm diameter steel casing. While the use of the Cable Percussion method of boring gives the maximum data on soil conditions, some mixing of laminated soil is inevitable. For this reason, thin lenses of granular material may not be noticed. Disturbed samples were taken from the boring tools at suitable depths, so that there is a representative sample at the top of each change in stratum and thereafter at regular intervals

down the borehole until the next stratum was encountered. The disturbed samples were then sealed and sent to the laboratory where they were visually examined to confirm the description of the relevant strata. Standard Penetration Tests were carried out in the boreholes. The results of these tests, together with the depths at which the tests were taken are shown on the accompanying borehole records. The test consists of a thick wall sampler tube, 50mm external diameter, being driven into the soil by a monkey weighing 63.5kg and with a free drop of 760mm. For gravels and glacial till the driving shoe was replaced by a solid 60° cone. The Standard Penetration Test number referred to as the 'N' value is the number of blows required to drive the tube 300mm, after an initial penetration of 150mm. The number gives a guide to the consistency of the soil and can also be used to estimate the relative strength/density at the depth of the test and also to estimate the bearing capacity and compressibility of the soil. The cable percussion borehole logs are provided in Appendix 5 of this Report.

3.6. Rotary Boreholes

The rotary coring was carried out by a track mounted T44 Beretta rig at the locations shown on the location plan in Appendix 1. The rotary boreholes were completed from the ground surface or alternatively, where noted on the individual borehole log, from the base of the cable percussion borehole where a temporary liner was installed to facilitate follow-on rotary coring.

The T44 Beretta is equipped with rubber tracks which allow for short travel on pavement surfaces avoiding any damage to the surface. The T44 Beretta utilises a triple tube core barrel system operated using a wireline drilling process. The outer barrel is rotated by the drill rods and at its lower end, carries the coring bit. The inner barrel is mounted on a swivel so that it does not rotate during the process. The third barrel or liner is placed within the second one to retain the core intact and to preserve as much as possible the fabric of the drilling stratum. The core is cut by the coring bit and passes to the inner liner. The core is brought up to the surface within the inner barrel on a small diameter wire rope or line attached to the "overshoot" recovery tool which is then placed into a core box in order of recovery. A drilling fluid, typically air mist or water flush is passed from the surface through hollow drill rods to the drill bit and is used to cool the drill bit. Temporary casing is used in some situations to support unstable ground or to seal off fissures or voids. It should be noted that the rotary coring can only achieve limited recovery in overburden, particularly granular or weakly cemented strata due to the flushing medium washing away the cohesive fraction during coring. The recovery achieved, where required is noted on the borehole logs and core photographs are provided to allow assessment of the core recovered. The rotary borehole logs are provided in Appendix 5 of this Report.

3.7. Surveying

The exploratory hole locations have been recorded using a KQ GEO Technologies KQ-M8 System which records the coordinates and elevation of the locations to ITM or Irish National Grid as required by the project specification. The coordinates and elevations are provided on the exploratory hole logs in the appendices of this Report.

3.8. Groundwater/Gas Monitoring Installations

Groundwater and or Gas Monitoring Installation were installed upon the completion of the boreholes to enable sampling and the determination of the equilibrium groundwater level. The typical groundwater monitoring installation consists of a 50mm uPVC/HDPE slotted pipe with a pea gravel response zone and bentonite seal installed to the Engineers specification. Where required the standpipe is sealed with a gas tap and finished with a durable steel cover fixed in place with a concrete surround. The installation details are provided on the exploratory hole logs in the appendices of this Report.

3.9. Insitu Plate Bearing Test

The plate bearing tests were carried out using a 450mm diameter plate at the locations shown on the site plan in Appendix 1. The plate was loaded in increments using a hydraulic jack and an excavator to provide a reaction and the displacement was monitored in accordance with BS1377 Part 9 using independently mounted digital strain gauges. The constrained modulus and equivalent CBR are calculated in accordance with HD29/75 and are provided on the test reports in Appendix 6 of this Report.

3.10. TRL Dynamic Cone Penetrometer

The TRL DCP tests were carried out at locations specified by the Consulting Engineer to determine a CBR design value for the design of external pavements. The testing was carried out below the Topsoil or existing pavement at the depths detailed on the test report. The test consists of dropping a 10kg weight on an anvil to drive a small diameter cone and recording the blows for a given penetration. The results of the DCP testing are included in Appendix 7 of this Report.

3.11. Laboratory Testing

Samples were selected from the exploratory holes for a range of geotechnical and environmental testing to assist in the classification of soils and to provide information for the proposed design.

Environmental & Chemical testing as required by the specification, including the Rilta Suite and Engineers Ireland Suites E and D was carried out by Element Materials Technology Laboratory in the UK. The Rilta suite testing includes both Solid Waste and Leachate Waste Acceptance Criteria. Chemical testing including organic matter content, sulphate content, chloride content and pH was carried out in Professional Soils Laboratory (PSL Ltd) in the UK.

Geotechnical testing consisting of moisture content, Atterberg limits, Particle Size Distribution (PSD), hydrometer and Moisture Condition Value (MCV) tests were also carried out in Professional Soils Laboratory (PSL Ltd). Specialist shear strength testing consisting of quick undrained, shear box and consolidation testing was carried out on undisturbed U100 or piston samples where recovered.

Rock strength testing including Point Load (Is_{50}) and Unconfined Compressive Strength (UCS) testing was carried out by Construction Materials Testing Laboratories (CTML) Ireland. The results of the laboratory testing are included in Appendix 8 of this Report.

4.0 Ground Conditions

4.1. General

The ground conditions encountered during the investigation are summarised below with reference to in situ and laboratory test results. The full details of the strata encountered during the ground investigation are provided in the exploratory hole logs included in the appendices of this report.

The sequence of strata encountered were generally consistent across the site and generally comprised;

- Surfacing
- Made Ground
- Organic Deposits
- Soft Cohesive Deposits
- Cohesive Deposits
- Granular Deposits
- Bedrock

SURFACING: Tarmac surfacing was present typically to a depth of 0.06m BGL.

MADE GROUND: Made Ground deposits were encountered beneath the Surfacing and were generally present to depths of between 0.5m and 1.0m BGL and a maximum of 3.4m BGL in BRC04. These deposits were described generally as *grey Sand and Gravel FILL and contained occasional fragments of tarmacadam* occasionally overlying *grey slightly sandy gravelly Clay* and *brownish black gravelly Peat with occasional red brick, ceramic and rubbish fragments*.

ORGANIC DEPOSITS: Organic deposits were generally encountered beneath the Made Ground and were described typically as *brownish black slightly clayey slightly gravelly PEAT*. The secondary constituents varied across the site, with silt and clay lenses occasionally present in the peat. The strength of the deposits was typically very soft based on SPT N values.

SOFT COHESIVE DEPOSITS: Soft Cohesive deposits were encountered beneath the organic deposits and were generally described as *beige or cream clayey SILT with frequent shell fragments* occasionally onto *light grey slightly sandy slightly gravelly clayey SILT with occasional cobbles*. The secondary sand and gravel constituents varied across the site and with depth, and peat lenses were occasionally present within the deposits. The strength of the soft cohesive deposits was typically very soft to soft.

COHESIVE DEPOSITS: Cohesive deposits were encountered beneath the soft cohesive deposits at some locations and were described typically as *light grey to grey slightly sandy slightly gravelly silty CLAY with occasional cobbles*. The secondary sand and gravel constituents varied across the site and with depth. The strength of the cohesive deposits typically increased with depth and was stiff or very stiff below 6.0m BGL

in the majority of the exploratory holes. These deposits had some occasional cobble content, where noted on the exploratory hole logs.

GRANULAR DEPOSITS: Granular deposits were occasionally encountered at the base of the cohesive deposits and were typically described as *grey very sandy subangular to subrounded fine to coarse GRAVEL with occasional cobbles*. The secondary sand constituents varied across the site while occasional cobble content was also present where noted on the exploratory hole logs.

Based on the SPT N values the deposits are typically medium dense to dense and become dense with depth. Groundwater strikes were occasionally noted in the boreholes on encountering the granular deposits.

BEDROCK: The rotary core boreholes recovered Strong thinly to medium bedded grey fine to medium grained fossiliferous LIMESTONE, with the exception of BRC04 which recovered strong to very strong thinly to thickly banded dark green medium to coarsely crystalline METAGABBRO. Occasional calcite veins were noted during logging.

The depth to rock increases to the southeast from 11.2m BGL in BH01 in the north western corner of the site to a maximum of 15.3m BGL in BRC03 in the centre of the site. The depth to rock decreases to 9.4m BGL in BRC06, and further decreases to between 6.6m and 6.1m BGL respectively in BRC04 and BRC05 in the southeastern portion of the site. The total core recovery is typically 100% within bedrock. The SCR and RQD are generally poorer in the upper weathered zone, however both indices show an increase with depth in each of the boreholes.

4.2. Groundwater

Groundwater strikes are noted on the exploratory hole logs where they occurred and where possible drilling was suspended for twenty minutes to allow the subsequent rise in groundwater to be recorded. We would point out that these exploratory holes did not remain open for sufficiently long periods of time to establish the hydrogeological regime and groundwater levels would be expected to vary with the tide, time of year, rainfall, nearby construction and other factors. For this reason, standpipes were installed in BRC1, BRC02, BRC04 and BRC05 to allow the equilibrium groundwater level to be determined. The groundwater monitoring is included in Appendix 9 of this Report.

4.3. Laboratory Testing

4.3.1. Geotechnical Laboratory Testing

The geotechnical testing carried out on soil samples recovered generally confirm the descriptions on the logs with the primary constituent of the cohesive deposits found to be a CLAY of high plasticity. The Particle Size Distribution tests confirm that generally the cohesive deposits are well-graded to gap graded with percentages of sands and gravels ranging between 1% and 48% generally with fines contents of 14 to 97%.

The Particle Size Distribution tests confirm that generally the granular deposits are gap graded with percentages of silt/clay typically between 1% and 2% with a sand content of typically 7% to 14%. gravel content of typically 53% to 68%.

Undrained shear strength testing on undisturbed samples gave results ranging between 3kPa and 13 kPa which correlated with results of laboratory vane tests which gave peak shear strengths ranging from 6 to 27 kPa.

MCV Values ranged between <1 to 4.5 in the cohesive deposits meaning the material would be considered unsuitable for reuse.

4.3.2. Chemical Laboratory Testing

The pH and sulphate testing carried out indicate that pH results are near neutral to alkaline, with values ranging from 7.1 to 9.6, and that the water soluble sulphate results range from low to elevated (DS-1 to DS-2) when compared to the guideline values from BRE Special Digest 1:2005. Samples of the Peat (TP3 0.6-1.1m BGL) have elevated Total Sulphur results (0.52%) which result in a high Total Potential Sulphate (TPS) and a subsequent higher classification of the Design Sulphate values in accordance with the BRE digest. Caution is recommended and the removal of the Peat and Made ground stratum to reduce the risk of sulphate attack adjacent to concrete elements as part of the permanent works. Piles in the organic and very soft deposits are recommended to incorporate appropriate measures to resist sulphate attack.

4.3.3. Environmental Laboratory Testing

A number of samples were analysed for a suite of parameters which allows for the assessment of the sampled material in terms of total pollutant content for classification of materials as *hazardous* or *non-hazardous*. The suite also allows for the assessment of the sampled material in terms of suitability for placement at licenced landfills (inert, stable non-reactive, hazardous etc.). The parameter list for the suite includes analysis of the solid samples for arsenic, barium, cadmium, chromium, copper, cyanide, lead, nickel, mercury, zinc, speciated aliphatic and aromatic petroleum hydrocarbons, pH, sulphate, sulphide, moisture content, soil organic matter and an asbestos screen.

The suite also includes those parameters specified in the EU Council Decision establishing criteria for the acceptance of waste at Landfills (Council Decision 2003/33/EC), which for the solid samples are total organic carbon (TOC), speciated aliphatic and aromatic petroleum hydrocarbons, BTEX, phenol, polychlorinated biphenyls (PCB) and PAH.

As part of the suite a leachate is generated from the solid sample which is analysed for antimony, arsenic, barium, cadmium, chromium, copper, lead, mercury, molybdenum, nickel, selenium, zinc, chloride, fluoride, soluble sulphate, sulphide, phenols, dissolved organic carbon (DOC) and total dissolved solids (TDS).

While the laboratory report provides a comparison with the waste acceptance criteria limits it does not provide a waste classification of the material sampled nor does it comment on any potentially hazardous properties of the materials tested. The possibility for contamination, not revealed by the testing undertaken should be borne in mind particularly where Made Ground deposits are present or the previous site use or

location indicate a risk of environmental variation. The waste classification report is included under the cover of a separate report by Ground Investigations Ireland.

4.3.4. Rock Laboratory Testing

The rock testing carried out on samples recovered from the boreholes reported Unconfined Compressive Strength (UCS) values ranging between 88.1 and 140.7 MPa while the point load testing gave Is_{50} values ranging between 2.93 to 8.79 MPa. The Is_{50} results correlate to the UCS values using a factor of approximately 20, giving values of 58.6 MPa and 175.8 MPa. These results correlate to the strength descriptions ranging between of Strong to very strong and confirming the descriptions on the logs.

The results from the completed laboratory testing are included in Appendix 8 of this report

4.4. Geotechnical Design Parameters

Preliminary geotechnical design parameters for the materials encountered during the ground investigation have been summarised in Table 1 Geotechnical Design Parameters. Both laboratory test and SPT N results, using standard empirical relationships, have been used to determine the geotechnical parameters of the overburden strata.

Shear strength parameters have been determined using laboratory testing and established empirical relationships for the relevant materials. Based on the relationship published by Stroud, the correlation of $C_u = f_1 \times N$ is used to estimate the undrained shear strength of the cohesive deposits, where f_1 is determined using a correlation with the plasticity index.

The shear strength parameters from the granular stratum are provided using the effective shear strength parameters determined from the uncorrected SPT N values after Peck et al. reported by Tomlinson Foundation Design and Construction 7th Ed. (2001).

A range is provided for the compressibility parameter m_v based on correlations with the SPT N value based on the relationship published by Stroud, the correlation of $M_v = 1/(f_2 \times N)$ where f_2 is determined using a correlation with the plasticity index

Table 1 Recommended Geotechnical Parameters based on GII GI Data

Stratum	Bulk Density (kN/m ³)	SPT 'N' Correlated	Undrained Shear Strength C_u (kN/m ²)	Effective Strength Parameters		Poisson's Ratio ν (ν_u)	Co-efficient of Compressibility m_v (m ² /MN)
				Cohesion c' (kN/m ²)	ϕ' degrees		
Granular Made Ground Deposits	16 – 20* ¹	1 - 20	n/a	-	28 – 30* ⁴	0.1 – 0.3	n/a
Cohesive Made Ground Deposits	16 – 20* ¹	1 - 20	5 – 100* ²	0	25 - 30* ⁴	0.2 (0.5)	0.1-1.5* ³
Soft Peat	10	n/a	0 – 5 kPa	0 – 2	n/a	n/a	>1.5
Soft Cohesive Deposits	16 – 20* ¹	1 - 8	5 - 40* ²	0 - 1	25 - 28* ⁴	0.2 (0.5)	0.1 – 1.5* ³
Firm Cohesive Deposits	18 – 20* ¹	8 – 15	40 - 75* ²	0 - 3	28 – 30* ⁴	0.2 (0.5)	0.1 – 0.3* ³
Stiff Cohesive Deposits	19 – 20* ¹	15 – 25	75 - 150* ²	0 - 5	30 - 33* ⁴	0.2 (0.5)	0.05 – 0.1* ³
Very Stiff Cohesive Deposits	20 – 22	25+	150+	0 – 5	30 – 33+* ⁴	0.2 (0.5)	0.05 – 0.1* ³
Loose Granular Deposits ¹	16 – 18* ¹	1 - 10	n/a	n/a	28 – 30 * ⁴	0.1 – 0.3	n/a
Medium Dense Granular Deposits ¹	18 – 21* ¹	10 - 30	n/a	n/a	30 – 36 * ⁴	0.1 – 0.3	n/a
Dense Granular Deposits ¹	21 - 23* ¹	30+	n/a	n/a	36+ * ⁴	0.1 – 0.3	n/a

*1 Values for bulk density assumed

*2 Based on correlated SPT N values

*3 Based on correlated SPT N values and published data. Caution should be exercised when selecting design values for the variable Made Ground Stratum.

*4 Testing on undisturbed samples is recommended to determine the design value of this parameter for detailed design.

NOTE: The values in Table 1 represent a range of recommended values based on the typical soil types, insitu testing and laboratory testing scheduled by the Consulting Engineer. The values presented are recommended for outline guidance only and specific designs should derive design values based on the exploratory hole logs and lab testing for each specific site. To determine specific design values relevant to the design being undertaken in a particular area, reference should be made to the relevant specific exploratory hole logs. Further testing is recommended to determine the specific geotechnical parameters required for foundation design and temporary works design

5.0 Recommendations & Conclusions

5.1. General

The recommendations given and opinions expressed in this report are based on the findings as detailed in the exploratory hole records. Where an opinion is expressed on the material between exploratory hole locations, this is for guidance only and no liability can be accepted for its accuracy. No responsibility can be accepted for conditions which have not been revealed by the exploratory holes. Limited information has been provided at the ground investigation stage and any designs based on the recommendations or conclusions should be completed in accordance with the current design codes, taking into account the variation and the specific details contained within the exploratory hole logs.

5.2. Foundations

Due to the presence of soft and compressible Cohesive deposits beneath the footprint of the proposed structure and high loading anticipated, piled foundations are recommended for the proposed building. The type, size and depth of the pile foundations should be confirmed by a specialist piling contractor based on the loading from the proposed building. The floor slab is recommended be suspended and also supported on the building piles.

Negative skin friction from the very soft cohesive deposits should be considered in the pile design due to the possibility of loading from working platforms or the adjacent pavement make up.

The pH and sulphate testing carried out indicate that pH results are near neutral to alkaline, with values ranging from 7.1 to 9.6, and that the water soluble sulphate results range from low to elevated (DS-1 to DS-2) when compared to the guideline values from BRE Special Digest 1:2005. Samples of the Peat (TP3 0.6-1.1m BGL) have elevated Total Sulphur results (0.52%) which result in a high Total Potential Sulphate (TPS) and a subsequent higher classification of the Design Sulphate values in accordance with the BRE digest. Caution is recommended and the removal of the Peat and Made ground stratum to reduce the risk of sulphate attack adjacent to concrete elements as part of the permanent works. Piles in the organic and very soft deposits are recommended to incorporate appropriate measures to resist sulphate attack.

5.3. External Pavements

The proposed pavements are recommended to be designed in accordance with the CBR test results included in the Appendices of this Report. The low CBR test results indicate that a capping layer or a sufficient depth of crushed stone fill may be required. Plate bearing tests are recommended at the time of construction to verify the design assumptions for the proposed pavement make up and to verify adequate compaction has been achieved.

The use of a geogrid and separation membrane may improve the performance of the proposed pavement and enable a more economical pavement design to be achieved, a specialist supplier is recommended to advise of the required strength, depth and type of geotextile for the proposed design.

5.4. Excavations

Short term temporary excavations in the cohesive deposits will remain stable for a limited time only and will require to be appropriately battered or the sides supported if the excavation is below 1.25m BGL or is required to permit man entry.

Excavations in the Made Ground, Peat or soft Cohesive Deposits will require to be appropriately battered or the sides supported due to the low strength of these deposits.

Any excavations which penetrate the granular deposits will require to be appropriately battered or the sides supported and are likely to require dewatering due to the groundwater seepages noted in the exploratory hole logs in the Appendices of this Report.

The groundwater and stability noted on the trial pit logs should be consulted when determining the most appropriate construction methods for excavations. Generally, where significant excavations are required in water bearing granular deposits a cut-off wall may be more cost effective than extensive dewatering. An assessment by a specialist dewatering contractor is recommended to determine the most cost effective approach to the proposed excavation.

Excavations in the upper cohesive are expected to be excavatable with conventional excavation equipment. Any waste material to be removed off site should be disposed of to a suitably licenced landfill.

The environmental testing completed during the ground investigation is reported under the cover of a separate GII Waste Classification/Subsoil Assessment Report.

5.5. Soakaway Design

At the locations of IT01 and IT02 the water level dropped too slowly to allow calculation of 'f' the soil infiltration rate. These locations are therefore not recommended as suitable for soakaway design and construction.

The recommendations provided in this report should be verified in the design of the proposed buildings, using the full details of the loading conditions and taking into consideration the allowable tolerable settlements/movements that the building can accommodate. The founding strata should be inspected and verified by a suitably qualified engineer prior to construction of the building foundations.

APPENDIX 1 - Site Location Plan



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529800E

529900E

530000E

726100N

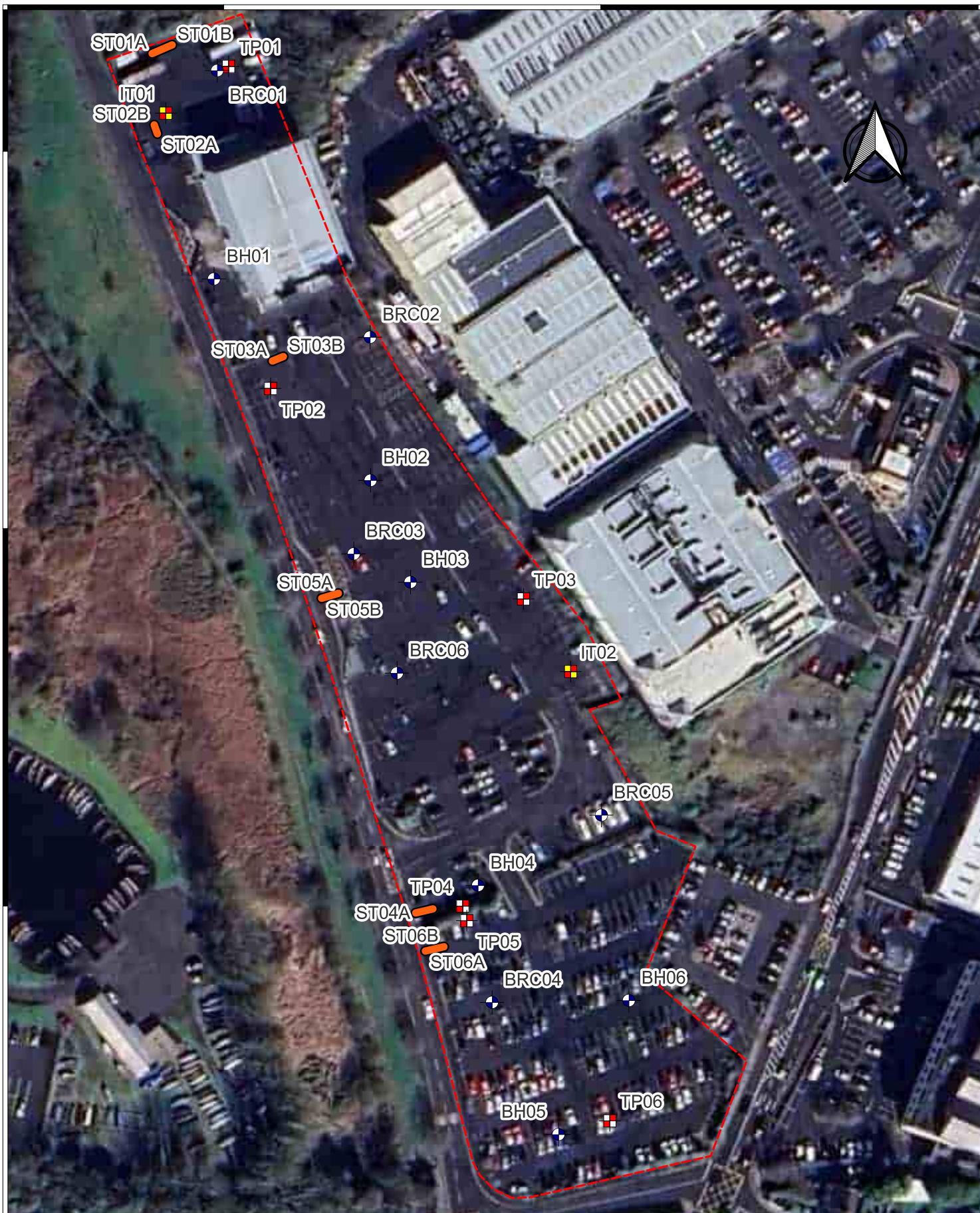
726100N

726000N

726000N

725900N

725900N



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Client:



0 11 22 33 44 55 m



Project Title:

Dyke Road Galway

Drawing Title:

Investigation Locations

GII Project Reference:

13614-02-24

Drawn By:
MS

Date:
24-06-24



Trial Pits



Slit Trench Locations



Borehole



Infiltration Test

APPENDIX 2 – Trial Pit Records





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Site
Dyke Road Galway

Trial Pit Number
TP01

Machine : 3T Tracked Excavator Method : Trial Pit	Dimensions 2.20 x 0.87 x 2.20m	Ground Level (mOD) 4.27	Client Aecom	Job Number 13614-02-24
	Location 529801.3 E 726122.3 N	Dates 17/04/2024	Engineer	Sheet 1/1

Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
0.50	B			4.20	(0.07)	TARMACADAM		
					(0.67)	MADE GROUND: Grey sandy angular to subrounded fine to coarse Gravel		
1.00	HV 34kPa B		44,38,20/Av. 34.00	3.53	0.74	MADE GROUND: Black plastic net membrane		
1.00				3.52	0.75	Brown fibrous PEAT		
					(1.45)			
				2.07	2.20	Complete at 2.20m		

Plan	Remarks No groundwater encountered during excavation Trial pit sidewalls stable DCP carried out at 0.8m BGL. Trial pit backfilled upon completion							
Scale (approx) 1:25						Logged By LB	Figure No. 13614-02-24.TP01	



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Site
Dyke Road Galway

Trial Pit Number
TP02

Machine : 3T Tracked Excavator Method : Trial Pit	Dimensions 2.10 x 0.88 x 2.20m	Ground Level (mOD) 5.01	Client Aecom	Job Number 13614-02-24
	Location 529812.4 E 726036.9 N	Dates 17/04/2024	Engineer	Sheet 1/1

Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
0.50	B			4.94	(0.07)	TARMACADAM		
0.70	HV 14kPa		10,16,16/Av. 14.00	4.42	(0.52)	MADE GROUND: Grey sandy subangular to subrounded fine to coarse Gravel with medium angular to subangular cobble content		
1.00	B		Slow(1) at 1.30m.	4.41	0.59	MADE GROUND: Black plastic net membrane		
2.00	HV 9kPa		10,10,8/Av. 9.33		0.60	MADE GROUND: Brown fibrous Peat with frequent wood fragments and occasional plastic glass and wire fragments		
2.00	B				(1.20)			
				3.21	1.80	Very soft cream clayey SILT with frequent shell fragments		
					(0.40)			
				2.81	2.20	Complete at 2.20m		

Plan	Remarks Groundwater encountered at 1.30m BGL. Slow Trial pit sidewalls stable DCP carried out at 0.7m BGL Trail pit backfilled upon completion							
Scale (approx) 1:25						Logged By LB	Figure No. 13614-02-24.TP02	



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Site Dyke Road Galway	Trial Pit Number TP03
Client Aecom	Job Number 13614-02-24
Engineer	Sheet 1/1

Machine : 3T Tracked Excavator Method : Trial Pit	Dimensions 1.80 x 0.63 x 2.30m	Ground Level (mOD) 5.06
	Location 529879.9 E 725980.6 N	Dates 15/04/2024

Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
0.50	B			4.96	(0.10)	TARMACADAM		
0.65	HV 53kPa		50,60,50/Av. 53.33	4.46	0.60	MADE GROUND: Grey sandy angular to subrounded fine to coarse Gravel		
				4.45	0.61	MADE GROUND: Membrane		
1.00	B				(0.49)	Dark brown fibrous PEAT with occasional rootlets		
1.50	HV 9kPa		10,10,6/Av. 8.67	3.96	1.10	Very soft cream clayey SILT with frequent shell fragments		
2.00	B				(1.20)			
				2.76	2.30	Complete at 2.30m		

Plan	Remarks
	No groundwater encountered during excavation Trial pit sidewalls stable Plate bearing test carried out at 0.20m BGL DCP carried out at 0.7m BGL Trial pit backfilled upon completion
	Scale (approx) 1:25
	Logged By LB
	Figure No. 13614-02-24.TP03



Site	Dyke Road Galway
-------------	------------------

**Trial Pit
Number
TP04**

Machine : 3T Tracked Excavator
Method : Trial Pit

Dimensions
2.40 x 1.00 x 1.20m

Ground Level (mOD)	6.24
--------------------	------

Client	Aecom
---------------	-------

Job Number	13614-02-24
------------	-------------

Location	529863.4 E 725899.1 N
-----------------	-----------------------

Dates	15/04/2024
--------------	------------

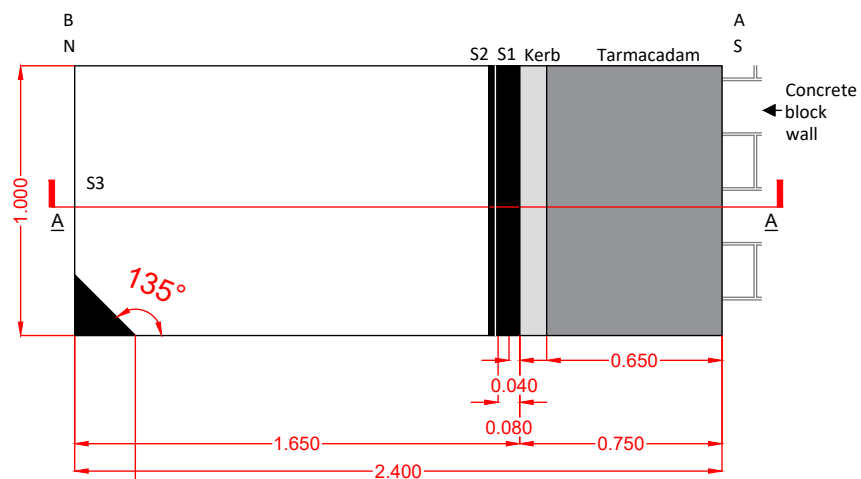
Engineer

Sheet
1/1

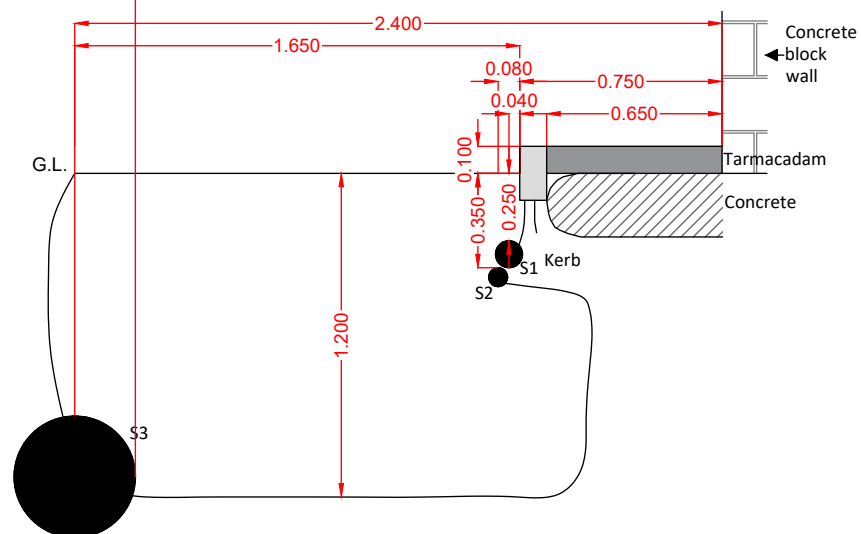
Plan 	Remarks No groundwater encountered during excavation Trial pit sidewalls collapsing DCP carried out at 0.9m BGL. Trial pit backfilled upon completion		
	Scale (approx) 1:25	Logged By LB	Figure No. 13614-02-24.TPO

TP04

PLAN VIEW



SECTION A-A



FOUNDATION PIT LOG

- 0.00 - 0.07 TARMACADAM.
 0.07 - 1.00 MADE GROUND: Grey slightly clayey sandy angular to subrounded fine to coarse Gravel with some cobbles and boulders.
 1.00 - 1.20 MADE GROUND: Brown slightly sandy slightly gravelly Clay with some red brick fragments.

SERVICES:

- S1 - Ø0.100m black duct 90°
 S2 - Ø0.070m black duct 90°
 S3 - Approximately Ø0.450m concrete 45°



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PROJECT:	13614-02-24 - Dyke Road Galway
DRAWING No.:	TP-04
DATE:	15/04/2024
CLIENT:	Aecom
SCALE:	NTS

Version:	Date:	Drawn By:	Checked By:
1	14/05/2024	J.S.	M.S.



Site	Dyke Road Galway
-------------	------------------

**Trial Pit
Number
TP05**

Machine : 3T Tracked Excavator
Method : Trial Pit

Dimensions
1.80 x 0.60 x 1.35m

Ground Level (mOD)	6.24
--------------------	------

Client	Aecom
---------------	-------

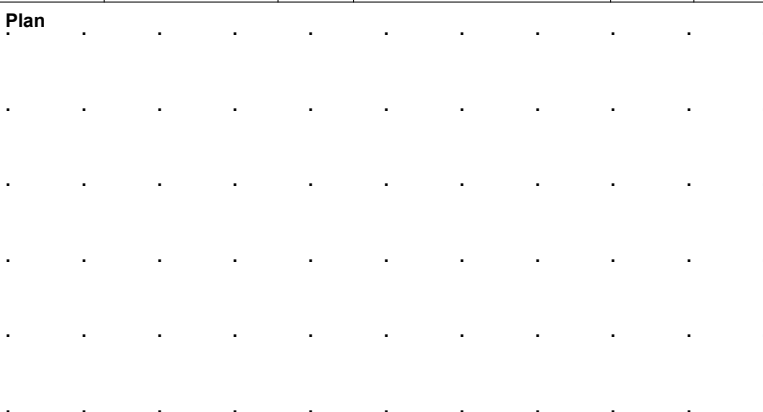
Job Number 13614-02-24

Location	529863.4 E 725899.1 N
-----------------	-----------------------

Dates	15/04/2024
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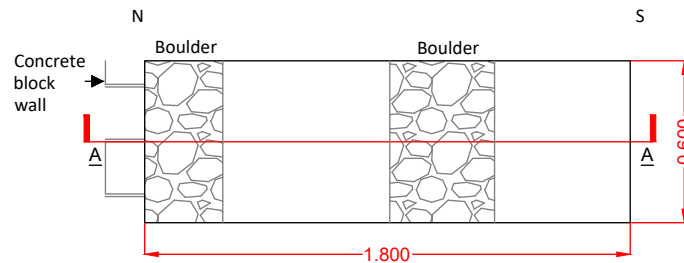
Engineer

Sheet
1/1

Plan 	Remarks		
	No groundwater encountered during excavation Trial pit sidewalls stable Trial pit backfilled upon completion		
	Scale (approx)	Logged By	Figure No.
	1:25	LB	13614-02-24.TPO

TP05

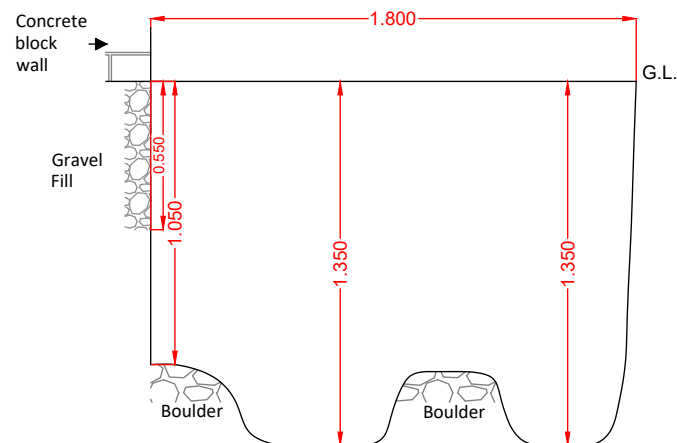
PLAN VIEW



FOUNDATION PIT LOG

0.00 - 0.07	TARMACADAM.
0.07 - 0.55	MADE GROUND: Grey slightly sandy angular to subrounded fine to coarse Gravel.
0.55 - 1.35	MADE GROUND: Light brown slightly clayey slightly gravelly fine to coarse Sand with some angular to subrounded cobbles and boulders.

SECTION A-A



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DRAWING No.:	TP-05
DATE:	15/04/2024
CLIENT:	Aecom
SCALE:	NTS

Version:	Date:	Drawn By:	Checked By:
1	14/05/2024	J.S.	M.S.



Site	Dyke Road Galway
-------------	------------------

**Trial Pit
Number**
TP06

Machine : 3T Tracked Excavator
Method : Trial Pit

Dimensions
2.30 x 0.80 x 0.80m

Ground Level (mOD)	7.16
--------------------	------

Client	Aecom
---------------	-------

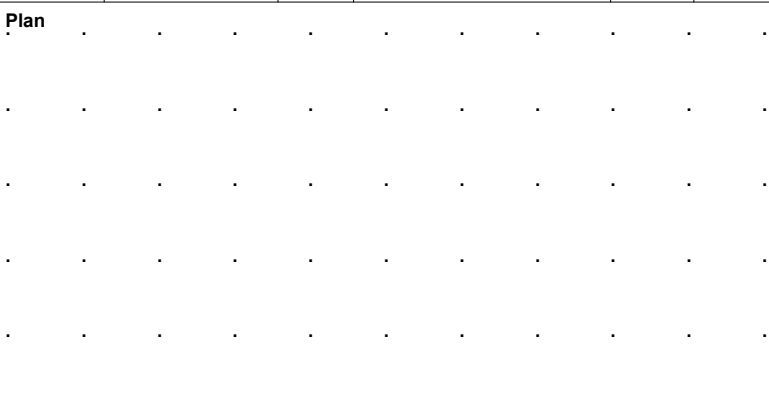
Job Number	13614-02-24
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Location	529902 4 E 725842 8 N
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Dates	17/04/2024
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Engineer

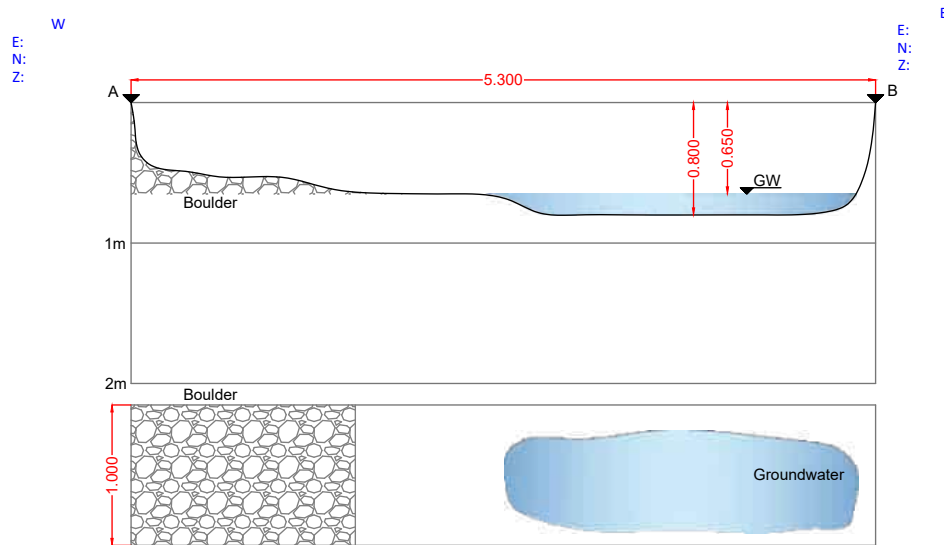
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1/1

<div>Plan</div> 	Remarks		
	No groundwater encountered during excavation Trial pit sidewalls stable Trial pit backfilled upon completion		
Scale (approx)		Logged By	Figure No.
1:25		LB	13614-02-24.TP06

APPENDIX 3 – Slit Trench Records



ST-01



Service No	ø (m)	Colour - Material	Utility	Angle to trench	Coordinates		Level
					East	North	

Surface from/to (m)	Surface type
0.00	5.30
	TARMACADAM

Sample depth (m)	Sample type
0.5	B

From (m)	To (m)	Description
0.00	0.05	TARMACADAM.
0.05	0.20	MADE GROUND: Dark grey slightly sandy angular to subangular fine to coarse Gravel.
0.20	0.80	MADE GROUND: Greyish brown sandy gravelly Clay with some fragments of metal, cans and some angular to subangular cobble content.

Groundwater	Y/N	Depth	Notes
Slow	Y	0.65	



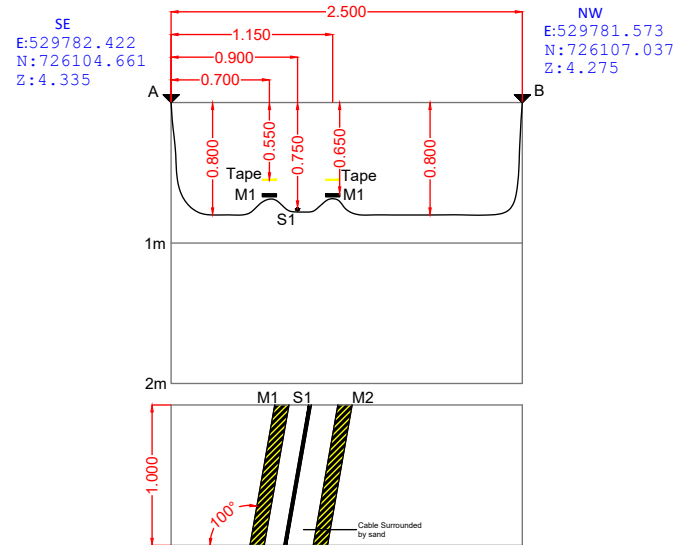
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PROJECT:	13614-02-24 - Dyke Road Galway
DRAWING No.:	ST-01
DATE:	11/04/2024
CLIENT:	Aecom
SCALE:	NTS

Version:	Date:	Drawn By:	Checked By:
1	23/04/2024	J.S.	L.B.

ST-02



Service No	ø (m)	Colour - Material	Utility	Angle to trench	Coordinates		Level
					East	North	
S1	0.020	Black	ESB	100°	529782.061	726105.5	3.559
M1	0.100	Black rubber	-	100°	529782.112	726105.334	3.698
M2	0.100	Black rubber	-	100°	529782.016	726105.736	3.676

Surface from/to (m)	Surface type
0.00	2.50
	TARMACADAM

Sample depth (m)	Sample type
0.5	B

From (m)	To (m)	Description
0.00	0.05	TARMACADAM.
0.05	0.15	MADE GROUND: Dark grey slightly sandy angular to subrounded fine to coarse Gravel.
0.15	0.80	MADE GROUND: Grey sandy gravelly Clay.

Groundwater	Y/N	Depth	Notes
	N		



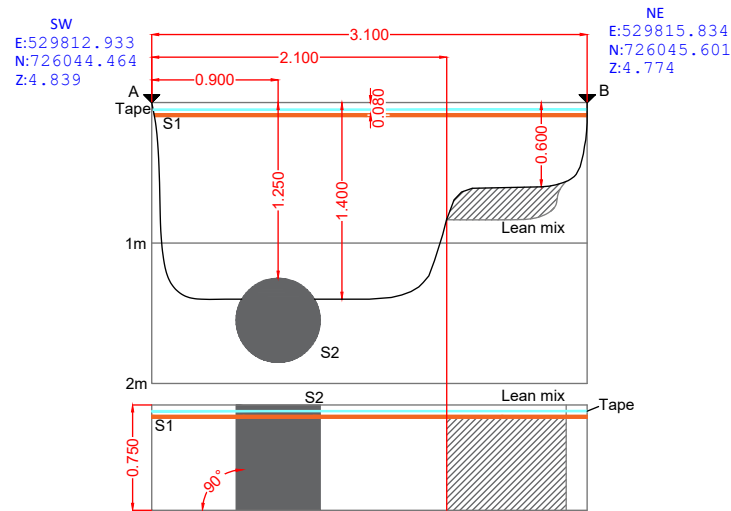
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PROJECT:	13614-02-24 - Dyke Road Galway
DRAWING No.:	ST-02
DATE:	11/04/2024
CLIENT:	Aecom
SCALE:	NTS

Version:	Date:	Drawn By:	Checked By:
1	23/04/2024	J.S.	L.B.

ST-03



Service No	ø (m)	Colour - Material	Utility	Angle to trench	Coordinates		Level
					East	North	
S1	0.020	Orange - Plastic	Fibre optic	0°	529813.155	726044.790	4.710
S1	0.600?	Concrete	Storm	90°	529813.831	726044.651	3.539

Surface from/to (m)	Surface type
0.00	3.10
	TARMACADAM

Sample depth (m)	Sample type
0.5	B

From (m)	To (m)	Description
0.00	0.07	TARMACADAM.
0.07	1.40	MADE GROUND: Grey clayey angular to subrounded fine to coarse Sand and Gravel.

Groundwater	Y/N	Depth	Notes
	N		



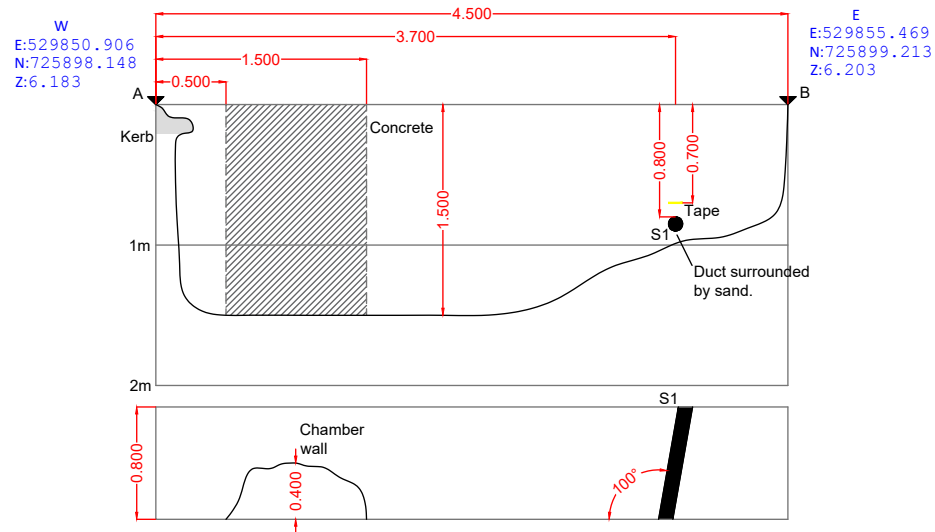
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PROJECT:	13614-02-24 - Dyke Road Galway
DRAWING No.:	ST-03
DATE:	12/04/2024
CLIENT:	Aecom
SCALE:	NTS

Version:	Date:	Drawn By:	Checked By:
1	23/04/2024	J.S.	L.B.

ST-04



Service No	ø (m)	Colour - Material	Utility	Angle to trench	Coordinates		Level
					East	North	
S1	0.100	Black - Duct	ESB	100°	529854.585	725898.986	5.482

Surface from/to (m)	Surface type
0.00	4.50
	TARMACADAM

Sample depth (m)	Sample type
1.0	B

From (m)	To (m)	Description
0.00	0.08	TARMACADAM.
0.08	0.30	MADE GROUND: Grey sandy angular to subrounded fine to coarse Gravel.
0.30	0.50	MADE GROUND: Brown clayey gravelly fine to coarse Sand.
0.50	1.50	MADE GROUND: Grey sandy angular to subrounded fine to coarse Gravel with some plastic fragments and plastic bags.

Groundwater	Y/N	Depth	Notes
	N		



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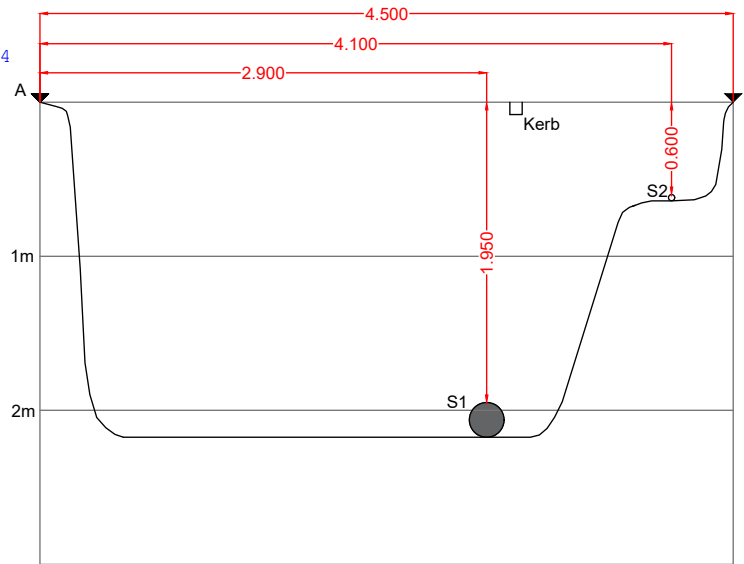
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DRAWING No.:	ST-04
DATE:	17/04/2024
CLIENT:	Aecom
SCALE:	NTS

Version:	Date:	Drawn By:	Checked By:
1	23/06/2024	S.F.	L.B.

ST-05

E
E:529830.397
N:725982.844
Z:5.685

W
E:529825.979
N:725981.532
Z:5.701



Service No	ø (m)	Colour - Material	Utility	Angle to trench	Coordinates		Level
					East	North	
S1	0.225	Grey	Foul	100°	529827.542	725981.833	3.884
S2	0.040	Steel crushed	-	85°	529826.391	725981.661	4.869

Surface from/to (m)		Surface type
0.00	3.05	Concrete
3.05	3.13	Kerb
3.15	4.50	Tarmacadam

Sample depth (m)	Sample type

From (m)	To (m)	Description
0.00	0.10	MADE GROUND: Concrete
0.10	0.20	Tarmacadam
0.20	0.90	MADE GROUND: 804
0.90	0.91	MADE GROUND: Membrane
0.20	0.90	Black fibrous PEAT.

Groundwater	Y/N	Depth	Notes
?	Y	1.50	



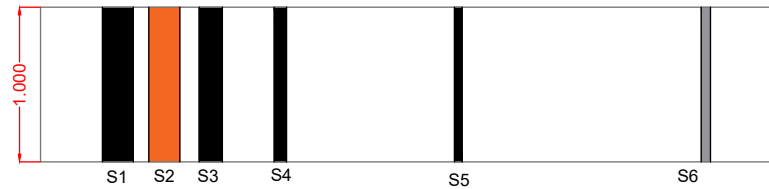
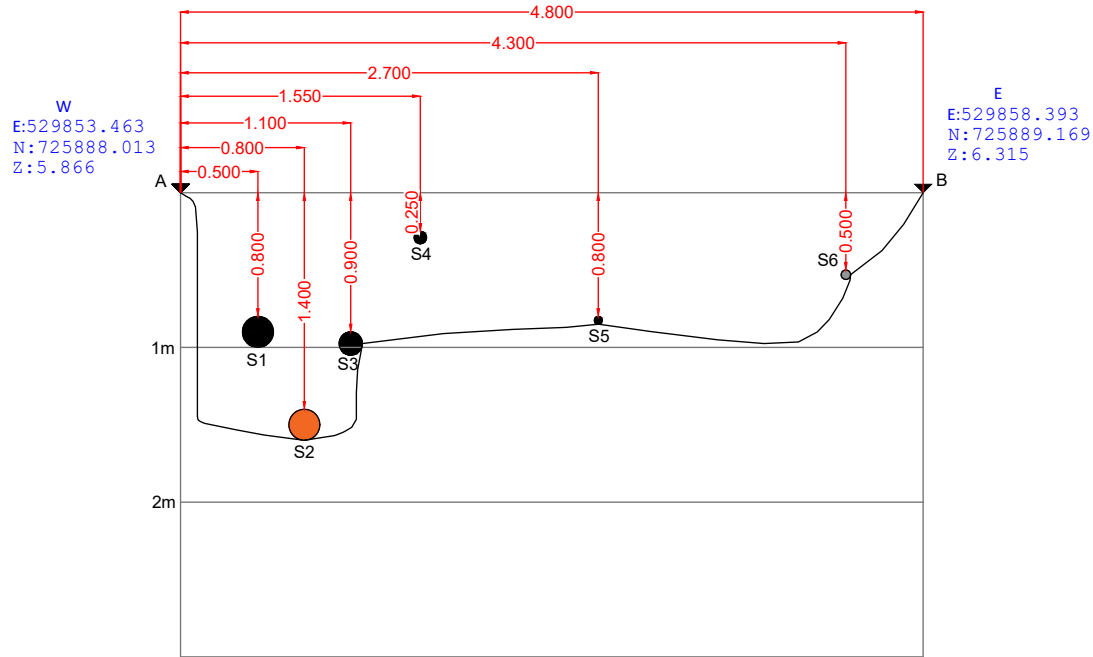
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PROJECT:	13614-02-24 - Dyke Road Galway
DRAWING No.:	ST-05
DATE:	13/06/2024
CLIENT:	Aecom
SCALE:	NTS

Version:	Date:	Drawn By:	Checked By:
1	23/04/2024	S.F	?

ST-06



Service No	ø (m)	Colour - Material	Utility	Angle to trench	Coordinates		Level
					East	North	
S1	0.200	Black PVC		90°	529853.931	725888.107	5.167
S2	0.200	Orange PVC	Foul?	90°	529854.062	725888.283	4.477
S3	0.150	Black PVC		90°	529854.473	725888.281	4.929
S4	0.080	Black Duct	ESB?	90°	529854.87	725888.265	5.760
S5	0.050	Black PVC		90°	529856.04	725888.724	5.411
S6	0.060	Steel bent	?	90°	529857.489	725889.112	5.844

Surface from/to (m)		Surface type
0.00	2.70	TARMACADAM Foot path
2.70	4.80	TARMACADAM Car park

From (m)	To (m)	Description
0.00	0.30	MADE GROUND: Tarmacadam
0.30	0.60	MADE GROUND: 804
0.60	0.61	MADE GROUND: Membrane
0.61	1.5	MADE GROUND: Grey slightly clayey slightly sandy subangular to subrounded fine to coarse Gravel.

Groundwater	Y/N	Depth	Notes
Slow	Y	0.65	



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PROJECT:	13614-02-24 - Dyke Road Galway
DRAWING No.:	ST-01
DATE:	13/06/2024
CLIENT:	Aecom
SCALE:	NTS

Version:	Date:	Drawn By:	Checked By:
1	16/06/2024	S.F.	?

Dyke Road Galway – Slit Trench Photographs

ST06



ST06



Dyke Road Galway – Slit Trench Photographs

ST06



APPENDIX 4 – Soakaway Records





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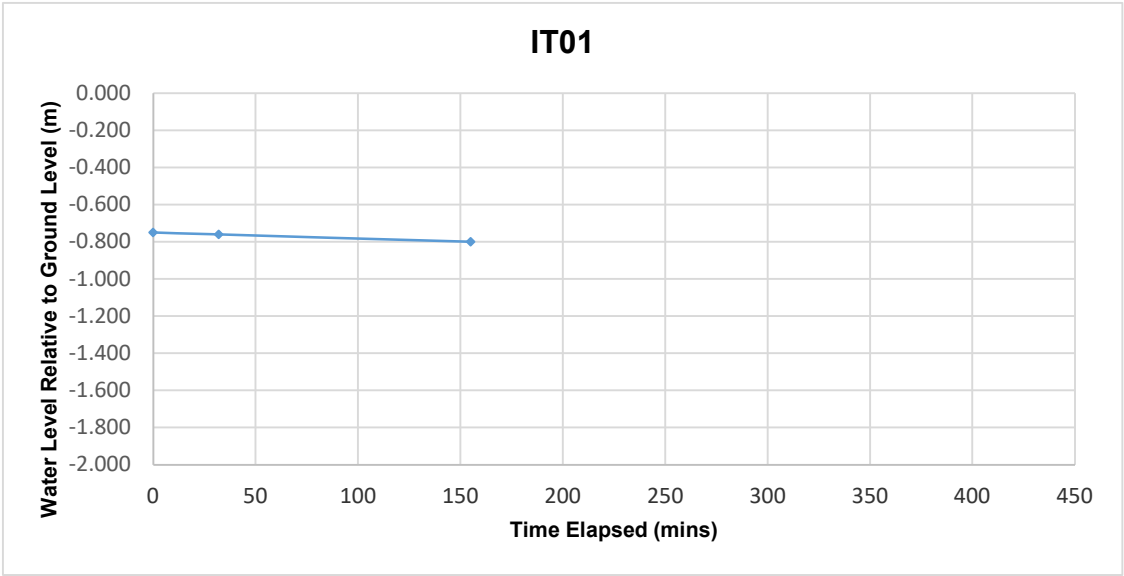
IT01

Soakaway Test to BRE Digest 365
Trial Pit Dimensions: 1.60m x 0.60m x 0.85m (L x W x D)

Date	Time	Water level (m bgl)
15/04/2024	Groundwater at 0.75m BGL	
15/04/2024	0	-0.750
15/04/2024	32	-0.760
15/04/2024	155	-0.800

***Soakaway failed - Pit backfilled**

Start depth	Depth of Pit	Diff	75% full	25%full
0.75	0.850	0.100	0.775	0.825





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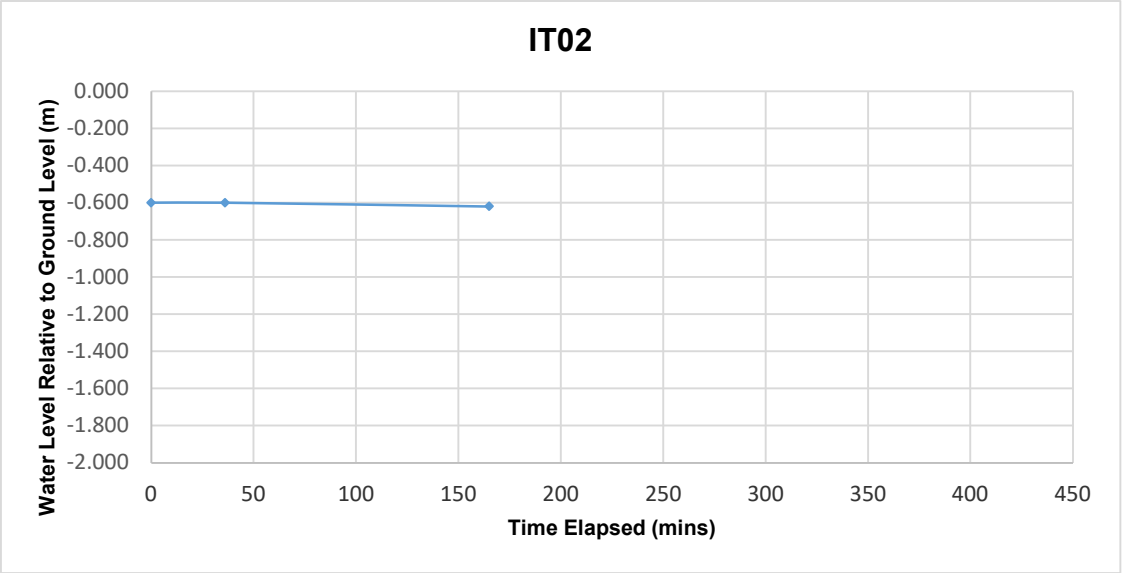
IT02

Soakaway Test to BRE Digest 365
Trial Pit Dimensions: 1.70m x 0.75m x 1.60m (L x W x D)

Date	Time	Water level (m bgl)
15/04/2024	0	-0.600
15/04/2024	36	-0.600
15/04/2024	165	-0.620

*Soakaway failed - Pit backfilled

Start depth	Depth of Pit	Diff	75% full	25%full
0.60	1.600	1.000	0.85	1.35



APPENDIX 5 – Borehole Records





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Site

Dyke Road Galway

Borehole Number

BH01

Machine : Dando 2000 & Beretta T-44

Method : Cable Percussion with Rotary follow on

Casing Diameter

200mm cased to 10.50m
96mm cased to 16.30m

Ground Level (mOD)

4.73

Client

Aecom

Job Number

13614-02-24

Location

529797.4 E 726065.9 N

Dates

26/04/2024-
22/05/2024

Engineer

Sheet

1/2

Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
0.25	ES				4.67	0.06	TARMACADAM		
0.50	ES				4.23	0.50	MADE GROUND: Grey slightly clayey sandy angular to subrounded fine to coarse Gravel		
1.00-1.45	SPT(C) N=2			2,2/1,0,0,1		(1.00)	MADE GROUND: Brownish black slightly clayey slightly sandy gravelly Peat with occasional red brick and ceramic fragments. Gravel is subangular to subrounded fine to coarse		
1.50	ES				3.23	1.50	MADE GROUND: Brownish black clayey slightly sandy slightly gravelly Peat with occasional ceramic and red brick fragments and rubbish. Gravel is subangular to subrounded fine to coarse		
2.00-2.45	SPT(C) N=0			1,0/0,0,0,0		(1.00)			
2.50	B				2.23	2.50	Very soft beige clayey SILT and brown CLAY with frequent shell fragments		
3.00-3.45	UT 100					(1.00)			
3.50	B				1.23	3.50	Very soft beige clayey SILT with frequent shell fragments		
4.00-4.45	SPT(C) N=0			0,0/0,0,0,0					
4.50	B					(2.20)			
5.00-5.45	UT 0								
5.70	B				-0.97	5.70	Very soft dark grey silty CLAY with occasional shell fragments		
6.00-6.45	SPT(C) N=2			1,1/1,0,1,0					
6.70	B								
7.50-7.95	SPT(C) N=1			0,1/0,0,0,1		(3.80)			
7.70	B								
8.70	B								
9.00-9.45	SPT(C) N=2			1,0/0,1,1,0					
9.50	B			Water strike(1) at 9.50m, rose to 4.00m in 20 mins.	-4.77	9.50	Grey slightly sandy slightly gravelly silty CLAY. Gravel is subangular to subrounded fine to coarse		
10.00-10.45	SPT(C) N=1			0,0/0,1,0,0					

Remarks

Cable percussion drilling completed at 10.50m BGL, Rotary drilling completed at 16.30m BGL.
Groundwater encountered at 9.50m BGL
Borehole backfilled upon completion

Scale (approx)

1:50

Logged By

AM

Figure No.

13614-02-24.BH01



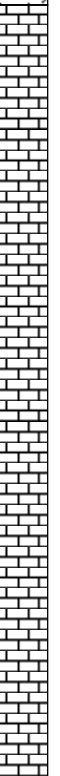


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Site
Dyke Road Galway

Borehole
Number
BH01

Machine : Dando 2000 & Beretta T-44 Method : Cable Percussion with Rotary follow on	Casing Diameter 200mm cased to 10.50m 96mm cased to 16.30m	Ground Level (mOD) 4.73	Client Aecom	Job Number 13614-02-24
	Location 529797.4 E 726065.9 N	Dates 26/04/2024- 22/05/2024	Engineer	Sheet 2/2

Depth (m)	Sample / Tests		Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
10.50					1,10/50 SPT(C) 50/5	-5.77	(1.00)	Very dense grey subangular to subrounded fine to coarse GRAVEL with low cobble content		
	TCR	SCR	RQD	FI			10.50			
11.00-11.16	74						-6.47	(0.70)	Strong thinly to medium bedded grey finely to medium grained fossiliferous LIMESTONE. Fresh	
11.00				11.20		(11.20 - 16.30m BGL) 1 fracture set. 10-30 degrees, closely to medium spaced, undulating, rough with occasional Clay staining				
11.20	100	87	61							
12.50				6						
	100	88	72							
14.00									(5.10)	
	100	87	69							
15.50	100	100	84							
16.30						-11.57	16.30	Complete at 16.30m		

Remarks	Scale (approx)	Logged By
	1:50	AM
	Figure No. 13614-02-24.BH01	



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Site
Dyke Road Galway

Borehole Number
BH02

Machine : Dando 2000	Casing Diameter 200mm cased to 9.70m	Ground Level (mOD) 5.08	Client Aecom	Job Number 13614-02-24
Method : Cable Percussion	Location 529838.9 E 726012.6 N	Dates 10/04/2024- 11/04/2024	Engineer	Sheet 1/1

Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
0.50	ES				5.03	0.05	TARMACADAM		
						(0.45)	Light grey slightly sandy subangular to subrounded fine to coarse gravel FILL		
					4.58	0.50	Brownish black mottled light brown slightly clayey slightly sandy slightly gravelly PEAT. Gravel is subangular to subrounded fine to coarse		
1.00	B			2 blows		(1.00)			
1.00	ES								
1.00-1.45	UT 60%								
1.50	B				3.58	1.50	Very soft greyish light brown slightly sandy clayey SILT		
2.00-2.45	SPT(C) N=1			0,0/0,0,1,0		(1.50)			
2.50	B								
3.00	B			3 blows	2.08	3.00	Grey slightly sandy slightly gravelly clayey SILT. Gravel is subangular to subrounded fine to coarse		
3.00-3.45	UT 100%					(0.60)			
3.60	B				1.48	3.60	Soft to firm grey slightly sandy slightly gravelly clayey SILT with occasional cobbles. Gravel is subangular to subrounded fine to coarse		
4.00-4.45	SPT(C) N=7			0,1/1,0,2,4					
4.60	B					(2.40)			
5.00-5.45	SPT(C) N=12			2,2/1,3,2,6					
5.60	B								
6.00-6.45	SPT(C) N=27			9,6/5,6,8,8	-0.92	6.00	Stiff to very stiff grey slightly sandy slightly gravelly CLAY with occasional cobbles. Gravel is subangular to subrounded fine to coarse		
6.00	B								
7.00	B								
7.50-7.78	SPT(C) 50/125			9,15/37,13		(3.40)			
8.00	B								
9.00-9.45	SPT(C) N=32			14,15/13,3,2,14					
9.00	B								
9.40	B			Water strike(1) at 9.50m, rose to 2.80m in 20 mins.	-4.32	9.40	Grey slightly clayey very sandy subangular to subrounded fine to coarse GRAVEL with occasional cobbles		
					-4.62	9.70	Complete at 9.70m		

Remarks

Cable percussion drilling refused at 9.70m BGL
Groundwater encountered at 9.50m BGL
Borehole backfilled upon completion
Chiselling from 7.50m to 8.00m for 0.5 hours.

Scale (approx)
1:50

Logged By
AM

Figure No.
13614-02-24.BH02




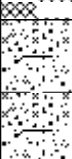
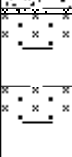

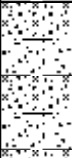


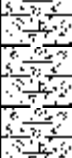
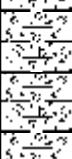
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Site
Dyke Road Galway

Borehole Number
BH03

Machine : Dando 2000	Casing Diameter 200mm cased to 9.10m	Ground Level (mOD) 5.21	Client Aecom	Job Number 13614-02-24
Method : Cable Percussion	Location 529849.5 E 725985.6 N	Dates 12/04/2024- 15/04/2024	Engineer	Sheet 1/1

Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
1.00 1.00-1.45	B UT 100			2 blows	5.15 4.21	0.06 1.00 (0.94) (1.00)	TARMACADAM Grey sandy angular to subrounded fine to coarse gravel FILL Greyish brown and beige slightly sandy slightly gravelly clayey SILT with pockets of brownish black Peat. Gravel is subangular to subrounded fine to coarse	 	
2.00-2.45 2.00	SPT(C) N=1 B			0,0/0,0,0,1	3.21	2.00 (1.00)	Very soft greyish beige clayey SILT with occasional shell fragments		
3.00 3.00-3.45 3.40	B UT 100 B			3 blows	2.21 1.81	3.00 (0.40) 3.40	Brownish grey peaty silty CLAY with frequent organics Very soft to soft light grey slightly sandy slightly gravelly clayey SILT with occasional cobbles. Gravel is subangular to subrounded fine to coarse	 	
4.00-4.45 4.40	SPT(C) N=4 B			0,1/0,0,1,3		(1.60)			
5.00-5.45 5.40	SPT(C) N=12 B			2,5/4,4,3,1	0.21	5.00 (1.00)	Firm light grey slightly sandy slightly gravelly clayey SILT with occasional cobbles. Gravel is subangular to subrounded fine to coarse		
6.00-6.45 6.00	SPT(C) N=32 B			3,5/9,7,8,8	-0.79	6.00 (2.00)	Stiff to very stiff light grey slightly sandy slightly gravelly CLAY with occasional cobbles. Gravel is subangular to subrounded fine to coarse		
7.00 7.50-7.95 8.00	B SPT(C) N=41 B			2,8/10,8,9,14	-2.79	8.00 (1.10)	Very stiff light grey sandy gravelly CLAY with occasional cobbles. Gravel is subangular to subrounded fine to coarse		
9.00-9.08	SPT(C) 25*/75 50/0			25/50	-3.89	9.10	Complete at 9.10m		

Remarks

Cable percussion drilling refused at 9.10m BGL
No groundwater encountered during drilling
Borehole backfilled upon completion
Chiselling from 9.10m to 9.10m for 1 hour.

Scale (approx)
1:50

Logged By
AM

Figure No.
13614-02-24.BH03



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Site
Dyke Road Galway

Borehole
Number
BH04

Machine : Dando 2000 Method : Cable Percussion	Casing Diameter 200mm cased to 2.80m	Ground Level (mOD) 6.09	Client Aecom	Job Number 13614-02-24
	Location 529867.4 E 725905.2 N	Dates 19/04/2024	Engineer	Sheet 1/1

Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
0.30	B				6.03	0.06 (0.24)	TARMACADAM		
0.50	ES				5.79	0.30 (0.50)	Grey Sand and Gravel FILL. Gravel is subangular to subrounded fine to coarse		
0.80	B				5.29	0.80	Brownish black slightly gravelly PEAT. Gravel is subangular to subrounded fine to coarse		
1.00-1.45	SPT(C) N=38			14,11/18,8,5,7		(1.00)	Very stiff greenish grey mottled brown sandy gravelly CLAY with occasional cobbles. Possible residual soil. Gravel is subangular to subrounded fine to coarse		
1.50	ES								
1.80	B				4.29	1.80	Dense greenish grey sandy angular to subrounded fine to coarse GRAVEL. Possible weathered rock		
2.30-2.68	SPT(C) 50/230			5,7/9,10,18,13		(1.00)			
					3.29	2.80	Complete at 2.80m		

Remarks Cable percussion drilling refused at 2.80m BGL No groundwater encountered during drilling Borehole backfilled upon completion Chiselling from 0.80m to 1.00m for 0.25 hours. Chiselling from 1.00m to 2.30m for 1 hour. Chiselling from 2.80m to 2.80m for 1 hour.	Scale (approx) 1:50	Logged By AM
	Figure No. 13614-02-24.BH04	



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Site
Dyke Road Galway

Borehole Number
BH05

Machine : Dando 2000	Casing Diameter 200mm cased to 4.50m	Ground Level (mOD) 7.05	Client Aecom	Job Number 13614-02-24
Method : Cable Percussion	Location 529888.8 E 725839.2 N	Dates 25/04/2024	Engineer	Sheet 1/1

Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
0.25	ES				6.99	0.06 (0.34)	TARMACADAM		
0.50	ES				6.65	0.40	Grey Sand and Gravel FILL. Gravel is subangular to subrounded fine to coarse		
1.00-1.30	SPT(C) 50/145 B			14,11/24,26		(1.60)	MADE GROUND: Greyish light brown slightly sandy slightly gravelly CLAY with occasional cobbles. Gravel is subangular to subrounded fine to coarse		
1.50	ES								
2.00-2.45	SPT(C) N=7 B			10,8/3,2,1,1	5.05	2.00 (0.40)	Soft greyish light brown slightly sandy slightly gravelly CLAY with occasional cobbles. Gravel is subangular to subrounded fine to coarse		
2.40	B				4.65	2.40 (0.60)	Soft brownish black slightly clayey slightly gravelly PEAT. Gravel is subangular to subrounded fine to coarse		
3.00-3.45	SPT(C) N=43			7,10/6,5,15,17	4.05	3.00 (0.50)	Brownish black slightly clayey slightly gravelly PEAT. Gravel is subangular to subrounded fine to coarse		
3.50	B			Water strike(1) at 3.50m.	3.55	3.50 (1.00)	Dense bluish grey very sandy angular to subrounded fine to coarse GRAVEL		V1
4.00-4.24	SPT(C) 50/85 B			17,8/31,19					
4.00					2.55	4.50	Terminated on possible bedrock or large boulder		
							Complete at 4.50m		

Remarks Cable percussion drilling refused at 4.50m BGL Groundwater encountered at 3.50m BGL Borehole backfilled upon completion Chiselling from 1.00m to 2.00m for 1 hour. Chiselling from 4.50m to 4.50m for 1 hour.	Scale (approx)	Logged By
	1:50	AM
	Figure No. 13614-02-24.BH05	



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Site
Dyke Road Galway

Borehole Number
BH06

Machine : Dando 2000	Casing Diameter 200mm cased to 5.70m	Ground Level (mOD) 6.89	Client Aecom	Job Number 13614-02-24
Method : Cable Percussion	Location 529907.4 E 725874.7 N	Dates 24/04/2024	Engineer	Sheet 1/1

Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
1.00-1.09	SPT(C) 25*/75			25/50	6.79	0.10	TARMACADAM		
1.00	50/15 B					(0.50)	Grey sandy angular to subrounded fine to coarse Gravel FILL		
1.80	B				6.29	0.60	MADE GROUND: Grey slightly sandy gravelly Clay with occasional cobbles. Gravel is subangular to subrounded fine to coarse		
2.00-2.45	SPT(C) N=9			17,8/2,3,2,2		(1.20)			
2.50	B				5.09	1.80	MADE GROUND: Greyish brown slightly sandy slightly gravelly Clay with occasional cobbles and occasional red brick and charcoal fragments. Gravel is subangular to subrounded fine to coarse		
2.70	B				4.39	2.50	MADE GROUND: Brownish black slightly gravelly Peat with rare red brick fragments. Gravel is subangular to subrounded fine to coarse		
3.00-3.45	UT 100			1 blows	4.19	(0.20)	Brownish black PEAT		
3.40	B					(0.70)			
4.00-4.45	SPT(C) N=32			Water strike(1) at 3.50m, rose to 3.00m in 20 mins.	3.49	3.40	Stiff to very stiff light grey slightly sandy slightly gravelly silty CLAY with occasional cobbles. Gravel is subangular to subrounded fine to coarse		
4.50	B			4,6/8,7,8,9		(1.10)			
5.00-5.25	SPT(C) 50/95			16,9/33,17	2.39	4.50	Very stiff light grey sandy slightly gravelly silty CLAY with occasional cobbles. Gravel is subangular to subrounded fine to coarse		
5.70-5.70	SPT(C) 25*/0			25/50		(1.20)			
	50/0				1.19	5.70	Complete at 5.70m		

Remarks

Cable percussion drilling refused at 5.70m BGL
Groundwater encountered at 3.50m BGL
Borehole backfilled upon completion
Chiselling from 1.00m to 1.80m for 1 hour. Chiselling from 5.00m to 5.70m for 0.5 hours. Chiselling from 5.70m to 5.70m for 1 hour.

Scale (approx)
1:50

Logged By
AM

Figure No.
13614-02-24.BH06



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Site

Dyke Road Galway

Borehole Number

BRC01

Machine : Dando 2000 & Beretta T-44 Method : Cable Percussion with Rotary follow on	Casing Diameter 200mm cased to 10.50m 96mm cased to 16.50m	Ground Level (mOD) 4.15	Client Aecom	Job Number 13614-02-24
	Location 529798.3 E 726121.2 N	Dates 25/04/2024- 21/05/2024	Engineer	Sheet 1/2

Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water	Instr
0.25	ES				4.09	0.06	TARMACADAM			
0.50	ES					(0.64)	Grey slightly clayey Sand and Gravel FILL with occasional tarmacadam fragments. Gravel is subangular to subrounded fine to coarse			
1.00	B			6 blows	3.45	0.70	Very soft brownish black slightly clayey slightly gravelly PEAT. Gravel is subangular to subrounded fine to coarse			
1.00-1.45	UT 60									
1.50	ES					(1.80)				
2.00-2.45	SPT(C) N=0			1,0/0,0,0,0						
2.00	B				1.65	2.50	Beige clayey SILT and brownish black slightly clayey PEAT with frequent shell fragments			
2.50	B			2 blows		(1.00)				
3.00-3.45	UT 0				0.65	3.50	Very soft beige clayey SILT with occasional shell fragments			
3.50	B			0,0/0,1,0,0		(2.00)				
4.00-4.45	SPT(C) N=1									
4.50	B									
5.30-5.75	UT 100			2 blows	-1.35	5.50	Soft brownish dark grey silty CLAY and beige clayey SILT with occasional shell fragments and occasional pockets of Peat			
5.50	B			2,1/2,1,2,2		(2.00)				
6.00-6.45	SPT(C) N=7									
6.50	B									
7.50	B			2 blows	-3.35	7.50	Very soft greyish brown silty CLAY			
7.50-7.95	UT 100					(1.50)				
8.50	B									
9.00	B			Water strike(1) at 9.00m, rose to 8.00m in 20 mins.	-4.85	9.00	Very soft to soft light grey slightly sandy slightly gravelly silty CLAY with occasional cobbles. Gravel is subangular to subrounded fine to coarse			
9.00-9.45	SPT(C) N=0			1,0/0,0,0,0		(1.50)				
10.00-10.45	SPT(C) N=4			1,0/0,1,2,1						

Remarks

Cable percussion drilling completed at 10.50m BGL, Rotary drilling completed at 16.50m BGL
 Groundwater encountered at 9.00m BGL
 50mm standpipe installed to 10.00m BGL. Slotted standpipe installed from 10.00 - 1.00m BGL. Solid standpipe installed from 1.00m BGL - GL with gas tap and flush cover

Scale (approx)

1:50

Logged By

AM

Figure No.

13614-02-24.BRC01



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Site
Dyke Road Galway

Borehole
Number
BRC01

Machine : Dando 2000 &
Beretta T-44
Method : Cable Percussion
with Rotary follow on

Casing Diameter
200mm cased to 10.50m
96mm cased to 16.50m

Ground Level (mOD)
4.15

Client
Aecom

**Job
Number**
13614-02-24

Location
529798.3 E 726121.2 N

Dates
25/04/2024-
21/05/2024

Engineer

Sheet
2/2

Depth (m)	Sample / Tests		Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water	Instr
10.50	TCR	SCR	RQD	FI	0,0/0,1,4,3 SPT(C) N=8	-6.35	10.50 (0.50)	Poor recovery. Recovery consists of grey peaty clayey SILT onto grey clayey silty subangular to subrounded fine to coarse GRAVEL			
11.00-11.45 11.00	64					-6.85	11.00 (0.50)	Poor recovery. Recovery consists of slightly clayey slightly sandy subangular to subrounded fine to coarse GRAVEL.			
11.50	80	50	45			-7.35	11.50	Strong thinly to medium bedded grey finely to medium grained fossiliferous LIMESTONE. Slightly weathered			
12.50-12.60 12.50				9	C		(1.50)	(11.50 - 13.00m BGL) 2 fracture sets. FS1: 10-30 degrees, closely to medium spaced, undulating, rough with occasional Clay staining. FS2: 70-80 degrees, widely spaced, undulating, rough with occasional Clay staining			
13.00	100	97	77			-8.85	13.00	Strong thinly to medium bedded grey finely to medium grained fossiliferous LIMESTONE. Fresh			
14.00 14.65-14.88				7	C		(3.50)	(13.00 - 16.50m BGL) 3 fracture sets. FS1: 0-20 degrees, closely to medium spaced, undulating, rough with occasional Clay staining. FS2: 30-40 degrees, medium to widely spaced, undulating, rough. FS3: 70-80 degrees, undulating, rough with occasional Clay staining			
15.50 16.05-16.15	100	90	70		C						
16.50						-12.35	16.50	Complete at 16.50m			

Remarks

Scale
(approx)
1:50

Logged
By
AM

Figure No.
13614-02-24.BRC01



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Site
Dyke Road Galway

Borehole Number
BRC02

Machine : Dando 2000 & Beretta T-44
Method : Cable Percussion with Rotary follow on

Casing Diameter
200mm cased to 10.10m
146mm cased to 12.40m
96mm cased to 18.60m

Ground Level (mOD)
5.08

Client
Aecom

Job Number
13614-02-24

Location
529838.8 E 726050.5 N

Dates
15/04/2024-
15/05/2024

Engineer

Sheet
1/2

Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water	Instr
0.25	ES				5.02	0.06	TARMACADAM			
0.50	ES					(0.94)	Grey Sand and Gravel FILL with occasional tarmacadam fragments. Gravel is subangular to subrounded fine to coarse			
1.00	B				4.08	1.00	Very soft brownish black slightly gravelly PEAT with occasional pockets of beige clayey SILT. Gravel is subangular to subrounded fine to coarse			
1.50	ES			4 blows		(1.50)				
1.60-2.05	UT 60									
2.00-2.45	SPT N=0			0,0/0,0,0,0						
2.00	B				2.58	2.50	Very soft beige clayey SILT with frequent shell fragments			
2.50	B									
3.00-3.45	UT 100			3 blows						
3.50	B					(2.20)				
4.00-4.45	SPT N=0			1,0/0,0,0,0						
4.70	B				0.38	4.70	Brownish dark grey peaty silty CLAY			
5.00-5.45	UT 100			3 blows		(1.30)				
5.70	B									
6.00	B			Water strike(1) at 6.00m, rose to 3.00m in 20 mins.	-0.92	6.00	Very stiff light grey slightly sandy very gravelly silty CLAY with low cobble content. Gravel is subangular to subrounded fine to coarse.			
6.00-6.45	SPT N=31			2,1/1,10,5,15						
7.00	B									
7.50-7.79	SPT(C) 50/135			21,4/32,18						
8.00	B					(4.10)				
9.00-9.31	SPT(C) 50/160			10,13/20,24,6						
9.00	B									
9.70-9.95	SPT(C) 50/95			25/31,19						

Remarks

Cable percussion drilling completed at 10.10m BGL, Rotary drilling completed at 18.60m BGL
Groundwater encountered at 6.00m BGL
50mm standpipe installed to 6.00m BGL. Slotted standpipe installed from 6.00 - 0.50m BGL. Solid standpipe installed from 0.50m BGL - GL with flush cover
Chiselling from 7.50m to 8.00m for 0.5 hours. Chiselling from 8.30m to 8.70m for 0.5 hours. Chiselling from 9.50m to 9.70m for 0.5 hours.

Scale (approx)
1:50

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AM

Figure No.
13614-02-24.BRC02



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Site
Dyke Road Galway

Borehole
Number
BRC02

Machine : Dando 2000 & Beretta T-44 Method : Cable Percussion with Rotary follow on	Casing Diameter 200mm cased to 10.10m 146mm cased to 12.40m 96mm cased to 18.60m	Ground Level (mOD) 5.08	Client Aecom	Job Number 13614-02-24
	Location 529838.8 E 726050.5 N	Dates 15/04/2024- 15/05/2024	Engineer	Sheet 2/2

Depth (m)	Sample / Tests		Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water	Instr
10.10	TCR	SCR	RQD	FI		-5.02	10.10	Poor recovery. Recovery consists of grey slightly sandy slightly gravelly silty CLAY. Gravel is subangular to subrounded fine to coarse.			
	11						(0.90)				
11.00						-5.92	11.00	Poor recovery. Recovery consists of grey slightly sandy slightly gravelly silty CLAY with medium cobble content. Gravel is subangular to subrounded fine to coarse			
	29						(1.40)				
12.40						-7.32	12.40	Grey subangular to subrounded fine to coarse GRAVEL			
	25						(1.10)				
13.50	100	100	22	6	C	-8.42	13.50	Strong thinly to medium bedded grey fine to medium grained fossiliferous LIMESTONE with occasional calcite veins. Fresh			
14.00											
	100	91	50				(2.70)	(13.50 - 16.20m BGL) 3 fracture sets. FS1: 0-20 degrees, closely to medium spaced, undulating, rough with occasional Clay staining. FS2: 40-60 degrees, widely spaced, undulating, rough with occasional Clay staining. FS3: 80-90 degrees, undulating, rough with occasional Clay staining			
15.37-15.50											
15.50											
16.20	100	97	86	3	C	-11.12	16.20	Strong thinly to medium bedded grey fine to medium grained fossiliferous LIMESTONE with occasional calcite veins. Fresh			
17.00											
	100	100	97				(2.40)	(16.20 - 18.60m BGL) 1 fracture set. 0-20 degrees, medium to widely spaced, undulating, rough with occasional Clay staining			
18.20-18.35 18.37-18.60											
18.60						-13.52	18.60	Complete at 18.60m			

Remarks	Scale (approx)	Logged By
	1:50	AM
	Figure No. 13614-02-24.BRC02	



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Site
Dyke Road Galway

Borehole Number
BRC03

Machine : Dando 2000 & Beretta T-44 Method : Cable Percussion with Rotary follow on	Casing Diameter 200mm cased to 10.30m 96mm cased to 20.10m	Ground Level (mOD) 5.37	Client Aecom	Job Number 13614-02-24
	Location 529834.4 E 725993.1 N	Dates 17/04/2024- 16/05/2024	Engineer	Sheet 1/3

Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
0.25	ES				5.31	0.06	TARMACADAM		
0.50	ES						Grey very sandy subangular to subrounded fine to coarse Gravel FILL		
1.50	ES					(1.94)			
2.00-2.45	SPT(C) N=0			2,0/0,0,0,0	3.37	2.00	Very soft brownish grey slightly sandy slightly gravelly silty CLAY with occasional cobbles. Gravel is subangular to subrounded fine to coarse		
2.00	B					(1.00)			
3.00	B			2 blows	2.37	3.00	Very soft light grey slightly sandy slightly gravelly clayey SILT with occasional cobbles. Gravel is subangular to subrounded fine to coarse		
3.00-3.45	U4 100								
4.00-4.45	SPT(C) N=3			1,1/2,1,0,0		(2.00)			
4.00	B								
5.00-5.45	SPT(C) N=6			1,0/0,2,2,2	0.37	5.00	Soft to firm light grey slightly sandy slightly gravelly silty CLAY with occasional cobbles. Gravel is subangular to subrounded fine to coarse		
5.00	B					(1.00)			
6.00-6.45	SPT(C) N=16			1,1/2,4,5,5	-0.63	6.00	Firm to stiff light grey slightly sandy slightly gravelly silty CLAY with occasional cobbles. Gravel is subangular to subrounded fine to coarse		
6.00	B					(1.50)			
7.50-7.95	SPT(C) N=49			6,8/9,8,9,23	-2.13	7.50	Very stiff grey slightly sandy slightly gravelly CLAY with occasional cobbles. Gravel is subangular to subrounded fine to coarse		
7.50	B								
8.50	B								
9.00-9.23	SPT(C) 50/80			15,10/46,4		(2.80)			
9.50	B								
10.00-10.22	SPT(C) 50/70			23,2/50					

Remarks Cable percussion drilling refused at 10.30m BGL, Rotary drilling completed at 20.10m BGL No groundwater encountered during drilling Borehole backfilled upon completion Chiselling from 8.00m to 8.30m for 0.25 hours. Chiselling from 9.00m to 9.50m for 0.5 hours. Chiselling from 9.50m to 10.00m for 0.5 hours.	Scale (approx) 1:50	Logged By AM
	Figure No. 13614-02-24.BRC03	



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Site
Dyke Road Galway

Borehole Number
BRC03

Machine : Dando 2000 & Beretta T-44 Method : Cable Percussion with Rotary follow on	Casing Diameter 200mm cased to 10.30m 96mm cased to 20.10m	Ground Level (mOD) 5.37	Client Aecom	Job Number 13614-02-24
	Location 529834.4 E 725993.1 N	Dates 17/04/2024- 16/05/2024	Engineer	Sheet 2/3

Depth (m)	Sample / Tests		Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
10.30	TCR	SCR	RQD	FI	25/50 SPT(C) 25*/75 50/0	-4.93	10.30	Very stiff to hard brownish grey slightly sandy slightly gravelly CLAY with low cobble content. Gravel is subangular to subrounded fine to coarse		
11.00-11.08 11.00	100									
12.50-12.58 12.50	100						(5.00)			
14.00-14.08 14.00	100	12	7		25/50 SPT(C) 25*/75 50/0			Strong thinly to medium bedded grey finely to medium grained fossiliferous LIMESTONE with occasional calcite veins. Fresh (15.30 - 17.50m BGL) 1 fracture set, 0-30 degrees, closely spaced, undulating, rough with occasional Clay staining		
15.30 15.50						-9.93	15.30			
16.40-16.50	100	90	70	8	C		(2.20)			
17.17-17.45 17.00					C			Strong thinly to medium bedded grey finely to medium grained fossiliferous LIMESTONE with occasional calcite veins. Fresh (17.50 - 20.10m BGL) 2 fracture sets. FS1: 0-30 degrees, closely to medium spaced, undulating, rough with occasional Clay staining. FS2: 60-70 degrees, widely spaced, undulating, rough with occasional Clay staining		
17.50	100	97	71			-12.13	17.50			
18.50				7			(2.60)			
19.95-20.05	100	100	84		C					

Remarks	Scale (approx) 1:50	Logged By AM
	Figure No. 13614-02-24.BRC03	



Site	Dyke Road Galway
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Borehole
Number
BRC03

Machine :	Dando 2000 & Beretta T-44
Flush :	Water
Core Dia:	63.5 mm
Method :	Cable Percussion with Rotary follow on

Casing Diameter
200mm cased to 10.30m
96mm cased to 20.10m

Ground Level (mOD)	5.37
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Client	Aecom
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Job Number
13614-02-24

Location	529834.4 E 725993.1 N
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Dates	17/04/2024- 16/05/2024
--------------	---------------------------

Engineer

Sheet
3/3

Depth (m)	TCR (%)	SCR (%)	RQD (%)	FI	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
20.10						-14.73	20.10	Complete at 20.10m		

Remarks

Scale (approx)

1:50

Logged
By

AM

Figure No.
13614-02-24.BRC03



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Site
Dyke Road Galway

Borehole Number
BRC04

Machine : Dando 2000 and Beretta T-44 Method : Cable Percussion with Rotary follow on	Casing Diameter 200mm cased to 6.50m 96mm cased to 12.00m	Ground Level (mOD) 6.94	Client Aecom	Job Number 13614-02-24
	Location 529871.1 E 725874.2 N	Dates 22/04/2024-16/05/2024	Engineer	Sheet 1/2

Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water	Instr
0.25	ES				6.88	0.06	TARMACADAM			
0.50	ES					(0.94)	MADE GROUND: Grey Sand and Gravel. Gravel is angular to subangular fine to coarse			
1.00-1.45	SPT(C) N=15			5,4/6,4,2,3	5.94	1.00	MADE GROUND: Brownish grey sandy slightly gravelly Clay with occasional glass wire and rubbish fragments and occasional cobbles. Gravel is subangular to subrounded fine to coarse			
1.50	ES					(1.60)				
2.00-2.45	SPT(C) N=11			3,2/2,3,3,3						
2.00	B				4.34	2.60	MADE GROUND: Brownish black slightly gravelly Peat. Gravel is subangular to subrounded fine to coarse			
2.60	B				4.14	(0.20) 2.80				
3.00-3.45	UT 100			14 blows		(0.60)	MADE GROUND: Beige slightly gravelly clayey SILT with pockets of brownish black Peat and wire fragments. Gravel is subangular to subrounded fine to coarse			
3.40	B				3.54	3.40				
						(0.60)	Light grey slightly sandy gravelly silty CLAY with occasional cobbles. Gravel is subangular to subrounded fine to coarse.			
4.00	B			Water strike(1) at 4.00m, rose to 3.50m in 20 mins. 19,6/5,4,4,4	2.94	4.00	Medium dense to dense greenish grey very sandy subangular to subrounded fine to coarse GRAVEL with occasional cobbles			
4.00-4.45	SPT(C) N=17									
5.00-5.38	SPT(C) 50/226			8,12/14,14,21,1		(2.50)				
5.00	B									
6.00-6.30	SPT(C) 50/151			10,15/19,29,2						
6.40	TCR	SCR	RQD	FI						
6.50	100				0.44	6.50	Dark green angular to subangular fine to coarse GRAVEL. Possible weathered bedrock			
6.60					0.34	6.60	Strong to very strong thinly to thickly banded dark green medium to coarsely crystalline METAGABBRO with occasional calcite veins. Fresh			
	100	93	87				(6.60 - 12.00m BGL) 2 fracture sets. FS1: 10-30 degrees, closely to medium spaced, planar to undulating, rough to striated. FS2: 30-40 degrees, widely spaced, planar to undulating, rough			
8.00				C						
8.40-8.57	100	93	89							
9.50-9.60				5		(5.40)				
9.50										

Remarks Cable percussion drilling refused at 6.50m BGL on bedrock, Rotary drilling completed at 12.00m BGL Groundwater encountered at 4.00m BGL 50mm standpipe installed to 6.00m BGL. Slotted standpipe installed from 6.00 - 0.50m BGL. Solid standpipe installed from 0.5m BGL - GL with flush cover Chiselling from 4.00m to 4.50m for 0.5 hours. Chiselling from 6.50m to 6.50m for 1 hour.	Scale (approx) 1:50	Logged By AM
	Figure No. 13614-02-24.BRC04	



Machine : Dando 2000 and Beretta T-44 Flush : Water Core Dia : 63.5 mm Method : Cable Percussion with Rotary follow on		Casing Diameter 200mm cased to 6.50m 96mm cased to 12.00m		Ground Level (mOD) 6.94	Client Aecom	Job Number 13614-02-24
		Location 529871.1 E 725874.2 N		Dates 22/04/2024- 16/05/2024	Engineer	Sheet 2/2

Depth (m)	TCR (%)	SCR (%)	RQD (%)	FI	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water	Instr
10.11-10.30	100	93	75		C						
11.00	100	100	83								
12.00						-5.06	12.00	Complete at 12.00m			

Remarks	Scale (approx)	Logged By
	1:50	AM
	Figure No. 13614-02-24.BRC04	



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Site
Dyke Road Galway

Borehole Number
BRC05

Machine : Dando 2000 & Beretta T-44 Method : Cable Percussion with Rotary follow on	Casing Diameter 200mm cased to 6.10m 96mm cased to 11.00m	Ground Level (mOD) 5.53	Client Aecom	Job Number 13614-02-24
	Location 529900.1 E 725923.8 N	Dates 18/04/2024-20/05/2024	Engineer	Sheet 1/2

Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water	Instr
0.50	ES				5.47	0.06	TARMACADAM			
0.80	B					(0.74)	MADE GROUND: Grey slightly clayey Sand and Gravel. Gravel is subangular to subrounded fine to coarse			
1.00-1.45	UT 80			3 blows	4.73	0.80	Brownish black slightly clayey slightly gravelly PEAT. Gravel is subangular to subrounded fine to coarse			
1.50	B				4.03	1.50	Very soft beige clayey SILT with occasional shell fragments			
1.50	ES					(0.90)				
2.00-2.45	SPT N=0			0,0/0,0,0,0	3.13	2.40	Dark brown peaty CLAY			
2.40	B				2.93	(0.20)	Light grey slightly gravelly clayey SILT with occasional cobbles. Gravel is subangular to subrounded fine to coarse			
2.60	B					(0.90)				
3.00-3.45	UT 0			Water strike(1) at 3.00m, rose to 2.90m in 20 mins. 8 blows	2.03	3.50	Very stiff light grey slightly sandy slightly gravelly silty CLAY with occasional cobbles. Gravel is subangular to subrounded fine to coarse			
3.50	B									
4.00-4.45	SPT(C) N=39			7,8/9,9,11,10						
4.50	B					(2.30)				
5.00-5.45	SPT(C) N=40			Water strike(2) at 5.00m, rose to 3.00m in 20 mins. 6,11/9,14,7,10						
5.80	B				-0.27	5.80	Dense bluish grey sandy angular to subangular fine to coarse GRAVEL with occasional cobbles. Gravel is subangular to subrounded fine to coarse			
5.80-6.09	SPT(C) 50/135			8,12/10,40	-0.57	(0.30)				
6.10	TCR	SCR	RQD	FI		6.10	Strong thinly to medium bedded grey finely to medium grained fossiliferous LIMESTONE with occasional calcite veins. Slightly weathered (6.10 - 7.20m BGL) 1 fracture set. 10-30 degrees, very closely to closely spaced, undulating, rough with occasional Clay staining			
6.50	100	85	27	15		(1.10)				
7.20	100	82	56		-1.67	7.20	Strong thinly to medium bedded grey finely to medium grained fossiliferous LIMESTONE with occasional calcite veins. Fresh (7.20 - 11.00m BGL) 1 fracture set. 10-30 degrees, closely to medium spaced, undulating, rough with occasional Clay staining			
7.71-7.80				C						
8.00										
9.25-9.36	100	97	38	8		(3.80)				
9.65-9.88				C						
9.50										

Remarks

Cable percussion drilling refused at 6.10m BGL. Rotary drilling completed at 11.00m BGL
 Groundwater encountered at 3.00m and 5.00m BGL
 50mm standpipe installed to 11.00m BGL. Slotted standpipe installed from 11.00 - 1.00m BGL. Solid standpipe installed from 1.00m BGL - GL with gas tap and flush cover
 Chiselling from 5.50m to 5.80m for 0.5 hours. Chiselling from 6.10m to 6.10m for 1 hour.

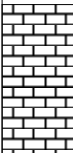

Scale (approx)
1:50

Logged By
AM

Figure No.
13614-02-24.BRC05



Machine : Dando 2000 & Beretta T-44 Flush : Water Core Dia : 63.50 mm Method : Cable Percussion with Rotary follow on			Casing Diameter 200mm cased to 6.10m 96mm cased to 11.00m			Ground Level (mOD) 5.53		Client Aecom		Job Number 13614-02-24	
			Location 529900.1 E 725923.8 N			Dates 18/04/2024- 20/05/2024		Engineer		Sheet 2/2	

Depth (m)	TCR (%)	SCR (%)	RQD (%)	FI	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water	Instr
11.00	100	97	73			-5.47	11.00				
								Complete at 11.00m			



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Site
Dyke Road Galway

Borehole Number
BRC06

Machine : Beretta T-44 Flush : Water Core Dia : 63.5 mm Method : Rotary Cored			Casing Diameter 96mm cased to 14.40m			Ground Level (mOD) 5.43		Client Aecom		Job Number 13614-02-24	
			Location 529845.9 E 725961.5 N			Dates 22/05/2024		Engineer		Sheet 1/2	

Depth (m)	TCR (%)	SCR (%)	RQD (%)	FI	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water		
2.00 2.00-2.45	33				1,0/0,0,0,0 SPT(C) N=0	5.40	0.03	TARMACADAM				
							(1.97)	Possible MADEGROUND: Poor recovery. Recovery consists of grey slightly clayey subangular to subrounded fine to coarse GRAVEL				
	10				0,0/0,0,0,0 SPT(C) N=0	3.43	2.00	Poor recovery. Recovery consists of grey sandy slightly gravelly silty CLAY with low cobble content. Gravel is subangular to subrounded fine to coarse. Driller notes: Grey silts (Very soft)				
							(4.50)					
	3.50 3.50-3.95	13						0,0/0,0,0,0 SPT(C) N=0				
5.00 5.00-5.45	27				0,0/1,0,0,0 SPT(C) N=1							
6.50 6.50-6.95	100				3,4/4,5,4,7 SPT(C) N=20	-1.07	6.50	Grey clayey sandy subangular to subrounded fine to coarse GRAVEL with low cobble content. Driller notes: sandy gravelly Cay (Stiff)				
8.00 8.00-8.45	37	7	0		0,5/7,8,8,12 SPT(C) N=35	-1.97	7.40	Grey gravelly fine to coarse SAND. Gravel is subangular to subrounded fine to medium				
9.40 9.50						-2.57	8.00	Poor recovery. Recovery consists of grey slightly clayey subangular to rounded fine to coarse GRAVEL with low cobble content (Dense)				
						-3.97	9.40	Medium strong to strong thinly to medium bedded grey finely to medium grained fossiliferous LIMESTONE with occasional calcite veins. Slightly weathered				

Remarks Rotary drilling completed at 14.40m BGL Borehole backfilled upon completion									Scale (approx) 1:50	Logged By AM
									Figure No. 13614-02-24.BRC06	



Site	Dyke Road Galway
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Borehole
Number
BRC06

Machine : Beretta T-44

Flush : Water

Core Dia: 63.5 mm

Method : Rotary Cored

Casing Diameter

96mm cased to 14.40m

Ground Level (mOD)

5.43

Client	
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Aecom

Job Number
13614-02-24

Sheet
2/2




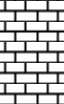

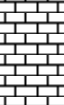

Location

529845 9 F 725961 5 N

Dates

22/05/2024

Engineer

Depth (m)	TCR (%)	SCR (%)	RQD (%)	FI	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water						
10.70 11.00 11.00-11.17 12.50 13.34-13.44 14.00 14.40	100	83	67	6	C	-5.27	(1.30)	(9.40 - 10.70m BGL) 1 fracture set. 10-30 degrees, closely to medium spaced, undulating, rough with occasional Clay staining								
	100	80	43	11			C	(3.70)	Medium strong to strong thinly to medium bedded grey and dark grey finely grained argillaceous LIMESTONE with frequent mudstone laminations and occasional calcite veins. Fresh							
					(10.70 - 14.40m BGL) 3 fracture sets. FS1: 0-20 degrees, closely to medium spaced, planar to undulating, rough with occasional Clay staining. FS2: 40-50 degrees, medium to widely spaced, planar to undulating, rough with occasional Clay staining. FS3: 60-70 degrees, widely spaced, undulating, rough											
						100			93		20	C	-8.97	14.40	Complete at 14.40m	
																

Remarks

Scale (approx)

1:50

Logged
By

AM

Figure No.

13614-02-24.BRC06

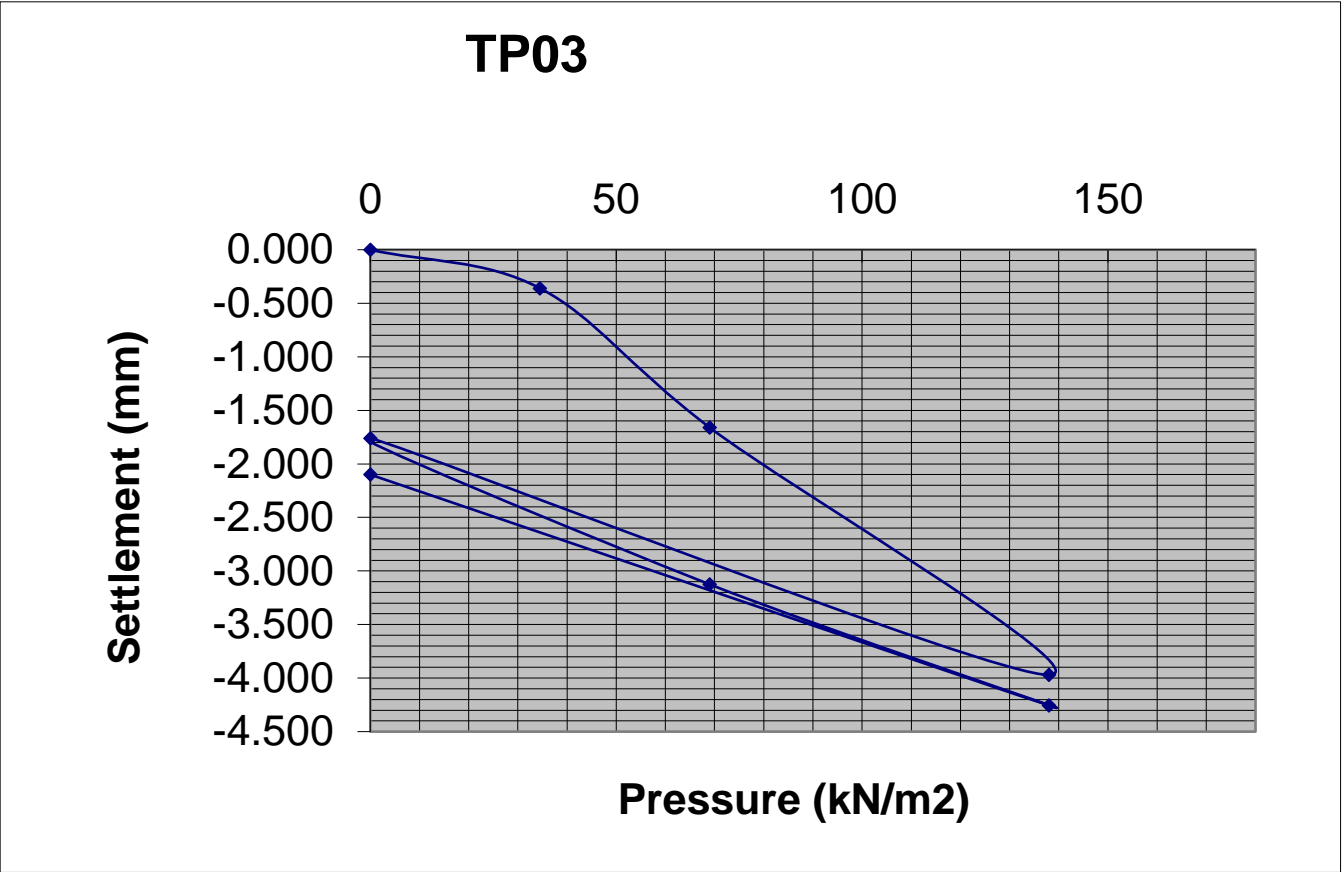
APPENDIX 6 – Insitu Plate Bearing Test Records



Applied Load	Gauge settlement
0	0.000
34.5	-0.36
69	-1.66
138	-3.97
0	-1.76
69	-3.125
138	-4.255
0	-2.1



LOCATION	Dyke Road, Galway	MATERIAL	MADE GROUND: Grey sandy angular to subrounded fine to coarse Gravel
CONTRACT NO.	13614-02-24		
DATE	15/04/2024		
CLIENT	Aecom	DEPTH	0.20m
PLATE DIAMETER	457mm	NOTES	
TEST NO.	TP03	SAMPLES	



Modulus of subgrade reaction, K (Initial) =	28.09 MN/m2/m
Modulus of subgrade reaction, K (Reload) =	34.16 MN/m2/m

Equivalent CBR(initial)in accordance with HD25/94 volume7 section2 =	3.12 %
Equivalent CBR(reload)in accordance with HD25/94 volume7 section2 =	4.38 %

APPENDIX 7 - TRL Dynamic Cone Penetrometer Records



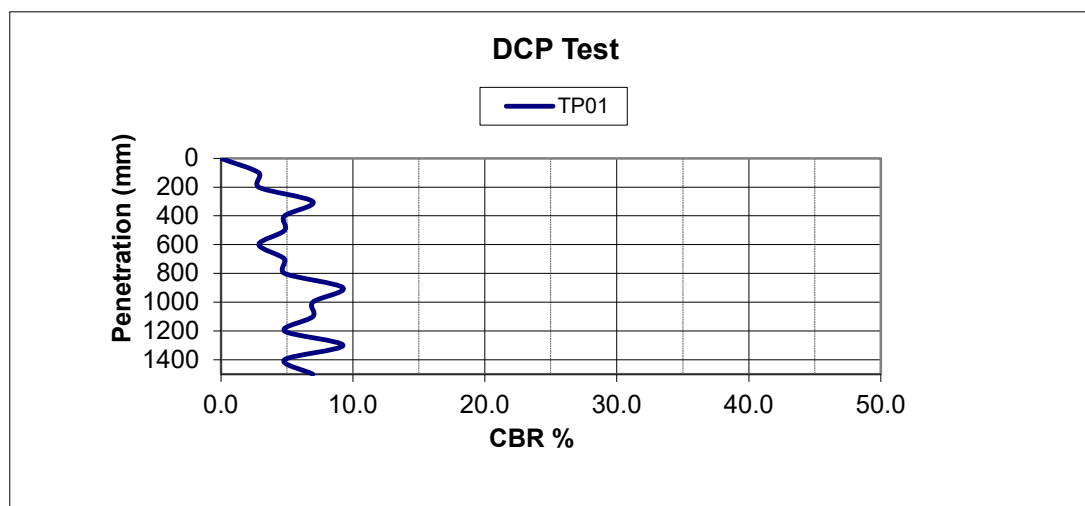
Job Name Dyke Road Galway
Job No. 13614-02-24
Client Aecom

Test Type Dynamic Cone Penetration Test
Test Reference TP01
By LB
Date 17/04/2024

Initial Depth 0.8

Depth below start depth (mm)	No. of Blows per 100mm	Penetration per Blow (mm)	CBR (%)
0	-	-	0.0
100	2	50.0	2.9
200	2	50.0	2.9
300	4	25.0	7.0
400	3	33.3	4.8
500	3	33.3	4.8
600	2	50.0	2.9
700	3	33.3	4.8
800	3	33.3	4.8
900	5	20.0	9.3
1000	4	25.0	7.0
1100	4	25.0	7.0
1200	3	33.3	4.8
1300	5	20.0	9.3
1400	3	33.3	4.8
1500	4	25.0	7.0

Reference Kleyn and Van Heerden (60° Cone)
Formula $\text{Log}_{10}(\text{CBR}) = 2.632 - 1.28 \text{ Log}_{10}(\text{mm/blow})$



Job Name Dyke Road Galway

Job No. 13614-02-24

Client Aecom

Test Type Dynamic Cone Penetration Test

Test Reference TP02

By LB

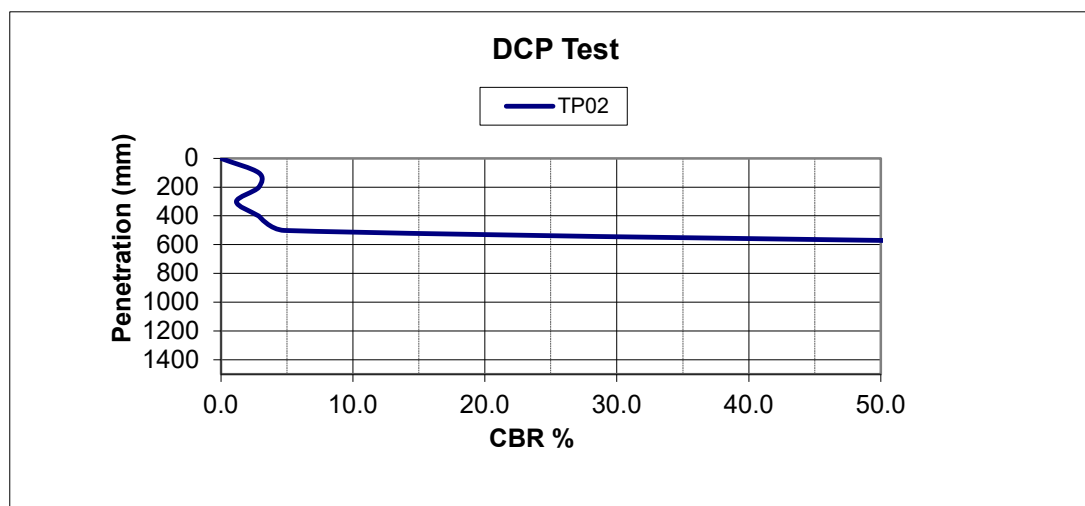
Date 17/04/2024

Initial Depth 0.7

Depth below start depth (mm)	No. of Blows per 100mm	Penetration per Blow (mm)	CBR (%)
0	-	-	0.0
100	2	50.0	2.9
200	2	50.0	2.9
300	1	100.0	1.2
400	2	50.0	2.9
500	3	33.3	4.8
600	25	4.0	72.7
700			
800			
900			
1000			
1100			
1200			
1300			
1400			
1500			

Reference Kleyn and Van Heerden (60° Cone)

Formula $\text{Log}_{10}(\text{CBR}) = 2.632 - 1.28 \text{ Log}_{10}(\text{mm/blow})$



Job Name Dyke Road Galway

Job No. 13614-02-24

Client Aecom

Initial Depth 0.7

Test Type Dynamic Cone Penetration Test

Test Reference TP02A

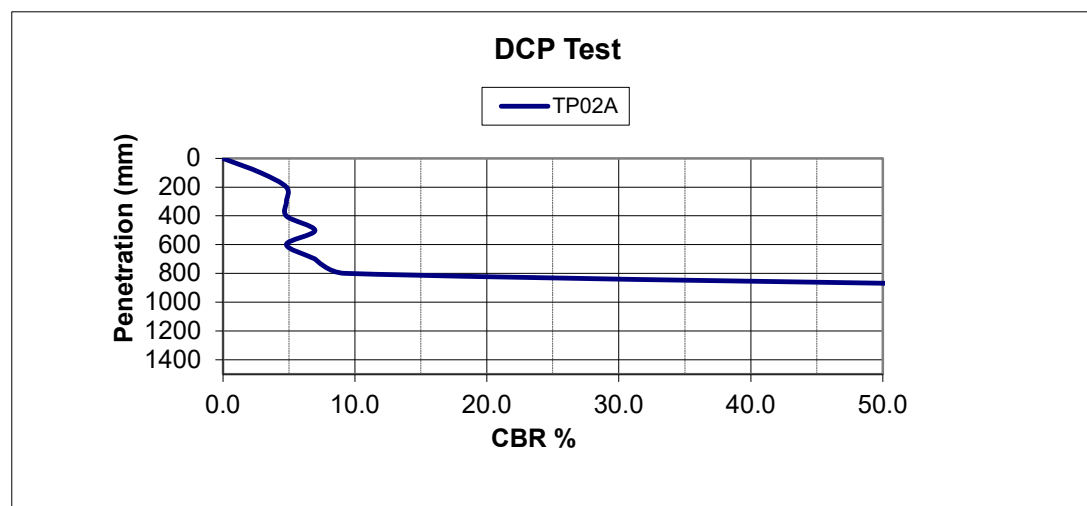
By LB

Date 17/04/2024

Depth below start depth (mm)	No. of Blows per 100mm	Penetration per Blow (mm)	CBR (%)
0	-	-	0.0
100	2	50.0	2.9
200	3	33.3	4.8
300	3	33.3	4.8
400	3	33.3	4.8
500	4	25.0	7.0
600	3	33.3	4.8
700	4	25.0	7.0
800	5	20.0	9.3
900	25	4.0	72.7
1000			
1100			
1200			
1300			
1400			
1500			

Reference Kleyn and Van Heerden (60° Cone)

Formula $\text{Log}_{10}(\text{CBR}) = 2.632 - 1.28 \text{ Log}_{10}(\text{mm/blow})$



Job Name Dyke Road Galway

Job No. 13614-02-24

Client Aecom

Initial Depth 0.7

Test Type Dynamic Cone Penetration Test

Test Reference TP03

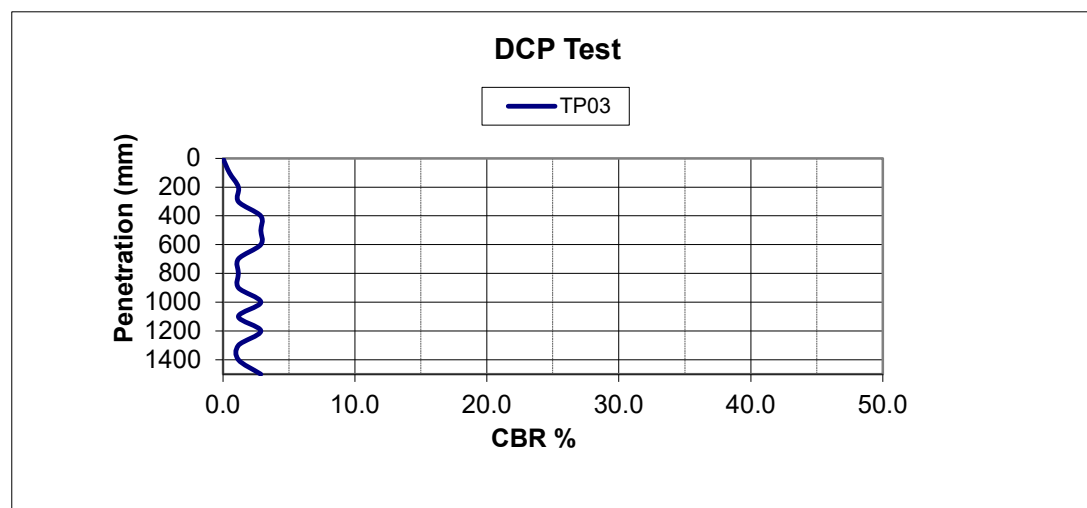
By LB

Date 15/04/2024

Depth below start depth (mm)	No. of Blows per 100mm	Penetration per Blow (mm)	CBR (%)
0	-	-	0.0
100	0	200.0	0.5
200	1	100.0	1.2
300	1	100.0	1.2
400	2	50.0	2.9
500	2	50.0	2.9
600	2	50.0	2.9
700	1	100.0	1.2
800	1	100.0	1.2
900	1	100.0	1.2
1000	2	50.0	2.9
1100	1	100.0	1.2
1200	2	50.0	2.9
1300	1	100.0	1.2
1400	1	100.0	1.2
1500	2	50.0	2.9

Reference Kleyn and Van Heerden (60° Cone)

Formula $\text{Log}_{10}(\text{CBR}) = 2.632 - 1.28 \text{ Log}_{10}(\text{mm/blow})$



Job Name Dyke Road Galway

Job No. 13614-02-24

Client Aecom

Initial Depth 0.9

Test Type Dynamic Cone Penetration Test

Test Reference TP04

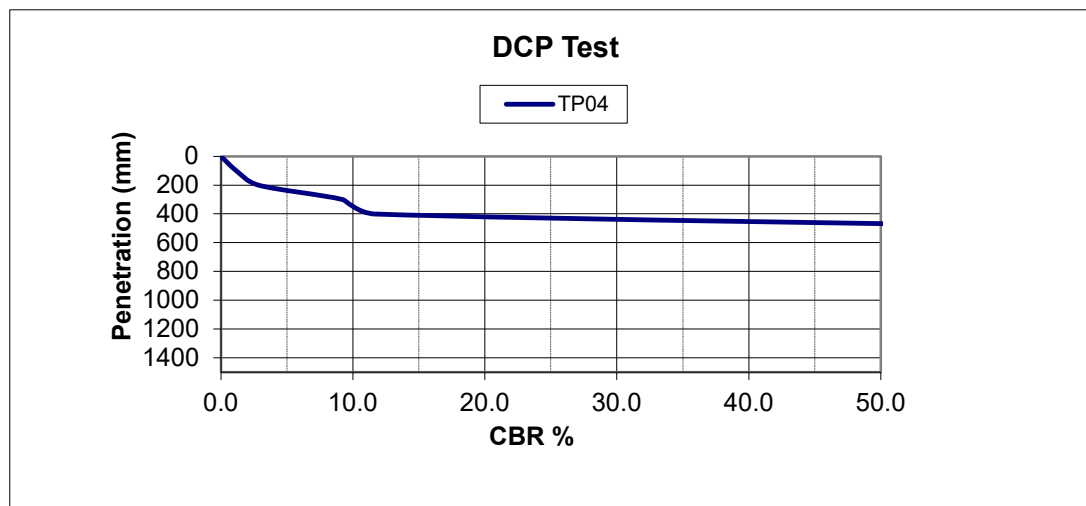
By LB

Date 15/04/2024

Depth below start depth (mm)	No. of Blows per 100mm	Penetration per Blow (mm)	CBR (%)
0	-	-	0.0
100	4	100.0	1.2
200	2	50.0	2.9
300	5	20.0	9.3
400	6	16.7	11.7
500	25	4.0	72.7
600			
700			
800			
900			
1000			
1100			
1200			
1300			
1400			
1500			

Reference Kleyn and Van Heerden (60° Cone)

Formula $\text{Log}_{10}(\text{CBR}) = 2.632 - 1.28 \text{ Log}_{10}(\text{mm/blow})$



APPENDIX 8 – Laboratory Results



Ground Investigations Ireland
Catherinstown House
Hazelhatch Road
Newcastle
Co. Dublin
Ireland
D22 K5P8



Attention : Mike Sutton
Date : 30th April, 2024
Your reference : 13614-02-24
Our reference : Test Report 24/6265 Batch 1
Location : Dyke Road Galway
Date samples received : 12th April, 2024
Status : Final Report
Issue : 202404301446

Two samples were received for analysis on 12th April, 2024 of which two were scheduled for analysis. Please find attached our Test Report which should be read with notes at the end of the report and should include all sections if reproduced. Interpretations and opinions are outside the scope of any accreditation, and all results relate only to samples supplied.

All analysis is carried out on as received samples and reported on a dry weight basis unless stated otherwise. Results are not surrogate corrected.

The greenhouse gas emissions generated (in Carbon – Co2e) to obtain the results in this report are estimated as:

Scope 1&2 emissions - 9.412 kg of CO2

Scope 1&2&3 emissions - 22.243 kg of CO2

Authorised By:



Phil Sommerton BSc

Senior Project Manager

Please include all sections of this report if it is reproduced

Element Materials Technology

Client Name: Ground Investigations Ireland
Reference: 13614-02-24
Location: Dyke Road Galway
Contact: Mike Sutton
EMT Job No: 24/6265

Report : Solid

Solids: V=60g VOC jar, J=250g glass jar, T=plastic tub

EMT Sample No.	1-4	5-8									Please see attached notes for all abbreviations and acronyms						
Sample ID	BH02	BH02															
Depth	0.50	1.00															
COC No / misc																	
Containers	V J T	V J T															
Sample Date	10/04/2024	10/04/2024															
Sample Type	Soil	Soil															
Batch Number	1	1															
Date of Receipt	12/04/2024	12/04/2024									LOD/LOR	Units	Method No.				
TPH CWG																	
Aliphatics																	
>C5-C6 (HS_1D_AL) #	<0.1 ^{SV}	<0.1 ^{SV}									<0.1	mg/kg	TM36/PM12				
>C6-C8 (HS_1D_AL) #	<0.1 ^{SV}	<0.1 ^{SV}									<0.1	mg/kg	TM36/PM12				
>C8-C10 (HS_1D_AL)	<0.1 ^{SV}	<0.1 ^{SV}									<0.1	mg/kg	TM36/PM12				
>C10-C12 (EH_CU_1D_AL) #	<0.2	<0.2									<0.2	mg/kg	TM5/PM8/PM16				
>C12-C16 (EH_CU_1D_AL) #	<4	<4									<4	mg/kg	TM5/PM8/PM16				
>C16-C21 (EH_CU_1D_AL) #	<7	<7									<7	mg/kg	TM5/PM8/PM16				
>C21-C35 (EH_CU_1D_AL) #	<7	<7									<7	mg/kg	TM5/PM8/PM16				
>C35-C40 (EH_CU_1D_AL)	<7	<7									<7	mg/kg	TM5/PM8/PM16				
Total aliphatics C5-40 (EH_CU+HS_1D_AL)	<26	<26									<26	mg/kg	TM5/PM8/PM16/PM12/PM10				
>C6-C10 (HS_1D_AL)	<0.1 ^{SV}	<0.1 ^{SV}									<0.1	mg/kg	TM36/PM12				
>C10-C25 (EH_CU_1D_AL)	<10	<10									<10	mg/kg	TM5/PM8/PM16				
>C25-C35 (EH_CU_1D_AL)	<10	<10									<10	mg/kg	TM5/PM8/PM16				
Aromatics																	
>C5-EC7 (HS_1D_AR) #	<0.1 ^{SV}	<0.1 ^{SV}									<0.1	mg/kg	TM36/PM12				
>EC7-EC8 (HS_1D_AR) #	<0.1 ^{SV}	<0.1 ^{SV}									<0.1	mg/kg	TM36/PM12				
>EC8-EC10 (HS_1D_AR) #	<0.1 ^{SV}	<0.1 ^{SV}									<0.1	mg/kg	TM36/PM12				
>EC10-EC12 (EH_CU_1D_AR) #	<0.2	<0.2									<0.2	mg/kg	TM5/PM8/PM16				
>EC12-EC16 (EH_CU_1D_AR) #	<4	<4									<4	mg/kg	TM5/PM8/PM16				
>EC16-EC21 (EH_CU_1D_AR) #	23	<7									<7	mg/kg	TM5/PM8/PM16				
>EC21-EC35 (EH_CU_1D_AR) #	369	168									<7	mg/kg	TM5/PM8/PM16				
>EC35-EC40 (EH_CU_1D_AR)	48	25									<7	mg/kg	TM5/PM8/PM16				
Total aromatics C5-40 (EH_CU+HS_1D_AR)	440	193									<26	mg/kg	TM5/PM8/PM16/PM12/PM10				
Total aliphatics and aromatics(C5-40) (EH_CU+HS_1D_Total)	440	193									<52	mg/kg	TM5/PM8/PM16/PM12/PM10				
>EC6-EC10 (HS_1D_AR) #	<0.1 ^{SV}	<0.1 ^{SV}									<0.1	mg/kg	TM36/PM12				
>EC10-EC25 (EH_CU_1D_AR)	67	39									<10	mg/kg	TM5/PM8/PM16				
>EC25-EC35 (EH_CU_1D_AR)	394	211									<10	mg/kg	TM5/PM8/PM16				
MTBE #												<5 ^{SV}	<5 ^{SV}		<5	ug/kg	TM36/PM12
Benzene #												<5 ^{SV}	<5 ^{SV}		<5	ug/kg	TM36/PM12
Toluene #												<5 ^{SV}	<5 ^{SV}		<5	ug/kg	TM36/PM12
Ethylbenzene #												<5 ^{SV}	<5 ^{SV}		<5	ug/kg	TM36/PM12
m/p-Xylene #												<5 ^{SV}	<5 ^{SV}		<5	ug/kg	TM36/PM12
o-Xylene #												<5 ^{SV}	<5 ^{SV}		<5	ug/kg	TM36/PM12
PCB 28 #												<5	<5		<5	ug/kg	TM17/PM8
PCB 52 #												<5	<5		<5	ug/kg	TM17/PM8
PCB 101 #												<5	<5		<5	ug/kg	TM17/PM8
PCB 118 #												<5	<5		<5	ug/kg	TM17/PM8
PCB 138 #												<5	<5		<5	ug/kg	TM17/PM8
PCB 153 #												<5	<5		<5	ug/kg	TM17/PM8
PCB 180 #												<5	<5		<5	ug/kg	TM17/PM8
Total 7 PCBs #												<35	<35		<35	ug/kg	TM17/PM8

Element Materials Technology

Client Name: Ground Investigations Ireland
Reference: 13614-02-24
Location: Dyke Road Galway
Contact: Mike Sutton
EMT Job No: 24/6265

Report : Solid

Solids: V=60g VOC jar, J=250g glass jar, T=plastic tub

[illegible]

Solids: V=60g VOC jar. J=250g glass jar. T=plastic tub

EMT Sample No.	1-4	5-8								Please see attached notes for all abbreviations and acronyms			
Sample ID	BH02	BH02											
Depth	0.50	1.00											
COC No / misc													
Containers	V J T	V J T											
Sample Date	10/04/2024	10/04/2024											
Sample Type	Soil	Soil											
Batch Number	1	1											
Date of Receipt	12/04/2024	12/04/2024									LOD/LOR	Units	Method No.
Dissolved Antimony #	0.010	0.005									<0.002	mg/l	TM30/PM17
Dissolved Antimony (A10) #	0.10	0.05									<0.02	mg/kg	TM30/PM17
Dissolved Arsenic #	0.0039	0.0055									<0.0025	mg/l	TM30/PM17
Dissolved Arsenic (A10) #	0.039	0.055									<0.025	mg/kg	TM30/PM17
Dissolved Barium #	0.035	0.020									<0.003	mg/l	TM30/PM17
Dissolved Barium (A10) #	0.35	0.20									<0.03	mg/kg	TM30/PM17
Dissolved Cadmium #	<0.0005	<0.0005									<0.0005	mg/l	TM30/PM17
Dissolved Cadmium (A10) #	<0.005	<0.005									<0.005	mg/kg	TM30/PM17
Dissolved Chromium #	<0.0015	<0.0015									<0.0015	mg/l	TM30/PM17
Dissolved Chromium (A10) #	<0.015	<0.015									<0.015	mg/kg	TM30/PM17
Dissolved Copper #	<0.007	<0.007									<0.007	mg/l	TM30/PM17
Dissolved Copper (A10) #	<0.07	<0.07									<0.07	mg/kg	TM30/PM17
Dissolved Lead #	<0.005	<0.005									<0.005	mg/l	TM30/PM17
Dissolved Lead (A10) #	<0.05	<0.05									<0.05	mg/kg	TM30/PM17
Dissolved Molybdenum #	0.009	0.010									<0.002	mg/l	TM30/PM17
Dissolved Molybdenum (A10) #	0.09	0.10									<0.02	mg/kg	TM30/PM17
Dissolved Nickel #	<0.002	<0.002									<0.002	mg/l	TM30/PM17
Dissolved Nickel (A10) #	<0.02	<0.02									<0.02	mg/kg	TM30/PM17
Dissolved Selenium #	<0.003	<0.003									<0.003	mg/l	TM30/PM17
Dissolved Selenium (A10) #	<0.03	<0.03									<0.03	mg/kg	TM30/PM17
Dissolved Zinc #	0.007	0.004									<0.003	mg/l	TM30/PM17
Dissolved Zinc (A10) #	0.07	0.04									<0.03	mg/kg	TM30/PM17
Mercury Dissolved by CVA#	<0.00001	<0.00001									<0.00001	mg/l	TM61/PM0
Mercury Dissolved by CVA#	<0.0001	<0.0001									<0.0001	mg/kg	TM61/PM0
Phenol	<0.01	<0.01									<0.01	mg/l	TM26/PM0
Phenol	<0.1	<0.1									<0.1	mg/kg	TM26/PM0
Fluoride	<0.3	0.3									<0.3	mg/l	TM173/PM0
Fluoride	<3	<3									<3	mg/kg	TM173/PM0
Sulphate as SO4 #	37.6	9.5									<0.5	mg/l	TM38/PM0
Sulphate as SO4 #	376	95									<5	mg/kg	TM38/PM0
Mass of raw test portion	0.1295	0.1342										kg	NONE/PM17
Chloride #	2.9	3.1									<0.3	mg/l	TM38/PM0
Chloride #	29	31									<3	mg/kg	TM38/PM0
Mass of dried test portion	0.09	0.09										kg	NONE/PM17
Dissolved Organic Carbon	8	13									<2	mg/l	TM60/PM0
Dissolved Organic Carbon	80	130									<20	mg/kg	TM60/PM0
pH	8.26	8.34									<0.01	pH units	TM73/PM0

Element Materials Technology

Client Name: Ground Investigations Ireland
Reference: 13614-02-24
Location: Dyke Road Galway
Contact: Mike Sutton
EMT Job No: 24/6265

Report : CEN 10:1 1 Batch

Solids: V=60g VOC jar, J=250g glass jar, T=plastic tub

[illegible]

Solids: V=60g VOC jar, J=250g glass jar, T=plastic tub

EMT Sample No.	1-4	5-8						
Sample ID	BH02	BH02						
Depth	0.50	1.00						
COC No / misc								
Containers	V J T	V J T						
Sample Date	10/04/2024	10/04/2024						
Sample Type	Soil	Soil						
Batch Number	1	1						
Date of Receipt	12/04/2024	12/04/2024						
Please see attached notes for all abbreviations and acronyms								
Inert	Stable Non-reactive	Hazardous	LOD LOR	Units	Method No.			
Solid Waste Analysis								
Total Organic Carbon #	13.75	13.99	3	5	6	<0.02	%	TM21/PM24
Sum of BTEX	<0.025 ^{SV}	<0.025 ^{SV}	6	-	-	<0.025	mg/kg	TM36/PM12
Sum of 7 PCBs #	<0.035	<0.035	1	-	-	<0.035	mg/kg	TM17/PM8
Mineral Oil	<30	<30	500	-	-	<30	mg/kg	TM5/PM8/PM16
PAH Sum of 6 #	1.93	<0.22	-	-	-	<0.22	mg/kg	TM4/PM8
PAH Sum of 17	3.34	<0.64	100	-	-	<0.64	mg/kg	TM4/PM8
CEN 10:1 Leachate								
Arsenic #	0.039	0.055	0.5	2	25	<0.025	mg/kg	TM30/PM17
Barium #	0.35	0.20	20	100	300	<0.03	mg/kg	TM30/PM17
Cadmium #	<0.005	<0.005	0.04	1	5	<0.005	mg/kg	TM30/PM17
Chromium #	<0.015	<0.015	0.5	10	70	<0.015	mg/kg	TM30/PM17
Copper #	<0.07	<0.07	2	50	100	<0.07	mg/kg	TM30/PM17
Mercury #	<0.0001	<0.0001	0.01	0.2	2	<0.0001	mg/kg	TM61/PM0
Molybdenum #	0.09	0.10	0.5	10	30	<0.02	mg/kg	TM30/PM17
Nickel #	<0.02	<0.02	0.4	10	40	<0.02	mg/kg	TM30/PM17
Lead #	<0.05	<0.05	0.5	10	50	<0.05	mg/kg	TM30/PM17
Antimony #	0.10	0.05	0.06	0.7	5	<0.02	mg/kg	TM30/PM17
Selenium #	<0.03	<0.03	0.1	0.5	7	<0.03	mg/kg	TM30/PM17
Zinc #	0.07	0.04	4	50	200	<0.03	mg/kg	TM30/PM17
Total Dissolved Solids #	2000	1469	4000	60000	100000	<350	mg/kg	TM20/PM0
Dissolved Organic Carbon	80	130	500	800	1000	<20	mg/kg	TM60/PM0
Mass of raw test portion	0.1295	0.1342	-	-	-		kg	NONE/PM17
Dry Matter Content Ratio	69.3	66.9	-	-	-	<0.1	%	NONE/PM4
Leachant Volume	0.86	0.855	-	-	-		l	NONE/PM17
Moisture Content 105C (% Dry Weight)	44.4	49.5	-	-	-	<0.1	%	PM4/PM0
pH #	7.49	7.43	-	-	-	<0.01	pH units	TM73/PM11
Phenol	<0.1	<0.1	1	-	-	<0.1	mg/kg	TM26/PM0
Fluoride	<3	<3	10	150	500	<3	mg/kg	TM173/PM0
Sulphate as SO4 #	376	95	1000	20000	50000	<5	mg/kg	TM38/PM0
Chloride #	29	31	800	15000	25000	<3	mg/kg	TM38/PM0

Matrix : Solid

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Note:

The LOQ of the Asbestos Quantification is 0.001% dry fibre of dry mass of sample.

Where trace asbestos is reported the amount of asbestos will be <0.1%.

QF-PM 3.1.15 v10 Please include all sections of this report if it is reproduced 9 of 17

NOTES TO ACCOMPANY ALL SCHEDULES AND REPORTS

EMT Job No.: 24/6265

SOILS and ASH

Please note we are only MCERTS accredited (UK soils only) for sand, loam and clay and any other matrix is outside our scope of accreditation.

Where an MCERTS report has been requested, you will be notified within 48 hours of any samples that have been identified as being outside our MCERTS scope. As validation has been performed on clay, sand and loam, only samples that are predominantly these matrices, or combinations of them will be within our MCERTS scope. If samples are not one of a combination of the above matrices they will not be marked as MCERTS accredited.

It is assumed that you have taken representative samples on site and require analysis on a representative subsample. Stones will generally be included unless we are requested to remove them.

All samples will be discarded one month after the date of reporting, unless we are instructed to the contrary. Asbestos samples are retained for 6 months.

If you have not already done so, please send us a purchase order if this is required by your company.

Where appropriate please make sure that our detection limits are suitable for your needs, if they are not, please notify us immediately.

All analysis is reported on a dry weight basis unless stated otherwise. Limits of detection for analyses carried out on as received samples are not moisture content corrected. Results are not surrogate corrected. Samples are dried at 35°C ±5°C unless otherwise stated. Moisture content for CEN Leachate tests are dried at 105°C ±5°C. Ash samples are dried at 35°C ±5°C.

Where Mineral Oil is quoted, this refers to Total Aliphatics C10-C40.

Where a CEN 10:1 ZERO Headspace VOC test has been carried out, a 10:1 ratio of water to wet (as received) soil has been used.

% Asbestos in Asbestos Containing Materials (ACMs) is determined by reference to HSG 264 The Survey Guide - Appendix 2 : ACMs in buildings listed in order of ease of fibre release.

Sufficient amount of sample must be received to carry out the testing specified. Where an insufficient amount of sample has been received the testing may not meet the requirements of our accredited methods, as such accreditation may be removed.

Negative Neutralization Potential (NP) values are obtained when the volume of NaOH (0.1N) titrated (pH 8.3) is greater than the volume of HCl (1N) to reduce the pH of the sample to 2.0 - 2.5. Any negative NP values are corrected to 0.

The calculation of Pyrite content assumes that all oxidisable sulphides present in the sample are pyrite. This may not be the case. The calculation may be an overestimate when other sulphides such as Barite (Barium Sulphate) are present.

WATERS

Please note we are not a UK Drinking Water Inspectorate (DWI) Approved Laboratory .

ISO17025 accreditation applies to surface water and groundwater and usually one other matrix which is analysis specific, any other liquids are outside our scope of accreditation.

As surface waters require different sample preparation to groundwaters the laboratory must be informed of the water type when submitting samples.

Where Mineral Oil is quoted, this refers to Total Aliphatics C10-C40.

STACK EMISSIONS

Where an MCERTS report has been requested, you will be notified within 48 hours of any samples that have been identified as being outside our MCERTS scope. As validation for Dioxins and Furans and Dioxin like PCBs has been performed on XAD-2 Resin, only samples which use this resin will be within our MCERTS scope.

Where appropriate please make sure that our detection limits are suitable for your needs, if they are not, please notify us immediately.

DEVIATING SAMPLES

All samples should be submitted to the laboratory in suitable containers with sufficient ice packs to sustain an appropriate temperature for the requested analysis. The temperature of sample receipt is recorded on the confirmation schedules in order that the client can make an informed decision as to whether testing should still be undertaken.

SURROGATES

Surrogate compounds are added during the preparation process to monitor recovery of analytes. However low recovery in soils is often due to peat, clay or other organic rich matrices. For waters this can be due to oxidants, surfactants, organic rich sediments or remediation fluids. Acceptable limits for most organic methods are 70 - 130% and for VOCs are 50 - 150%. When surrogate recoveries are outside the performance criteria but the associated AQC passes this is assumed to be due to matrix effect. Results are not surrogate corrected.

DILUTIONS

A dilution suffix indicates a dilution has been performed and the reported result takes this into account. No further calculation is required.

BLANKS

Where analytes have been found in the blank, the sample will be treated in accordance with our laboratory procedure for dealing with contaminated blanks.

NOTE

Data is only reported if the laboratory is confident that the data is a true reflection of the samples analysed. Data is only reported as accredited when all the requirements of our Quality System have been met. In certain circumstances where all the requirements of the Quality System have not been met, for instance if the associated AQC has failed, the reason is fully investigated and documented. The sample data is then evaluated alongside the other quality control checks performed during analysis to determine its suitability. Following this evaluation, provided the sample results have not been effected, the data is reported but accreditation is removed. It is a requirement of our Accreditation Body for data not reported as accredited to be considered indicative only, but this does not mean the data is not valid.

Where possible, and if requested, samples will be re-extracted and a revised report issued with accredited results. Please do not hesitate to contact the laboratory if further details are required of the circumstances which have led to the removal of accreditation.

Laboratory records are kept for a period of no less than 6 years.

REPORTS FROM THE SOUTH AFRICA LABORATORY

Any method number not prefixed with SA has been undertaken in our UK laboratory unless reported as subcontracted.

Measurement Uncertainty

Measurement uncertainty defines the range of values that could reasonably be attributed to the measured quantity. This range of values has not been included within the reported results. Uncertainty expressed as a percentage can be provided upon request.

Customer Provided Information

Sample ID and depth is information provided by the customer.

Age of Diesel

The age of release estimation is based on the nC17/pristane ratio only as prescribed by Christensen and Larsen (1993) and Kaplan, Galperin, Alimi et al., (1996).

Age estimation should be treated with caution as it can be influenced by site specific factors of which the laboratory are not aware.

Tentatively Identified Compounds (TICs)

Where Tentatively Identified Compounds (TICs) are reported, up to 10 Tentatively Identified Compounds will be listed where there is found to be a greater than 80% match with the NIST library. The reported concentration is determined semi-quantitatively, with a matrix specific limit of detection.

Note, other compounds may be present but are not reported.

ABBREVIATIONS and ACRONYMS USED

#	ISO17025 (UKAS Ref No. 4225) accredited - UK.
SA	ISO17025 (SANAS Ref No.T0729) accredited - South Africa
B	Indicates analyte found in associated method blank.
DR	Dilution required.
M	MCERTS accredited.
NA	Not applicable
NAD	No Asbestos Detected.
ND	None Detected (usually refers to VOC and/SVOC TICs).
NDP	No Determination Possible
SS	Calibrated against a single substance
SV	Surrogate recovery outside performance criteria. This may be due to a matrix effect.
W	Results expressed on as received basis.
+	AQC failure, accreditation has been removed from this result, if appropriate, see 'Note' on previous page.
>>	Results above quantitative calibration range. The result should be considered the minimum value and is indicative only. The actual result could be significantly higher.
*	Analysis subcontracted to an Element Materials Technology approved laboratory.
CO	Suspected carry over
LOD/LOR	Limit of Detection (Limit of Reporting) in line with ISO 17025 and MCERTS
ME	Matrix Effect
NFD	No Fibres Detected
BS	AQC Sample
LB	Blank Sample
N	Client Sample
TB	Trip Blank Sample
OC	Outside Calibration Range

HWOL ACRONYMS AND OPERATORS USED

HS	Headspace Analysis.
EH	Extractable Hydrocarbons - i.e. everything extracted by the solvent.
CU	Clean-up - e.g. by florisil, silica gel.
1D	GC - Single coil gas chromatography.
Total	Aliphatics & Aromatics.
AL	Aliphatics only.
AR	Aromatics only.
2D	GC-GC - Double coil gas chromatography.
#1	EH_Total but with humics mathematically subtracted
#2	EU_Total but with fatty acids mathematically subtracted
_	Operator - underscore to separate acronyms (exception for +).
+	Operator to indicate cumulative e.g. EH+HS_Total or EH_CU+HS_Total
MS	Mass Spectrometry.

EMT Job No: 24/6265

Test Method No.	Description	Prep Method No. (if appropriate)	Description	ISO 17025 (UKAS/S ANAS)	MCERTS (UK soils only)	Analysis done on As Received (AR) or Dried (AD)	Reported on dry weight basis
PM4	Gravimetric measurement of Natural Moisture Content and % Moisture Content at either 35°C or 105°C. Calculation based on ISO 11465:1993(E) and BS1377-2:1990.	PM0	No preparation is required.			AR	
TM4	Modified USEPA 8270D v5:2014 method for the solvent extraction and determination of PAHs by GC-MS.	PM8	End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required.			AR	Yes
TM4	Modified USEPA 8270D v5:2014 method for the solvent extraction and determination of PAHs by GC-MS.	PM8	End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required.	Yes		AR	Yes
TM5	Modified 8015B v2:1996 method for the determination of solvent Extractable Petroleum Hydrocarbons (EPH) within the range C8-C40 by GCFID. For waters the solvent extracts dissolved phase plus a sheen if present.	PM16	Fractionation into aliphatic and aromatic fractions using a Rapid Trace SPE.			AR	
TM5	Modified 8015B v2:1996 method for the determination of solvent Extractable Petroleum Hydrocarbons (EPH) within the range C8-C40 by GCFID. For waters the solvent extracts dissolved phase plus a sheen if present.	PM8/PM16	End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required/Fractionation into aliphatic and aromatic fractions using a Rapid Trace SPE.			AR	Yes
TM5	Modified 8015B v2:1996 method for the determination of solvent Extractable Petroleum Hydrocarbons (EPH) within the range C8-C40 by GCFID. For waters the solvent extracts dissolved phase plus a sheen if present.	PM8/PM16	End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required/Fractionation into aliphatic and aromatic fractions using a Rapid Trace SPE.	Yes		AR	Yes
TM5/TM36	please refer to TM5 and TM36 for method details	PM8/PM12/PM16	please refer to PM8/PM16 and PM12 for method details			AR	Yes
TM17	Modified US EPA method 8270D v5:2014. Determination of specific Polychlorinated Biphenyl congeners by GC-MS.	PM8	End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required.	Yes		AR	Yes
TM20	Modified BS 1377-3:1990/USEPA 160.1/3 (TDS/TS: 1971) Gravimetric determination of Total Dissolved Solids/Total Solids	PM0	No preparation is required.	Yes		AR	Yes
TM21	Modified BS 7755-3:1995, ISO10694:1995 Determination of Total Organic Carbon or Total Carbon by combustion in an Eltra TOC furnace/analyser in the presence of oxygen. The CO2 generated is quantified using infra-red detection. Organic Matter (SOM) calculated as per EA MCERTS Chemical Testing of Soil.	PM24	Preparation of Soil and Marine Sediment Samples for Total Organic Carbon.	Yes		AD	Yes

EMT Job No: 24/6265

Test Method No.	Description	Prep Method No. (if appropriate)	Description	ISO 17025 (UKAS/ANAS)	MCERTS (UK soils only)	Analysis done on As Received (AR) or Dried (AD)	Reported on dry weight basis
TM26	Determination of phenols by Reversed Phased High Performance Liquid Chromatography and Electro-Chemical Detection.	PM0	No preparation is required.			AR	Yes
TM30	Determination of Trace Metals by ICP-OES (Inductively Coupled Plasma – Optical Emission Spectrometry): WATERS by Modified USEPA Method 200.7, Rev. 4.4, 1994; Modified EPA Method 6010B, Rev.2, Dec 1996; Modified BS EN ISO 11885:2009: SOILS by Modified USEP 6010B, Rev.2, Dec.1996; Modified EPA Method 3050B, Rev.2, Dec.1996	PM15	Acid digestion of dried and ground solid samples using Aqua Regia refluxed at 112.5 °C. Samples containing asbestos are not dried and ground.			AD	Yes
TM30	Determination of Trace Metals by ICP-OES (Inductively Coupled Plasma – Optical Emission Spectrometry): WATERS by Modified USEPA Method 200.7, Rev. 4.4, 1994; Modified EPA Method 6010B, Rev.2, Dec 1996; Modified BS EN ISO 11885:2009: SOILS by Modified USEP 6010B, Rev.2, Dec.1996; Modified EPA Method 3050B, Rev.2, Dec.1996	PM15	Acid digestion of dried and ground solid samples using Aqua Regia refluxed at 112.5 °C. Samples containing asbestos are not dried and ground.	Yes		AD	Yes
TM30	Determination of Trace Metals by ICP-OES (Inductively Coupled Plasma – Optical Emission Spectrometry): WATERS by Modified USEPA Method 200.7, Rev. 4.4, 1994; Modified EPA Method 6010B, Rev.2, Dec 1996; Modified BS EN ISO 11885:2009: SOILS by Modified USEP 6010B, Rev.2, Dec.1996; Modified EPA Method 3050B, Rev.2, Dec.1996	PM17	Modified method BS EN12457-2:2002 As received solid samples are leached with water in a 10:1 water to soil ratio for 24 hours, the moisture content of the sample is included in the ratio.	Yes		AR	Yes
TM36	Modified US EPA method 8015B v2:1996. Determination of Gasoline Range Organics (GRO) in the carbon chain range of C4-12 by headspace GC-FID. MTBE by GC/FID co-elutes with 3-methylpentane if present and therefore can give a false positive. Positive MTBE results will be re-run using GC-MS to double check, when requested.	PM12	Modified US EPA method 5021A v2:2014. Preparation of solid and liquid samples for GC headspace analysis.			AR	Yes
TM36	Modified US EPA method 8015B v2:1996. Determination of Gasoline Range Organics (GRO) in the carbon chain range of C4-12 by headspace GC-FID. MTBE by GC/FID co-elutes with 3-methylpentane if present and therefore can give a false positive. Positive MTBE results will be re-run using GC-MS to double check, when requested.	PM12	Modified US EPA method 5021A v2:2014. Preparation of solid and liquid samples for GC headspace analysis.	Yes		AR	Yes
TM38	Soluble Ion analysis using Discrete Analyser. Modified US EPA methods: Chloride 325.2 (1978), Sulphate 375.4 (Rev.2 1993), o-Phosphate 365.2 (Rev.2 1993), TON 353.1 (Rev.2 1993), Nitrite 354.1 (1971), Hex Cr 7196A (1992), NH4+ 350.1 (Rev.2 1993) – All anions comparable to BS ISO 15923-1: 2013I	PM0	No preparation is required.	Yes		AR	Yes
TM38	Soluble Ion analysis using Discrete Analyser. Modified US EPA methods: Chloride 325.2 (1978), Sulphate 375.4 (Rev.2 1993), o-Phosphate 365.2 (Rev.2 1993), TON 353.1 (Rev.2 1993), Nitrite 354.1 (1971), Hex Cr 7196A (1992), NH4+ 350.1 (Rev.2 1993) – All anions comparable to BS ISO 15923-1: 2013I	PM20	Extraction of dried and ground or as received samples with deionised water in a 2:1 water to solid ratio using a reciprocal shaker for all analytes except hexavalent chromium. Extraction of as received sample using 10:1 ratio of 0.2M sodium hydroxide to soil for hexavalent chromium using a reciprocal shaker.	Yes		AR	Yes
TM60	TC/TOC analysis of Waters by High Temperature Combustion followed by NDIR detection. Based on the following modified standard methods: USEPA 9060A (2002), APHA SMEWW 5310B:1999 22nd Edition, ASTM D 7573, and USEPA 415.1.	PM0	No preparation is required.			AR	Yes
TM61	Determination of Mercury by Cold Vapour Atomic Fluorescence - WATERS: Modified USEPA Method 245.7, Rev 2, Feb 2005. SOILS: Modified USEPA Method 7471B, Rev.2, Feb 2007	PM0	No preparation is required.	Yes		AR	Yes

EMT Job No: 24/6265

Test Method No.	Description	Prep Method No. (if appropriate)	Description	ISO 17025 (UKAS/S ANAS)	MCERTS (UK soils only)	Analysis done on As Received (AR) or Dried (AD)	Reported on dry weight basis
TM65	Asbestos Bulk Identification method based on HSG 248 Second edition (2021)	PM42	Modified SCA Blue Book V.12 draft 2017 and WM3 1st Edition v1.1:2018. Solid samples undergo a thorough visual inspection for asbestos fibres prior to asbestos identification using TM065.	Yes		AR	
TM73	Modified US EPA methods 150.1 (1982) and 9045D Rev. 4 - 2004) and BS1377-3:1990. Determination of pH by Metrohm automated probe analyser.	PM0	No preparation is required.			AR	Yes
TM73	Modified US EPA methods 150.1 (1982) and 9045D Rev. 4 - 2004) and BS1377-3:1990. Determination of pH by Metrohm automated probe analyser.	PM11	Extraction of as received solid samples using one part solid to 2.5 parts deionised water.	Yes		AR	No
TM173	Analysis of fluoride by ISE (Ion Selective Electrode) using modified ISE method 9214 - 340.2 (EPA 1998)	PM0	No preparation is required.			AR	Yes
NONE	No Method Code	NONE	No Method Code			AD	Yes
NONE	No Method Code	PM17	Modified method BS EN12457-2:2002 As received solid samples are leached with water in a 10:1 water to soil ratio for 24 hours, the moisture content of the sample is included in the ratio.				
NONE	No Method Code	PM4	Gravimetric measurement of Natural Moisture Content and % Moisture Content at either 35°C or 105°C. Calculation based on ISO 11465:1993(E) and BS1377-2:1990.			AR	

Ground Investigations Ireland
Catherinestown House
Hazelhatch Road
Newcastle
Co. Dublin
Ireland
D22 K5P8



Attention : Mike Sutton
Date : 30th April, 2024
Your reference : 13614-02-24
Our reference : Test Report 24/6663 Batch 1
Location : Dyke Road Galway
Date samples received : 18th April, 2024
Status : Final Report
Issue : 202404301121

Two samples were received for analysis on 18th April, 2024 of which two were scheduled for analysis. Please find attached our Test Report which should be read with notes at the end of the report and should include all sections if reproduced. Interpretations and opinions are outside the scope of any accreditation, and all results relate only to samples supplied.

All analysis is carried out on as received samples and reported on a dry weight basis unless stated otherwise. Results are not surrogate corrected.

The greenhouse gas emissions generated (in Carbon – Co2e) to obtain the results in this report are estimated as:

Scope 1&2 emissions - 6.371 kg of CO2

Scope 1&2&3 emissions - 15.057 kg of CO2

Authorised By:



Bruce Leslie
Project Manager

Please include all sections of this report if it is reproduced

Element Materials Technology

Client Name: Ground Investigations Ireland
Reference: 13614-02-24
Location: Dyke Road Galway
Contact: Mike Sutton
EMT Job No: 24/6663

Report : Solid

Solids: V=60g VOC jar, J=250g glass jar, T=plastic tub

EMT Sample No.	1-4	5-8									Please see attached notes for all abbreviations and acronyms		
Sample ID	BRC-02	BRC-02											
Depth	0.25	1.50											
COC No / misc													
Containers	V J T	V J T											
Sample Date	16/04/2024	16/04/2024											
Sample Type	Soil	Soil											
Batch Number	1	1											
Date of Receipt	18/04/2024	18/04/2024											
Arsenic #	1.6	1.6									<0.5	mg/kg	TM30/PM15
Cadmium #	0.4	0.4									<0.1	mg/kg	TM30/PM15
Chromium #	13.0	11.6									<0.5	mg/kg	TM30/PM15
Copper #	6	6									<1	mg/kg	TM30/PM15
Lead #	<5	<5									<5	mg/kg	TM30/PM15
Mercury #	<0.1	<0.1									<0.1	mg/kg	TM30/PM15
Nickel #	8.5	7.7									<0.7	mg/kg	TM30/PM15
Selenium #	<1	<1									<1	mg/kg	TM30/PM15
Water Soluble Boron #	0.3	0.3									<0.1	mg/kg	TM74/PM32
Zinc #	11	10									<5	mg/kg	TM30/PM15
PAH MS													
Naphthalene #	<0.04	<0.04									<0.04	mg/kg	TM4/PM8
Acenaphthylene	<0.03	<0.03									<0.03	mg/kg	TM4/PM8
Acenaphthene #	<0.05	<0.05									<0.05	mg/kg	TM4/PM8
Fluorene #	<0.04	<0.04									<0.04	mg/kg	TM4/PM8
Phenanthrene #	<0.03	<0.03									<0.03	mg/kg	TM4/PM8
Anthracene #	<0.04	<0.04									<0.04	mg/kg	TM4/PM8
Fluoranthene #	<0.03	<0.03									<0.03	mg/kg	TM4/PM8
Pyrene #	0.05	<0.03									<0.03	mg/kg	TM4/PM8
Benzo(a)anthracene #	<0.06	<0.06									<0.06	mg/kg	TM4/PM8
Chrysene #	<0.02	<0.02									<0.02	mg/kg	TM4/PM8
Benzo(bk)fluoranthene #	<0.07	<0.07									<0.07	mg/kg	TM4/PM8
Benzo(a)pyrene #	<0.04	<0.04									<0.04	mg/kg	TM4/PM8
Indeno(123cd)pyrene #	<0.04	<0.04									<0.04	mg/kg	TM4/PM8
Dibenzo(ah)anthracene #	<0.04	<0.04									<0.04	mg/kg	TM4/PM8
Benzo(ghi)perylene #	<0.04	<0.04									<0.04	mg/kg	TM4/PM8
Coronene	<0.04	<0.04									<0.04	mg/kg	TM4/PM8
PAH 17 Total	<0.64	<0.64									<0.64	mg/kg	TM4/PM8
Benzo(b)fluoranthene	<0.05	<0.05									<0.05	mg/kg	TM4/PM8
Benzo(k)fluoranthene	<0.02	<0.02									<0.02	mg/kg	TM4/PM8
PAH Surrogate % Recovery	90	89									<0	%	TM4/PM8
EPH (C8-C40) (EH_1D_Total) #	877	1033									<30	mg/kg	TM5/PM8
Phenol #	<0.01	<0.01									<0.01	mg/kg	TM26/PM21B
Natural Moisture Content	3.9	187.8									<0.1	%	PM4/PM0
Sulphate as SO4 (2:1 Ext) #	0.0158	0.0387									<0.0015	g/l	TM38/PM20
Total Cyanide #	<0.5	<0.5									<0.5	mg/kg	TM89/PM45
Organic Matter	0.8	1.1									<0.2	%	TM21/PM24

Element Materials Technology

Client Name: Ground Investigations Ireland
Reference: 13614-02-24
Location: Dyke Road Galway
Contact: Mike Sutton
EMT Job No: 24/6663

Report : Solid

Solids: V=60g VOC jar, J=250g glass jar, T=plastic tub

[illegible]

Note:

The LOQ of the Asbestos Quantification is 0.001% dry fibre of dry mass of sample.

Where trace asbestos is reported the amount of asbestos will be <0.1%.

QF-PM 3.1.15 v10 Please include all sections of this report if it is reproduced 4 of 11

NOTES TO ACCOMPANY ALL SCHEDULES AND REPORTS

EMT Job No.: 24/6663

SOILS and ASH

Please note we are only MCERTS accredited (UK soils only) for sand, loam and clay and any other matrix is outside our scope of accreditation.

Where an MCERTS report has been requested, you will be notified within 48 hours of any samples that have been identified as being outside our MCERTS scope. As validation has been performed on clay, sand and loam, only samples that are predominantly these matrices, or combinations of them will be within our MCERTS scope. If samples are not one of a combination of the above matrices they will not be marked as MCERTS accredited.

It is assumed that you have taken representative samples on site and require analysis on a representative subsample. Stones will generally be included unless we are requested to remove them.

All samples will be discarded one month after the date of reporting, unless we are instructed to the contrary. Asbestos samples are retained for 6 months.

If you have not already done so, please send us a purchase order if this is required by your company.

Where appropriate please make sure that our detection limits are suitable for your needs, if they are not, please notify us immediately.

All analysis is reported on a dry weight basis unless stated otherwise. Limits of detection for analyses carried out on as received samples are not moisture content corrected. Results are not surrogate corrected. Samples are dried at 35°C ±5°C unless otherwise stated. Moisture content for CEN Leachate tests are dried at 105°C ±5°C. Ash samples are dried at 35°C ±5°C.

Where Mineral Oil is quoted, this refers to Total Aliphatics C10-C40.

Where a CEN 10:1 ZERO Headspace VOC test has been carried out, a 10:1 ratio of water to wet (as received) soil has been used.

% Asbestos in Asbestos Containing Materials (ACMs) is determined by reference to HSG 264 The Survey Guide - Appendix 2 : ACMs in buildings listed in order of ease of fibre release.

Sufficient amount of sample must be received to carry out the testing specified. Where an insufficient amount of sample has been received the testing may not meet the requirements of our accredited methods, as such accreditation may be removed.

Negative Neutralization Potential (NP) values are obtained when the volume of NaOH (0.1N) titrated (pH 8.3) is greater than the volume of HCl (1N) to reduce the pH of the sample to 2.0 - 2.5. Any negative NP values are corrected to 0.

The calculation of Pyrite content assumes that all oxidisable sulphides present in the sample are pyrite. This may not be the case. The calculation may be an overestimate when other sulphides such as Barite (Barium Sulphate) are present.

WATERS

Please note we are not a UK Drinking Water Inspectorate (DWI) Approved Laboratory .

ISO17025 accreditation applies to surface water and groundwater and usually one other matrix which is analysis specific, any other liquids are outside our scope of accreditation.

As surface waters require different sample preparation to groundwaters the laboratory must be informed of the water type when submitting samples.

Where Mineral Oil is quoted, this refers to Total Aliphatics C10-C40.

STACK EMISSIONS

Where an MCERTS report has been requested, you will be notified within 48 hours of any samples that have been identified as being outside our MCERTS scope. As validation for Dioxins and Furans and Dioxin like PCBs has been performed on XAD-2 Resin, only samples which use this resin will be within our MCERTS scope.

Where appropriate please make sure that our detection limits are suitable for your needs, if they are not, please notify us immediately.

DEVIATING SAMPLES

All samples should be submitted to the laboratory in suitable containers with sufficient ice packs to sustain an appropriate temperature for the requested analysis. The temperature of sample receipt is recorded on the confirmation schedules in order that the client can make an informed decision as to whether testing should still be undertaken.

SURROGATES

Surrogate compounds are added during the preparation process to monitor recovery of analytes. However low recovery in soils is often due to peat, clay or other organic rich matrices. For waters this can be due to oxidants, surfactants, organic rich sediments or remediation fluids. Acceptable limits for most organic methods are 70 - 130% and for VOCs are 50 - 150%. When surrogate recoveries are outside the performance criteria but the associated AQC passes this is assumed to be due to matrix effect. Results are not surrogate corrected.

DILUTIONS

A dilution suffix indicates a dilution has been performed and the reported result takes this into account. No further calculation is required.

BLANKS

Where analytes have been found in the blank, the sample will be treated in accordance with our laboratory procedure for dealing with contaminated blanks.

NOTE

Data is only reported if the laboratory is confident that the data is a true reflection of the samples analysed. Data is only reported as accredited when all the requirements of our Quality System have been met. In certain circumstances where all the requirements of the Quality System have not been met, for instance if the associated AQC has failed, the reason is fully investigated and documented. The sample data is then evaluated alongside the other quality control checks performed during analysis to determine its suitability. Following this evaluation, provided the sample results have not been effected, the data is reported but accreditation is removed. It is a requirement of our Accreditation Body for data not reported as accredited to be considered indicative only, but this does not mean the data is not valid.

Where possible, and if requested, samples will be re-extracted and a revised report issued with accredited results. Please do not hesitate to contact the laboratory if further details are required of the circumstances which have led to the removal of accreditation.

Laboratory records are kept for a period of no less than 6 years.

REPORTS FROM THE SOUTH AFRICA LABORATORY

Any method number not prefixed with SA has been undertaken in our UK laboratory unless reported as subcontracted.

Measurement Uncertainty

Measurement uncertainty defines the range of values that could reasonably be attributed to the measured quantity. This range of values has not been included within the reported results. Uncertainty expressed as a percentage can be provided upon request.

Customer Provided Information

Sample ID and depth is information provided by the customer.

Age of Diesel

The age of release estimation is based on the nC17/pristane ratio only as prescribed by Christensen and Larsen (1993) and Kaplan, Galperin, Alimi et al., (1996).

Age estimation should be treated with caution as it can be influenced by site specific factors of which the laboratory are not aware.

Tentatively Identified Compounds (TICs)

Where Tentatively Identified Compounds (TICs) are reported, up to 10 Tentatively Identified Compounds will be listed where there is found to be a greater than 80% match with the NIST library. The reported concentration is determined semi-quantitatively, with a matrix specific limit of detection.

Note, other compounds may be present but are not reported.

ABBREVIATIONS and ACRONYMS USED

#	ISO17025 (UKAS Ref No. 4225) accredited - UK.
SA	ISO17025 (SANAS Ref No.T0729) accredited - South Africa
B	Indicates analyte found in associated method blank.
DR	Dilution required.
M	MCERTS accredited.
NA	Not applicable
NAD	No Asbestos Detected.
ND	None Detected (usually refers to VOC and/SVOC TICs).
NDP	No Determination Possible
SS	Calibrated against a single substance
SV	Surrogate recovery outside performance criteria. This may be due to a matrix effect.
W	Results expressed on as received basis.
+	AQC failure, accreditation has been removed from this result, if appropriate, see 'Note' on previous page.
>>	Results above quantitative calibration range. The result should be considered the minimum value and is indicative only. The actual result could be significantly higher.
*	Analysis subcontracted to an Element Materials Technology approved laboratory.
CO	Suspected carry over
LOD/LOR	Limit of Detection (Limit of Reporting) in line with ISO 17025 and MCERTS
ME	Matrix Effect
NFD	No Fibres Detected
BS	AQC Sample
LB	Blank Sample
N	Client Sample
TB	Trip Blank Sample
OC	Outside Calibration Range

HWOL ACRONYMS AND OPERATORS USED

HS	Headspace Analysis.
EH	Extractable Hydrocarbons - i.e. everything extracted by the solvent.
CU	Clean-up - e.g. by florisil, silica gel.
1D	GC - Single coil gas chromatography.
Total	Aliphatics & Aromatics.
AL	Aliphatics only.
AR	Aromatics only.
2D	GC-GC - Double coil gas chromatography.
#1	EH_Total but with humics mathematically subtracted
#2	EU_Total but with fatty acids mathematically subtracted
_	Operator - underscore to separate acronyms (exception for +).
+	Operator to indicate cumulative e.g. EH+HS_Total or EH_CU+HS_Total
MS	Mass Spectrometry.

EMT Job No: 24/6663

Test Method No.	Description	Prep Method No. (if appropriate)	Description	ISO 17025 (UKAS/ANAS)	MCERTS (UK soils only)	Analysis done on As Received (AR) or Dried (AD)	Reported on dry weight basis
PM4	Gravimetric measurement of Natural Moisture Content and % Moisture Content at either 35°C or 105°C. Calculation based on ISO 11465:1993(E) and BS1377-2:1990.	PM0	No preparation is required.			AR	
TM4	Modified USEPA 8270D v5:2014 method for the solvent extraction and determination of PAHs by GC-MS.	PM8	End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required.			AR	Yes
TM4	Modified USEPA 8270D v5:2014 method for the solvent extraction and determination of PAHs by GC-MS.	PM8	End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required.	Yes		AR	Yes
TM5	Modified 8015B v2:1996 method for the determination of solvent Extractable Petroleum Hydrocarbons (EPH) within the range C8-C40 by GCFID. For waters the solvent extracts dissolved phase plus a sheen if present.	PM8	End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required.	Yes		AR	Yes
TM21	Modified BS 7755-3:1995, ISO10694:1995 Determination of Total Organic Carbon or Total Carbon by combustion in an Eltra TOC furnace/analyser in the presence of oxygen. The CO2 generated is quantified using infra-red detection. Organic Matter (SOM) calculated as per EA MCERTS Chemical Testing of Soil.	PM24	Preparation of Soil and Marine Sediment Samples for Total Organic Carbon.			AD	Yes
TM26	Determination of phenols by Reversed Phased High Performance Liquid Chromatography and Electro-Chemical Detection.	PM21B	As Received samples are extracted in Methanol: Water (60:40) by reciprocal shaker.	Yes		AR	Yes
TM30	Determination of Trace Metals by ICP-OES (Inductively Coupled Plasma – Optical Emission Spectrometry): WATERS by Modified USEPA Method 200.7, Rev. 4.4, 1994; Modified EPA Method 6010B, Rev.2, Dec 1996; Modified BS EN ISO 11885:2009: SOILS by Modified USEP 6010B, Rev.2, Dec.1996; Modified EPA Method 3050B, Rev.2, Dec.1996	PM15	Acid digestion of dried and ground solid samples using Aqua Regia refluxed at 112.5 °C. Samples containing asbestos are not dried and ground.	Yes		AD	Yes
TM38	Soluble Ion analysis using Discrete Analyser. Modified US EPA methods: Chloride 325.2 (1978), Sulphate 375.4 (Rev.2 1993), o-Phosphate 365.2 (Rev.2 1993), TON 353.1 (Rev.2 1993), Nitrite 354.1 (1971), Hex Cr 7196A (1992), NH4+ 350.1 (Rev.2 1993) – All anions comparable to BS ISO 15923-1: 2013l	PM20	Extraction of dried and ground or as received samples with deionised water in a 2:1 water to solid ratio using a reciprocal shaker for all analytes except hexavalent chromium. Extraction of as received sample using 10:1 ratio of 0.2M sodium hydroxide to soil for hexavalent chromium using a reciprocal shaker.	Yes		AD	Yes
TM65	Asbestos Bulk Identification method based on HSG 248 Second edition (2021)	PM42	Modified SCA Blue Book V.12 draft 2017 and WM3 1st Edition v1.1:2018. Solid samples undergo a thorough visual inspection for asbestos fibres prior to asbestos identification using TM065.	Yes		AR	
TM73	Modified US EPA methods 150.1 (1982) and 9045D Rev. 4 - 2004) and BS1377-3:1990. Determination of pH by Metrohm automated probe analyser.	PM11	Extraction of as received solid samples using one part solid to 2.5 parts deionised water.	Yes		AR	No

EMT Job No: 24/6663

Test Method No.	Description	Prep Method No. (if appropriate)	Description	ISO 17025 (UKAS/S ANAS)	MCERTS (UK soils only)	Analysis done on As Received (AR) or Dried (AD)	Reported on dry weight basis
TM74	Analysis of water soluble boron (20:1 extract) by ICP-OES.	PM32	Hot water soluble boron is extracted from dried and ground samples using a 20:1 ratio.	Yes		AD	Yes
TM89	Modified USEPA method OIA-1667 (1999). Determination of cyanide by Flow Injection Analyser. Where WAD cyanides are required a Ligand displacement step is carried out before analysis.	PM45	As received solid samples are extracted with 1M NaOH by orbital shaker for Cyanide, Sulphide and Thiocyanate analysis.	Yes		AR	Yes

Ground Investigations Ireland
Catherinestown House
Hazelhatch Road
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Co. Dublin
Ireland
D22 K5P8



Attention : Mike Sutton
Date : 16th May, 2024
Your reference : 13614-02-24
Our reference : Test Report 24/6996 Batch 1
Location : Dyke Road, Galway
Date samples received : 24th April, 2024
Status : Final Report
Issue : 202405161600

Twenty samples were received for analysis on 24th April, 2024 of which nine were scheduled for analysis. Please find attached our Test Report which should be read with notes at the end of the report and should include all sections if reproduced. Interpretations and opinions are outside the scope of any accreditation, and all results relate only to samples supplied.

All analysis is carried out on as received samples and reported on a dry weight basis unless stated otherwise. Results are not surrogate corrected.

The greenhouse gas emissions generated (in Carbon – Co2e) to obtain the results in this report are estimated as:

Scope 1&2 emissions - 43.223 kg of CO2

Scope 1&2&3 emissions - 102.147 kg of CO2

Authorised By:



Bruce Leslie
Project Manager

Please include all sections of this report if it is reproduced

Element Materials Technology

Client Name: Ground Investigations Ireland
Reference: 13614-02-24
Location: Dyke Road, Galway
Contact: Mike Sutton
EMT Job No: 24/6996

Report : Solid

Solids: V=60g VOC jar, J=250g glass jar, T=plastic tub

EMT Sample No.	1-4	5-8	9-12	33-36	37-40	41-44	45-48	49-52	57-60		Please see attached notes for all abbreviations and acronyms		
Sample ID	ST01	ST02	ST03	TP03	TP03	TP03	TP04	TP04	TP05				
Depth	0.20-0.80	0.15-0.80	0.07-1.40	0.10-0.60	0.61-1.10	1.10-2.30	0.07-1.00	1.00-1.20	0.55-1.35				
COC No / misc													
Containers	V J T	V J T	V J T	V J T	V J T	V J T	V J T	V J T	V J T				
Sample Date	11/04/2024	11/04/2024	12/04/2024	15/04/2024	15/04/2024	15/04/2024	15/04/2024	15/04/2024	15/04/2024				
Sample Type	Soil	Soil	Soil	Soil	Soil	Soil	Soil	Soil	Soil				
Batch Number	1	1	1	1	1	1	1	1	1				
Date of Receipt	24/04/2024	24/04/2024	24/04/2024	24/04/2024	24/04/2024	24/04/2024	24/04/2024	24/04/2024	24/04/2024		LOD/LOR	Units	Method No.
Antimony	<1	<1	<1	<1	<1	<1	<1	<1	<1		<1	mg/kg	TM30/PM15
Arsenic #	<0.5	2.4	1.6	1.3	7.7	1.3	1.7	2.6	1.7		<0.5	mg/kg	TM30/PM15
Barium #	8	12	7	7	34	99	13	12	10		<1	mg/kg	TM30/PM15
Cadmium #	0.2	0.4	0.4	0.3	0.6	0.2	0.4	0.4	0.9		<0.1	mg/kg	TM30/PM15
Chromium #	13.3	16.2	11.7	4.1	11.6	17.9	23.0	10.2	24.5		<0.5	mg/kg	TM30/PM15
Copper #	4	9	5	3	26	6	5	6	5		<1	mg/kg	TM30/PM15
Lead #	<5	9	<5	<5	30	<5	<5	7	<5		<5	mg/kg	TM30/PM15
Mercury #	<0.1	<0.1	<0.1	<0.1	0.2	<0.1	<0.1	<0.1	<0.1		<0.1	mg/kg	TM30/PM15
Molybdenum #	0.7	1.5	0.9	0.4	1.0	1.3	1.4	0.9	1.5		<0.1	mg/kg	TM30/PM15
Nickel #	3.4	9.4	7.2	2.8	8.7	3.0	6.3	7.6	7.0		<0.7	mg/kg	TM30/PM15
Selenium #	<1	<1	<1	<1	3	<1	<1	<1	<1		<1	mg/kg	TM30/PM15
Sulphur as S	-	-	-	-	0.52	0.13	-	0.07	-		<0.01	%	TM30/PM15
Total Sulphate as SO4 BRE	-	-	-	-	0.58	0.10	-	0.03	-		<0.01	%	TM50/PM29
Zinc #	6	16	11	<5	29	6	11	10	16		<5	mg/kg	TM30/PM15
Magnesium	-	-	-	-	0.0106	0.0035	-	0.0019	-		<0.0001	g/l	TM30/PM20
PAH MS													
Naphthalene #	<0.04	<0.04	<0.04	<0.04	<0.04	<0.04	<0.04	<0.04	<0.04		<0.04	mg/kg	TM4/PM8
Acenaphthylene	<0.03	<0.03	<0.03	<0.03	<0.03	<0.03	<0.03	<0.03	<0.03		<0.03	mg/kg	TM4/PM8
Acenaphthene #	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05		<0.05	mg/kg	TM4/PM8
Fluorene #	<0.04	<0.04	<0.04	<0.04	<0.04	<0.04	<0.04	<0.04	<0.04		<0.04	mg/kg	TM4/PM8
Phenanthrene #	<0.03	<0.03	<0.03	<0.03	0.12	<0.03	<0.03	<0.03	<0.03		<0.03	mg/kg	TM4/PM8
Anthracene #	<0.04	<0.04	<0.04	<0.04	<0.04	<0.04	<0.04	<0.04	<0.04		<0.04	mg/kg	TM4/PM8
Fluoranthene #	<0.03	<0.03	<0.03	<0.03	0.21	<0.03	<0.03	0.05	0.07		<0.03	mg/kg	TM4/PM8
Pyrene #	<0.03	<0.03	<0.03	<0.03	0.18	<0.03	<0.03	0.04	0.08		<0.03	mg/kg	TM4/PM8
Benzo(a)anthracene #	<0.06	<0.06	<0.06	<0.06	0.21	<0.06	<0.06	<0.06	0.08		<0.06	mg/kg	TM4/PM8
Chrysene #	<0.02	<0.02	<0.02	<0.02	0.15	<0.02	<0.02	<0.02	0.07		<0.02	mg/kg	TM4/PM8
Benzo(k)fluoranthene #	<0.07	<0.07	<0.07	<0.07	<0.07	<0.07	<0.07	<0.07	0.15		<0.07	mg/kg	TM4/PM8
Benzo(a)pyrene #	<0.04	<0.04	<0.04	<0.04	0.12	<0.04	<0.04	<0.04	0.10		<0.04	mg/kg	TM4/PM8
Indeno(123cd)pyrene #	<0.04	<0.04	<0.04	<0.04	<0.04	<0.04	<0.04	<0.04	0.06		<0.04	mg/kg	TM4/PM8
Dibenzo(ah)anthracene #	<0.04	<0.04	<0.04	<0.04	<0.04	<0.04	<0.04	<0.04	<0.04		<0.04	mg/kg	TM4/PM8
Benzo(ghi)perylene #	<0.04	<0.04	<0.04	<0.04	<0.04	<0.04	<0.04	<0.04	0.06		<0.04	mg/kg	TM4/PM8
Coronene	<0.04	<0.04	<0.04	<0.04	<0.04	<0.04	<0.04	<0.04	<0.04		<0.04	mg/kg	TM4/PM8
PAH 6 Total #	<0.22	<0.22	<0.22	<0.22	0.33	<0.22	<0.22	<0.22	0.44		<0.22	mg/kg	TM4/PM8
PAH 17 Total	<0.64	<0.64	<0.64	<0.64	0.99	<0.64	<0.64	<0.64	0.67		<0.64	mg/kg	TM4/PM8
Benzo(b)fluoranthene	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	0.11		<0.05	mg/kg	TM4/PM8
Benzo(k)fluoranthene	<0.02	<0.02	<0.02	<0.02	<0.02	<0.02	<0.02	<0.02	0.04		<0.02	mg/kg	TM4/PM8
Benzo(j)fluoranthene	<1	<1	<1	<1	<1	<1	<1	<1	<1		<1	mg/kg	TM4/PM8
PAH Surrogate % Recovery	95	94	97	97	94	96	95	94	96		<0	%	TM4/PM8
Mineral Oil (C10-C40) (EH_CU_1D_AL)	<30	<30	69	<30	124	173	<30	<30	<30		<30	mg/kg	TM5/PM8/PM16

Element Materials Technology

Client Name: Ground Investigations Ireland
Reference: 13614-02-24
Location: Dyke Road, Galway
Contact: Mike Sutton
EMT Job No: 24/6996

Report : Solid

Solids: V=60g VOC jar, J=250g glass jar, T=plastic tub

EMT Sample No.	1-4	5-8	9-12	33-36	37-40	41-44	45-48	49-52	57-60		Please see attached notes for all abbreviations and acronyms		
Sample ID	ST01	ST02	ST03	TP03	TP03	TP03	TP04	TP04	TP05				
Depth	0.20-0.80	0.15-0.80	0.07-1.40	0.10-0.60	0.61-1.10	1.10-2.30	0.07-1.00	1.00-1.20	0.55-1.35				
COC No / misc													
Containers	V J T	V J T	V J T	V J T	V J T	V J T	V J T	V J T	V J T				
Sample Date	11/04/2024	11/04/2024	12/04/2024	15/04/2024	15/04/2024	15/04/2024	15/04/2024	15/04/2024	15/04/2024				
Sample Type	Soil	Soil	Soil	Soil	Soil	Soil	Soil	Soil	Soil				
Batch Number	1	1	1	1	1	1	1	1	1				
Date of Receipt	24/04/2024	24/04/2024	24/04/2024	24/04/2024	24/04/2024	24/04/2024	24/04/2024	24/04/2024	24/04/2024		LOD/LOR	Units	Method No.
TPH CWG													
Aliphatics													
>C5-C6 (HS_1D_AL) #	<0.1	<0.1	<0.1	<0.1	<0.1 ^{SV}	<0.1	<0.1	<0.1	<0.1		<0.1	mg/kg	TM36/PM12
>C6-C8 (HS_1D_AL) #	<0.1	<0.1	<0.1	<0.1	<0.1 ^{SV}	<0.1	<0.1	<0.1	<0.1		<0.1	mg/kg	TM36/PM12
>C8-C10 (HS_1D_AL)	<0.1	<0.1	<0.1	<0.1	<0.1 ^{SV}	<0.1	<0.1	<0.1	<0.1		<0.1	mg/kg	TM36/PM12
>C10-C12 (EH_CU_1D_AL) #	<0.2	<0.2	<0.2	<0.2	<0.2	<0.2	<0.2	<0.2	<0.2		<0.2	mg/kg	TM5/PM8/PM16
>C12-C16 (EH_CU_1D_AL) #	<4	<4	<4	<4	<4	<4	<4	<4	<4		<4	mg/kg	TM5/PM8/PM16
>C16-C21 (EH_CU_1D_AL) #	<7	<7	<7	<7	21	71	<7	<7	<7		<7	mg/kg	TM5/PM8/PM16
>C21-C35 (EH_CU_1D_AL) #	<7	<7	69	<7	103	102	19	<7	<7		<7	mg/kg	TM5/PM8/PM16
>C35-C40 (EH_CU_1D_AL)	<7	<7	<7	<7	<7	<7	<7	<7	<7		<7	mg/kg	TM5/PM8/PM16
Total aliphatics C5-40 (EH_CU+HS_1D_AL)	<26	<26	69	<26	124	173	<26	<26	<26		<26	mg/kg	TM5/PM8/PM16/12/PM18
>C6-C10 (HS_1D_AL)	<0.1	<0.1	<0.1	<0.1	<0.1 ^{SV}	<0.1	<0.1	<0.1	<0.1		<0.1	mg/kg	TM36/PM12
>C10-C25 (EH_CU_1D_AL)	<10	<10	<10	<10	35	112	<10	<10	<10		<10	mg/kg	TM5/PM8/PM16
>C25-C35 (EH_CU_1D_AL)	<10	<10	56	<10	73	61	19	<10	<10		<10	mg/kg	TM5/PM8/PM16
Aromatics													
>C5-EC7 (HS_1D_AR) #	<0.1	<0.1	<0.1	<0.1	<0.1 ^{SV}	<0.1	<0.1	<0.1	<0.1		<0.1	mg/kg	TM36/PM12
>EC7-EC8 (HS_1D_AR) #	<0.1	<0.1	<0.1	<0.1	<0.1 ^{SV}	<0.1	<0.1	<0.1	<0.1		<0.1	mg/kg	TM36/PM12
>EC8-EC10 (HS_1D_AR) #	<0.1	<0.1	<0.1	<0.1	<0.1 ^{SV}	<0.1	<0.1	<0.1	<0.1		<0.1	mg/kg	TM36/PM12
>EC10-EC12 (EH_CU_1D_AR) #	<0.2	<0.2	<0.2	<0.2	<0.2	<0.2	<0.2	<0.2	<0.2		<0.2	mg/kg	TM5/PM8/PM16
>EC12-EC16 (EH_CU_1D_AR) #	<4	<4	<4	<4	<4	<4	<4	<4	<4		<4	mg/kg	TM5/PM8/PM16
>EC16-EC21 (EH_CU_1D_AR) #	<7	<7	<7	<7	<7	<7	<7	<7	<7		<7	mg/kg	TM5/PM8/PM16
>EC21-EC35 (EH_CU_1D_AR) #	<7	<7	68	<7	237	156	56	<7	<7		<7	mg/kg	TM5/PM8/PM16
>EC35-EC40 (EH_CU_1D_AR)	<7	<7	9	<7	21	<7	13	<7	<7		<7	mg/kg	TM5/PM8/PM16
Total aromatics C5-40 (EH_CU+HS_1D_AR)	<26	<26	77	<26	258	156	69	<26	<26		<26	mg/kg	TM5/PM8/PM16/12/PM18
Total aliphatics and aromatics(C5-40) (EH_CU+HS_1D_Total)	<52	<52	146	<52	382	329	69	<52	<52		<52	mg/kg	TM5/PM8/PM16/12/PM18
>EC6-EC10 (HS_1D_AR) #	<0.1	<0.1	<0.1	<0.1	<0.1 ^{SV}	<0.1	<0.1	<0.1	<0.1		<0.1	mg/kg	TM36/PM12
>EC10-EC25 (EH_CU_1D_AR)	<10	<10	<10	<10	53	54	<10	<10	<10		<10	mg/kg	TM5/PM8/PM16
>EC25-EC35 (EH_CU_1D_AR)	<10	<10	68	<10	176	115	56	<10	<10		<10	mg/kg	TM5/PM8/PM16
MTBE #	<5	<5	<5	<5	<5 ^{SV}	<5	<5	<5	<5		<5	ug/kg	TM36/PM12
Benzene #	<5	<5	<5	<5	<5 ^{SV}	<5	<5	<5	<5		<5	ug/kg	TM36/PM12
Toluene #	<5	7	<5	<5	<5 ^{SV}	<5	<5	<5	<5		<5	ug/kg	TM36/PM12
Ethylbenzene #	<5	<5	<5	<5	<5 ^{SV}	<5	<5	<5	<5		<5	ug/kg	TM36/PM12
m/p-Xylene #	<5	7	<5	<5	<5 ^{SV}	<5	<5	<5	<5		<5	ug/kg	TM36/PM12
o-Xylene #	<5	<5	<5	<5	<5 ^{SV}	<5	<5	<5	<5		<5	ug/kg	TM36/PM12
PCB 28 #	<5	<5	<5	<5	<5	<5	<5	<5	<5		<5	ug/kg	TM17/PM8
PCB 52 #	<5	<5	<5	<5	<5	<5	<5	<5	<5		<5	ug/kg	TM17/PM8
PCB 101 #	<5	<5	<5	<5	<5	<5	<5	<5	<5		<5	ug/kg	TM17/PM8
PCB 118 #	<5	<5	<5	<5	<5	<5	<5	<5	<5		<5	ug/kg	TM17/PM8
PCB 138 #	<5	<5	<5	<5	<5	<5	<5	<5	<5		<5	ug/kg	TM17/PM8
PCB 153 #	<5	<5	<5	<5	<5	<5	<5	<5	<5		<5	ug/kg	TM17/PM8
PCB 180 #	<5	<5	<5	<5	<5	<5	<5	<5	<5		<5	ug/kg	TM17/PM8
Total 7 PCBs #	<35	<35	<35	<35	<35	<35	<35	<35	<35		<35	ug/kg	TM17/PM8

Element Materials Technology

Client Name: Ground Investigations Ireland
Reference: 13614-02-24
Location: Dyke Road, Galway
Contact: Mike Sutton
EMT Job No: 24/6996

Report : CEN 10:1 1 Batch

Solids: V=60g VOC jar, J=250g glass jar, T=plastic tub

EMT Sample No.	1-4	5-8	9-12	33-36	37-40	41-44	45-48	49-52	57-60		Please see attached notes for all abbreviations and acronyms		
Sample ID	ST01	ST02	ST03	TP03	TP03	TP03	TP04	TP04	TP05				
Depth	0.20-0.80	0.15-0.80	0.07-1.40	0.10-0.60	0.61-1.10	1.10-2.30	0.07-1.00	1.00-1.20	0.55-1.35				
COC No / misc													
Containers	V J T	V J T	V J T	V J T	V J T	V J T	V J T	V J T	V J T				
Sample Date	11/04/2024	11/04/2024	12/04/2024	15/04/2024	15/04/2024	15/04/2024	15/04/2024	15/04/2024	15/04/2024				
Sample Type	Soil	Soil	Soil	Soil	Soil	Soil	Soil	Soil	Soil				
Batch Number	1	1	1	1	1	1	1	1	1				
Date of Receipt	24/04/2024	24/04/2024	24/04/2024	24/04/2024	24/04/2024	24/04/2024	24/04/2024	24/04/2024	24/04/2024		LOD/LOR	Units	Method No.
Dissolved Antimony [#]	<0.002	<0.002	<0.002	<0.002	0.007	0.002	<0.002	<0.002	<0.002		<0.002	mg/l	TM30/PM17
Dissolved Antimony (A10) [#]	<0.02	<0.02	<0.02	<0.02	0.07	<0.02	<0.02	<0.02	<0.02		<0.02	mg/kg	TM30/PM17
Dissolved Arsenic [#]	<0.0025	<0.0025	<0.0025	<0.0025	0.0068	0.0039	<0.0025	<0.0025	<0.0025		<0.0025	mg/l	TM30/PM17
Dissolved Arsenic (A10) [#]	<0.025	<0.025	<0.025	<0.025	0.068	0.039	<0.025	<0.025	<0.025		<0.025	mg/kg	TM30/PM17
Dissolved Barium [#]	<0.003	<0.003	<0.003	<0.003	0.021	0.008	<0.003	<0.003	<0.003		<0.003	mg/l	TM30/PM17
Dissolved Barium (A10) [#]	<0.03	<0.03	<0.03	<0.03	0.21	0.08	<0.03	<0.03	<0.03		<0.03	mg/kg	TM30/PM17
Dissolved Cadmium [#]	<0.0005	<0.0005	<0.0005	<0.0005	<0.0005	<0.0005	<0.0005	<0.0005	<0.0005		<0.0005	mg/l	TM30/PM17
Dissolved Cadmium (A10) [#]	<0.005	<0.005	<0.005	<0.005	<0.005	<0.005	<0.005	<0.005	<0.005		<0.005	mg/kg	TM30/PM17
Dissolved Chromium [#]	<0.0015	<0.0015	<0.0015	<0.0015	<0.0015	<0.0015	0.0036	<0.0015	<0.0015		<0.0015	mg/l	TM30/PM17
Dissolved Chromium (A10) [#]	<0.015	<0.015	<0.015	<0.015	<0.015	<0.015	0.036	<0.015	<0.015		<0.015	mg/kg	TM30/PM17
Dissolved Copper [#]	<0.007	<0.007	<0.007	<0.007	<0.007	<0.007	<0.007	<0.007	<0.007		<0.007	mg/l	TM30/PM17
Dissolved Copper (A10) [#]	<0.07	<0.07	<0.07	<0.07	<0.07	<0.07	<0.07	<0.07	<0.07		<0.07	mg/kg	TM30/PM17
Dissolved Lead [#]	<0.005	<0.005	<0.005	<0.005	<0.005	<0.005	<0.005	<0.005	<0.005		<0.005	mg/l	TM30/PM17
Dissolved Lead (A10) [#]	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05		<0.05	mg/kg	TM30/PM17
Dissolved Molybdenum [#]	<0.002	0.004	<0.002	<0.002	0.005	0.005	<0.002	<0.002	<0.002		<0.002	mg/l	TM30/PM17
Dissolved Molybdenum (A10) [#]	<0.02	0.04	<0.02	<0.02	0.05	0.05	<0.02	<0.02	<0.02		<0.02	mg/kg	TM30/PM17
Dissolved Nickel [#]	<0.002	<0.002	<0.002	<0.002	<0.002	<0.002	<0.002	<0.002	<0.002		<0.002	mg/l	TM30/PM17
Dissolved Nickel (A10) [#]	<0.02	<0.02	<0.02	<0.02	<0.02	<0.02	<0.02	<0.02	<0.02		<0.02	mg/kg	TM30/PM17
Dissolved Selenium [#]	<0.003	<0.003	<0.003	<0.003	<0.003	<0.003	<0.003	<0.003	<0.003		<0.003	mg/l	TM30/PM17
Dissolved Selenium (A10) [#]	<0.03	<0.03	<0.03	<0.03	<0.03	<0.03	<0.03	<0.03	<0.03		<0.03	mg/kg	TM30/PM17
Dissolved Zinc [#]	0.003	<0.003	<0.003	<0.003	0.006	0.004	<0.003	0.003	0.003		<0.003	mg/l	TM30/PM17
Dissolved Zinc (A10) [#]	0.03	<0.03	<0.03	<0.03	0.06	0.04	<0.03	<0.03	0.03		<0.03	mg/kg	TM30/PM17
Mercury Dissolved by CVA [#]	<0.00001	<0.00001	<0.00001	<0.00001	<0.00001	<0.00001	0.00001	<0.00001	<0.00001		<0.00001	mg/l	TM61/PM0
Mercury Dissolved by CVA [#]	<0.0001	<0.0001	<0.0001	<0.0001	<0.0001	<0.0001	0.0001	<0.0001	<0.0001		<0.0001	mg/kg	TM61/PM0
Phenol	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01		<0.01	mg/l	TM26/PM0
Phenol	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1		<0.1	mg/kg	TM26/PM0
Fluoride	<0.3	0.3	<0.3	0.4	0.4	<0.3	<0.3	<0.3	<0.3		<0.3	mg/l	TM173/PM0
Fluoride	<3	3	<3	4	4	<3	<3	<3	<3		<3	mg/kg	TM173/PM0
Sulphate as SO ₄ [#]	0.7	3.4	1.6	203.0	63.6	<0.5	11.6	0.9	<0.5		<0.5	mg/l	TM38/PM0
Sulphate as SO ₄ [#]	7	34	16	2031	636	<5	116	9	<5		<5	mg/kg	TM38/PM0
Mass of raw test portion	0.0914	0.0954	0.0942	0.0923	0.369	0.1394	0.0961	0.0922	0.096		kg	NONE/PM17	
Chloride [#]	<0.3	<0.3	0.4	0.7	17.8	2.4	0.8	<0.3	0.3		<0.3	mg/l	TM38/PM0
Chloride [#]	<3	<3	4	7	178	24	8	<3	3		<3	mg/kg	TM38/PM0
Mass of dried test portion	0.09	0.09	0.09	0.09	0.09	0.09	0.09	0.09	0.09		kg	NONE/PM17	
Dissolved Organic Carbon	<2	<2	<2	<2	35	9	<2	<2	<2		<2	mg/l	TM60/PM0
Dissolved Organic Carbon	<20	<20	<20	<20	350	90	<20	<20	<20		<20	mg/kg	TM60/PM0
pH	7.88	7.93	7.50	8.23	8.21	8.17	10.43	8.13	8.09		<0.01	pH units	TM73/PM0

Element Materials Technology

Client Name: Ground Investigations Ireland
Reference: 13614-02-24
Location: Dyke Road, Galway
Contact: Mike Sutton
EMT Job No: 24/6996

Report : CEN 10:1 1 Batch

Solids: V=60g VOC jar, J=250g glass jar, T=plastic tub

[illegible]

Client Name:	Ground Investigations Ireland	Report :	EN12457_2
Reference:	13614-02-24		
Location:	Dyke Road, Galway	Solids:	V=60g VOC jar, J=250g glass jar, T=plastic tub
Contact:	Mike Sutton		
EMT Job No:	24/6996		

Please see attached notes for all abbreviations and acronyms

Matrix : Solid

8 of 17

Matrix : Solid

Location: Dyke Road, Galway

Contact: Mike Sutton

Please note that only samples that are deviating are mentioned in this report. If no samples are listed it is because none were deviating. Only analyses which are accredited are recorded as deviating if set criteria are not met.

It is a requirement under ISO 17025 that we inform clients if samples are deviating i.e. outside what is expected. A deviating sample indicates that the sample 'may' be compromised but not necessarily will be compromised. The result is still accredited and our analytical reports will still show accreditation on the relevant analytes.

NOTES TO ACCOMPANY ALL SCHEDULES AND REPORTS

EMT Job No.: 24/6996

SOILS and ASH

Please note we are only MCERTS accredited (UK soils only) for sand, loam and clay and any other matrix is outside our scope of accreditation.

Where an MCERTS report has been requested, you will be notified within 48 hours of any samples that have been identified as being outside our MCERTS scope. As validation has been performed on clay, sand and loam, only samples that are predominantly these matrices, or combinations of them will be within our MCERTS scope. If samples are not one of a combination of the above matrices they will not be marked as MCERTS accredited.

It is assumed that you have taken representative samples on site and require analysis on a representative subsample. Stones will generally be included unless we are requested to remove them.

All samples will be discarded one month after the date of reporting, unless we are instructed to the contrary. Asbestos samples are retained for 6 months.

If you have not already done so, please send us a purchase order if this is required by your company.

Where appropriate please make sure that our detection limits are suitable for your needs, if they are not, please notify us immediately.

All analysis is reported on a dry weight basis unless stated otherwise. Limits of detection for analyses carried out on as received samples are not moisture content corrected. Results are not surrogate corrected. Samples are dried at 35°C ±5°C unless otherwise stated. Moisture content for CEN Leachate tests are dried at 105°C ±5°C. Ash samples are dried at 35°C ±5°C.

Where Mineral Oil is quoted, this refers to Total Aliphatics C10-C40.

Where a CEN 10:1 ZERO Headspace VOC test has been carried out, a 10:1 ratio of water to wet (as received) soil has been used.

% Asbestos in Asbestos Containing Materials (ACMs) is determined by reference to HSG 264 The Survey Guide - Appendix 2 : ACMs in buildings listed in order of ease of fibre release.

Sufficient amount of sample must be received to carry out the testing specified. Where an insufficient amount of sample has been received the testing may not meet the requirements of our accredited methods, as such accreditation may be removed.

Negative Neutralization Potential (NP) values are obtained when the volume of NaOH (0.1N) titrated (pH 8.3) is greater than the volume of HCl (1N) to reduce the pH of the sample to 2.0 - 2.5. Any negative NP values are corrected to 0.

The calculation of Pyrite content assumes that all oxidisable sulphides present in the sample are pyrite. This may not be the case. The calculation may be an overestimate when other sulphides such as Barite (Barium Sulphate) are present.

WATERS

Please note we are not a UK Drinking Water Inspectorate (DWI) Approved Laboratory .

ISO17025 accreditation applies to surface water and groundwater and usually one other matrix which is analysis specific, any other liquids are outside our scope of accreditation.

As surface waters require different sample preparation to groundwaters the laboratory must be informed of the water type when submitting samples.

Where Mineral Oil is quoted, this refers to Total Aliphatics C10-C40.

STACK EMISSIONS

Where an MCERTS report has been requested, you will be notified within 48 hours of any samples that have been identified as being outside our MCERTS scope. As validation for Dioxins and Furans and Dioxin like PCBs has been performed on XAD-2 Resin, only samples which use this resin will be within our MCERTS scope.

Where appropriate please make sure that our detection limits are suitable for your needs, if they are not, please notify us immediately.

DEVIATING SAMPLES

All samples should be submitted to the laboratory in suitable containers with sufficient ice packs to sustain an appropriate temperature for the requested analysis. The temperature of sample receipt is recorded on the confirmation schedules in order that the client can make an informed decision as to whether testing should still be undertaken.

SURROGATES

Surrogate compounds are added during the preparation process to monitor recovery of analytes. However low recovery in soils is often due to peat, clay or other organic rich matrices. For waters this can be due to oxidants, surfactants, organic rich sediments or remediation fluids. Acceptable limits for most organic methods are 70 - 130% and for VOCs are 50 - 150%. When surrogate recoveries are outside the performance criteria but the associated AQC passes this is assumed to be due to matrix effect. Results are not surrogate corrected.

DILUTIONS

A dilution suffix indicates a dilution has been performed and the reported result takes this into account. No further calculation is required.

BLANKS

Where analytes have been found in the blank, the sample will be treated in accordance with our laboratory procedure for dealing with contaminated blanks.

NOTE

Data is only reported if the laboratory is confident that the data is a true reflection of the samples analysed. Data is only reported as accredited when all the requirements of our Quality System have been met. In certain circumstances where all the requirements of the Quality System have not been met, for instance if the associated AQC has failed, the reason is fully investigated and documented. The sample data is then evaluated alongside the other quality control checks performed during analysis to determine its suitability. Following this evaluation, provided the sample results have not been effected, the data is reported but accreditation is removed. It is a requirement of our Accreditation Body for data not reported as accredited to be considered indicative only, but this does not mean the data is not valid.

Where possible, and if requested, samples will be re-extracted and a revised report issued with accredited results. Please do not hesitate to contact the laboratory if further details are required of the circumstances which have led to the removal of accreditation.

Laboratory records are kept for a period of no less than 6 years.

REPORTS FROM THE SOUTH AFRICA LABORATORY

Any method number not prefixed with SA has been undertaken in our UK laboratory unless reported as subcontracted.

Measurement Uncertainty

Measurement uncertainty defines the range of values that could reasonably be attributed to the measured quantity. This range of values has not been included within the reported results. Uncertainty expressed as a percentage can be provided upon request.

Customer Provided Information

Sample ID and depth is information provided by the customer.

Age of Diesel

The age of release estimation is based on the nC17/pristane ratio only as prescribed by Christensen and Larsen (1993) and Kaplan, Galperin, Alimi et al., (1996).

Age estimation should be treated with caution as it can be influenced by site specific factors of which the laboratory are not aware.

Tentatively Identified Compounds (TICs)

Where Tentatively Identified Compounds (TICs) are reported, up to 10 Tentatively Identified Compounds will be listed where there is found to be a greater than 80% match with the NIST library. The reported concentration is determined semi-quantitatively, with a matrix specific limit of detection.

Note, other compounds may be present but are not reported.

ABBREVIATIONS and ACRONYMS USED

#	ISO17025 (UKAS Ref No. 4225) accredited - UK.
SA	ISO17025 (SANAS Ref No.T0729) accredited - South Africa
B	Indicates analyte found in associated method blank.
DR	Dilution required.
M	MCERTS accredited.
NA	Not applicable
NAD	No Asbestos Detected.
ND	None Detected (usually refers to VOC and/SVOC TICs).
NDP	No Determination Possible
SS	Calibrated against a single substance
SV	Surrogate recovery outside performance criteria. This may be due to a matrix effect.
W	Results expressed on as received basis.
+	AQC failure, accreditation has been removed from this result, if appropriate, see 'Note' on previous page.
>>	Results above quantitative calibration range. The result should be considered the minimum value and is indicative only. The actual result could be significantly higher.
*	Analysis subcontracted to an Element Materials Technology approved laboratory.
CO	Suspected carry over
LOD/LOR	Limit of Detection (Limit of Reporting) in line with ISO 17025 and MCERTS
ME	Matrix Effect
NFD	No Fibres Detected
BS	AQC Sample
LB	Blank Sample
N	Client Sample
TB	Trip Blank Sample
OC	Outside Calibration Range

HWOL ACRONYMS AND OPERATORS USED

HS	Headspace Analysis.
EH	Extractable Hydrocarbons - i.e. everything extracted by the solvent.
CU	Clean-up - e.g. by florisil, silica gel.
1D	GC - Single coil gas chromatography.
Total	Aliphatics & Aromatics.
AL	Aliphatics only.
AR	Aromatics only.
2D	GC-GC - Double coil gas chromatography.
#1	EH_Total but with humics mathematically subtracted
#2	EU_Total but with fatty acids mathematically subtracted
_	Operator - underscore to separate acronyms (exception for +).
+	Operator to indicate cumulative e.g. EH+HS_Total or EH_CU+HS_Total
MS	Mass Spectrometry.

EMT Job No: 24/6996

Test Method No.	Description	Prep Method No. (if appropriate)	Description	ISO 17025 (UKAS/S ANAS)	MCERTS (UK soils only)	Analysis done on As Received (AR) or Dried (AD)	Reported on dry weight basis
PM4	Gravimetric measurement of Natural Moisture Content and % Moisture Content at either 35°C or 105°C. Calculation based on ISO 11465:1993(E) and BS1377-2:1990.	PM0	No preparation is required.			AR	
TM4	Modified USEPA 8270D v5:2014 method for the solvent extraction and determination of PAHs by GC-MS.	PM8	End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required.			AR	Yes
TM4	Modified USEPA 8270D v5:2014 method for the solvent extraction and determination of PAHs by GC-MS.	PM8	End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required.	Yes		AR	Yes
TM5	Modified 8015B v2:1996 method for the determination of solvent Extractable Petroleum Hydrocarbons (EPH) within the range C8-C40 by GCFID. For waters the solvent extracts dissolved phase plus a sheen if present.	PM16	Fractionation into aliphatic and aromatic fractions using a Rapid Trace SPE.			AR	
TM5	Modified 8015B v2:1996 method for the determination of solvent Extractable Petroleum Hydrocarbons (EPH) within the range C8-C40 by GCFID. For waters the solvent extracts dissolved phase plus a sheen if present.	PM8/PM16	End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required/Fractionation into aliphatic and aromatic fractions using a Rapid Trace SPE.			AR	Yes
TM5	Modified 8015B v2:1996 method for the determination of solvent Extractable Petroleum Hydrocarbons (EPH) within the range C8-C40 by GCFID. For waters the solvent extracts dissolved phase plus a sheen if present.	PM8/PM16	End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required/Fractionation into aliphatic and aromatic fractions using a Rapid Trace SPE.	Yes		AR	Yes
TM5/TM36	please refer to TM5 and TM36 for method details	PM8/PM12/PM16	please refer to PM8/PM16 and PM12 for method details			AR	Yes
TM17	Modified US EPA method 8270D v5:2014. Determination of specific Polychlorinated Biphenyl congeners by GC-MS.	PM8	End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required.	Yes		AR	Yes
TM20	Modified BS 1377-3:1990/USEPA 160.1/3 (TDS/TS: 1971) Gravimetric determination of Total Dissolved Solids/Total Solids	PM0	No preparation is required.	Yes		AR	Yes
TM21	Modified BS 7755-3:1995, ISO10694:1995 Determination of Total Organic Carbon or Total Carbon by combustion in an Eltra TOC furnace/analyser in the presence of oxygen. The CO2 generated is quantified using infra-red detection. Organic Matter (SOM) calculated as per EA MCERTS Chemical Testing of Soil.	PM24	Preparation of Soil and Marine Sediment Samples for Total Organic Carbon.	Yes		AD	Yes

EMT Job No: 24/6996

Test Method No.	Description	Prep Method No. (if appropriate)	Description	ISO 17025 (UKAS/ANAS)	MCERTS (UK soils only)	Analysis done on As Received (AR) or Dried (AD)	Reported on dry weight basis
TM26	Determination of phenols by Reversed Phased High Performance Liquid Chromatography and Electro-Chemical Detection.	PM0	No preparation is required.			AR	Yes
TM30	Determination of Trace Metals by ICP-OES (Inductively Coupled Plasma – Optical Emission Spectrometry): WATERS by Modified USEPA Method 200.7, Rev. 4.4, 1994; Modified EPA Method 6010B, Rev.2, Dec 1996; Modified BS EN ISO 11885:2009: SOILS by Modified USEP 6010B, Rev.2, Dec.1996; Modified EPA Method 3050B, Rev.2, Dec.1996	PM15	Acid digestion of dried and ground solid samples using Aqua Regia refluxed at 112.5 °C. Samples containing asbestos are not dried and ground.			AD	Yes
TM30	Determination of Trace Metals by ICP-OES (Inductively Coupled Plasma – Optical Emission Spectrometry): WATERS by Modified USEPA Method 200.7, Rev. 4.4, 1994; Modified EPA Method 6010B, Rev.2, Dec 1996; Modified BS EN ISO 11885:2009: SOILS by Modified USEP 6010B, Rev.2, Dec.1996; Modified EPA Method 3050B, Rev.2, Dec.1996	PM15	Acid digestion of dried and ground solid samples using Aqua Regia refluxed at 112.5 °C. Samples containing asbestos are not dried and ground.	Yes		AD	Yes
TM30	Determination of Trace Metals by ICP-OES (Inductively Coupled Plasma – Optical Emission Spectrometry): WATERS by Modified USEPA Method 200.7, Rev. 4.4, 1994; Modified EPA Method 6010B, Rev.2, Dec 1996; Modified BS EN ISO 11885:2009: SOILS by Modified USEP 6010B, Rev.2, Dec.1996; Modified EPA Method 3050B, Rev.2, Dec.1996	PM17	Modified method BS EN12457-2:2002 As received solid samples are leached with water in a 10:1 water to soil ratio for 24 hours, the moisture content of the sample is included in the ratio.	Yes		AR	Yes
TM30	Determination of Trace Metals by ICP-OES (Inductively Coupled Plasma – Optical Emission Spectrometry): WATERS by Modified USEPA Method 200.7, Rev. 4.4, 1994; Modified EPA Method 6010B, Rev.2, Dec 1996; Modified BS EN ISO 11885:2009: SOILS by Modified USEP 6010B, Rev.2, Dec.1996; Modified EPA Method 3050B, Rev.2, Dec.1996	PM20	Extraction of dried and ground or as received samples with deionised water in a 2:1 water to solid ratio using a reciprocal shaker for all analytes except hexavalent chromium. Extraction of as received sample using 10:1 ratio of 0.2M sodium hydroxide to soil for hexavalent chromium using a reciprocal shaker.			AD	Yes
TM36	Modified US EPA method 8015B v2:1996. Determination of Gasoline Range Organics (GRO) in the carbon chain range of C4-12 by headspace GC-FID. MTBE by GC/FID co-elutes with 3-methylpentane if present and therefore can give a false positive. Positive MTBE results will be re-run using GC-MS to double check, when requested.	PM12	Modified US EPA method 5021A v2:2014. Preparation of solid and liquid samples for GC headspace analysis.			AR	Yes
TM36	Modified US EPA method 8015B v2:1996. Determination of Gasoline Range Organics (GRO) in the carbon chain range of C4-12 by headspace GC-FID. MTBE by GC/FID co-elutes with 3-methylpentane if present and therefore can give a false positive. Positive MTBE results will be re-run using GC-MS to double check, when requested.	PM12	Modified US EPA method 5021A v2:2014. Preparation of solid and liquid samples for GC headspace analysis.	Yes		AR	Yes
TM38	Soluble Ion analysis using Discrete Analyser. Modified US EPA methods: Chloride 325.2 (1978), Sulphate 375.4 (Rev.2 1993), o-Phosphate 365.2 (Rev.2 1993), TON 353.1 (Rev.2 1993), Nitrite 354.1 (1971), Hex Cr 7196A (1992), NH4+ 350.1 (Rev.2 1993) – All anions comparable to BS ISO 15923-1: 2013	PM0	No preparation is required.	Yes		AR	Yes
TM38	Soluble Ion analysis using Discrete Analyser. Modified US EPA methods: Chloride 325.2 (1978), Sulphate 375.4 (Rev.2 1993), o-Phosphate 365.2 (Rev.2 1993), TON 353.1 (Rev.2 1993), Nitrite 354.1 (1971), Hex Cr 7196A (1992), NH4+ 350.1 (Rev.2 1993) – All anions comparable to BS ISO 15923-1: 2013	PM20	Extraction of dried and ground or as received samples with deionised water in a 2:1 water to solid ratio using a reciprocal shaker for all analytes except hexavalent chromium. Extraction of as received sample using 10:1 ratio of 0.2M sodium hydroxide to soil for hexavalent chromium using a reciprocal shaker.			AD	Yes
TM38	Soluble Ion analysis using Discrete Analyser. Modified US EPA methods: Chloride 325.2 (1978), Sulphate 375.4 (Rev.2 1993), o-Phosphate 365.2 (Rev.2 1993), TON 353.1 (Rev.2 1993), Nitrite 354.1 (1971), Hex Cr 7196A (1992), NH4+ 350.1 (Rev.2 1993) – All anions comparable to BS ISO 15923-1: 2013	PM20	Extraction of dried and ground or as received samples with deionised water in a 2:1 water to solid ratio using a reciprocal shaker for all analytes except hexavalent chromium. Extraction of as received sample using 10:1 ratio of 0.2M sodium hydroxide to soil for hexavalent chromium using a reciprocal shaker.	Yes		AD	Yes

EMT Job No: 24/6996

Test Method No.	Description	Prep Method No. (if appropriate)	Description	ISO 17025 (UKAS/ANAS)	MCERTS (UK soils only)	Analysis done on As Received (AR) or Dried (AD)	Reported on dry weight basis
TM38	Soluble Ion analysis using Discrete Analyser. Modified US EPA methods: Chloride 325.2 (1978), Sulphate 375.4 (Rev.2 1993), o-Phosphate 365.2 (Rev.2 1993), TON 353.1 (Rev.2 1993), Nitrite 354.1 (1971), Hex Cr 7196A (1992), NH4+ 350.1 (Rev.2 1993) – All anions comparable to BS ISO 15923-1: 2013	PM20	Extraction of dried and ground or as received samples with deionised water in a 2:1 water to solid ratio using a reciprocal shaker for all analytes except hexavalent chromium. Extraction of as received sample using 10:1 ratio of 0.2M sodium hydroxide to soil for hexavalent chromium using a reciprocal shaker.	Yes		AR	Yes
TM50	Acid soluble sulphate (Total Sulphate) analysed by ICP-OES	PM29	A hot hydrochloric acid digest is performed on a dried and ground sample, and the resulting liquor is analysed.			AD	Yes
TM60	TC/TOC analysis of Waters by High Temperature Combustion followed by NDIR detection. Based on the following modified standard methods: USEPA 9060A (2002), APHA SMEWW 5310B:1999 22nd Edition, ASTM D 7573, and USEPA 415.1.	PM0	No preparation is required.			AR	Yes
TM61	Determination of Mercury by Cold Vapour Atomic Fluorescence - WATERS: Modified USEPA Method 245.7, Rev 2, Feb 2005. SOILS: Modified USEPA Method 7471B, Rev.2, Feb 2007	PM0	No preparation is required.	Yes		AR	Yes
TM73	Modified US EPA methods 150.1 (1982) and 9045D Rev. 4 - 2004) and BS1377-3:1990. Determination of pH by Metrohm automated probe analyser.	PM0	No preparation is required.			AR	Yes
TM73	Modified US EPA methods 150.1 (1982) and 9045D Rev. 4 - 2004) and BS1377-3:1990. Determination of pH by Metrohm automated probe analyser.	PM11	Extraction of as received solid samples using one part solid to 2.5 parts deionised water.	Yes		AR	No
TM173	Analysis of fluoride by ISE (Ion Selective Electrode) using modified ISE method 9214 - 340.2 (EPA 1998)	PM0	No preparation is required.			AR	Yes
NONE	No Method Code	NONE	No Method Code			AD	Yes
NONE	No Method Code	PM17	Modified method BS EN12457-2:2002 As received solid samples are leached with water in a 10:1 water to soil ratio for 24 hours, the moisture content of the sample is included in the ratio.				
NONE	No Method Code	PM4	Gravimetric measurement of Natural Moisture Content and % Moisture Content at either 35°C or 105°C. Calculation based on ISO 11465:1993(E) and BS1377-2:1990.			AR	

EMT Job No: 24/6996

Test Method No.	Description	Prep Method No. (if appropriate)	Description	ISO 17025 (UKAS/S ANAS)	MCERTS (UK soils only)	Analysis done on As Received (AR) or Dried (AD)	Reported on dry weight basis
Subcontracted	See attached subcontractor report for accreditation status and provider.					AR	

Ground Investigations Ireland
Catherinstown House
Hazelhatch Road
Newcastle
Co. Dublin
Ireland
D22 K5P8



Attention : Mike Sutton
Date : 21st May, 2024
Your reference : 13614-02-24
Our reference : Test Report 24/7795 Batch 1
Location : Dykes Road Galway
Date samples received : 8th May, 2024
Status : Final Report
Issue : 202405211146

Fifteen samples were received for analysis on 8th May, 2024 of which fifteen were scheduled for analysis. Please find attached our Test Report which should be read with notes at the end of the report and should include all sections if reproduced. Interpretations and opinions are outside the scope of any accreditation, and all results relate only to samples supplied.

All analysis is carried out on as received samples and reported on a dry weight basis unless stated otherwise. Results are not surrogate corrected.

The greenhouse gas emissions generated (in Carbon – Co2e) to obtain the results in this report are estimated as:

Scope 1&2 emissions - 69.721 kg of CO2

Scope 1&2&3 emissions - 164.769 kg of CO2

Authorised By:



Bruce Leslie
Project Manager

Please include all sections of this report if it is reproduced

Element Materials Technology

Client Name: Ground Investigations Ireland
Reference: 13614-02-24
Location: Dykes Road Galway
Contact: Mike Sutton
EMT Job No: 24/7795

Report : Solid

Solids: V=60g VOC jar, J=250g glass jar, T=plastic tub

EMT Sample No.	1-4	5-8	9-12	13-16	17-20	21-24	25-28	29-32	33-36	37-40	Please see attached notes for all abbreviations and acronyms		
Sample ID	BH01	BH01	BH04	BH05	BH05	BH05	BRC01	BRC01	BRC01	BRC02			
Depth	0.50	1.50	1.50	0.25	0.50	1.50	0.50	1.50	3.50	0.50			
COC No / misc													
Containers	V J T	V J T	V J T	V J T	V J T	V J T	V J T	V J T	V J T	V J T			
Sample Date	06/05/2024	06/05/2024	06/05/2024	06/05/2024	06/05/2024	06/05/2024	06/05/2024	06/05/2024	06/05/2024	06/05/2024			
Sample Type	Soil	Soil	Soil	Soil	Soil	Soil	Soil	Soil	Soil	Soil			
Batch Number	1	1	1	1	1	1	1	1	1	1			
Date of Receipt	08/05/2024	08/05/2024	08/05/2024	08/05/2024	08/05/2024	08/05/2024	08/05/2024	08/05/2024	08/05/2024	08/05/2024	LOD/LOR	Units	Method No.
TPH CWG													
Aliphatics													
>C5-C6 (HS_1D_AL) #	<0.1 ^{SV}	<0.1 ^{SV}	<0.1	<0.1 ^{SV}	<0.1	<0.1	<0.1 ^{SV}	<0.1 ^{SV}	<0.1	<0.1	<0.1	mg/kg	TM36/PM12
>C6-C8 (HS_1D_AL) #	<0.1 ^{SV}	<0.1 ^{SV}	<0.1	<0.1 ^{SV}	<0.1	<0.1	<0.1 ^{SV}	<0.1 ^{SV}	<0.1	<0.1	<0.1	mg/kg	TM36/PM12
>C8-C10 (HS_1D_AL)	<0.1 ^{SV}	<0.1 ^{SV}	<0.1	0.2 ^{SV}	<0.1	<0.1	<0.1 ^{SV}	<0.1 ^{SV}	<0.1	<0.1	<0.1	mg/kg	TM36/PM12
>C10-C12 (EH_CU_1D_AL) #	<0.2	<0.2	<0.2	<0.2	<0.2	<0.2	<0.2	<0.2	<0.2	<0.2	<0.2	mg/kg	TM5/PM8/PM16
>C12-C16 (EH_CU_1D_AL) #	<4	<4	<4	<4	<4	<4	<4	<4	<4	<4	<4	mg/kg	TM5/PM8/PM16
>C16-C21 (EH_CU_1D_AL) #	<7	22	<7	61	<7	<7	<7	<7	16	21	<7	mg/kg	TM5/PM8/PM16
>C21-C35 (EH_CU_1D_AL) #	88	300	<7	887	19	29	118	52	<7	246	<7	mg/kg	TM5/PM8/PM16
>C35-C40 (EH_CU_1D_AL)	<7	22	<7	99	<7	<7	11	<7	<7	29	<7	mg/kg	TM5/PM8/PM16
Total aliphatics C5-40 (EH_CU+HS_1D_AL)	88	344	<26	1047	<26	29	129	52	<26	296	<26	mg/kg	TM5/PM8/PM16/12/PM18
>C6-C10 (HS_1D_AL)	<0.1 ^{SV}	<0.1 ^{SV}	<0.1	0.2 ^{SV}	<0.1	<0.1	<0.1 ^{SV}	<0.1 ^{SV}	<0.1	<0.1	<0.1	mg/kg	TM36/PM12
>C10-C25 (EH_CU_1D_AL)	<10	76	<10	198	<10	<10	10	<10	23	58	<10	mg/kg	TM5/PM8/PM16
>C25-C35 (EH_CU_1D_AL)	76	246	<10	753	19	24	108	45	<10	209	<10	mg/kg	TM5/PM8/PM16
Aromatics													
>C5-EC7 (HS_1D_AR) #	<0.1 ^{SV}	<0.1 ^{SV}	<0.1	<0.1 ^{SV}	<0.1	<0.1	<0.1 ^{SV}	<0.1 ^{SV}	<0.1	<0.1	<0.1	mg/kg	TM36/PM12
>EC7-EC8 (HS_1D_AR) #	<0.1 ^{SV}	<0.1 ^{SV}	<0.1	<0.1 ^{SV}	<0.1	<0.1	<0.1 ^{SV}	<0.1 ^{SV}	<0.1	<0.1	<0.1	mg/kg	TM36/PM12
>EC8-EC10 (HS_1D_AR) #	<0.1 ^{SV}	<0.1 ^{SV}	<0.1	<0.1 ^{SV}	<0.1	<0.1	<0.1 ^{SV}	<0.1 ^{SV}	<0.1	<0.1	<0.1	mg/kg	TM36/PM12
>EC10-EC12 (EH_CU_1D_AR) #	<0.2	<0.2	<0.2	<0.2	<0.2	<0.2	<0.2	<0.2	<0.2	<0.2	<0.2	mg/kg	TM5/PM8/PM16
>EC12-EC16 (EH_CU_1D_AR) #	<4	<4	<4	<4	<4	<4	<4	<4	<4	<4	<4	mg/kg	TM5/PM8/PM16
>EC16-EC21 (EH_CU_1D_AR) #	<7	64	<7	91	<7	<7	<7	<7	<7	28	<7	mg/kg	TM5/PM8/PM16
>EC21-EC35 (EH_CU_1D_AR) #	256	917	<7	1788	<7	86	238	141	<7	562	<7	mg/kg	TM5/PM8/PM16
>EC35-EC40 (EH_CU_1D_AR)	43	122	<7	266	<7	<7	39	20	<7	100	<7	mg/kg	TM5/PM8/PM16
Total aromatics C5-40 (EH_CU+HS_1D_AR)	299	1103	<26	2145	<26	86	277	161	<26	690	<26	mg/kg	TM5/PM8/PM16/12/PM18
Total aliphatics and aromatics(C5-40) (EH_CU+HS_1D_Total)	387	1447	<52	3192	<52	115	406	213	<52	986	<52	mg/kg	TM5/PM8/PM16/12/PM18
>EC6-EC10 (HS_1D_AR) #	<0.1 ^{SV}	<0.1 ^{SV}	<0.1	<0.1 ^{SV}	<0.1	<0.1	<0.1 ^{SV}	<0.1 ^{SV}	<0.1	<0.1	<0.1	mg/kg	TM36/PM12
>EC10-EC25 (EH_CU_1D_AR)	40	224	<10	369	<10	24	36	<10	<10	115	<10	mg/kg	TM5/PM8/PM16
>EC25-EC35 (EH_CU_1D_AR)	224	757	<10	1512	<10	68	208	131	<10	477	<10	mg/kg	TM5/PM8/PM16
MTBE #	<5 ^{SV}	<5 ^{SV}	<5	<5 ^{SV}	<5	<5	<5 ^{SV}	<5 ^{SV}	<5	<5	<5	ug/kg	TM36/PM12
Benzene #	<5 ^{SV}	<5 ^{SV}	<5	<5 ^{SV}	<5	<5	<5 ^{SV}	<5 ^{SV}	<5	<5	<5	ug/kg	TM36/PM12
Toluene #	<5 ^{SV}	<5 ^{SV}	<5	<5 ^{SV}	<5	<5	<5 ^{SV}	<5 ^{SV}	<5	<5	<5	ug/kg	TM36/PM12
Ethylbenzene #	<5 ^{SV}	<5 ^{SV}	<5	<5 ^{SV}	<5	<5	<5 ^{SV}	<5 ^{SV}	<5	<5	<5	ug/kg	TM36/PM12
m/p-Xylene #	<5 ^{SV}	<5 ^{SV}	<5	7 ^{SV}	<5	<5	<5 ^{SV}	<5 ^{SV}	<5	<5	<5	ug/kg	TM36/PM12
o-Xylene #	<5 ^{SV}	<5 ^{SV}	<5	<5 ^{SV}	<5	<5	<5 ^{SV}	<5 ^{SV}	<5	<5	<5	ug/kg	TM36/PM12
PCB 28 #	<5	<50 ^{AA}	<5	<50 ^{AA}	<5	<5	<5	<5	<5	<5	<5	ug/kg	TM17/PM8
PCB 52 #	<5	<50 ^{AA}	<5	<50 ^{AA}	<5	<5	<5	<5	<5	<5	<5	ug/kg	TM17/PM8
PCB 101 #	<5	<50 ^{AA}	<5	<50 ^{AA}	<5	<5	<5	<5	<5	<5	<5	ug/kg	TM17/PM8
PCB 118 #	<5	<50 ^{AA}	<5	<50 ^{AA}	<5	<5	<5	<5	<5	<5	<5	ug/kg	TM17/PM8
PCB 138 #	<5	<50 ^{AA}	<5	<50 ^{AA}	<5	<5	<5	<5	<5	<5	<5	ug/kg	TM17/PM8
PCB 153 #	<5	<50 ^{AA}	<5	<50 ^{AA}	<5	<5	<5	<5	<5	<5	<5	ug/kg	TM17/PM8
PCB 180 #	<5	<50 ^{AA}	<5	<50 ^{AA}	<5	<5	<5	<5	<5	<5	<5	ug/kg	TM17/PM8
Total 7 PCBs #	<35	<350 ^{AA}	<35	<350 ^{AA}	<35	<35	<35	<35	<35	<35	<35	ug/kg	TM17/PM8

Element Materials Technology

Client Name: Ground Investigations Ireland
Reference: 13614-02-24
Location: Dykes Road Galway
Contact: Mike Sutton
EMT Job No: 24/7795

Report : Solid

Solids: V=60g VOC jar, J=250g glass jar, T=plastic tub

EMT Sample No.	41-44	45-48	49-52	53-56	57-60						Please see attached notes for all abbreviations and acronyms		
Sample ID	BRC02	BRC04	BRC04	BRC05	BRC05								
Depth	1.50	0.50	1.50	0.50	3.50								
COC No / misc													
Containers	V J T	V J T	V J T	V J T	V J T								
Sample Date	06/05/2024	06/05/2024	06/05/2024	06/05/2024	06/05/2024								
Sample Type	Soil	Soil	Soil	Soil	Soil								
Batch Number	1	1	1	1	1								
Date of Receipt	08/05/2024	08/05/2024	08/05/2024	08/05/2024	08/05/2024						LOD/LOR	Units	Method No.
Antimony	<1	<1	<1	<1	<1						<1	mg/kg	TM30/PM15
Arsenic #	3.5	1.0	3.3	2.2	1.8						<0.5	mg/kg	TM30/PM15
Barium #	39	6	68	12	45						<1	mg/kg	TM30/PM15
Cadmium #	0.6	0.2	0.6	0.2	0.4						<0.1	mg/kg	TM30/PM15
Chromium #	23.4	8.7	29.7	13.2	34.2						<0.5	mg/kg	TM30/PM15
Copper #	17	3	11	4	13						<1	mg/kg	TM30/PM15
Lead #	28	<5	38	<5	8						<5	mg/kg	TM30/PM15
Mercury #	0.2	<0.1	<0.1	<0.1	<0.1						<0.1	mg/kg	TM30/PM15
Molybdenum #	2.1	0.6	1.8	1.2	1.5						<0.1	mg/kg	TM30/PM15
Nickel #	11.7	4.1	8.9	6.4	10.0						<0.7	mg/kg	TM30/PM15
Selenium #	2	<1	<1	<1	<1						<1	mg/kg	TM30/PM15
Zinc #	34	<5	106	<5	28						<5	mg/kg	TM30/PM15
PAH MS													
Naphthalene #	<0.04	<0.40 ^{AA}	<0.04	<0.04	<0.04						<0.04	mg/kg	TM4/PM8
Acenaphthylene	<0.03	<0.30 ^{AA}	<0.03	<0.03	<0.03						<0.03	mg/kg	TM4/PM8
Acenaphthene #	<0.05	<0.50 ^{AA}	<0.05	<0.05	<0.05						<0.05	mg/kg	TM4/PM8
Fluorene #	<0.04	<0.40 ^{AA}	<0.04	<0.04	<0.04						<0.04	mg/kg	TM4/PM8
Phenanthrene #	<0.03	<0.30 ^{AA}	0.28	<0.03	<0.03						<0.03	mg/kg	TM4/PM8
Anthracene #	<0.04	<0.40 ^{AA}	0.11	<0.04	<0.04						<0.04	mg/kg	TM4/PM8
Fluoranthene #	<0.03	<0.30 ^{AA}	0.70	<0.03	<0.03						<0.03	mg/kg	TM4/PM8
Pyrene #	<0.03	<0.30 ^{AA}	0.59	<0.03	<0.03						<0.03	mg/kg	TM4/PM8
Benzo(a)anthracene #	<0.06	<0.60 ^{AA}	0.41	<0.06	<0.06						<0.06	mg/kg	TM4/PM8
Chrysene #	<0.02	<0.20 ^{AA}	0.38	<0.02	<0.02						<0.02	mg/kg	TM4/PM8
Benzo(bk)fluoranthene #	<0.07	<0.70 ^{AA}	0.70	<0.07	<0.07						<0.07	mg/kg	TM4/PM8
Benzo(a)pyrene #	<0.04	<0.40 ^{AA}	0.44	<0.04	<0.04						<0.04	mg/kg	TM4/PM8
Indeno(123cd)pyrene #	<0.04	<0.40 ^{AA}	0.26	<0.04	<0.04						<0.04	mg/kg	TM4/PM8
Dibenzo(ah)anthracene #	<0.04	<0.40 ^{AA}	0.06	<0.04	<0.04						<0.04	mg/kg	TM4/PM8
Benzo(ghi)perylene #	<0.04	<0.40 ^{AA}	0.24	<0.04	<0.04						<0.04	mg/kg	TM4/PM8
Coronene	<0.04	<0.40 ^{AA}	<0.04	<0.04	<0.04						<0.04	mg/kg	TM4/PM8
PAH 6 Total #	<0.22	<2.20 ^{AA}	2.34	<0.22	<0.22						<0.22	mg/kg	TM4/PM8
PAH 17 Total	<0.64	<6.40 ^{AA}	4.17	<0.64	<0.64						<0.64	mg/kg	TM4/PM8
Benzo(b)fluoranthene	<0.05	<0.50 ^{AA}	0.50	<0.05	<0.05						<0.05	mg/kg	TM4/PM8
Benzo(k)fluoranthene	<0.02	<0.20 ^{AA}	0.20	<0.02	<0.02						<0.02	mg/kg	TM4/PM8
Benzo(j)fluoranthene	<1	<10 ^{AA}	<1	<1	<1						<1	mg/kg	TM4/PM8
PAH Surrogate % Recovery	99	98 ^{AA}	100	99	94						<0	%	TM4/PM8
Mineral Oil (C10-C40) (EH_CU_1D_AL)	117	613	<30	137	<30						<30	mg/kg	TM5/PM8/PM16

Element Materials Technology

Client Name: Ground Investigations Ireland
Reference: 13614-02-24
Location: Dykes Road Galway
Contact: Mike Sutton
EMT Job No: 24/7795

Report : Solid

Solids: V=60g VOC jar, J=250g glass jar, T=plastic tub

EMT Sample No.	41-44	45-48	49-52	53-56	57-60								
Sample ID	BRC02	BRC04	BRC04	BRC05	BRC05								
Depth	1.50	0.50	1.50	0.50	3.50								
COC No / misc													
Containers	V J T	V J T	V J T	V J T	V J T								
Sample Date	06/05/2024	06/05/2024	06/05/2024	06/05/2024	06/05/2024								
Sample Type	Soil	Soil	Soil	Soil	Soil								
Batch Number	1	1	1	1	1								
Date of Receipt	08/05/2024	08/05/2024	08/05/2024	08/05/2024	08/05/2024								
Please see attached notes for all abbreviations and acronyms											LOD/LOR	Units	Method No.
TPH CWG													
Aliphatics													
>C5-C6 (HS_1D_AL) #	<0.1 ^{SV}	<0.1 ^{SV}	<0.1	<0.1 ^{SV}	<0.1						<0.1	mg/kg	TM36/PM12
>C6-C8 (HS_1D_AL) #	<0.1 ^{SV}	<0.1 ^{SV}	<0.1	<0.1 ^{SV}	<0.1						<0.1	mg/kg	TM36/PM12
>C8-C10 (HS_1D_AL)	<0.1 ^{SV}	<0.1 ^{SV}	<0.1	0.5 ^{SV}	1.0						<0.1	mg/kg	TM36/PM12
>C10-C12 (EH_CU_1D_AL) #	<0.2	<0.2	<0.2	<0.2	<0.2						<0.2	mg/kg	TM5/PM8/PM16
>C12-C16 (EH_CU_1D_AL) #	<4	<4	<4	<4	<4						<4	mg/kg	TM5/PM8/PM16
>C16-C21 (EH_CU_1D_AL) #	<7	22	<7	<7	<7						<7	mg/kg	TM5/PM8/PM16
>C21-C35 (EH_CU_1D_AL) #	117	521	<7	123	<7						<7	mg/kg	TM5/PM8/PM16
>C35-C40 (EH_CU_1D_AL)	<7	70	<7	14	<7						<7	mg/kg	TM5/PM8/PM16
Total aliphatics C5-40 (EH_CU+HS_1D_AL)	117	613	<26	138	<26						<26	mg/kg	TM5/PM8/PM16
>C6-C10 (HS_1D_AL)	<0.1 ^{SV}	<0.1 ^{SV}	<0.1	0.5 ^{SV}	1.0						<0.1	mg/kg	TM36/PM12
>C10-C25 (EH_CU_1D_AL)	31	92	<10	14	<10						<10	mg/kg	TM5/PM8/PM16
>C25-C35 (EH_CU_1D_AL)	90	450	<10	110	<10						<10	mg/kg	TM5/PM8/PM16
Aromatics													
>C5-EC7 (HS_1D_AR) #	<0.1 ^{SV}	<0.1 ^{SV}	<0.1	<0.1 ^{SV}	<0.1						<0.1	mg/kg	TM36/PM12
>EC7-EC8 (HS_1D_AR) #	<0.1 ^{SV}	<0.1 ^{SV}	<0.1	<0.1 ^{SV}	<0.1						<0.1	mg/kg	TM36/PM12
>EC8-EC10 (HS_1D_AR) #	<0.1 ^{SV}	<0.1 ^{SV}	<0.1	<0.1 ^{SV}	<0.1						<0.1	mg/kg	TM36/PM12
>EC10-EC12 (EH_CU_1D_AR) #	<0.2	<0.2	<0.2	<0.2	<0.2						<0.2	mg/kg	TM5/PM8/PM16
>EC12-EC16 (EH_CU_1D_AR) #	<4	5	<4	<4	<4						<4	mg/kg	TM5/PM8/PM16
>EC16-EC21 (EH_CU_1D_AR) #	<7	65	8	<7	<7						<7	mg/kg	TM5/PM8/PM16
>EC21-EC35 (EH_CU_1D_AR) #	263	1271	9	266	<7						<7	mg/kg	TM5/PM8/PM16
>EC35-EC40 (EH_CU_1D_AR)	34	230	<7	47	<7						<7	mg/kg	TM5/PM8/PM16
Total aromatics C5-40 (EH_CU+HS_1D_AR)	297	1571	<26	313	<26						<26	mg/kg	TM5/PM8/PM16
Total aliphatics and aromatics(C5-40) (EH_CU+HS_1D_Total)	414	2184	<52	451	<52						<52	mg/kg	TM5/PM8/PM16
>EC6-EC10 (HS_1D_AR) #	<0.1 ^{SV}	<0.1 ^{SV}	<0.1	<0.1 ^{SV}	<0.1						<0.1	mg/kg	TM36/PM12
>EC10-EC25 (EH_CU_1D_AR)	56	254	<10	42	<10						<10	mg/kg	TM5/PM8/PM16
>EC25-EC35 (EH_CU_1D_AR)	222	1087	<10	231	<10						<10	mg/kg	TM5/PM8/PM16
MTBE #	<5 ^{SV}	<5 ^{SV}	<5	<5 ^{SV}	<5						<5	ug/kg	TM36/PM12
Benzene #	<5 ^{SV}	<5 ^{SV}	<5	<5 ^{SV}	<5						<5	ug/kg	TM36/PM12
Toluene #	<5 ^{SV}	<5 ^{SV}	<5	<5 ^{SV}	<5						<5	ug/kg	TM36/PM12
Ethylbenzene #	<5 ^{SV}	<5 ^{SV}	<5	<5 ^{SV}	<5						<5	ug/kg	TM36/PM12
m/p-Xylene #	<5 ^{SV}	<5 ^{SV}	<5	<5 ^{SV}	<5						<5	ug/kg	TM36/PM12
o-Xylene #	<5 ^{SV}	<5 ^{SV}	<5	<5 ^{SV}	19						<5	ug/kg	TM36/PM12
PCB 28 #	<5	<50 ^{AA}	<5	<5	<5						<5	ug/kg	TM17/PM8
PCB 52 #	<5	<50 ^{AA}	<5	<5	<5						<5	ug/kg	TM17/PM8
PCB 101 #	<5	<50 ^{AA}	<5	<5	<5						<5	ug/kg	TM17/PM8
PCB 118 #	<5	<50 ^{AA}	<5	<5	<5						<5	ug/kg	TM17/PM8
PCB 138 #	<5	<50 ^{AA}	<5	<5	<5						<5	ug/kg	TM17/PM8
PCB 153 #	<5	<50 ^{AA}	<5	<5	<5						<5	ug/kg	TM17/PM8
PCB 180 #	<5	<50 ^{AA}	<5	<5	<5						<5	ug/kg	TM17/PM8
Total 7 PCBs #	<35	<350 ^{AA}	<35	<35	<35						<35	ug/kg	TM17/PM8

Element Materials Technology

Client Name: Ground Investigations Ireland
Reference: 13614-02-24
Location: Dykes Road Galway
Contact: Mike Sutton
EMT Job No: 24/7795

Report : CEN 10:1 1 Batch

Solids: V=60g VOC jar, J=250g glass jar, T=plastic tub

EMT Sample No.	1-4	5-8	9-12	13-16	17-20	21-24	25-28	29-32	33-36	37-40	Please see attached notes for all abbreviations and acronyms		
Sample ID	BH01	BH01	BH04	BH05	BH05	BH05	BRC01	BRC01	BRC01	BRC02			
Depth	0.50	1.50	1.50	0.25	0.50	1.50	0.50	1.50	3.50	0.50			
COC No / misc													
Containers	V J T	V J T	V J T	V J T	V J T	V J T	V J T	V J T	V J T	V J T			
Sample Date	06/05/2024	06/05/2024	06/05/2024	06/05/2024	06/05/2024	06/05/2024	06/05/2024	06/05/2024	06/05/2024	06/05/2024			
Sample Type	Soil	Soil	Soil	Soil	Soil	Soil	Soil	Soil	Soil	Soil			
Batch Number	1	1	1	1	1	1	1	1	1	1			
Date of Receipt	08/05/2024	08/05/2024	08/05/2024	08/05/2024	08/05/2024	08/05/2024	08/05/2024	08/05/2024	08/05/2024	08/05/2024	LOD/LOR	Units	Method No.
Dissolved Antimony [#]	0.009	0.013	<0.002	<0.002	<0.002	<0.002	<0.002	<0.002	<0.002	<0.002	<0.002	mg/l	TM30/PM17
Dissolved Antimony (A10) [#]	0.09	0.13	<0.02	<0.02	<0.02	<0.02	<0.02	<0.02	<0.02	<0.02	<0.02	mg/kg	TM30/PM17
Dissolved Arsenic [#]	0.0026	0.0051	<0.0025	<0.0025	<0.0025	<0.0025	<0.0025	0.0071	0.0035	<0.0025	<0.0025	mg/l	TM30/PM17
Dissolved Arsenic (A10) [#]	0.026	0.051	<0.025	<0.025	<0.025	<0.025	<0.025	0.071	0.035	<0.025	<0.025	mg/kg	TM30/PM17
Dissolved Barium [#]	0.055	0.068	0.010	<0.003	<0.003	<0.003	<0.003	0.011	0.021	<0.003	<0.003	mg/l	TM30/PM17
Dissolved Barium (A10) [#]	0.55	0.68	0.10	<0.03	<0.03	<0.03	<0.03	0.11	0.21	<0.03	<0.03	mg/kg	TM30/PM17
Dissolved Cadmium [#]	<0.0005	<0.0005	<0.0005	<0.0005	<0.0005	<0.0005	<0.0005	<0.0005	<0.0005	<0.0005	<0.0005	mg/l	TM30/PM17
Dissolved Cadmium (A10) [#]	<0.005	<0.005	<0.005	<0.005	<0.005	<0.005	<0.005	<0.005	<0.005	<0.005	<0.005	mg/kg	TM30/PM17
Dissolved Chromium [#]	<0.0015	<0.0015	<0.0015	<0.0015	<0.0015	<0.0015	<0.0015	<0.0015	<0.0015	<0.0015	<0.0015	mg/l	TM30/PM17
Dissolved Chromium (A10) [#]	<0.015	<0.015	<0.015	<0.015	<0.015	<0.015	<0.015	<0.015	<0.015	<0.015	<0.015	mg/kg	TM30/PM17
Dissolved Copper [#]	<0.007	<0.007	<0.007	<0.007	<0.007	<0.007	<0.007	<0.007	<0.007	<0.007	<0.007	mg/l	TM30/PM17
Dissolved Copper (A10) [#]	<0.07	<0.07	<0.07	<0.07	<0.07	<0.07	<0.07	<0.07	<0.07	<0.07	<0.07	mg/kg	TM30/PM17
Dissolved Lead [#]	<0.005	<0.005	<0.005	<0.005	<0.005	<0.005	<0.005	<0.005	<0.005	<0.005	<0.005	mg/l	TM30/PM17
Dissolved Lead (A10) [#]	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	mg/kg	TM30/PM17
Dissolved Molybdenum [#]	0.015	0.023	<0.002	<0.002	<0.002	<0.002	0.004	0.005	0.003	<0.002	<0.002	mg/l	TM30/PM17
Dissolved Molybdenum (A10) [#]	0.15	0.23	<0.02	<0.02	<0.02	<0.02	0.04	0.05	0.03	<0.02	<0.02	mg/kg	TM30/PM17
Dissolved Nickel [#]	<0.002	<0.002	<0.002	<0.002	<0.002	<0.002	<0.002	<0.002	<0.002	<0.002	<0.002	mg/l	TM30/PM17
Dissolved Nickel (A10) [#]	<0.02	<0.02	<0.02	<0.02	<0.02	<0.02	<0.02	<0.02	<0.02	<0.02	<0.02	mg/kg	TM30/PM17
Dissolved Selenium [#]	<0.003	<0.003	<0.003	<0.003	<0.003	<0.003	<0.003	<0.003	<0.003	<0.003	<0.003	mg/l	TM30/PM17
Dissolved Selenium (A10) [#]	<0.03	<0.03	<0.03	<0.03	<0.03	<0.03	<0.03	<0.03	<0.03	<0.03	<0.03	mg/kg	TM30/PM17
Dissolved Zinc [#]	<0.003	0.005	<0.003	<0.003	<0.003	<0.003	<0.003	<0.003	<0.003	<0.003	<0.003	mg/l	TM30/PM17
Dissolved Zinc (A10) [#]	<0.03	0.05	<0.03	<0.03	<0.03	<0.03	<0.03	<0.03	<0.03	<0.03	<0.03	mg/kg	TM30/PM17
Mercury Dissolved by CVAF [#]	<0.00001	<0.00001	<0.00001	<0.00001	<0.00001	<0.00001	<0.00001	<0.00001	<0.00001	<0.00001	<0.00001	mg/l	TM61/PM0
Mercury Dissolved by CVAF [#]	<0.0001	<0.0001	<0.0001	<0.0001	<0.0001	<0.0001	<0.0001	<0.0001	<0.0001	<0.0001	<0.0001	mg/kg	TM61/PM0
Phenol	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	mg/l	TM26/PM0
Phenol	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1	mg/kg	TM26/PM0
Fluoride	<0.3	<0.3	<0.3	0.3	0.6	0.6	0.5	<0.3	<0.3	<0.3	<0.3	mg/l	TM173/PM0
Fluoride	<3	<3	<3	<3	6	6	5	<3	<3	<3	<3	mg/kg	TM173/PM0
Sulphate as SO4 [#]	93.2	83.4	1.1	4.4	3.4	3.1	2.4	12.6	<0.5	2.5	<0.5	mg/l	TM38/PM0
Sulphate as SO4 [#]	933	834	11	44	34	31	24	126	<5	25	<5	mg/kg	TM38/PM0
Mass of raw test portion	0.1038	0.0984	0.0972	0.094	0.0974	0.0967	0.095	0.1067	0.1052	0.0943		kg	NONE/PM17
Chloride [#]	3.1	9.2	0.4	0.6	0.6	0.7	0.5	2.0	1.8	0.5	<0.3	mg/l	TM38/PM0
Chloride [#]	31	92	4	6	6	7	5	20	18	5	<3	mg/kg	TM38/PM0
Mass of dried test portion	0.09	0.09	0.09	0.09	0.09	0.09	0.09	0.09	0.09	0.09		kg	NONE/PM17
Dissolved Organic Carbon	4	6	5	<2	<2	<2	<2	15	8	<2	<2	mg/l	TM60/PM0
Dissolved Organic Carbon	40	60	50	<20	<20	<20	<20	150	80	<20	<20	mg/kg	TM60/PM0
pH	8.09	8.22	8.17	8.06	7.93	8.02	8.03	8.34	8.19	8.10	<0.01	pH units	TM73/PM0

Element Materials Technology

Client Name: Ground Investigations Ireland
Reference: 13614-02-24
Location: Dykes Road Galway
Contact: Mike Sutton
EMT Job No: 24/7795

Report : CEN 10:1 1 Batch

Solids: V=60g VOC jar, J=250g glass jar, T=plastic tub

EMT Sample No.	41-44	45-48	49-52	53-56	57-60						Please see attached notes for all abbreviations and acronyms		
Sample ID	BRC02	BRC04	BRC04	BRC05	BRC05								
Depth	1.50	0.50	1.50	0.50	3.50								
COC No / misc													
Containers	V J T	V J T	V J T	V J T	V J T								
Sample Date	06/05/2024	06/05/2024	06/05/2024	06/05/2024	06/05/2024								
Sample Type	Soil	Soil	Soil	Soil	Soil								
Batch Number	1	1	1	1	1								
Date of Receipt	08/05/2024	08/05/2024	08/05/2024	08/05/2024	08/05/2024						LOD/LOR	Units	Method No.
Dissolved Antimony [#]	<0.002	<0.002	<0.002	<0.002	<0.002						<0.002	mg/l	TM30/PM17
Dissolved Antimony (A10) [#]	<0.02	<0.02	<0.02	<0.02	<0.02						<0.02	mg/kg	TM30/PM17
Dissolved Arsenic [#]	0.0051	<0.0025	<0.0025	<0.0025	<0.0025						<0.0025	mg/l	TM30/PM17
Dissolved Arsenic (A10) [#]	0.051	<0.025	<0.025	<0.025	<0.025						<0.025	mg/kg	TM30/PM17
Dissolved Barium [#]	0.015	<0.003	0.014	0.009	0.016						<0.003	mg/l	TM30/PM17
Dissolved Barium (A10) [#]	0.15	<0.03	0.14	0.09	0.16						<0.03	mg/kg	TM30/PM17
Dissolved Cadmium [#]	<0.0005	<0.0005	<0.0005	<0.0005	<0.0005						<0.0005	mg/l	TM30/PM17
Dissolved Cadmium (A10) [#]	<0.005	<0.005	<0.005	<0.005	<0.005						<0.005	mg/kg	TM30/PM17
Dissolved Chromium [#]	<0.0015	<0.0015	<0.0015	<0.0015	<0.0015						<0.0015	mg/l	TM30/PM17
Dissolved Chromium (A10) [#]	<0.015	<0.015	<0.015	<0.015	<0.015						<0.015	mg/kg	TM30/PM17
Dissolved Copper [#]	<0.007	<0.007	<0.007	<0.007	<0.007						<0.007	mg/l	TM30/PM17
Dissolved Copper (A10) [#]	<0.07	<0.07	<0.07	<0.07	<0.07						<0.07	mg/kg	TM30/PM17
Dissolved Lead [#]	<0.005	<0.005	<0.005	<0.005	<0.005						<0.005	mg/l	TM30/PM17
Dissolved Lead (A10) [#]	<0.05	<0.05	<0.05	<0.05	<0.05						<0.05	mg/kg	TM30/PM17
Dissolved Molybdenum [#]	0.008	<0.002	<0.002	<0.002	<0.002						<0.002	mg/l	TM30/PM17
Dissolved Molybdenum (A10) [#]	0.08	<0.02	<0.02	<0.02	<0.02						<0.02	mg/kg	TM30/PM17
Dissolved Nickel [#]	<0.002	<0.002	<0.002	<0.002	<0.002						<0.002	mg/l	TM30/PM17
Dissolved Nickel (A10) [#]	<0.02	<0.02	<0.02	<0.02	<0.02						<0.02	mg/kg	TM30/PM17
Dissolved Selenium [#]	<0.003	<0.003	<0.003	<0.003	<0.003						<0.003	mg/l	TM30/PM17
Dissolved Selenium (A10) [#]	<0.03	<0.03	<0.03	<0.03	<0.03						<0.03	mg/kg	TM30/PM17
Dissolved Zinc [#]	0.003	<0.003	<0.003	<0.003	<0.003						<0.003	mg/l	TM30/PM17
Dissolved Zinc (A10) [#]	<0.03	<0.03	<0.03	<0.03	<0.03						<0.03	mg/kg	TM30/PM17
Mercury Dissolved by CVA [#]	<0.00001	<0.00001	<0.00001	<0.00001	<0.00001						<0.00001	mg/l	TM61/PM0
Mercury Dissolved by CVA [#]	<0.0001	<0.0001	<0.0001	<0.0001	<0.0001						<0.0001	mg/kg	TM61/PM0
Phenol	<0.01	<0.01	<0.01	<0.01	<0.01						<0.01	mg/l	TM26/PM0
Phenol	<0.1	<0.1	<0.1	<0.1	<0.1						<0.1	mg/kg	TM26/PM0
Fluoride	0.4	<0.3	0.4	0.7	0.4						<0.3	mg/l	TM173/PM0
Fluoride	4	<3	4	7	4						<3	mg/kg	TM173/PM0
Sulphate as SO ₄ [#]	21.0	4.5	6.1	149.4	4.5						<0.5	mg/l	TM38/PM0
Sulphate as SO ₄ [#]	210	45	61	1493	45						<5	mg/kg	TM38/PM0
Mass of raw test portion	0.1029	0.0936	0.0968	0.0958	0.0983							kg	NONE/PM17
Chloride [#]	1.7	0.8	0.6	0.9	0.7						<0.3	mg/l	TM38/PM0
Chloride [#]	17	8	6	9	7						<3	mg/kg	TM38/PM0
Mass of dried test portion	0.09	0.09	0.09	0.09	0.09							kg	NONE/PM17
Dissolved Organic Carbon	10	<2	3	<2	<2						<2	mg/l	TM60/PM0
Dissolved Organic Carbon	100	<20	30	<20	<20						<20	mg/kg	TM60/PM0
pH	8.21	8.00	8.34	7.74	8.03						<0.01	pH units	TM73/PM0

Element Materials Technology

Client Name: Ground Investigations Ireland
Reference: 13614-02-24
Location: Dykes Road Galway
Contact: Mike Sutton
EMT Job No: 24/7795

Report : CEN 10:1 1 Batch

Solids: V=60g VOC jar, J=250g glass jar, T=plastic tub

[illegible]

Solids: V=60g VOC jar, J=250g glass jar, T=plastic tub

[illegible]

Please include all sections of this report if it is reproduced

QF-PM 3.1.17 v3 All solid results are expressed on a dry weight basis unless stated otherwise. 12 of 22

Solids: V=60g VOC jar, J=250g glass jar, T=plastic tub

Please see attached notes for all abbreviations and acronyms

Matrix : Solid

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NOTES TO ACCOMPANY ALL SCHEDULES AND REPORTS

EMT Job No.: 24/7795

SOILS and ASH

Please note we are only MCERTS accredited (UK soils only) for sand, loam and clay and any other matrix is outside our scope of accreditation.

Where an MCERTS report has been requested, you will be notified within 48 hours of any samples that have been identified as being outside our MCERTS scope. As validation has been performed on clay, sand and loam, only samples that are predominantly these matrices, or combinations of them will be within our MCERTS scope. If samples are not one of a combination of the above matrices they will not be marked as MCERTS accredited.

It is assumed that you have taken representative samples on site and require analysis on a representative subsample. Stones will generally be included unless we are requested to remove them.

All samples will be discarded one month after the date of reporting, unless we are instructed to the contrary. Asbestos samples are retained for 6 months.

If you have not already done so, please send us a purchase order if this is required by your company.

Where appropriate please make sure that our detection limits are suitable for your needs, if they are not, please notify us immediately.

All analysis is reported on a dry weight basis unless stated otherwise. Limits of detection for analyses carried out on as received samples are not moisture content corrected. Results are not surrogate corrected. Samples are dried at 35°C ±5°C unless otherwise stated. Moisture content for CEN Leachate tests are dried at 105°C ±5°C. Ash samples are dried at 35°C ±5°C.

Where Mineral Oil is quoted, this refers to Total Aliphatics C10-C40.

Where a CEN 10:1 ZERO Headspace VOC test has been carried out, a 10:1 ratio of water to wet (as received) soil has been used.

% Asbestos in Asbestos Containing Materials (ACMs) is determined by reference to HSG 264 The Survey Guide - Appendix 2 : ACMs in buildings listed in order of ease of fibre release.

Sufficient amount of sample must be received to carry out the testing specified. Where an insufficient amount of sample has been received the testing may not meet the requirements of our accredited methods, as such accreditation may be removed.

Negative Neutralization Potential (NP) values are obtained when the volume of NaOH (0.1N) titrated (pH 8.3) is greater than the volume of HCl (1N) to reduce the pH of the sample to 2.0 - 2.5. Any negative NP values are corrected to 0.

The calculation of Pyrite content assumes that all oxidisable sulphides present in the sample are pyrite. This may not be the case. The calculation may be an overestimate when other sulphides such as Barite (Barium Sulphate) are present.

WATERS

Please note we are not a UK Drinking Water Inspectorate (DWI) Approved Laboratory .

ISO17025 accreditation applies to surface water and groundwater and usually one other matrix which is analysis specific, any other liquids are outside our scope of accreditation.

As surface waters require different sample preparation to groundwaters the laboratory must be informed of the water type when submitting samples.

Where Mineral Oil is quoted, this refers to Total Aliphatics C10-C40.

STACK EMISSIONS

Where an MCERTS report has been requested, you will be notified within 48 hours of any samples that have been identified as being outside our MCERTS scope. As validation for Dioxins and Furans and Dioxin like PCBs has been performed on XAD-2 Resin, only samples which use this resin will be within our MCERTS scope.

Where appropriate please make sure that our detection limits are suitable for your needs, if they are not, please notify us immediately.

DEVIATING SAMPLES

All samples should be submitted to the laboratory in suitable containers with sufficient ice packs to sustain an appropriate temperature for the requested analysis. The temperature of sample receipt is recorded on the confirmation schedules in order that the client can make an informed decision as to whether testing should still be undertaken.

SURROGATES

Surrogate compounds are added during the preparation process to monitor recovery of analytes. However low recovery in soils is often due to peat, clay or other organic rich matrices. For waters this can be due to oxidants, surfactants, organic rich sediments or remediation fluids. Acceptable limits for most organic methods are 70 - 130% and for VOCs are 50 - 150%. When surrogate recoveries are outside the performance criteria but the associated AQC passes this is assumed to be due to matrix effect. Results are not surrogate corrected.

DILUTIONS

A dilution suffix indicates a dilution has been performed and the reported result takes this into account. No further calculation is required.

BLANKS

Where analytes have been found in the blank, the sample will be treated in accordance with our laboratory procedure for dealing with contaminated blanks.

NOTE

Data is only reported if the laboratory is confident that the data is a true reflection of the samples analysed. Data is only reported as accredited when all the requirements of our Quality System have been met. In certain circumstances where all the requirements of the Quality System have not been met, for instance if the associated AQC has failed, the reason is fully investigated and documented. The sample data is then evaluated alongside the other quality control checks performed during analysis to determine its suitability. Following this evaluation, provided the sample results have not been effected, the data is reported but accreditation is removed. It is a requirement of our Accreditation Body for data not reported as accredited to be considered indicative only, but this does not mean the data is not valid.

Where possible, and if requested, samples will be re-extracted and a revised report issued with accredited results. Please do not hesitate to contact the laboratory if further details are required of the circumstances which have led to the removal of accreditation.

Laboratory records are kept for a period of no less than 6 years.

REPORTS FROM THE SOUTH AFRICA LABORATORY

Any method number not prefixed with SA has been undertaken in our UK laboratory unless reported as subcontracted.

Measurement Uncertainty

Measurement uncertainty defines the range of values that could reasonably be attributed to the measured quantity. This range of values has not been included within the reported results. Uncertainty expressed as a percentage can be provided upon request.

Customer Provided Information

Sample ID and depth is information provided by the customer.

Age of Diesel

The age of release estimation is based on the nC17/pristane ratio only as prescribed by Christensen and Larsen (1993) and Kaplan, Galperin, Alimi et al., (1996).

Age estimation should be treated with caution as it can be influenced by site specific factors of which the laboratory are not aware.

Tentatively Identified Compounds (TICs)

Where Tentatively Identified Compounds (TICs) are reported, up to 10 Tentatively Identified Compounds will be listed where there is found to be a greater than 80% match with the NIST library. The reported concentration is determined semi-quantitatively, with a matrix specific limit of detection.

Note, other compounds may be present but are not reported.

ABBREVIATIONS and ACRONYMS USED

#	ISO17025 (UKAS Ref No. 4225) accredited - UK.
SA	ISO17025 (SANAS Ref No.T0729) accredited - South Africa
B	Indicates analyte found in associated method blank.
DR	Dilution required.
M	MCERTS accredited.
NA	Not applicable
NAD	No Asbestos Detected.
ND	None Detected (usually refers to VOC and/SVOC TICs).
NDP	No Determination Possible
SS	Calibrated against a single substance
SV	Surrogate recovery outside performance criteria. This may be due to a matrix effect.
W	Results expressed on as received basis.
+	AQC failure, accreditation has been removed from this result, if appropriate, see 'Note' on previous page.
>>	Results above quantitative calibration range. The result should be considered the minimum value and is indicative only. The actual result could be significantly higher.
*	Analysis subcontracted to an Element Materials Technology approved laboratory.
CO	Suspected carry over
LOD/LOR	Limit of Detection (Limit of Reporting) in line with ISO 17025 and MCERTS
ME	Matrix Effect
NFD	No Fibres Detected
BS	AQC Sample
LB	Blank Sample
N	Client Sample
TB	Trip Blank Sample
OC	Outside Calibration Range
AA	x10 Dilution
AB	x20 Dilution
BA	x10 Dilution

HWOL ACRONYMS AND OPERATORS USED

HS	Headspace Analysis.
EH	Extractable Hydrocarbons - i.e. everything extracted by the solvent.
CU	Clean-up - e.g. by florisil, silica gel.
1D	GC - Single coil gas chromatography.
Total	Aliphatics & Aromatics.
AL	Aliphatics only.
AR	Aromatics only.
2D	GC-GC - Double coil gas chromatography.
#1	EH_Total but with humics mathematically subtracted
#2	EU_Total but with fatty acids mathematically subtracted
_	Operator - underscore to separate acronyms (exception for +).
+	Operator to indicate cumulative e.g. EH+HS_Total or EH_CU+HS_Total
MS	Mass Spectrometry.

EMT Job No: 24/7795

Test Method No.	Description	Prep Method No. (if appropriate)	Description	ISO 17025 (UKAS/ANAS)	MCERTS (UK soils only)	Analysis done on As Received (AR) or Dried (AD)	Reported on dry weight basis
PM4	Gravimetric measurement of Natural Moisture Content and % Moisture Content at either 35°C or 105°C. Calculation based on ISO 11465:1993(E) and BS1377-2:1990.	PM0	No preparation is required.			AR	
TM4	Modified USEPA 8270D v5:2014 method for the solvent extraction and determination of PAHs by GC-MS.	PM8	End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required.			AR	Yes
TM4	Modified USEPA 8270D v5:2014 method for the solvent extraction and determination of PAHs by GC-MS.	PM8	End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required.	Yes		AR	Yes
TM5	Modified 8015B v2:1996 method for the determination of solvent Extractable Petroleum Hydrocarbons (EPH) within the range C8-C40 by GCFID. For waters the solvent extracts dissolved phase plus a sheen if present.	PM16	Fractionation into aliphatic and aromatic fractions using a Rapid Trace SPE.			AR	
TM5	Modified 8015B v2:1996 method for the determination of solvent Extractable Petroleum Hydrocarbons (EPH) within the range C8-C40 by GCFID. For waters the solvent extracts dissolved phase plus a sheen if present.	PM8/PM16	End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required/Fractionation into aliphatic and aromatic fractions using a Rapid Trace SPE.			AR	Yes
TM5	Modified 8015B v2:1996 method for the determination of solvent Extractable Petroleum Hydrocarbons (EPH) within the range C8-C40 by GCFID. For waters the solvent extracts dissolved phase plus a sheen if present.	PM8/PM16	End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required/Fractionation into aliphatic and aromatic fractions using a Rapid Trace SPE.	Yes		AR	Yes
TM5/TM36	please refer to TM5 and TM36 for method details	PM8/PM12/PM16	please refer to PM8/PM16 and PM12 for method details			AR	Yes
TM17	Modified US EPA method 8270D v5:2014. Determination of specific Polychlorinated Biphenyl congeners by GC-MS.	PM8	End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required.	Yes		AR	Yes
TM20	Modified BS 1377-3:1990/USEPA 160.1/3 (TDS/TS: 1971) Gravimetric determination of Total Dissolved Solids/Total Solids	PM0	No preparation is required.	Yes		AR	Yes
TM21	Modified BS 7755-3:1995, ISO10694:1995 Determination of Total Organic Carbon or Total Carbon by combustion in an Eltra TOC furnace/analyser in the presence of oxygen. The CO2 generated is quantified using infra-red detection. Organic Matter (SOM) calculated as per EA MCERTS Chemical Testing of Soil.	PM24	Preparation of Soil and Marine Sediment Samples for Total Organic Carbon.	Yes		AD	Yes

EMT Job No: 24/7795

Test Method No.	Description	Prep Method No. (if appropriate)	Description	ISO 17025 (UKAS/ANAS)	MCERTS (UK soils only)	Analysis done on As Received (AR) or Dried (AD)	Reported on dry weight basis
TM26	Determination of phenols by Reversed Phased High Performance Liquid Chromatography and Electro-Chemical Detection.	PM0	No preparation is required.			AR	Yes
TM30	Determination of Trace Metals by ICP-OES (Inductively Coupled Plasma – Optical Emission Spectrometry): WATERS by Modified USEPA Method 200.7, Rev. 4.4, 1994; Modified EPA Method 6010B, Rev.2, Dec 1996; Modified BS EN ISO 11885:2009: SOILS by Modified USEP 6010B, Rev.2, Dec.1996; Modified EPA Method 3050B, Rev.2, Dec.1996	PM15	Acid digestion of dried and ground solid samples using Aqua Regia refluxed at 112.5 °C. Samples containing asbestos are not dried and ground.			AD	Yes
TM30	Determination of Trace Metals by ICP-OES (Inductively Coupled Plasma – Optical Emission Spectrometry): WATERS by Modified USEPA Method 200.7, Rev. 4.4, 1994; Modified EPA Method 6010B, Rev.2, Dec 1996; Modified BS EN ISO 11885:2009: SOILS by Modified USEP 6010B, Rev.2, Dec.1996; Modified EPA Method 3050B, Rev.2, Dec.1996	PM15	Acid digestion of dried and ground solid samples using Aqua Regia refluxed at 112.5 °C. Samples containing asbestos are not dried and ground.	Yes		AD	Yes
TM30	Determination of Trace Metals by ICP-OES (Inductively Coupled Plasma – Optical Emission Spectrometry): WATERS by Modified USEPA Method 200.7, Rev. 4.4, 1994; Modified EPA Method 6010B, Rev.2, Dec 1996; Modified BS EN ISO 11885:2009: SOILS by Modified USEP 6010B, Rev.2, Dec.1996; Modified EPA Method 3050B, Rev.2, Dec.1996	PM17	Modified method BS EN12457-2:2002 As received solid samples are leached with water in a 10:1 water to soil ratio for 24 hours, the moisture content of the sample is included in the ratio.	Yes		AR	Yes
TM36	Modified US EPA method 8015B v2:1996. Determination of Gasoline Range Organics (GRO) in the carbon chain range of C4-12 by headspace GC-FID. MTBE by GC/FID co-elutes with 3-methylpentane if present and therefore can give a false positive. Positive MTBE results will be re-run using GC-MS to double check, when requested.	PM12	Modified US EPA method 5021A v2:2014. Preparation of solid and liquid samples for GC headspace analysis.			AR	Yes
TM36	Modified US EPA method 8015B v2:1996. Determination of Gasoline Range Organics (GRO) in the carbon chain range of C4-12 by headspace GC-FID. MTBE by GC/FID co-elutes with 3-methylpentane if present and therefore can give a false positive. Positive MTBE results will be re-run using GC-MS to double check, when requested.	PM12	Modified US EPA method 5021A v2:2014. Preparation of solid and liquid samples for GC headspace analysis.	Yes		AR	Yes
TM38	Soluble Ion analysis using Discrete Analyser. Modified US EPA methods: Chloride 325.2 (1978), Sulphate 375.4 (Rev.2 1993), o-Phosphate 365.2 (Rev.2 1993), TON 353.1 (Rev.2 1993), Nitrite 354.1 (1971), Hex Cr 7196A (1992), NH4+ 350.1 (Rev.2 1993) – All anions comparable to BS ISO 15923-1: 2013I	PM0	No preparation is required.	Yes		AR	Yes
TM38	Soluble Ion analysis using Discrete Analyser. Modified US EPA methods: Chloride 325.2 (1978), Sulphate 375.4 (Rev.2 1993), o-Phosphate 365.2 (Rev.2 1993), TON 353.1 (Rev.2 1993), Nitrite 354.1 (1971), Hex Cr 7196A (1992), NH4+ 350.1 (Rev.2 1993) – All anions comparable to BS ISO 15923-1: 2013I	PM20	Extraction of dried and ground or as received samples with deionised water in a 2:1 water to solid ratio using a reciprocal shaker for all analytes except hexavalent chromium. Extraction of as received sample using 10:1 ratio of 0.2M sodium hydroxide to soil for hexavalent chromium using a reciprocal shaker.	Yes		AR	Yes
TM60	TC/TOC analysis of Waters by High Temperature Combustion followed by NDIR detection. Based on the following modified standard methods: USEPA 9060A (2002), APHA SMEWW 5310B:1999 22nd Edition, ASTM D 7573, and USEPA 415.1.	PM0	No preparation is required.			AR	Yes
TM61	Determination of Mercury by Cold Vapour Atomic Fluorescence - WATERS: Modified USEPA Method 245.7, Rev 2, Feb 2005. SOILS: Modified USEPA Method 7471B, Rev.2, Feb 2007	PM0	No preparation is required.	Yes		AR	Yes

EMT Job No: 24/7795

Test Method No.	Description	Prep Method No. (if appropriate)	Description	ISO 17025 (UKAS/S ANAS)	MCERTS (UK soils only)	Analysis done on As Received (AR) or Dried (AD)	Reported on dry weight basis
TM73	Modified US EPA methods 150.1 (1982) and 9045D Rev. 4 - 2004) and BS1377-3:1990. Determination of pH by Metrohm automated probe analyser.	PM0	No preparation is required.			AR	Yes
TM73	Modified US EPA methods 150.1 (1982) and 9045D Rev. 4 - 2004) and BS1377-3:1990. Determination of pH by Metrohm automated probe analyser.	PM11	Extraction of as received solid samples using one part solid to 2.5 parts deionised water.	Yes		AR	No
TM173	Analysis of fluoride by ISE (Ion Selective Electrode) using modified ISE method 9214 - 340.2 (EPA 1998)	PM0	No preparation is required.			AR	Yes
NONE	No Method Code	NONE	No Method Code			AD	Yes
NONE	No Method Code	PM17	Modified method BS EN12457-2:2002 As received solid samples are leached with water in a 10:1 water to soil ratio for 24 hours, the moisture content of the sample is included in the ratio.				
NONE	No Method Code	PM4	Gravimetric measurement of Natural Moisture Content and % Moisture Content at either 35°C or 105°C. Calculation based on ISO 11465:1993(E) and BS1377-2:1990.			AR	
Subcontracted	See attached subcontractor report for accreditation status and provider.					AR	



LABORATORY REPORT



Contract Number: PSL24/3965

Report Date: 28 June 2024

Client's Reference: 13614-02-24

Client Name: Ground Investigations Ireland Ltd
Catherinestown House
Hazelhatch Road
Newcastle
Co Dublin
D22 YD52

For the attention of: Mike Sutton

Contract Title: Dyke Road Galway

Date Received: 5/6/2024

Date Commenced: 5/6/2024

Date Completed: 28/6/2024

Notes: Opinions and Interpretations are outside the UKAS Accreditation

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Checked and Approved Signatories:

A Watkins
(Managing Director)

R Berriman
(Associate Director)

S Royle
(Laboratory Manager)

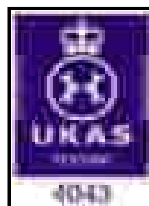
L Knight
(Assistant Laboratory Manager) (Senior Technical Coordinator)

S Eyre

T Watkins
(Senior Technician)

SUMMARY OF LABORATORY SOIL DESCRIPTIONS

Hole Number	Sample Number	Sample Type	Top Depth m	Base Depth m	Description of Sample
BH01		UT	3.00	3.45	Very soft brown organic CLAY.
BH01		B	3.50		Brown slightly sandy slightly gravelly organic CLAY.
BH01		B	6.70		Brown slightly sandy slightly gravelly organic CLAY.
BH02		UT	1.00	1.27	Dark brown fibrous PEAT.
BH02		B	3.00		Grey slightly sandy gravelly CLAY with some organic material.
BH02		UT	3.00	3.45	Very soft dark grey organic CLAY.
BH03		UT	1.00	1.45	Dark brown organic CLAY.
BH03		UT	3.00	3.45	Brown organic CLAY.
BH05		B	4.00		Grey slightly silty sandy GRAVEL with many cobbles.
BH06		UT	3.00	3.45	Brown slightly sandy organic CLAY.
BRC01		UT	5.30	5.75	Very soft brown organic CLAY.
BRC01		B	5.50		Brown slightly sandy slightly gravelly organic CLAY.
BRC01		B	7.50		Brown slightly sandy slightly gravelly organic CLAY.
BRC01		UT	7.50	7.95	Very soft brown organic CLAY.
BRC02		UT	1.60	1.87	Dark brown PEAT.
BRC02		B	2.00		Dark brown slightly sandy gravelly organic CLAY.
BRC02		UT	3.00	3.45	Very soft light brown organic CLAY.
BRC02		B	3.50		Brown slightly sandy organic CLAY.
BRC02		UT	5.00	5.45	Very soft brown organic CLAY.



Dyke Road Galway

Contract No:

PSL24/3965

Client Ref:

13614-02-24

SUMMARY OF LABORATORY SOIL DESCRIPTIONS

Hole Number	Sample Number	Sample Type	Top Depth m	Base Depth m	Description of Sample
BRC02		B	7.00		Grey silty sandy GRAVEL with many cobbles.
BRC03		UT	3.00	3.45	Very soft brown slightly gravelly organic CLAY.
BRC03		B	4.00		Brown slightly sandy slightly gravelly CLAY with many cobbles.
BRC03		B	7.50		Brown slightly sandy slightly gravelly CLAY.
BRC03		B	8.50		Grey sandy slightly gravelly CLAY.
BRC03		B	9.50		Grey slightly sandy slightly gravelly CLAY.
BRC04		B	4.00		Grey slightly silty sandy GRAVEL with many cobbles.
BRC05		B	2.60		Grey slightly sandy slightly gravelly CLAY with many cobbles.
BRC05		B	3.50		Grey slightly sandy slightly gravelly CLAY with many cobbles.
BRC05		B	4.50		Grey slightly sandy gravelly SILT.
TP01		B	0.50		Grey clayey sandy GRAVEL.
TP01		B	1.00		Dark brown fibrous PEAT.
TP02		B	2.00		Brown peaty SILT.
TP03		B	2.00		Brown peaty SILT.
TP05		B	1.00		Grey very sandy very silty GRAVEL.



Dyke Road Galway

Contract No:

PSL24/3965

Client Ref:

13614-02-24

SUMMARY OF SOIL CLASSIFICATION TESTS

(BS1377 : PART 2 : 1990)

Hole Number	Sample Number	Sample Type	Top Depth m	Base Depth m	Moisture Content % Clause 3.2	Linear Shrinkage % Clause 6.5	Particle Density Mg/m ³ Clause 8.2	Liquid Limit % Clause 4.3/4	Plastic Limit % Clause 5.3	Plasticity Index % Clause 5.4	Passing .425mm %	Remarks
BH01		UT	3.00	3.45	136			150	55	95	100	Extremely High Plasticity CE
BH01		B	3.50		145			141	45	96	96	Extremely High Plasticity CE
BH01		B	6.70		123			151	55	96	88	Extremely High Plasticity CE
BH02		UT	1.00	1.27	80				NP			
BH02		B	3.00		27			56	26	30	55	High Plasticity CH
BH02		UT	3.00	3.45	133			161	58	103	100	Extremely High Plasticity CE
BH06		UT	3.00	3.45	159			258	84	174	100	Extremely High Plasticity CE
BRC01		UT	5.30	5.75	141			237	78	159	100	Extremely High Plasticity CE
BRC01		B	5.50		160			243	80	163	94	Extremely High Plasticity CE
BRC01		B	7.50		197			274	88	186	51	Extremely High Plasticity CE
BRC01		UT	7.50	7.95	192			233	77	156	100	Extremely High Plasticity CE
BRC02		UT	1.60	1.87	92			302	135	167	92	Extremely High Plasticity ME
BRC02		B	2.00		88			321	101	220	38	Extremely High Plasticity CE
BRC02		UT	3.00	3.45	115			135	51	84	100	Extremely High Plasticity CE
BRC02		B	3.50		121			124	48	76	100	Extremely High Plasticity CE
BRC02		B	7.00		1.2				NP			
BRC03		UT	3.00	3.45	113			256	83	173	100	Extremely High Plasticity CE
BRC03		B	4.00		8.2			20	12	8	41	Low Plasticity CL
BRC03		B	7.50		8.2			24	13	11	65	Low Plasticity CL

SYMBOLS : NP : Non Plastic

* : Liquid Limit and Plastic Limit Wet Sieved.



Dyke Road Galway

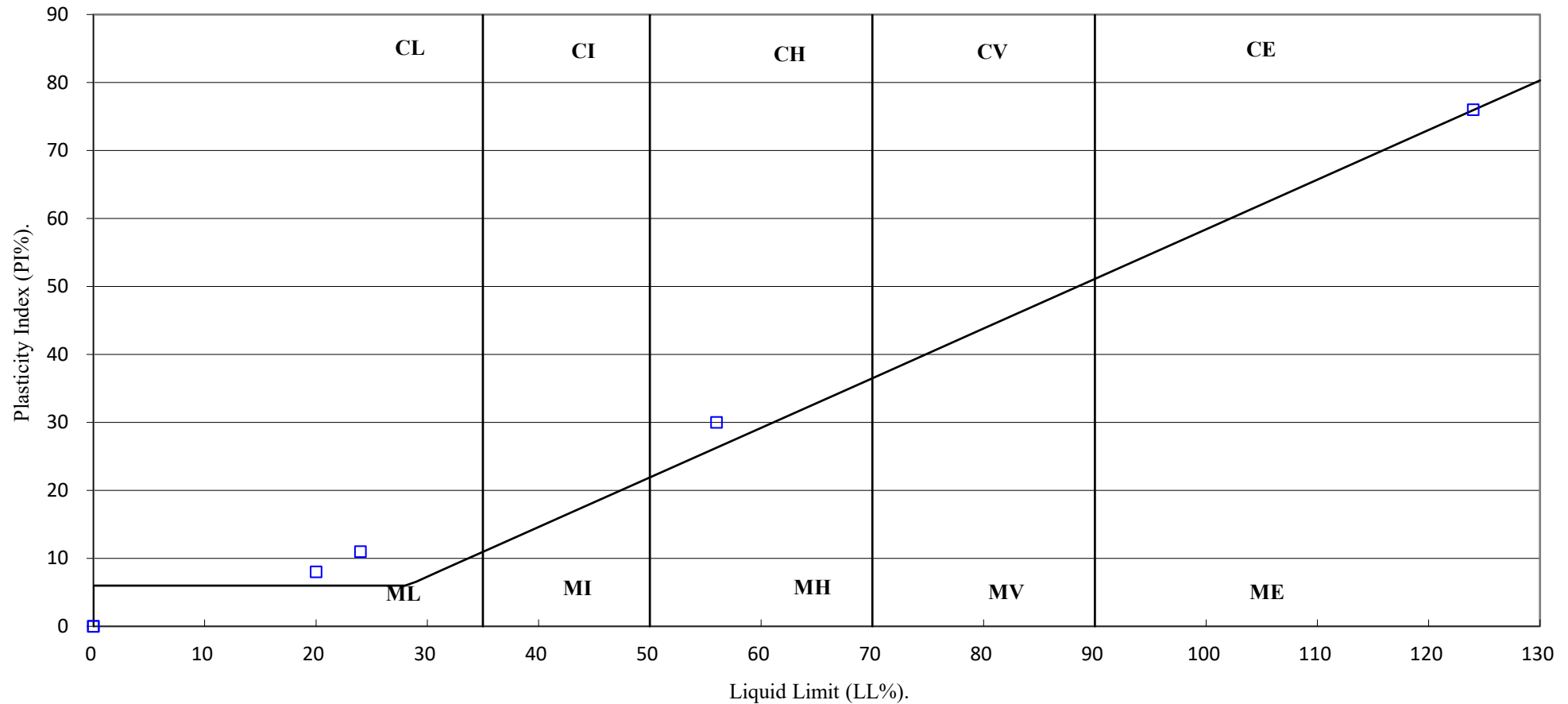
Contract No:

PSL24/3965

Client Ref:

13614-02-24

PLASTICITY CHART FOR CASAGRANDE CLASSIFICATION.



Dyke Road Galway

Contract No:

PSL24/3965

Client Ref:

13614-02-24

SUMMARY OF SOIL CLASSIFICATION TESTS

(BS1377 : PART 2 : 1990)

[illegible]

SYMBOLS : NP : Non Plastic

*** : Liquid Limit and Plastic Limit Wet Sieved.**



Dyke Road Galway

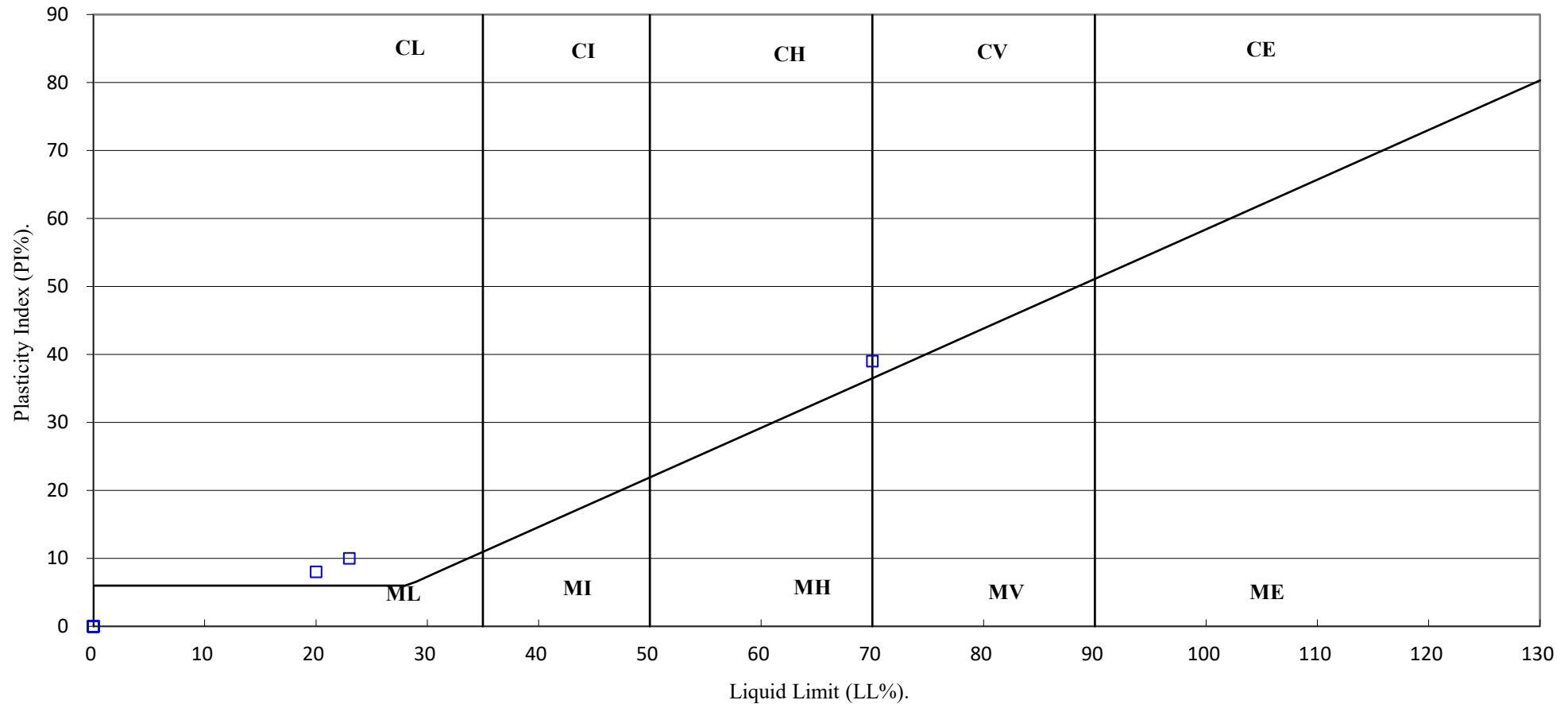
Contract No:

PSL24/3965

Client Ref:

13614-02-24

PLASTICITY CHART FOR CASAGRANDE CLASSIFICATION.



Dyke Road Galway

Contract No:

PSL24/3965

Client Ref:

13614-02-24

PARTICLE SIZE DISTRIBUTION TEST

BS1377 : Part 2 : 1990

Wet Sieve & Pipette Analysis, Clause 9.2 & 9.4

Hole Number:

BH01

Top Depth (m):

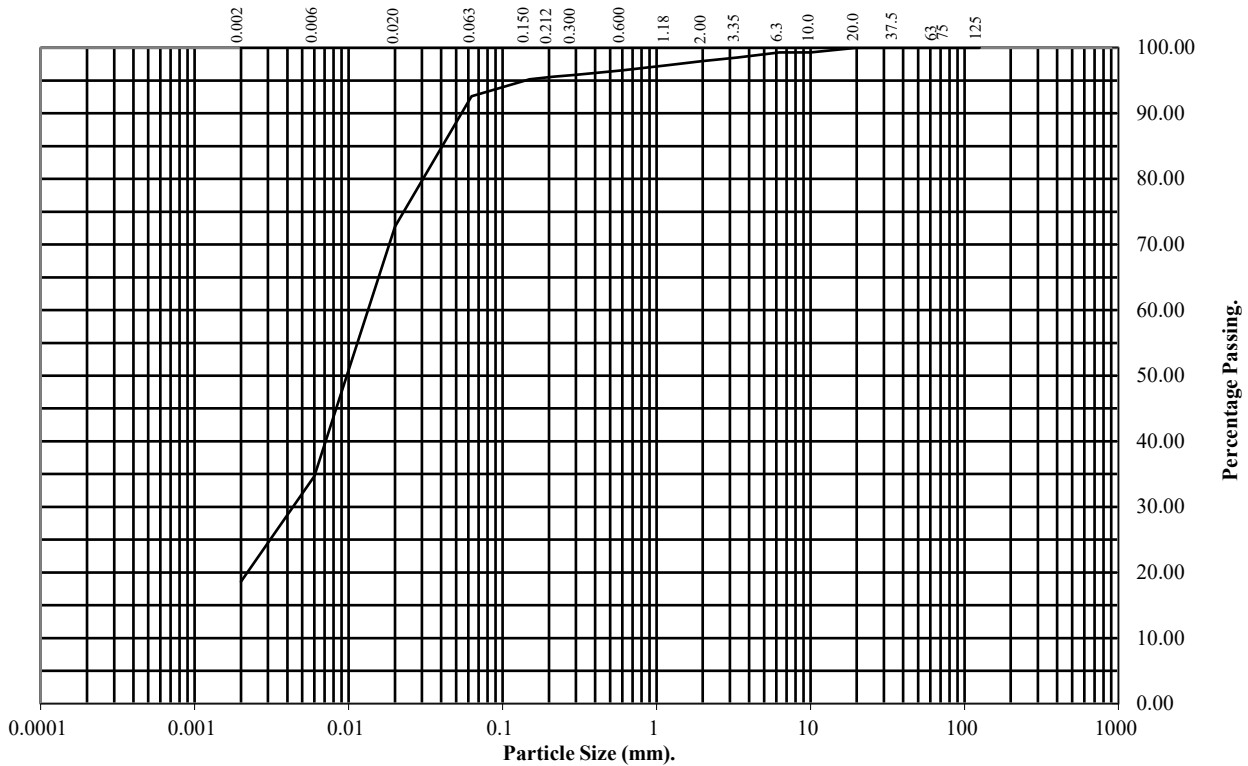
3.50

Sample Number:

Base Depth(m):

Sample Type:

B



BS Test Sieve (mm)	Percentage Passing
125	100
75	100
63	100
37.5	100
20	100
10	99
6.3	99
3.35	99
2	98
1.18	97
0.6	97
0.3	96
0.212	96
0.15	95
0.063	93

Particle Diameter	Percentage Passing
0.02	73
0.006	35
0.002	19

Soil Fraction	Total Percentage
Cobbles	0
Gravel	2
Sand	5
Silt	74
Clay	19

Remarks:

See Summary of Soil Descriptions



Dyke Road Galway

Contract No:

PSL24/3965

Client Ref:

13614-02-24

PARTICLE SIZE DISTRIBUTION TEST

BS1377 : Part 2 : 1990

Wet Sieve & Pipette Analysis, Clause 9.2 & 9.4

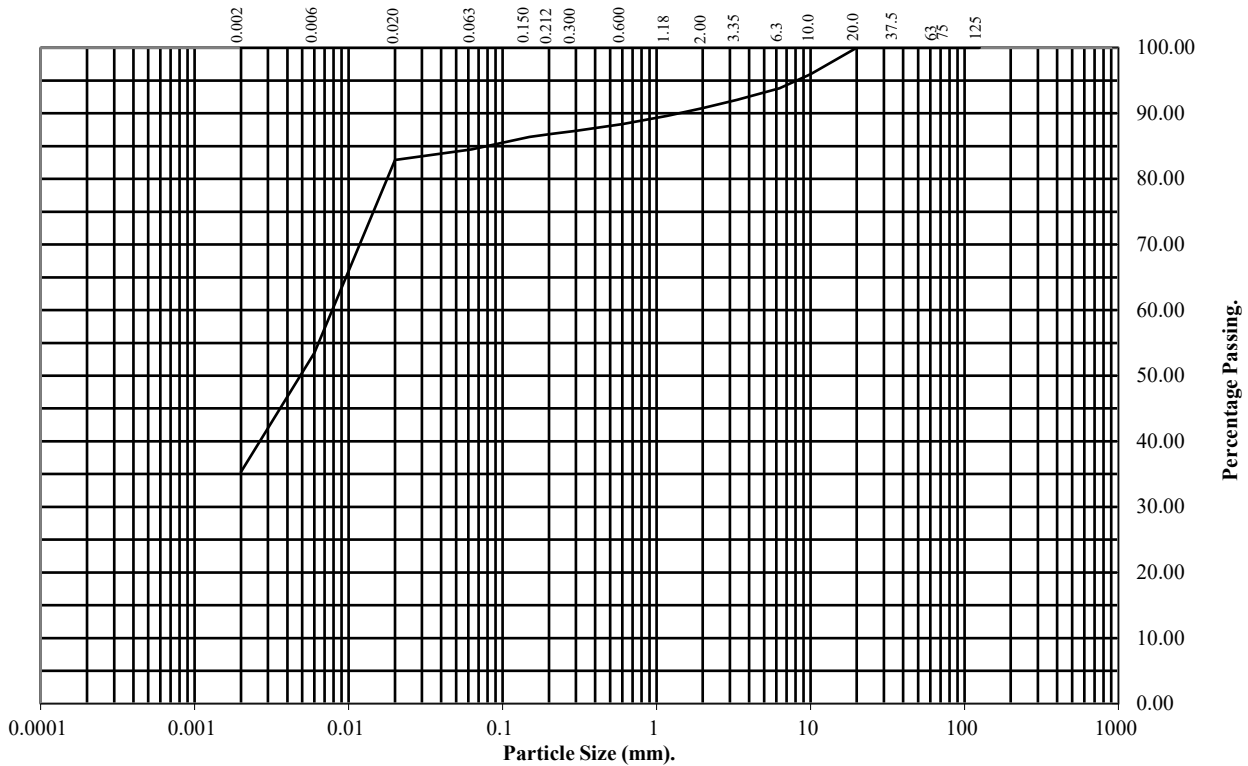
Hole Number: BH01

Top Depth (m): 6.70

Sample Number:

Base Depth(m):

Sample Type: B



BS Test Sieve (mm)	Percentage Passing
125	100
75	100
63	100
37.5	100
20	100
10	96
6.3	94
3.35	92
2	91
1.18	90
0.6	88
0.3	87
0.212	87
0.15	86
0.063	85

Particle Diameter	Percentage Passing
0.02	83
0.006	53
0.002	35

Soil Fraction	Total Percentage
Cobbles	0
Gravel	9
Sand	6
Silt	50
Clay	35

Remarks:

See Summary of Soil Descriptions



Dyke Road Galway

Contract No:
PSL24/3965
Client Ref:
13614-02-24

PARTICLE SIZE DISTRIBUTION TEST

BS1377 : Part 2 : 1990

Wet Sieve & Pipette Analysis, Clause 9.2 & 9.4

Hole Number:

BH02

Top Depth (m):

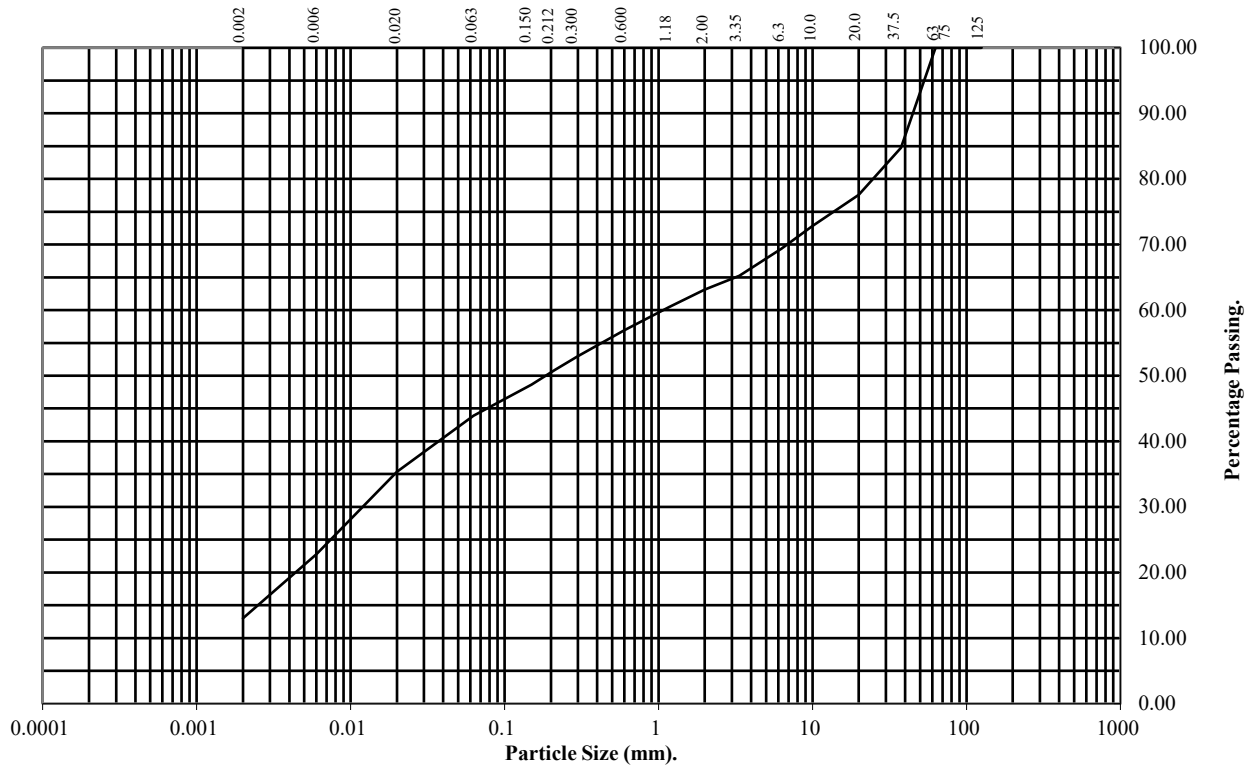
3.00

Sample Number:

Base Depth(m):

Sample Type:

B



BS Test Sieve (mm)	Percentage Passing
125	100
75	100
63	100
37.5	85
20	78
10	73
6.3	69
3.35	65
2	63
1.18	60
0.6	57
0.3	53
0.212	51
0.15	49
0.063	44

Particle Diameter	Percentage Passing
0.02	35
0.006	23
0.002	13

Soil Fraction	Total Percentage
Cobbles	0
Gravel	37
Sand	19
Silt	31
Clay	13

Remarks:

See Summary of Soil Descriptions



Dyke Road Galway

Contract No:
PSL24/3965
Client Ref:
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PARTICLE SIZE DISTRIBUTION TEST

BS1377 : Part 2 : 1990

Wet Sieve, Clause 9.2

Hole Number:

BH05

Top Depth (m):

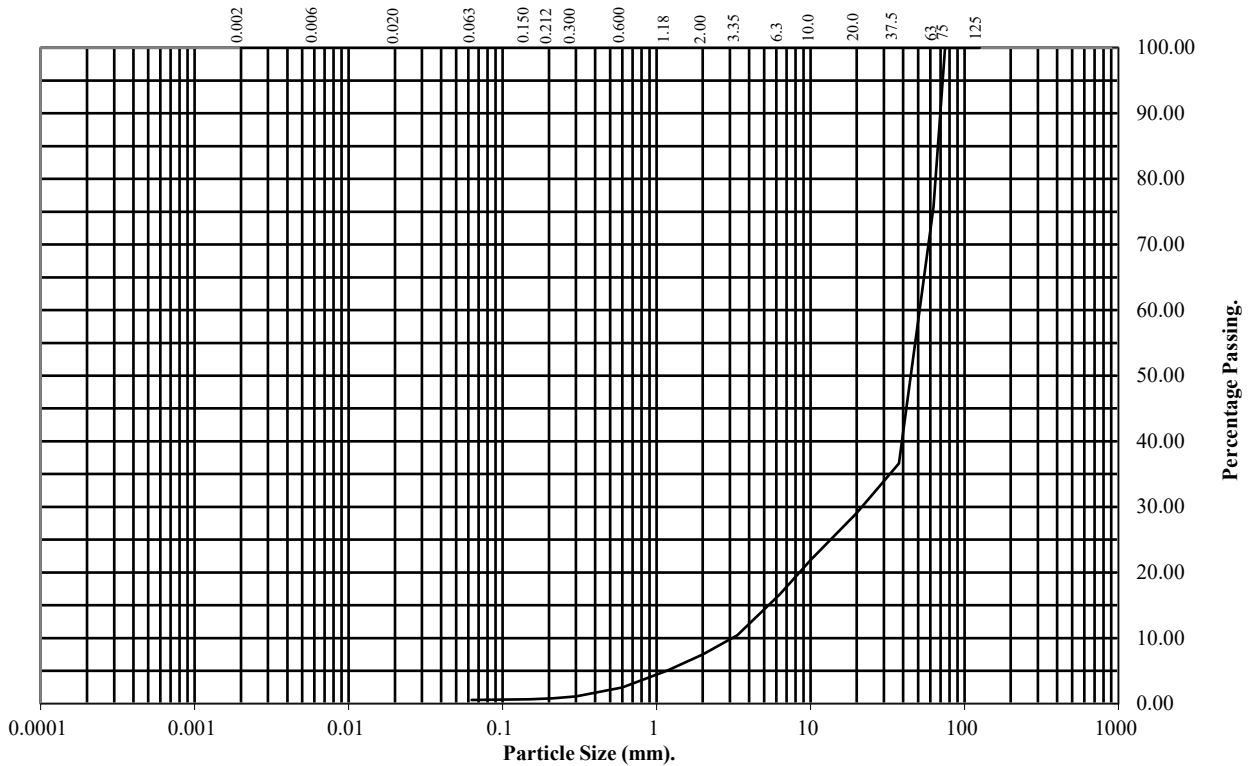
4.00

Sample Number:

Base Depth(m):

Sample Type:

B



BS Test Sieve (mm)	Percentage Passing
125	100
75	100
63	76
37.5	37
20	29
10	22
6.3	17
3.35	10
2	8
1.18	5
0.6	2
0.3	1
0.212	1
0.15	1
0.063	1

Soil Fraction	Total Percentage
Cobbles	24
Gravel	68
Sand	7
Silt/Clay	1

Remarks:

See Summary of Soil Descriptions



Dyke Road Galway

Contract No:

PSL24/3965

Client Ref:

13614-02-24

PARTICLE SIZE DISTRIBUTION TEST

BS1377 : Part 2 : 1990

Wet Sieve & Pipette Analysis, Clause 9.2 & 9.4

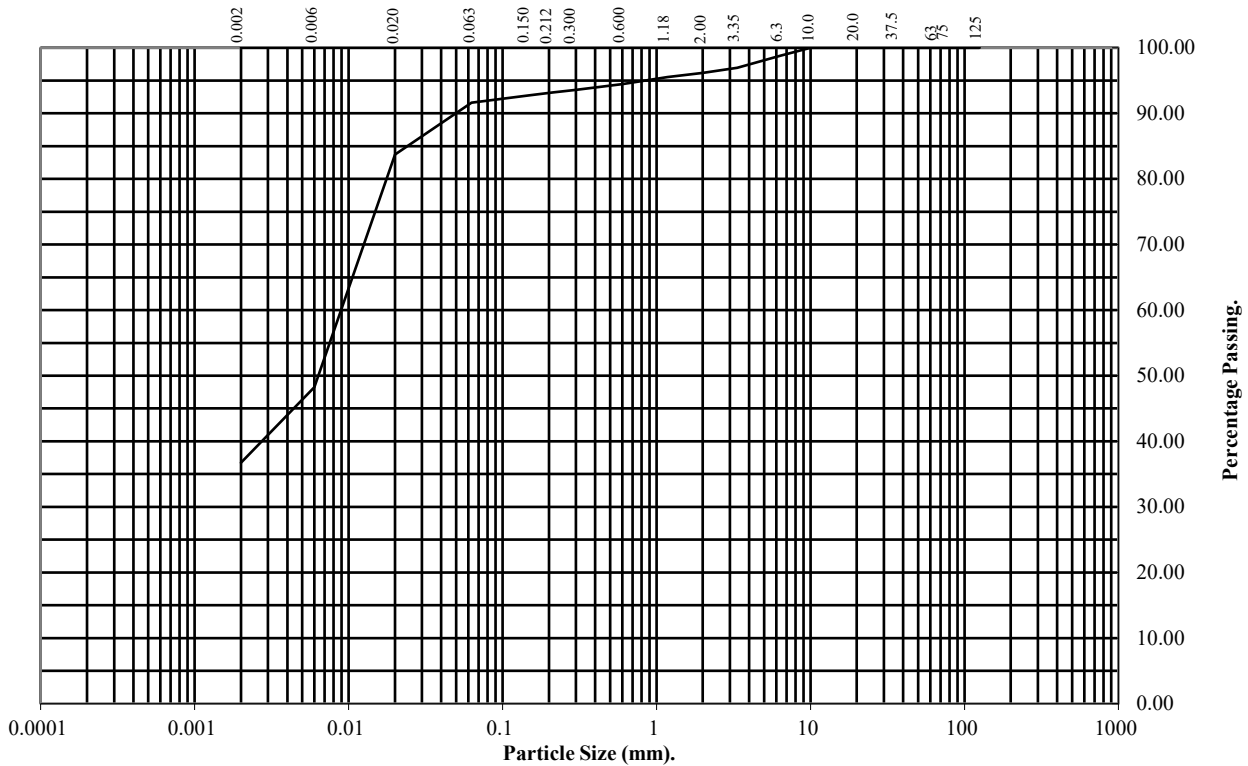
Hole Number: BRC01

Top Depth (m): 5.50

Sample Number:

Base Depth(m):

Sample Type: B



BS Test Sieve (mm)	Percentage Passing
125	100
75	100
63	100
37.5	100
20	100
10	100
6.3	99
3.35	97
2	96
1.18	96
0.6	94
0.3	94
0.212	93
0.15	93
0.063	92

Particle Diameter	Percentage Passing
0.02	84
0.006	48
0.002	37

Soil Fraction	Total Percentage
Cobbles	0
Gravel	4
Sand	4
Silt	55
Clay	37

Remarks:

See Summary of Soil Descriptions



Dyke Road Galway

Contract No:
PSL24/3965
Client Ref:
13614-02-24

PARTICLE SIZE DISTRIBUTION TEST

BS1377 : Part 2 : 1990

Wet Sieve & Pipette Analysis, Clause 9.2 & 9.4

Hole Number:

BRC01

Top Depth (m):

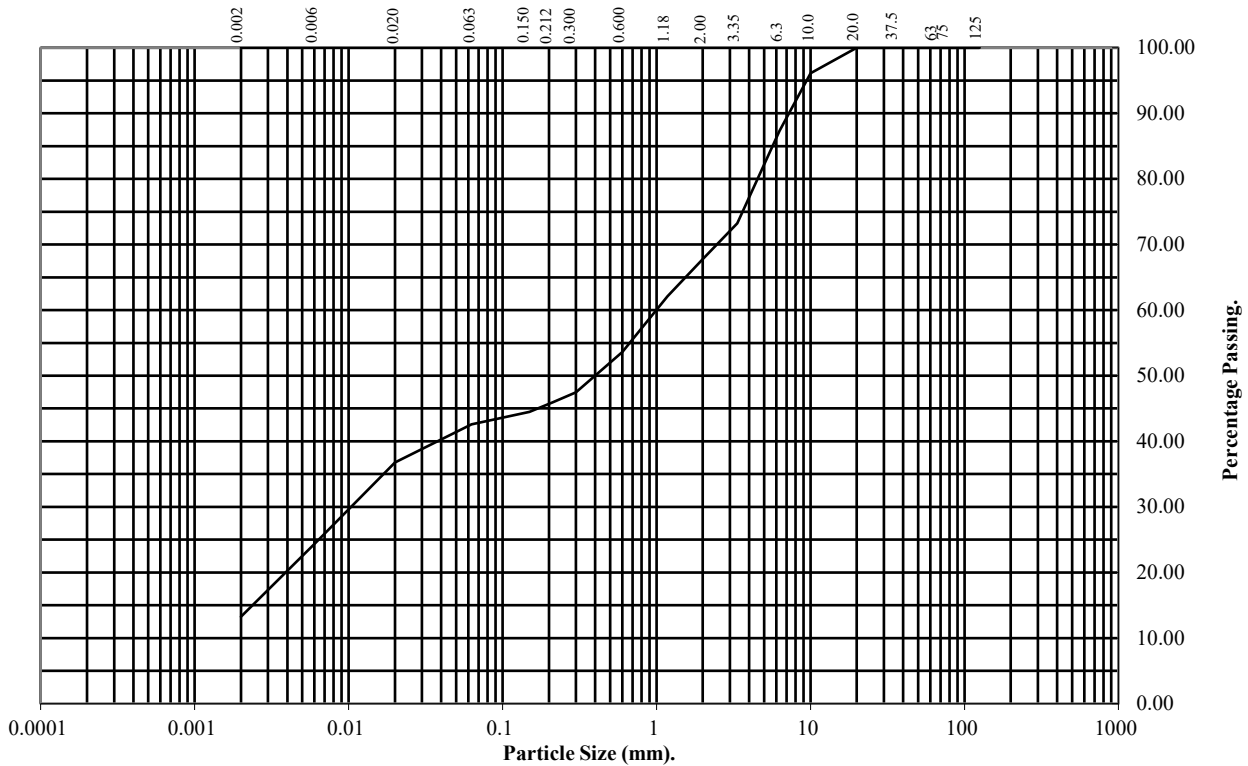
7.50

Sample Number:

Base Depth(m):

Sample Type:

B



BS Test Sieve (mm)	Percentage Passing
125	100
75	100
63	100
37.5	100
20	100
10	96
6.3	87
3.35	73
2	68
1.18	62
0.6	54
0.3	47
0.212	46
0.15	44
0.063	43

Particle Diameter	Percentage Passing
0.02	37
0.006	24
0.002	13

Soil Fraction	Total Percentage
Cobbles	0
Gravel	32
Sand	25
Silt	30
Clay	13

Remarks:

See Summary of Soil Descriptions



Dyke Road Galway

Contract No:

PSL24/3965

Client Ref:

13614-02-24

PARTICLE SIZE DISTRIBUTION TEST

BS1377 : Part 2 : 1990

Wet Sieve & Pipette Analysis, Clause 9.2 & 9.4

Hole Number:

BRC02

Top Depth (m):

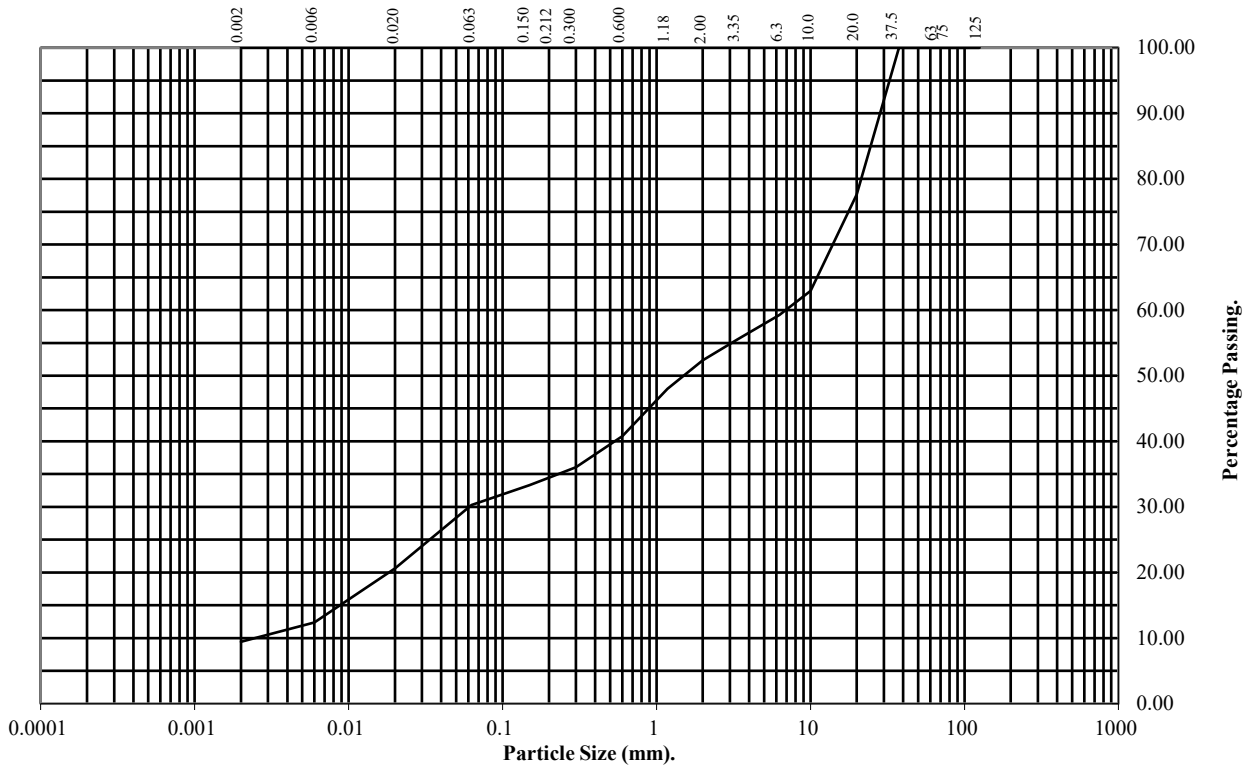
2.00

Sample Number:

Base Depth(m):

Sample Type:

B



BS Test Sieve (mm)	Percentage Passing
125	100
75	100
63	100
37.5	100
20	78
10	63
6.3	59
3.35	56
2	52
1.18	48
0.6	41
0.3	36
0.212	35
0.15	33
0.063	30

Particle Diameter	Percentage Passing
0.02	21
0.006	12
0.002	9

Soil Fraction	Total Percentage
Cobbles	0
Gravel	48
Sand	22
Silt	21
Clay	9

Remarks:

See Summary of Soil Descriptions



Dyke Road Galway

Contract No:

PSL24/3965

Client Ref:

13614-02-24

PARTICLE SIZE DISTRIBUTION TEST

BS1377 : Part 2 : 1990

Wet Sieve & Pipette Analysis, Clause 9.2 & 9.4

Hole Number:

BRC02

Top Depth (m):

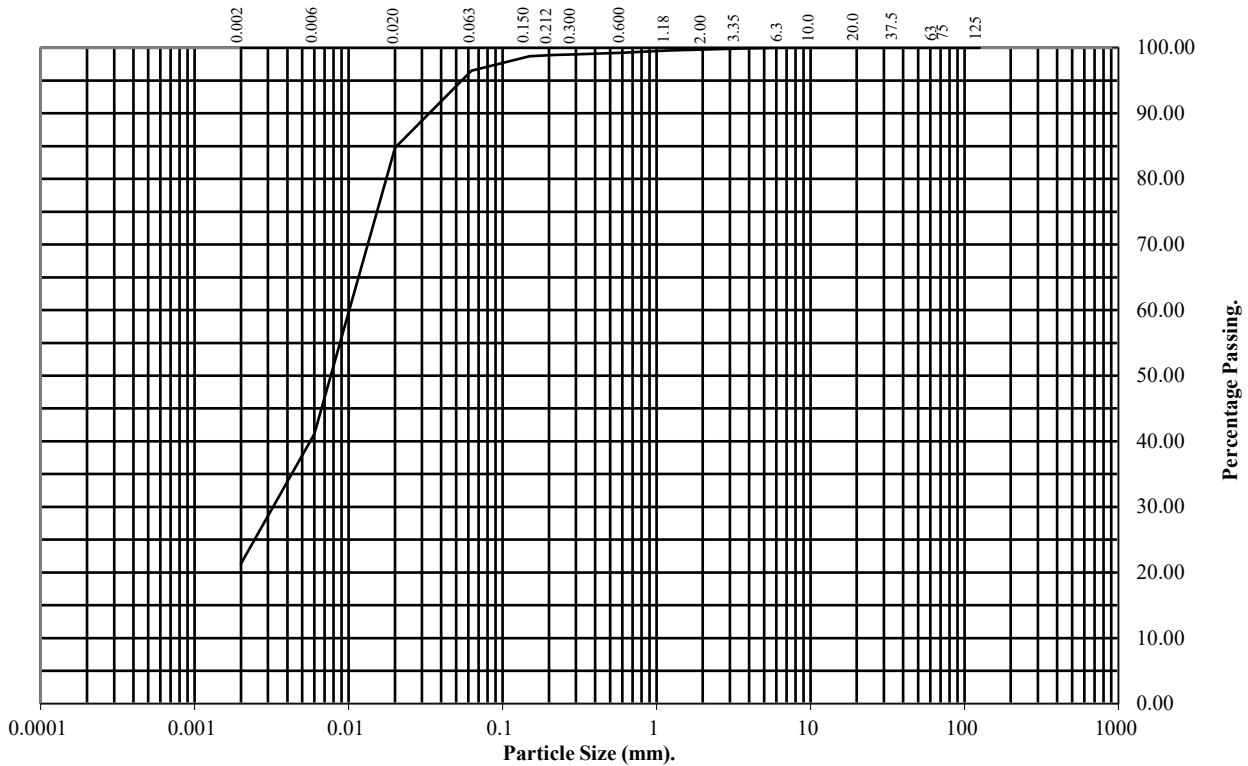
3.50

Sample Number:

Base Depth(m):

Sample Type:

B



BS Test Sieve (mm)	Percentage Passing
125	100
75	100
63	100
37.5	100
20	100
10	100
6.3	100
3.35	100
2	100
1.18	100
0.6	99
0.3	99
0.212	99
0.15	99
0.063	97

Particle Diameter	Percentage Passing
0.02	85
0.006	41
0.002	21

Soil Fraction	Total Percentage
Cobbles	0
Gravel	0
Sand	3
Silt	76
Clay	21

Remarks:

See Summary of Soil Descriptions



Dyke Road Galway

Contract No:
PSL24/3965
Client Ref:
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PARTICLE SIZE DISTRIBUTION TEST

BS1377 : Part 2 : 1990

Wet Sieve & Pipette Analysis, Clause 9.2 & 9.4

Hole Number:

BRC02

Top Depth (m):

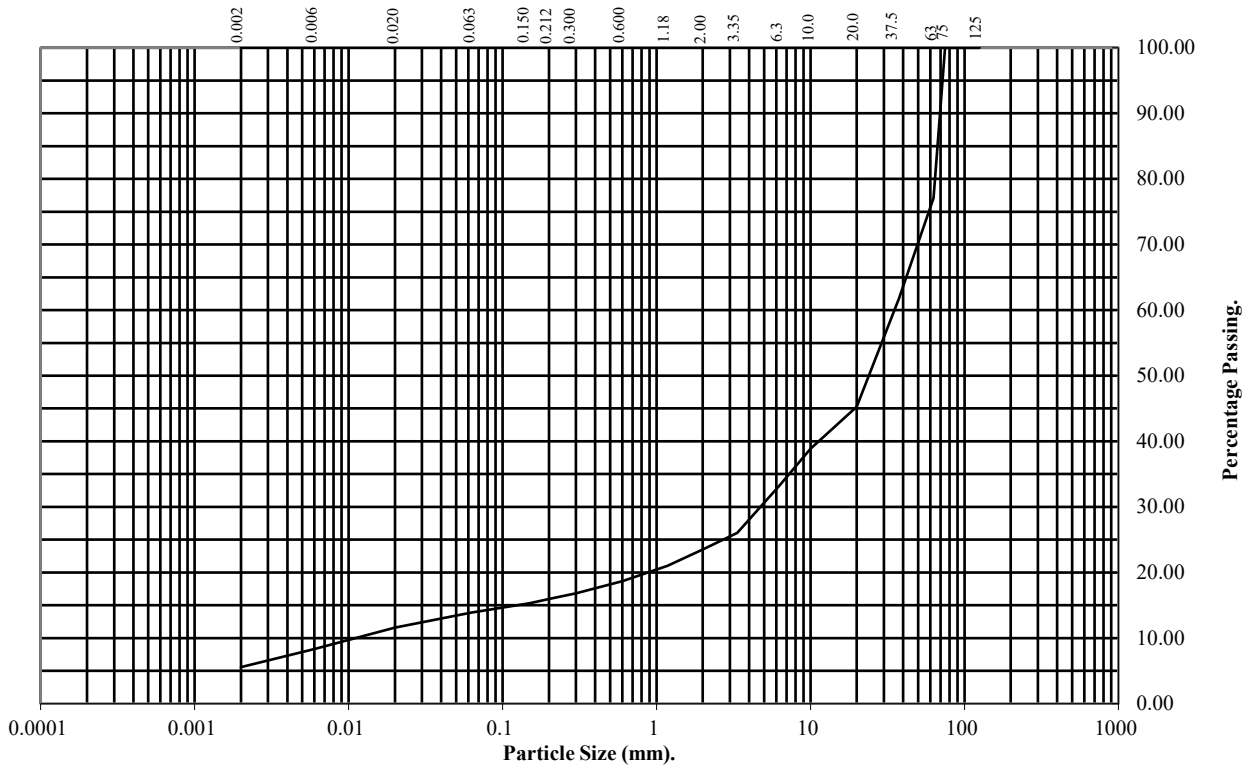
7.00

Sample Number:

Base Depth(m):

Sample Type:

B



BS Test Sieve (mm)	Percentage Passing
125	100
75	100
63	77
37.5	62
20	45
10	39
6.3	33
3.35	26
2	24
1.18	21
0.6	19
0.3	17
0.212	16
0.15	15
0.063	14

Particle Diameter	Percentage Passing
0.02	12
0.006	8
0.002	6

Soil Fraction	Total Percentage
Cobbles	23
Gravel	53
Sand	10
Silt	8
Clay	6

Remarks:

See Summary of Soil Descriptions



Dyke Road Galway

Contract No:
PSL24/3965
Client Ref:
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PARTICLE SIZE DISTRIBUTION TEST

BS1377 : Part 2 : 1990

Wet Sieve & Pipette Analysis, Clause 9.2 & 9.4

Hole Number:

BRC03

Top Depth (m):

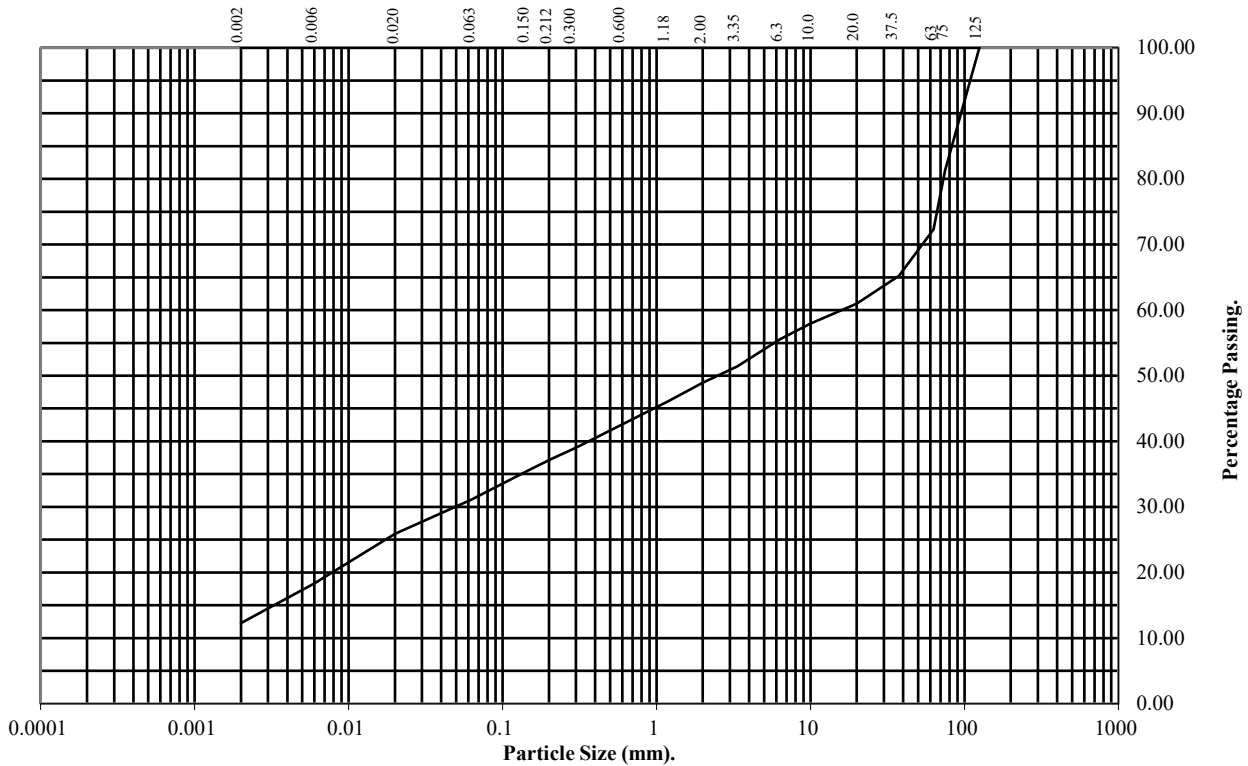
4.00

Sample Number:

Base Depth(m):

Sample Type:

B



BS Test Sieve (mm)	Percentage Passing
125	100
75	81
63	72
37.5	65
20	61
10	58
6.3	56
3.35	51
2	49
1.18	46
0.6	43
0.3	39
0.212	37
0.15	36
0.063	31

Particle Diameter	Percentage Passing
0.02	26
0.006	18
0.002	12

Soil Fraction	Total Percentage
Cobbles	28
Gravel	23
Sand	18
Silt	19
Clay	12

Remarks:

See Summary of Soil Descriptions



Dyke Road Galway

Contract No:

PSL24/3965

Client Ref:

13614-02-24

PARTICLE SIZE DISTRIBUTION TEST

BS1377 : Part 2 : 1990

Wet Sieve & Pipette Analysis, Clause 9.2 & 9.4

Hole Number:

BRC03

Top Depth (m):

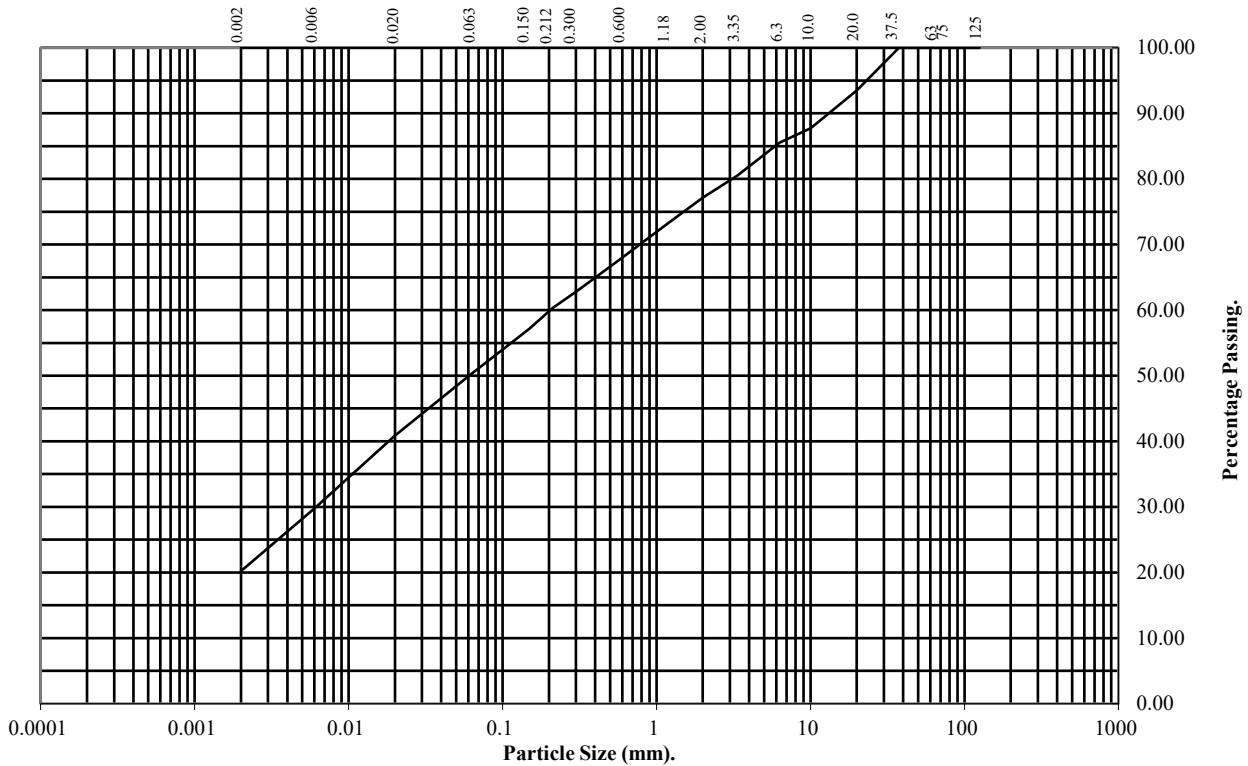
7.50

Sample Number:

Base Depth(m):

Sample Type:

B



BS Test Sieve (mm)	Percentage Passing
125	100
75	100
63	100
37.5	100
20	93
10	88
6.3	86
3.35	80
2	77
1.18	73
0.6	68
0.3	63
0.212	60
0.15	57
0.063	50

Particle Diameter	Percentage Passing
0.02	41
0.006	30
0.002	20

Soil Fraction	Total Percentage
Cobbles	0
Gravel	23
Sand	27
Silt	30
Clay	20

Remarks:

See Summary of Soil Descriptions



Dyke Road Galway

Contract No:

PSL24/3965

Client Ref:

13614-02-24

PARTICLE SIZE DISTRIBUTION TEST

BS1377 : Part 2 : 1990

Wet Sieve & Pipette Analysis, Clause 9.2 & 9.4

Hole Number:

BRC03

Top Depth (m):

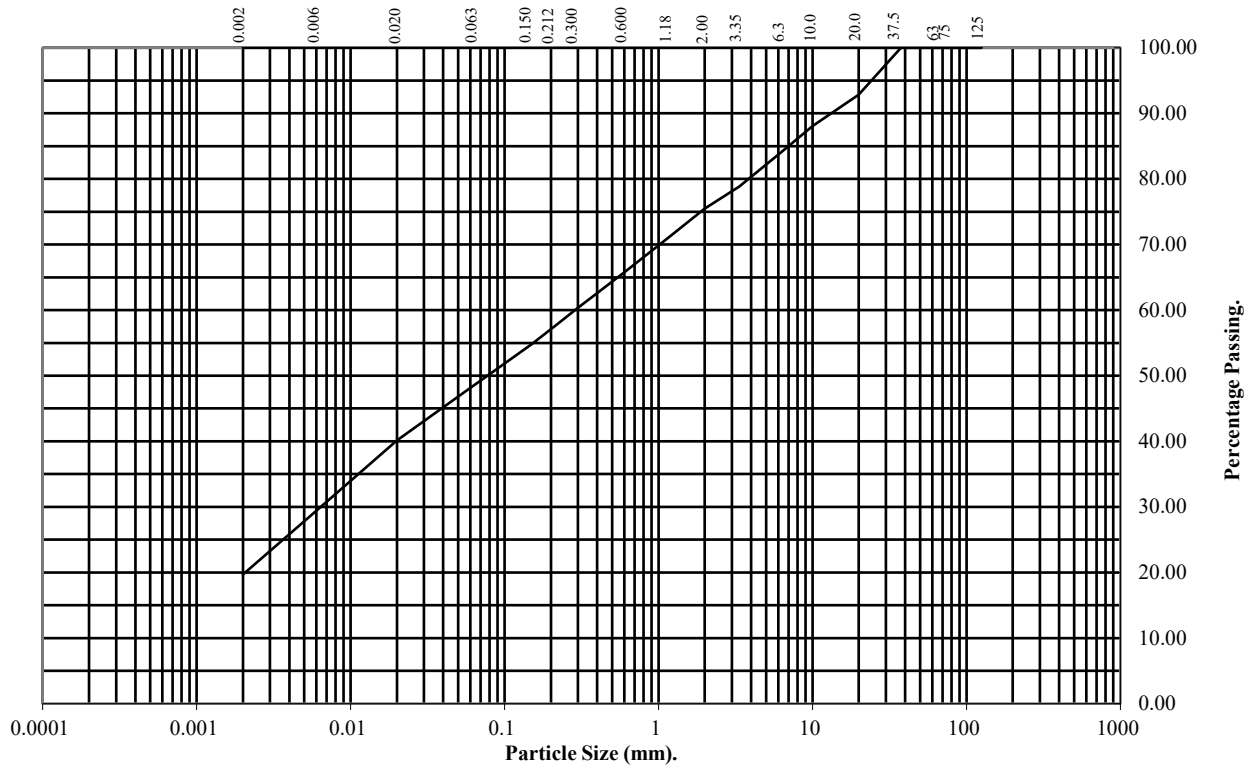
9.50

Sample Number:

Base Depth(m):

Sample Type:

B



BS Test Sieve (mm)	Percentage Passing
125	100
75	100
63	100
37.5	100
20	93
10	88
6.3	84
3.35	79
2	75
1.18	71
0.6	66
0.3	60
0.212	58
0.15	55
0.063	48

Particle Diameter	Percentage Passing
0.02	40
0.006	29
0.002	20

Soil Fraction	Total Percentage
Cobbles	0
Gravel	25
Sand	27
Silt	28
Clay	20

Remarks:

See Summary of Soil Descriptions



Dyke Road Galway

Contract No:

PSL24/3965

Client Ref:

13614-02-24

PARTICLE SIZE DISTRIBUTION TEST

BS1377 : Part 2 : 1990

Wet Sieve, Clause 9.2

Hole Number:

BRC04

Top Depth (m):

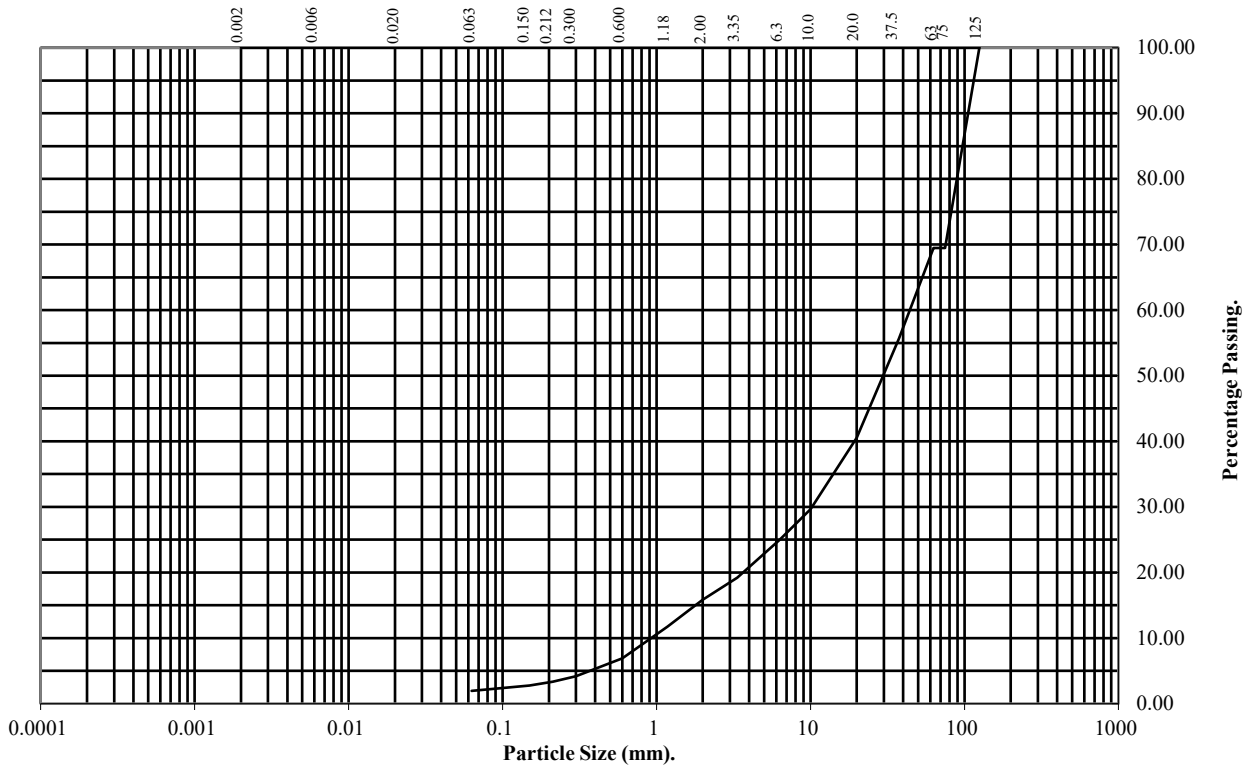
4.00

Sample Number:

Base Depth(m):

Sample Type:

B



BS Test Sieve (mm)	Percentage Passing
125	100
75	69
63	69
37.5	56
20	41
10	30
6.3	25
3.35	19
2	16
1.18	12
0.6	7
0.3	4
0.212	3
0.15	3
0.063	2

Soil Fraction	Total Percentage
Cobbles	31
Gravel	53
Sand	14
Silt/Clay	2

Remarks:

See Summary of Soil Descriptions



Dyke Road Galway

Contract No:

PSL24/3965

Client Ref:

13614-02-24

PARTICLE SIZE DISTRIBUTION TEST

BS1377 : Part 2 : 1990

Wet Sieve & Pipette Analysis, Clause 9.2 & 9.4

Hole Number:

BRC05

Top Depth (m):

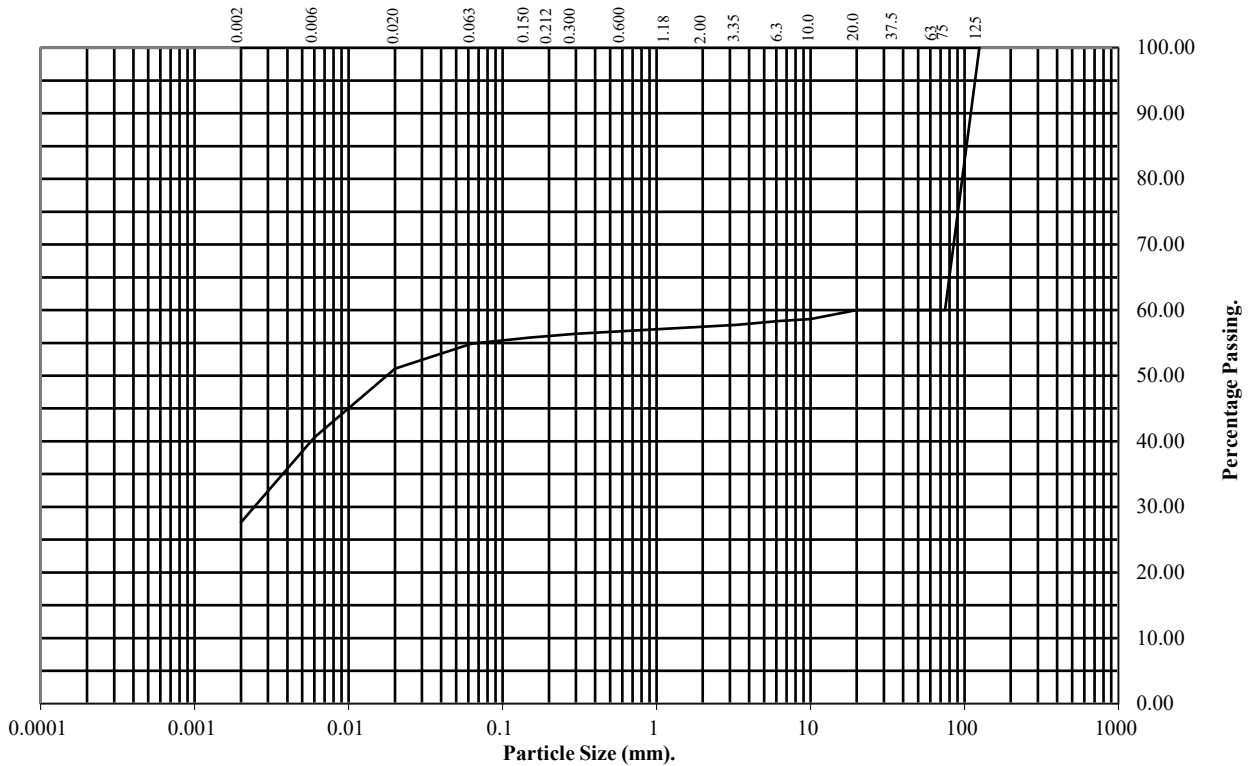
2.60

Sample Number:

Base Depth(m):

Sample Type:

B



BS Test Sieve (mm)	Percentage Passing
125	100
75	60
63	60
37.5	60
20	60
10	59
6.3	58
3.35	58
2	57
1.18	57
0.6	57
0.3	56
0.212	56
0.15	56
0.063	55

Particle Diameter	Percentage Passing
0.02	51
0.006	41
0.002	28

Soil Fraction	Total Percentage
Cobbles	40
Gravel	3
Sand	2
Silt	27
Clay	28

Remarks:

See Summary of Soil Descriptions



Dyke Road Galway

Contract No:

PSL24/3965

Client Ref:

13614-02-24

PARTICLE SIZE DISTRIBUTION TEST

BS1377 : Part 2 : 1990

Wet Sieve & Pipette Analysis, Clause 9.2 & 9.4

Hole Number:

BRC05

Top Depth (m):

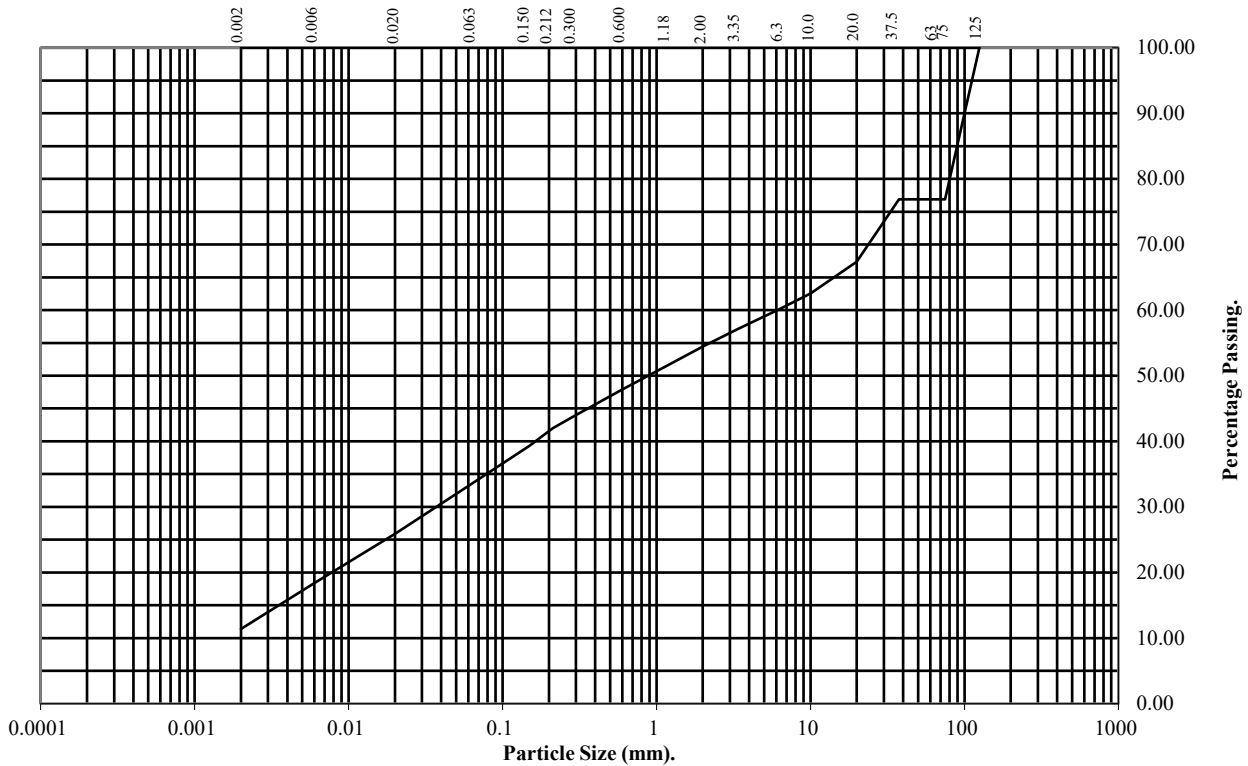
3.50

Sample Number:

Base Depth(m):

Sample Type:

B



BS Test Sieve (mm)	Percentage Passing
125	100
75	77
63	77
37.5	77
20	67
10	63
6.3	60
3.35	57
2	54
1.18	52
0.6	48
0.3	44
0.212	42
0.15	39
0.063	33

Particle Diameter	Percentage Passing
0.02	26
0.006	18
0.002	11

Soil Fraction	Total Percentage
Cobbles	23
Gravel	23
Sand	21
Silt	22
Clay	11

Remarks:

See Summary of Soil Descriptions



Dyke Road Galway

Contract No:

PSL24/3965

Client Ref:

13614-02-24

PARTICLE SIZE DISTRIBUTION TEST

BS1377 : Part 2 : 1990

Wet Sieve, Clause 9.2

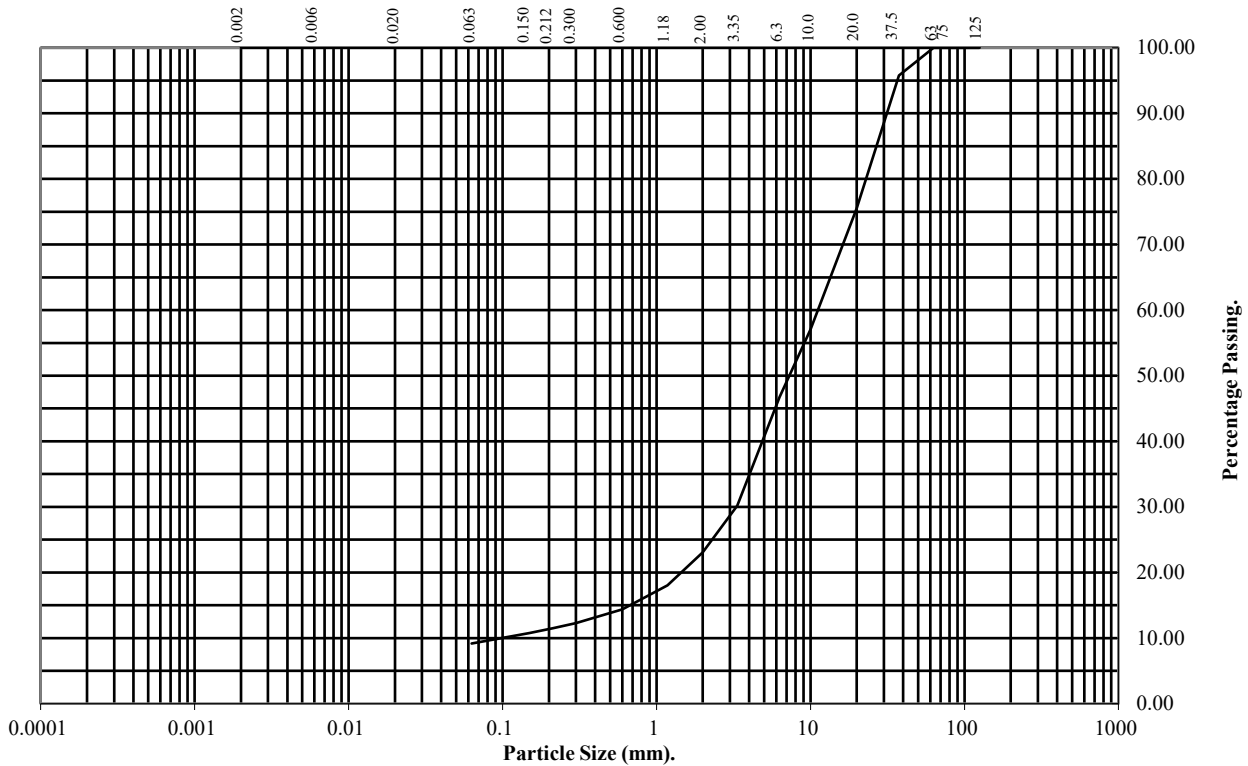
Hole Number: TP01

Top Depth (m): 0.50

Sample Number:

Base Depth(m):

Sample Type: B



BS Test Sieve (mm)	Percentage Passing
125	100
75	100
63	100
37.5	96
20	76
10	57
6.3	47
3.35	30
2	23
1.18	18
0.6	14
0.3	12
0.212	12
0.15	11
0.063	9

Soil Fraction	Total Percentage
Cobbles	0
Gravel	77
Sand	14
Silt/Clay	9

Remarks:

See Summary of Soil Descriptions



Dyke Road Galway

Contract No:
PSL24/3965
Client Ref:
13614-02-24

PARTICLE SIZE DISTRIBUTION TEST

BS1377 : Part 2 : 1990

Wet Sieve & Pipette Analysis, Clause 9.2 & 9.4

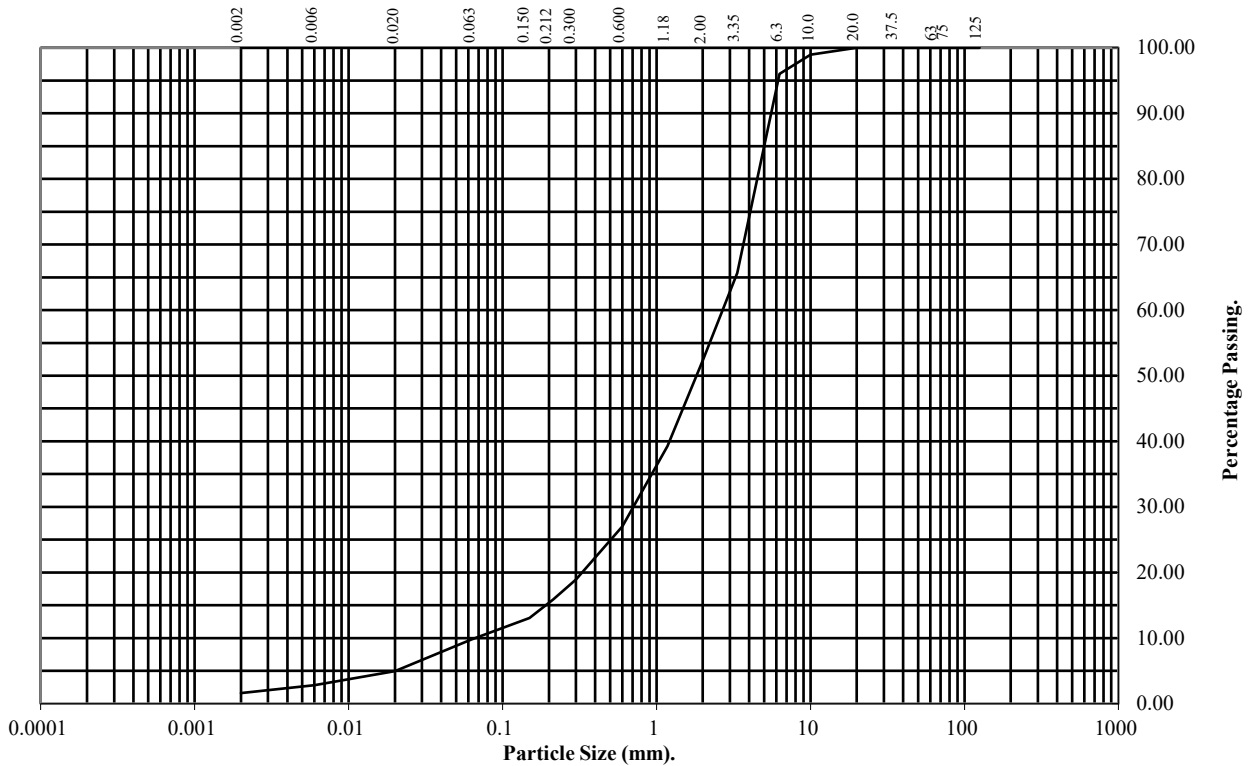
Hole Number: TP01

Top Depth (m): 1.00

Sample Number:

Base Depth(m):

Sample Type: B



BS Test Sieve (mm)	Percentage Passing
125	100
75	100
63	100
37.5	100
20	100
10	99
6.3	96
3.35	66
2	52
1.18	39
0.6	27
0.3	19
0.212	16
0.15	13
0.063	10

Particle Diameter	Percentage Passing
0.02	5
0.006	3
0.002	2

Soil Fraction	Total Percentage
Cobbles	0
Gravel	48
Sand	42
Silt	8
Clay	2

Remarks:

See Summary of Soil Descriptions



Dyke Road Galway

Contract No:
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Client Ref:
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PARTICLE SIZE DISTRIBUTION TEST

BS1377 : Part 2 : 1990

Wet Sieve & Pipette Analysis, Clause 9.2 & 9.4

Hole Number:

TP02

Top Depth (m):

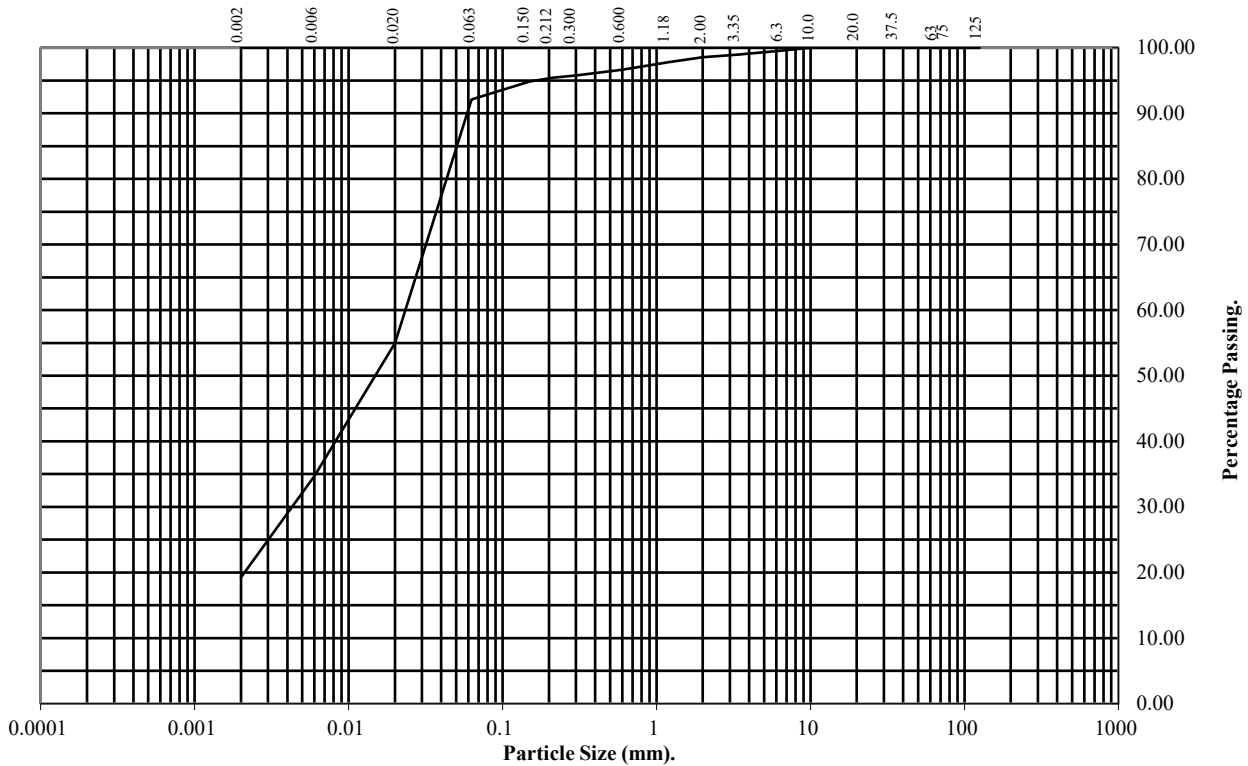
2.00

Sample Number:

Base Depth(m):

Sample Type:

B



BS Test Sieve (mm)	Percentage Passing
125	100
75	100
63	100
37.5	100
20	100
10	100
6.3	100
3.35	99
2	99
1.18	98
0.6	97
0.3	96
0.212	95
0.15	95
0.063	92

Particle Diameter	Percentage Passing
0.02	55
0.006	35
0.002	19

Soil Fraction	Total Percentage
Cobbles	0
Gravel	1
Sand	7
Silt	73
Clay	19

Remarks:

See Summary of Soil Descriptions



Dyke Road Galway

Contract No:

PSL24/3965

Client Ref:

13614-02-24

PARTICLE SIZE DISTRIBUTION TEST

BS1377 : Part 2 : 1990

Wet Sieve & Pipette Analysis, Clause 9.2 & 9.4

Hole Number:

TP03

Top Depth (m):

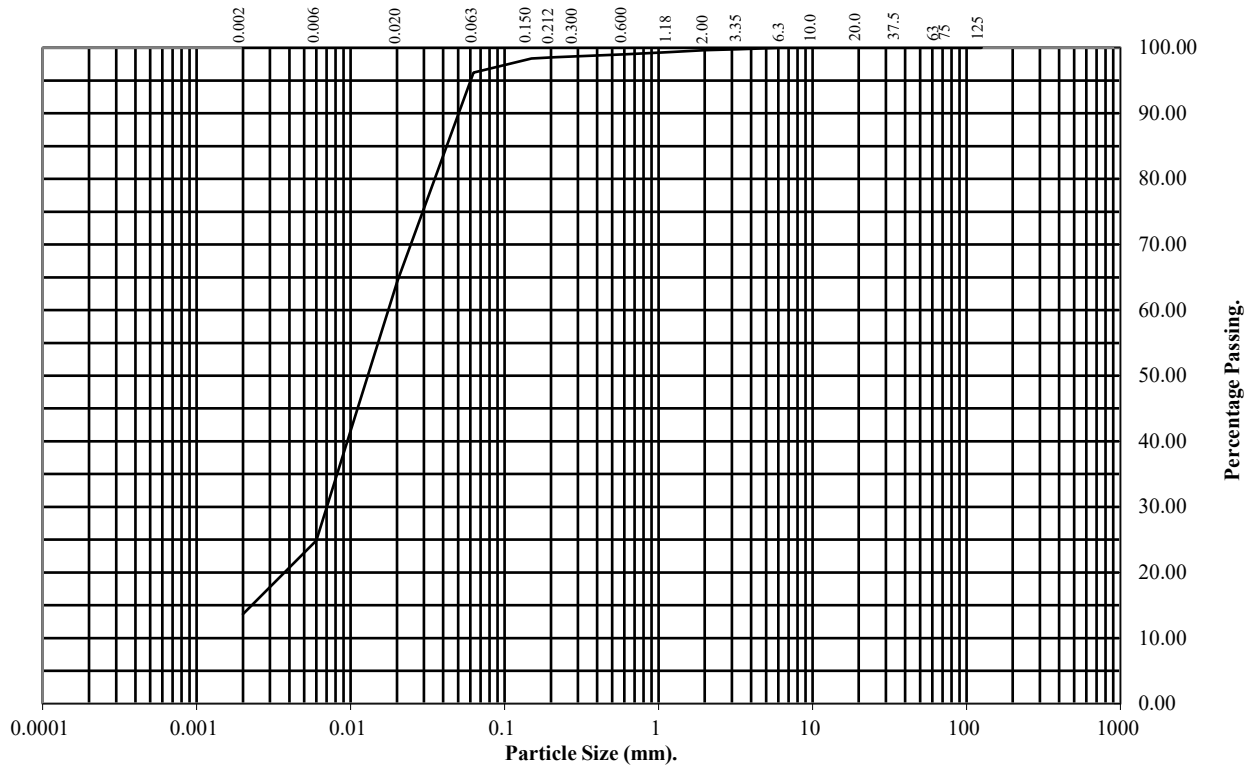
2.00

Sample Number:

Base Depth(m):

Sample Type:

B



BS Test Sieve (mm)	Percentage Passing
125	100
75	100
63	100
37.5	100
20	100
10	100
6.3	100
3.35	100
2	100
1.18	99
0.6	99
0.3	99
0.212	99
0.15	98
0.063	96

Particle Diameter	Percentage Passing
0.02	64
0.006	25
0.002	14

Soil Fraction	Total Percentage
Cobbles	0
Gravel	0
Sand	4
Silt	82
Clay	14

Remarks:

See Summary of Soil Descriptions



Dyke Road Galway

Contract No:
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Client Ref:
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PARTICLE SIZE DISTRIBUTION TEST

BS1377 : Part 2 : 1990

Wet Sieve, Clause 9.2

Hole Number:

TP05

Top Depth (m):

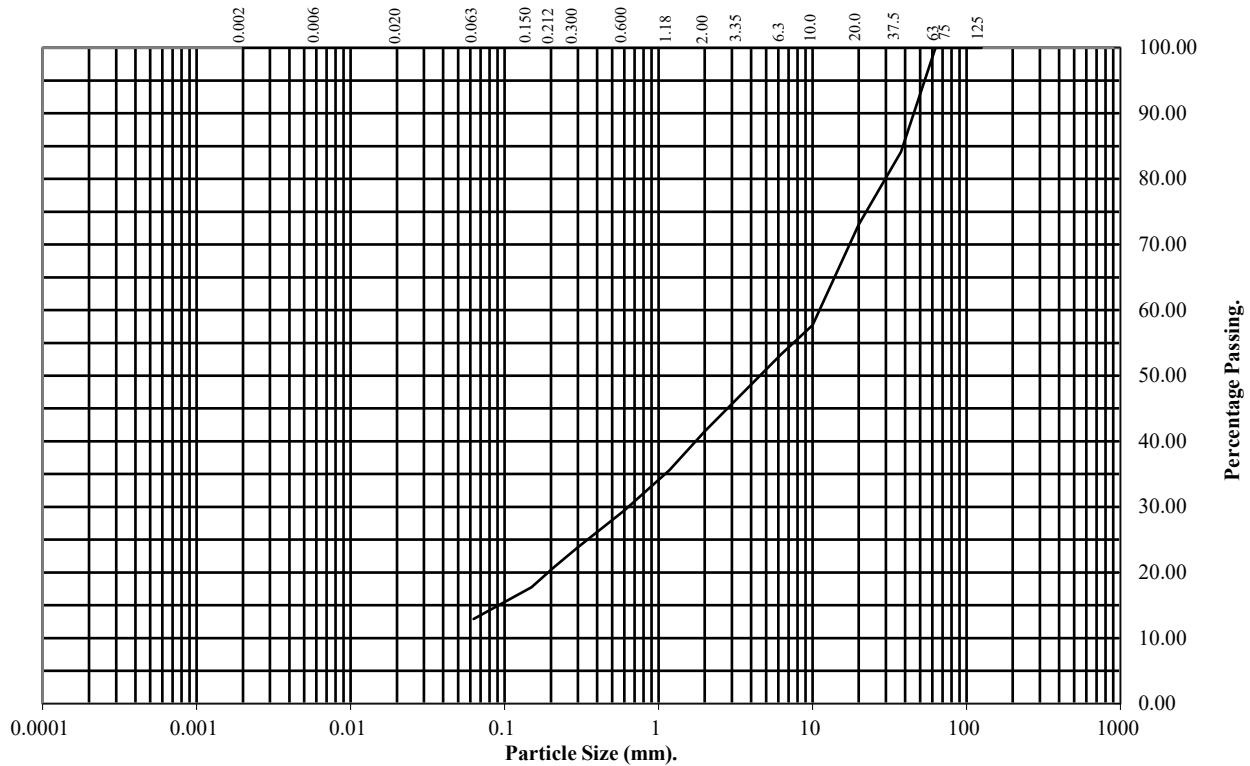
1.00

Sample Number:

Base Depth(m):

Sample Type:

B



BS Test Sieve (mm)	Percentage Passing
125	100
75	100
63	100
37.5	84
20	73
10	58
6.3	53
3.35	47
2	42
1.18	36
0.6	29
0.3	24
0.212	21
0.15	18
0.063	13

Soil Fraction	Total Percentage
Cobbles	0
Gravel	58
Sand	29
Silt/Clay	13

Remarks:

See Summary of Soil Descriptions



Dyke Road Galway

Contract No:

PSL24/3965

Client Ref:

13614-02-24

MOISTURE CONDITION VALUE

BS1377 : Part 4 : 1990 Clause 5.4

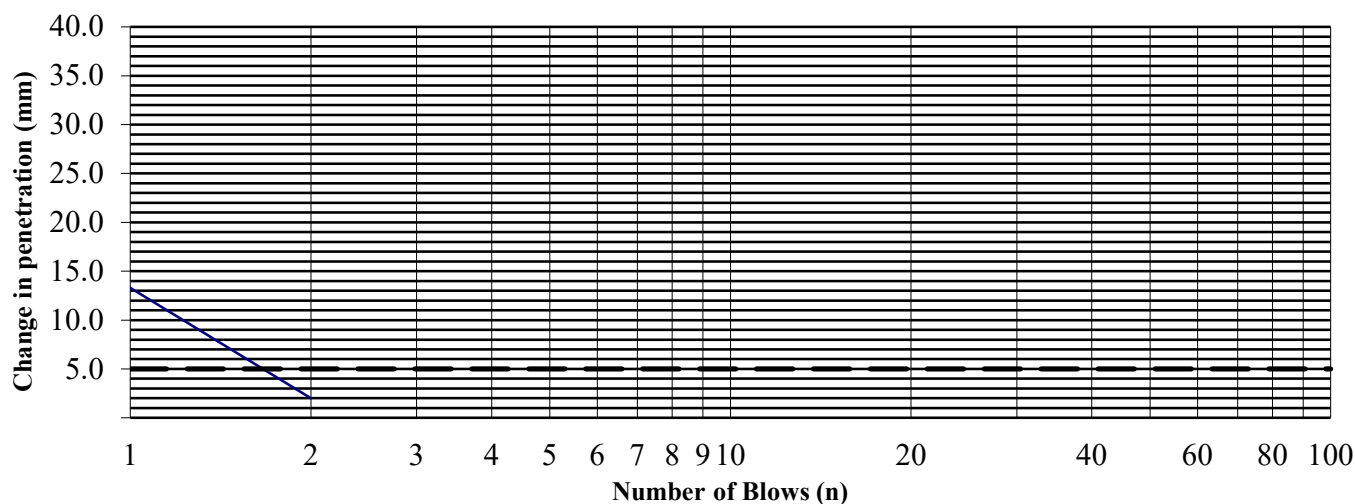
Hole Number: BRC03 Top Depth (m): 8.50

Sample Number: Base Depth (m):

Sample Type: B

Material Retained on the 20mm BS Test Sieve (%):	12
Interpretation based on steepest straight line intercept with 5mm change in penetration.	

MCV Determination



Blows (N)	Penetration (mm)	n to 4n (mm)
1	44.3	13.3
2	32.7	2.0
3	31.1	
4	31.0	
6	30.8	
8	30.7	
12		
16		
24		
32		
48		
64		
96		
128		
192		
256		

Test Results.

Moisture Content (%)	14
MCV	2.2



Dyke Road Galway

Contract No:
PSL24/3965
Client Ref:
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MOISTURE CONDITION VALUE

BS1377 : Part 4 : 1990 Clause 5.4

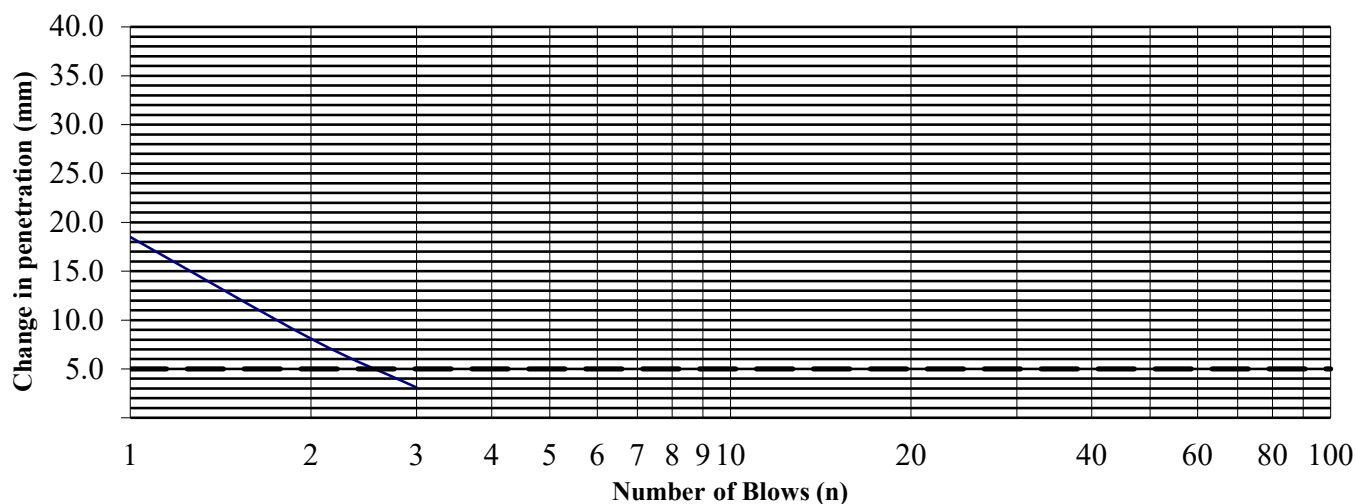
Hole Number: BRC05 Top Depth (m): 4.50

Sample Number: Base Depth (m):

Sample Type: B

Material Retained on the 20mm BS Test Sieve (%):	49
Interpretation based on steepest straight line intercept with 5mm change in penetration.	

MCV Determination



Blows (N)	Penetration (mm)	n to 4n (mm)
1	50.9	18.5
2	39.9	8.1
3	34.6	3.1
4	32.4	
6	32.0	
8	31.8	
12	31.5	
16		
24		
32		
48		
64		
96		
128		
192		
256		

Test Results.

Moisture Content (%)	10
MCV	4.1



Dyke Road Galway

Contract No:
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Client Ref:
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MOISTURE CONDITION VALUE

BS1377 : Part 4 : 1990 Clause 5.4

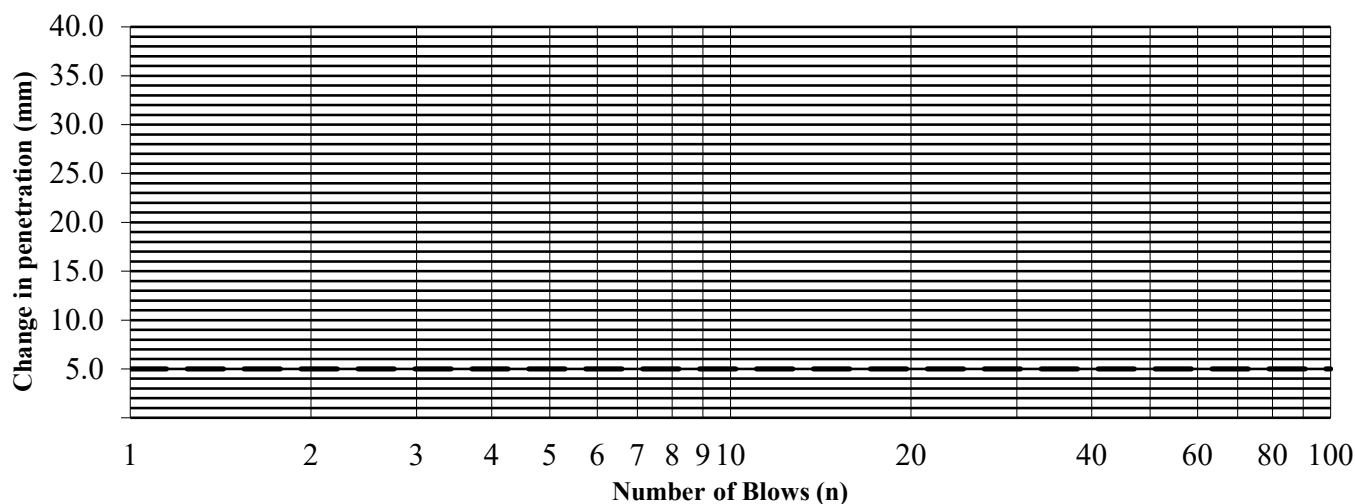
Hole Number: TP01 Top Depth (m): 1.00

Sample Number: Base Depth (m):

Sample Type: B

Material Retained on the 20mm BS Test Sieve (%):	0
Interpretation based on steepest straight line intercept with 5mm change in penetration.	

MCV Determination



Blows (N)	Penetration (mm)	n to 4n (mm)
1	136.8	2.6
2	135.1	
3	134.6	
4	134.2	
6		
8		
12		
16		
24		
32		
48		
64		
96		
128		
192		
256		

Test Results.

Moisture Content (%)	614
MCV	<1



Dyke Road Galway

Contract No:
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MOISTURE CONDITION VALUE

BS1377 : Part 4 : 1990 Clause 5.4

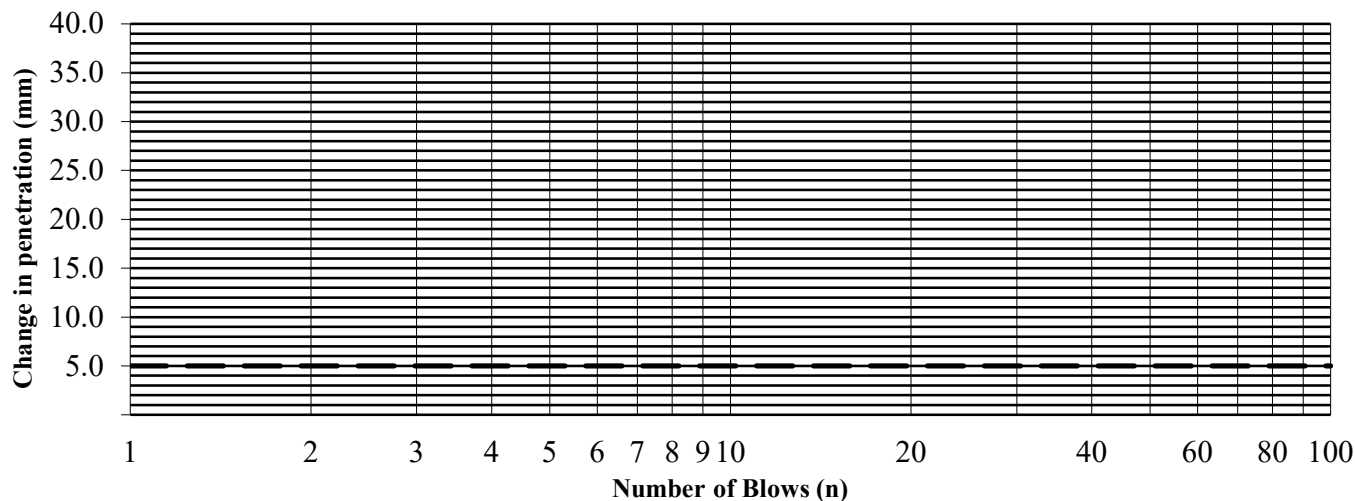
Hole Number: TP02 Top Depth (m): 2.00

Sample Number: Base Depth (m):

Sample Type: B

Material Retained on the 20mm BS Test Sieve (%):	0
Interpretation based on steepest straight line intercept with 5mm change in penetration.	

MCV Determination



Blows (N)	Penetration (mm)	n to 4n (mm)
1	97.3	3.2
2	95.0	
3	94.4	
4	94.1	
6		
8		
12		
16		
24		
32		
48		
64		
96		
128		
192		
256		

Test Results.

Moisture Content (%)	131
MCV	<1



Dyke Road Galway

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MOISTURE CONDITION VALUE

BS1377 : Part 4 : 1990 Clause 5.4

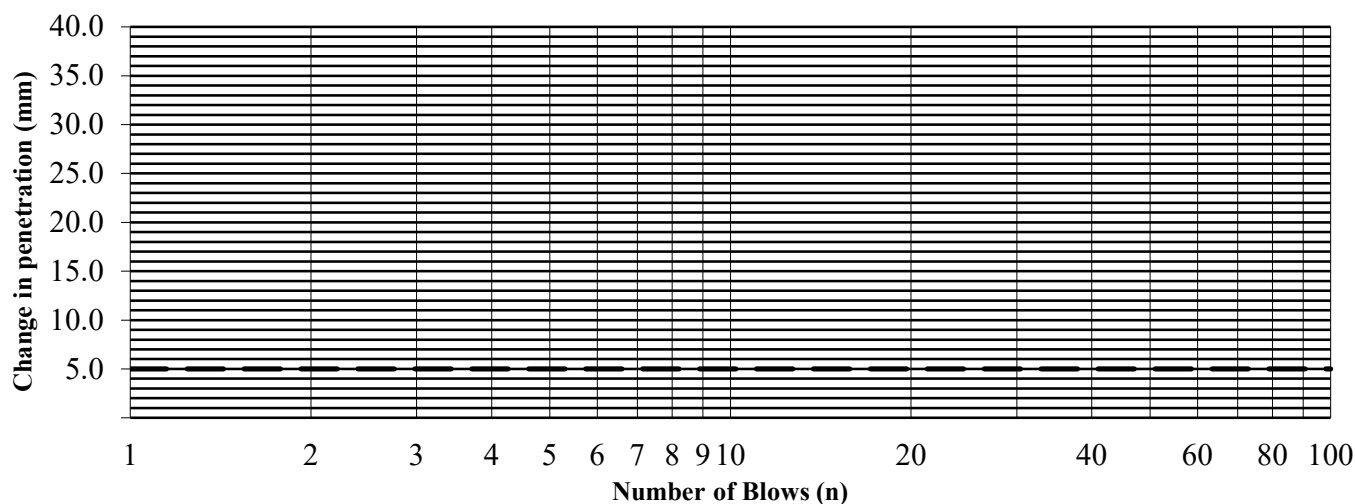
Hole Number: TP03 Top Depth (m): 2.00

Sample Number: Base Depth (m):

Sample Type: B

Material Retained on the 20mm BS Test Sieve (%):	0
Interpretation based on steepest straight line intercept with 5mm change in penetration.	

MCV Determination



Blows (N)	Penetration (mm)	n to 4n (mm)
1	94.1	2.2
2	92.9	
3	92.3	
4	91.9	
6		
8		
12		
16		
24		
32		
48		
64		
96		
128		
192		
256		

Test Results.

Moisture Content (%)	145
MCV	<1



Dyke Road Galway

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MOISTURE CONDITION VALUE

BS1377 : Part 4 : 1990 Clause 5.4

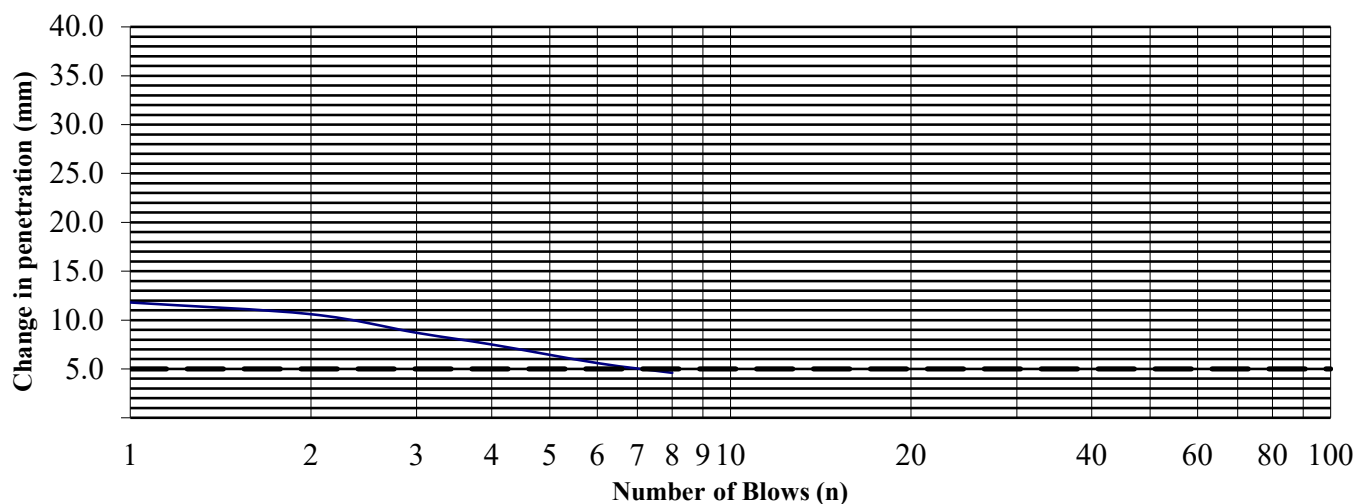
Hole Number: TP05 Top Depth (m): 1.00

Sample Number: Base Depth (m):

Sample Type: B

Material Retained on the 20mm BS Test Sieve (%):	27
Interpretation based on steepest straight line intercept with 5mm change in penetration.	

MCV Determination



Blows (N)	Penetration (mm)	n to 4n (mm)
1	50.0	11.8
2	44.8	10.6
3	40.5	8.7
4	38.2	7.5
6	35.6	5.6
8	34.2	4.6
12	31.8	
16	30.7	
24	30.0	
32	29.6	
48		
64		
96		
128		
192		
256		

Test Results.

Moisture Content (%)	8.1
MCV	8.5



Dyke Road Galway


Contract No:
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Client Ref:
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SUMMARY OF LABORATORY HAND VANES

(BS1377 : PART 7 : 1990)

[illegible]

* This test is out of our UKAS scope

	<p align="center">Dyke Road Galway</p>	Contract No:
		PSL24/3965
		Client Ref:
		13614-02-24

UNDRAINED SHEAR STRENGTH IN TRIAXIAL COMPRESSION

WITHOUT MEASUREMENT OF PORE PRESSURE

BS1377 : Part7 : 1990: Clause 8

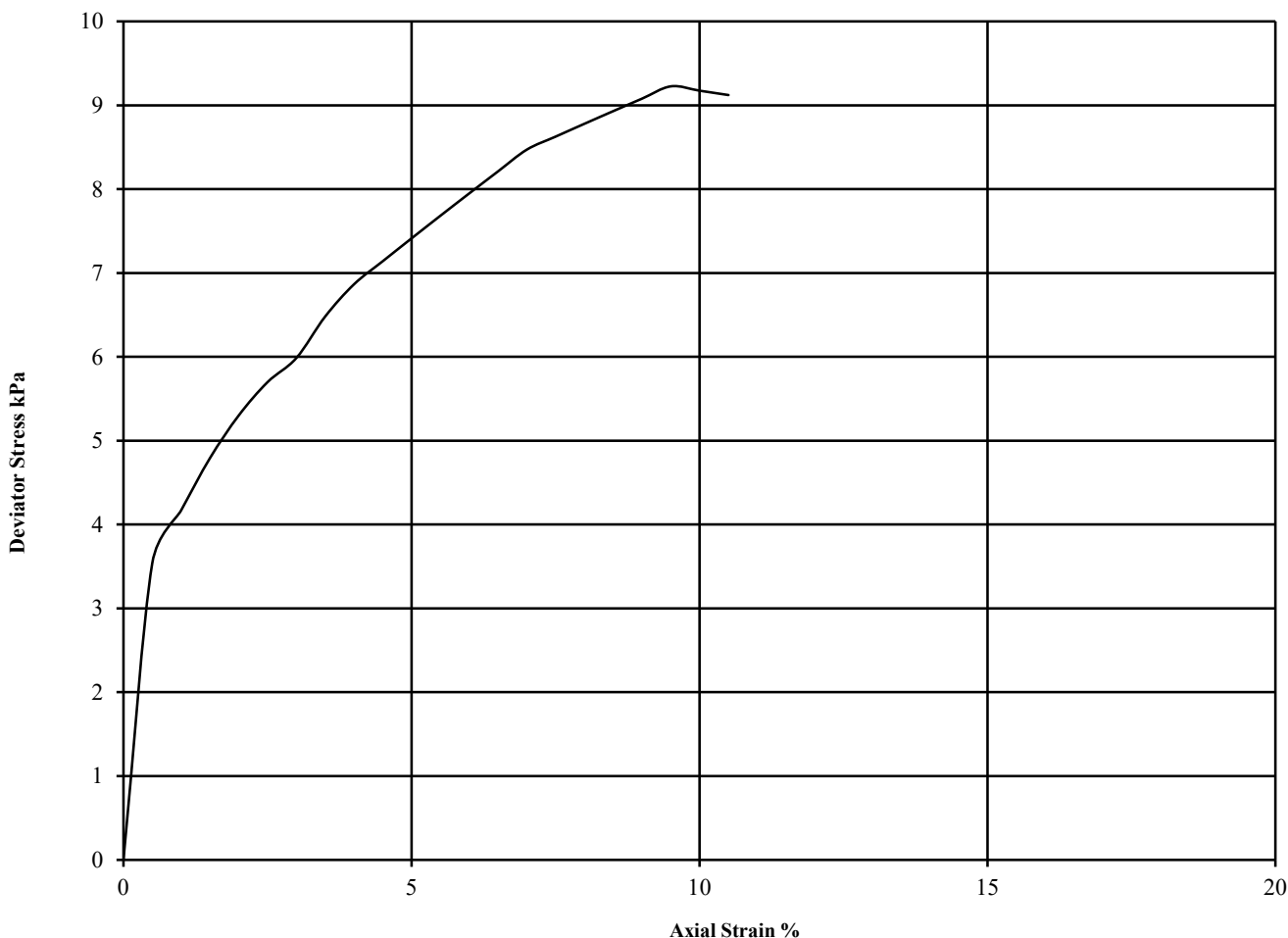
Hole Number: BH01

Top Depth (m): 3.00

Sample Number:

Base Depth (m): 3.45

Sample Type UT



Diameter (mm):		102	Height (mm):		204	Test:	UU Single Stage		Remarks:
Specimen	Moisture Content (%)	Bulk Density (Mg/m ³)	Dry Density (Mg/m ³)	Cell Pressure (kPa)	Corr. Max. Deviator Stress (kPa)	Shear Strength Cu (kPa)	Failure Strain (%)	Mode of Failure	Undisturbed Sample Sample taken from top of tube Rate of strain = 2 %/min Latex Membrane used 0.2 mm thick, Correction applied 0.35 See summary of soil descriptions
				θ_3	$(\theta_1 - \theta_3)_f$	$\frac{1}{2}(\theta_1 - \theta_3)_f$			
1	79	1.33	0.74	60	9	5	9.5	Intermediate	



Dyke Road Galway

Contract No:

PSL24/3965

Client Ref:

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UNDRAINED SHEAR STRENGTH IN TRIAXIAL COMPRESSION

WITHOUT MEASUREMENT OF PORE PRESSURE

BS1377 : Part7 : 1990: Clause 8

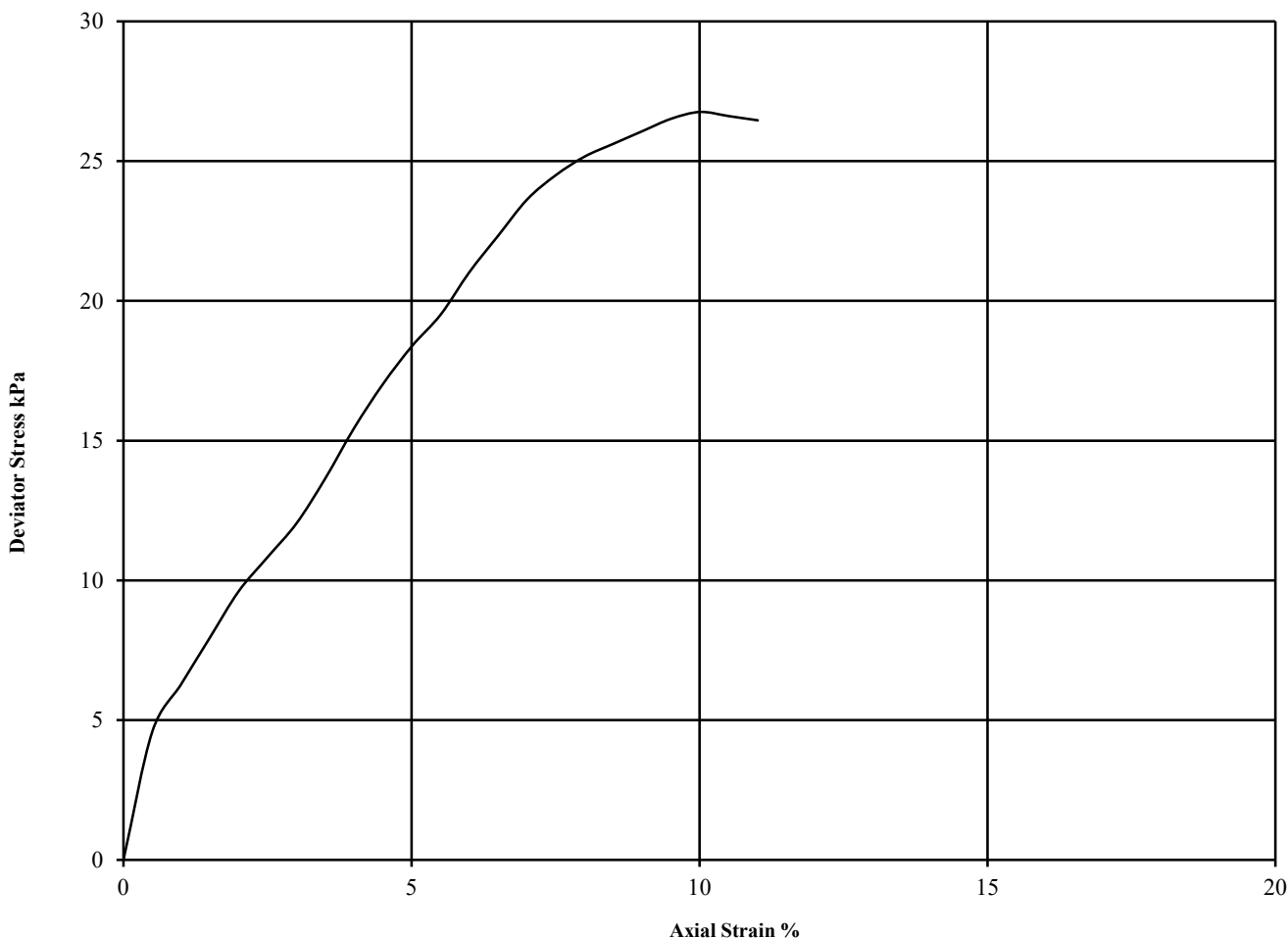
Hole Number: BH02

Top Depth (m): 3.00

Sample Number:

Base Depth (m): 3.45

Sample Type UT



Diameter (mm):		102	Height (mm):		204	Test:	UU Single Stage		Remarks:
Specimen	Moisture Content (%)	Bulk Density (Mg/m ³)	Dry Density (Mg/m ³)	Cell Pressure (kPa)	Corr. Max. Deviator Stress (kPa)	Shear Strength Cu (kPa)	Failure Strain (%)	Mode of Failure	Undisturbed Sample Sample taken from top of tube Rate of strain = 2 %/min Latex Membrane used 0.2 mm thick, Correction applied 0.35 See summary of soil descriptions
				θ_3	$(\theta_1 - \theta_3)_f$	$\frac{1}{2}(\theta_1 - \theta_3)_f$			
1	96	1.33	0.68	60	27	13	10.0	Intermediate	



Dyke Road Galway

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UNDRAINED SHEAR STRENGTH IN TRIAXIAL COMPRESSION

WITHOUT MEASUREMENT OF PORE PRESSURE

BS1377 : Part7 : 1990: Clause 8

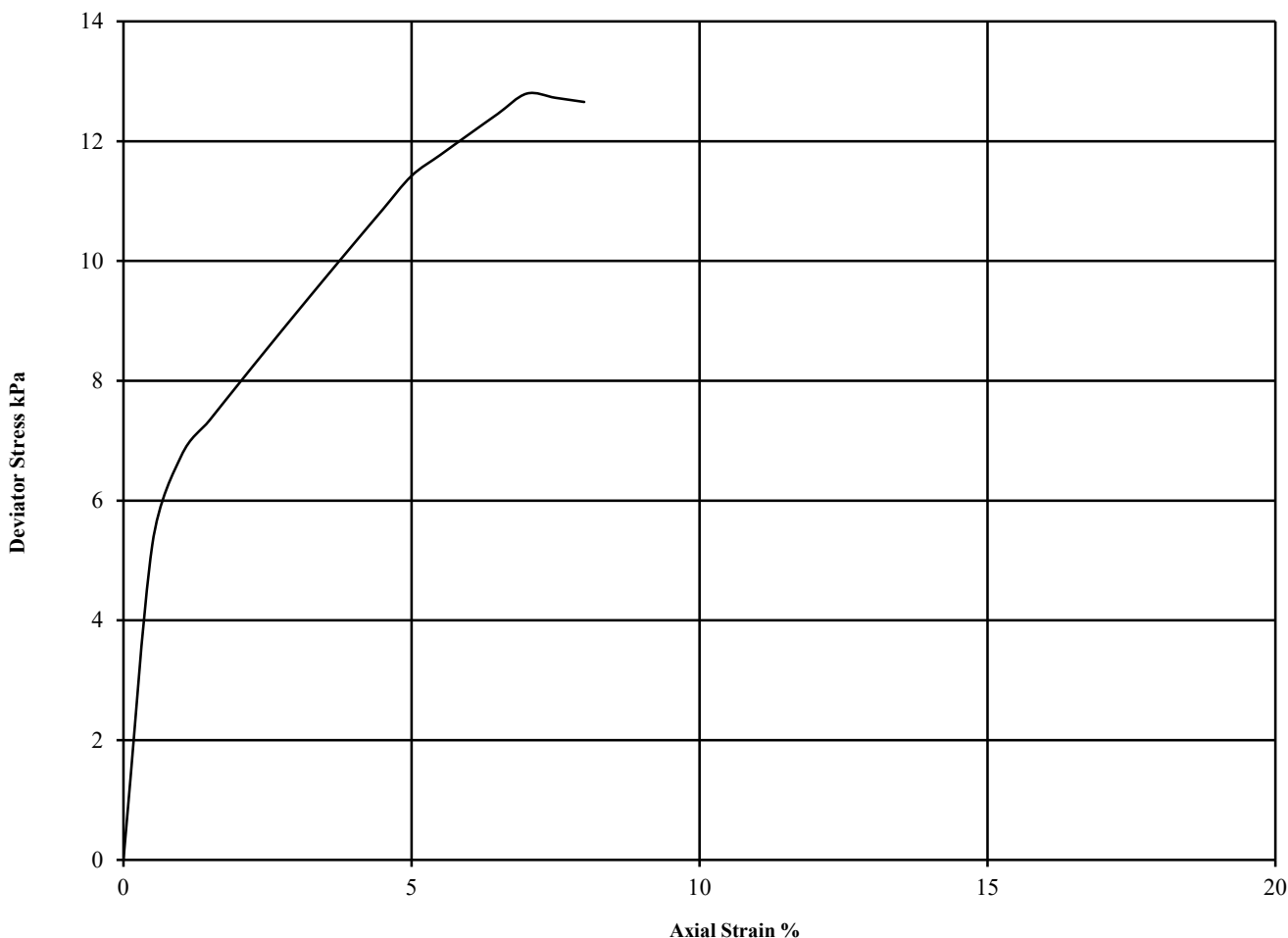
Hole Number: BRC01

Top Depth (m): 5.30

Sample Number:

Base Depth (m): 5.75

Sample Type UT



Diameter (mm):		102	Height (mm):		204	Test:	UU Single Stage		Remarks:
Specimen	Moisture Content (%)	Bulk Density (Mg/m3)	Dry Density (Mg/m3)	Cell Pressure (kPa)	Corr. Max. Deviator Stress (kPa)	Shear Strength Cu (kPa)	Failure Strain (%)	Mode of Failure	Undisturbed Sample Sample taken from top of tube Rate of strain = 2 %/min Latex Membrane used 0.2 mm thick, Correction applied 0.36 See summary of soil descriptions
				θ_3	$(\theta_1 - \theta_3)_f$	$\frac{1}{2}(\theta_1 - \theta_3)_f$			
1	123	1.27	0.57	110	13	6	7.0	Intermediate	



Dyke Road Galway

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UNDRAINED SHEAR STRENGTH IN TRIAXIAL COMPRESSION

WITHOUT MEASUREMENT OF PORE PRESSURE

BS1377 : Part7 : 1990: Clause 8

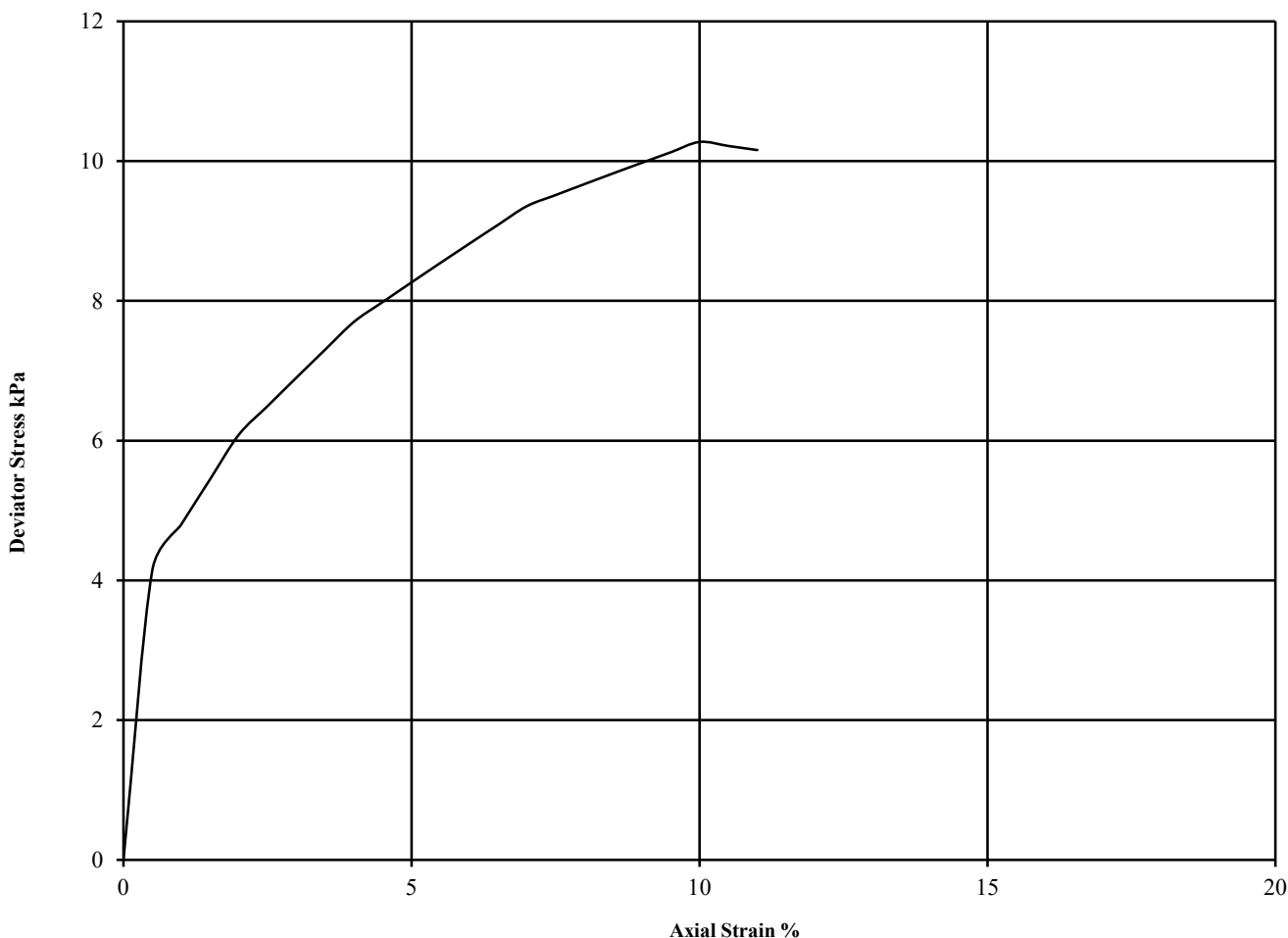
Hole Number: BRC01

Top Depth (m): 7.50

Sample Number:

Base Depth (m): 7.95

Sample Type UT



Diameter (mm):		100	Height (mm):		200	Test:	UU Single Stage		Remarks:
Specimen	Moisture Content (%)	Bulk Density (Mg/m ³)	Dry Density (Mg/m ³)	Cell Pressure (kPa)	Corr. Max. Deviator Stress (kPa)	Shear Strength Cu (kPa)	Failure Strain (%)	Mode of Failure	Undisturbed Sample Sample taken from top of tube Rate of strain = 2 %/min Latex Membrane used 0.2 mm thick, Correction applied 0.36 See summary of soil descriptions
				θ_3	$(\theta_1 - \theta_3)_f$	$\frac{1}{2}(\theta_1 - \theta_3)_f$			
1	160	1.26	0.48	150	10	5	10.0	Intermediate	



Dyke Road Galway

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Client Ref:

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UNDRAINED SHEAR STRENGTH IN TRIAXIAL COMPRESSION

WITHOUT MEASUREMENT OF PORE PRESSURE

BS1377 : Part7 : 1990: Clause 8

Hole Number:

BRC02

Top Depth (m):

3.00

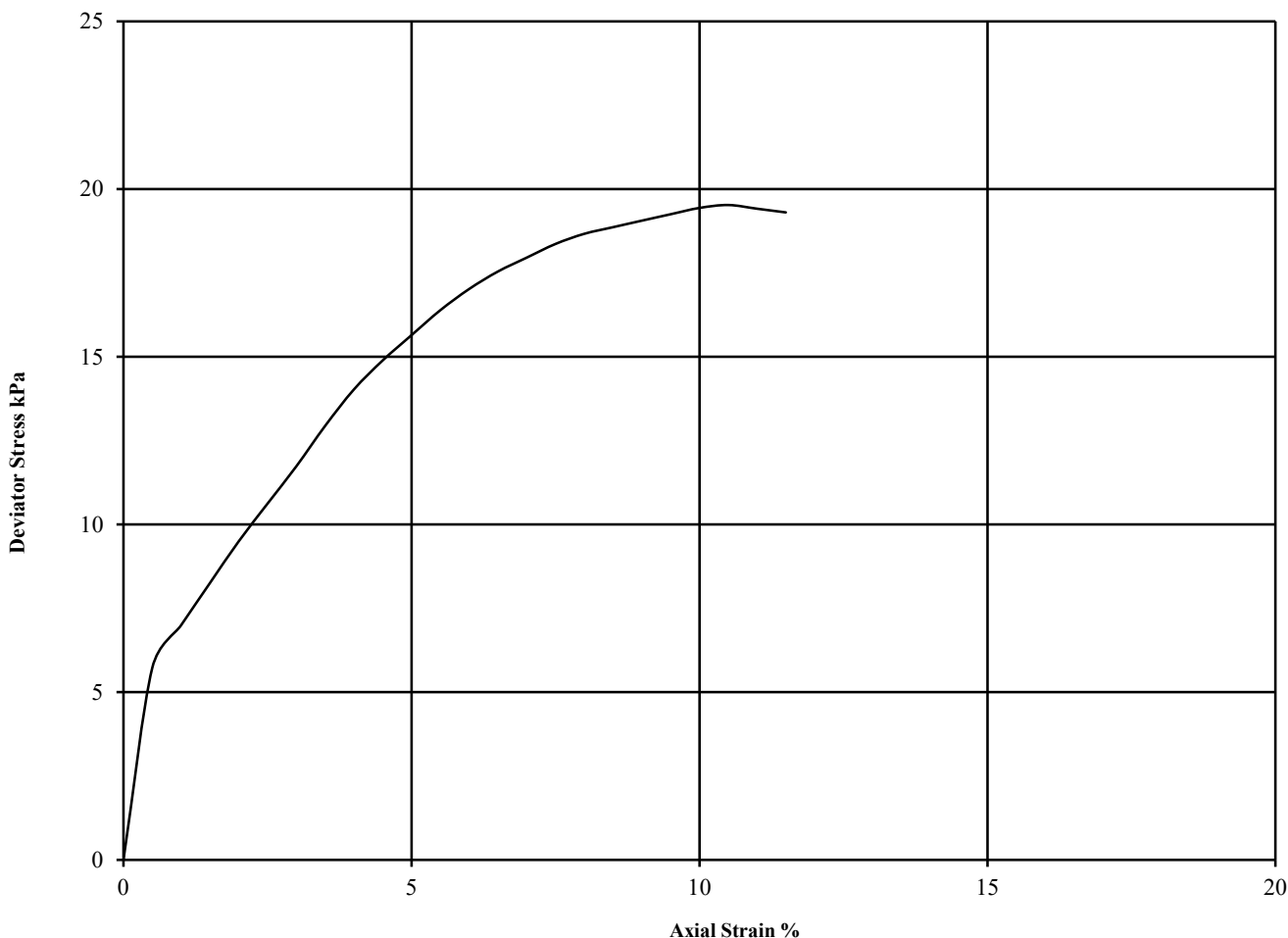
Sample Number:

Base Depth (m):

3.45

Sample Type

UT



Diameter (mm):		102	Height (mm):		204	Test:	UU Single Stage		Remarks:
Specimen	Moisture Content (%)	Bulk Density (Mg/m ³)	Dry Density (Mg/m ³)	Cell Pressure (kPa)	Corr. Max. Deviator Stress (kPa)	Shear Strength Cu (kPa)	Failure Strain (%)	Mode of Failure	Undisturbed Sample Sample taken from top of tube Rate of strain = 2 %/min Latex Membrane used 0.2 mm thick, Correction applied 0.35 See summary of soil descriptions
				θ_3	$(\theta_1 - \theta_3)_f$	$\frac{1}{2}(\theta_1 - \theta_3)_f$			
1	86	1.30	0.70	60	20	10	10.5	Intermediate	



Dyke Road Galway

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UNDRAINED SHEAR STRENGTH IN TRIAXIAL COMPRESSION

WITHOUT MEASUREMENT OF PORE PRESSURE

BS1377 : Part7 : 1990: Clause 8

Hole Number:

BRC02

Top Depth (m):

5.00

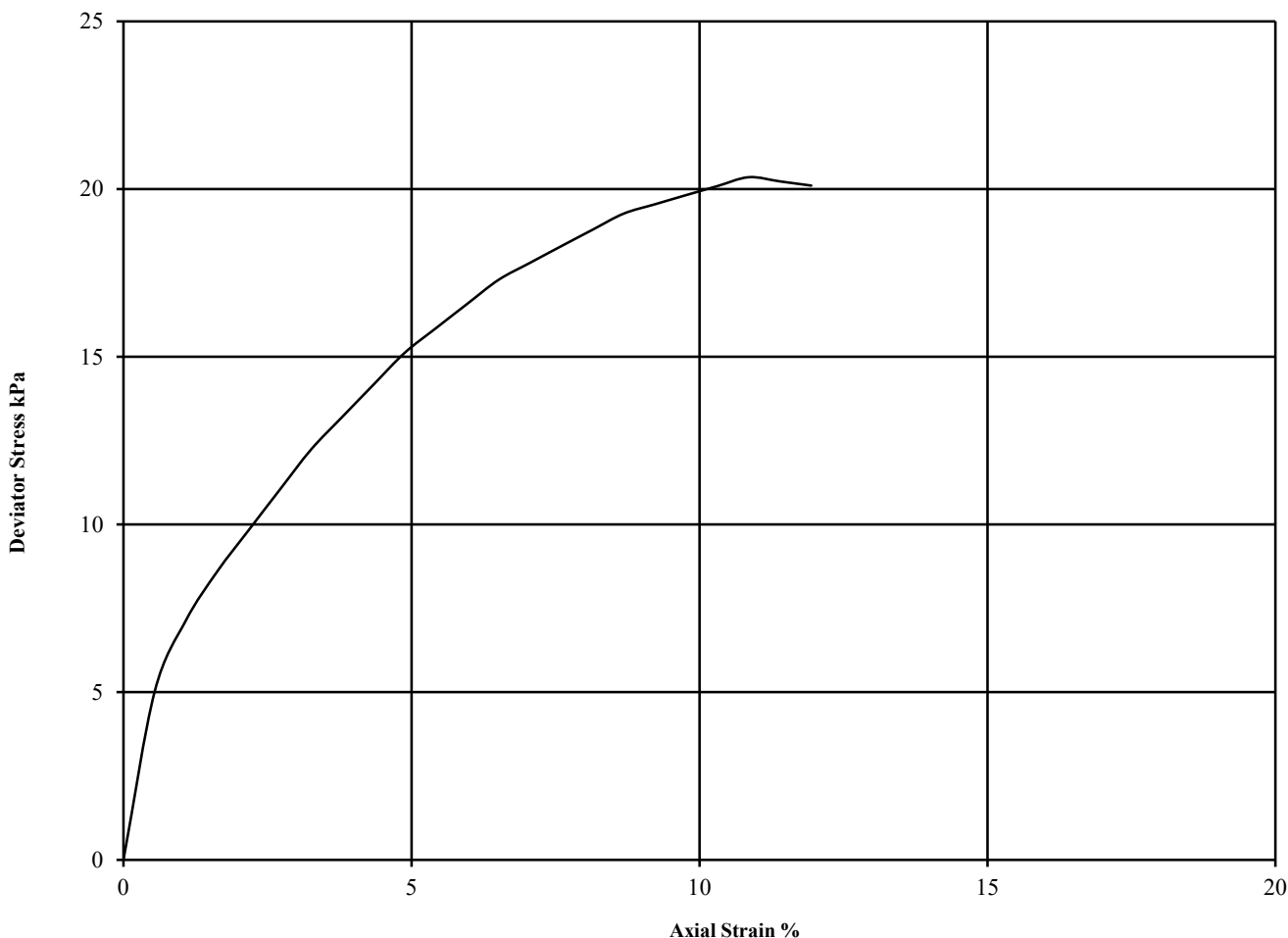
Sample Number:

Base Depth (m):

5.45

Sample Type

UT



Diameter (mm):		102	Height (mm):		188	Test:	UU Single Stage		Remarks:
Specimen	Moisture Content (%)	Bulk Density (Mg/m3)	Dry Density (Mg/m3)	Cell Pressure (kPa)	Corr. Max. Deviator Stress (kPa)	Shear Strength Cu (kPa)	Failure Strain (%)	Mode of Failure	Undisturbed Sample Sample taken from top of tube Rate of strain = 2 %/min Latex Membrane used 0.2 mm thick, Correction applied 0.35 See summary of soil descriptions
				θ_3	$(\theta_1 - \theta_3)_f$	$\frac{1}{2}(\theta_1 - \theta_3)_f$			
1	131	1.27	0.55	100	20	10	10.9	Intermediate	



Dyke Road Galway

Contract No:

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Client Ref:

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UNDRAINED SHEAR STRENGTH IN TRIAXIAL COMPRESSION

WITHOUT MEASUREMENT OF PORE PRESSURE

BS1377 : Part7 : 1990: Clause 8

Hole Number:

BRC03

Top Depth (m):

3.00

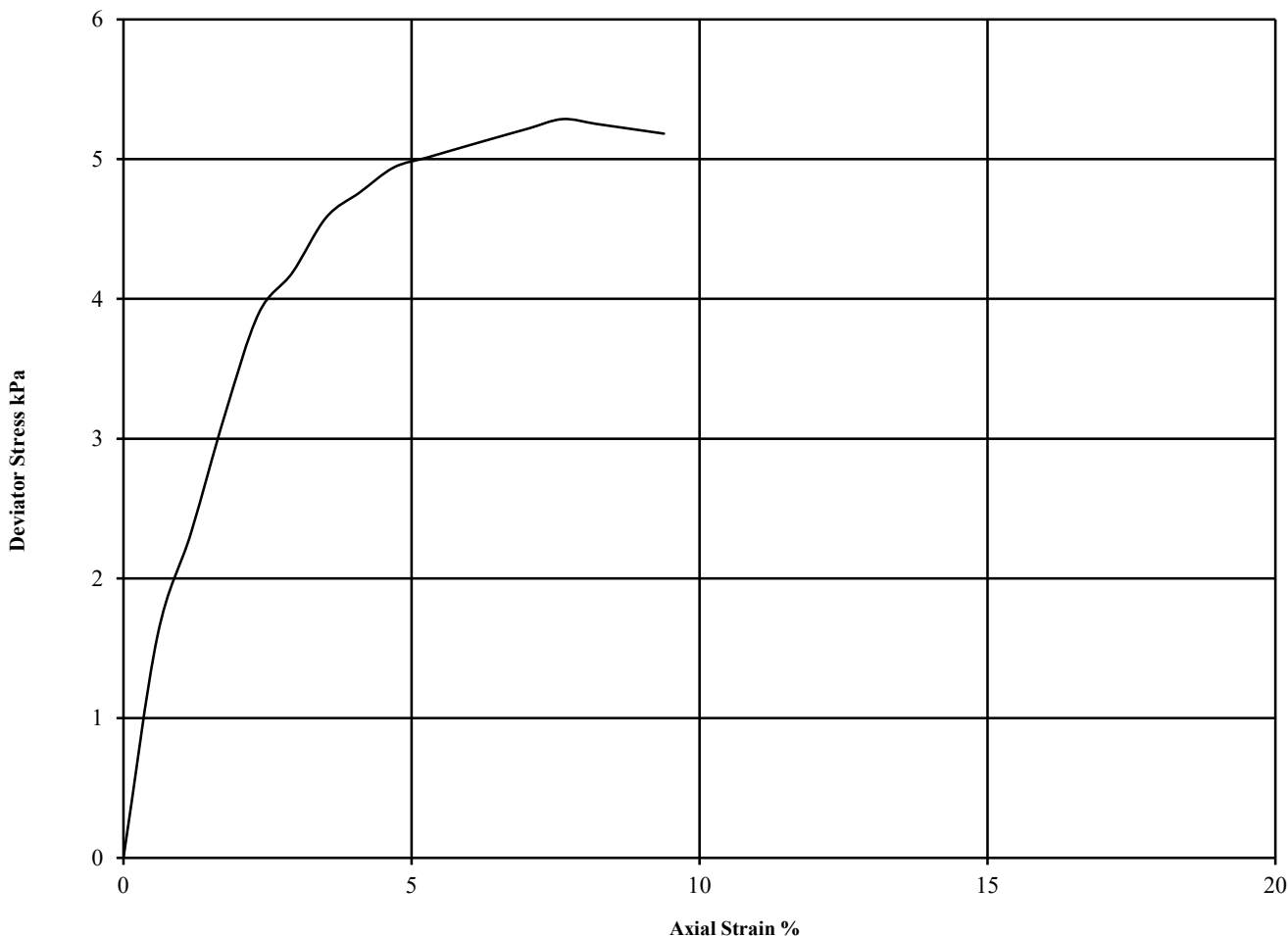
Sample Number:

Base Depth (m):

3.45

Sample Type

UT



Diameter (mm):		102	Height (mm):		174	Test:	UU Single Stage		Remarks:
Specimen	Moisture Content (%)	Bulk Density (Mg/m3)	Dry Density (Mg/m3)	Cell Pressure (kPa)	Corr. Max. Deviator Stress (kPa)	Shear Strength Cu (kPa)	Failure Strain (%)	Mode of Failure	Undisturbed Sample Sample taken from top of tube Rate of strain = 2 %/min Latex Membrane used 0.2 mm thick, Correction applied 0.36 See summary of soil descriptions
				θ_3	$(\theta_1 - \theta_3)_f$	$\frac{1}{2}(\theta_1 - \theta_3)_f$			
1	87	1.44	0.77	60	5	3	7.6	Intermediate	



Dyke Road Galway

Contract No:

PSL24/3965

Client Ref:

13614-02-24

ONE DIMENSIONAL CONSOLIDATION TEST

BS 1377: Part 5: 1990: Clause 3

Hole Number: BH02

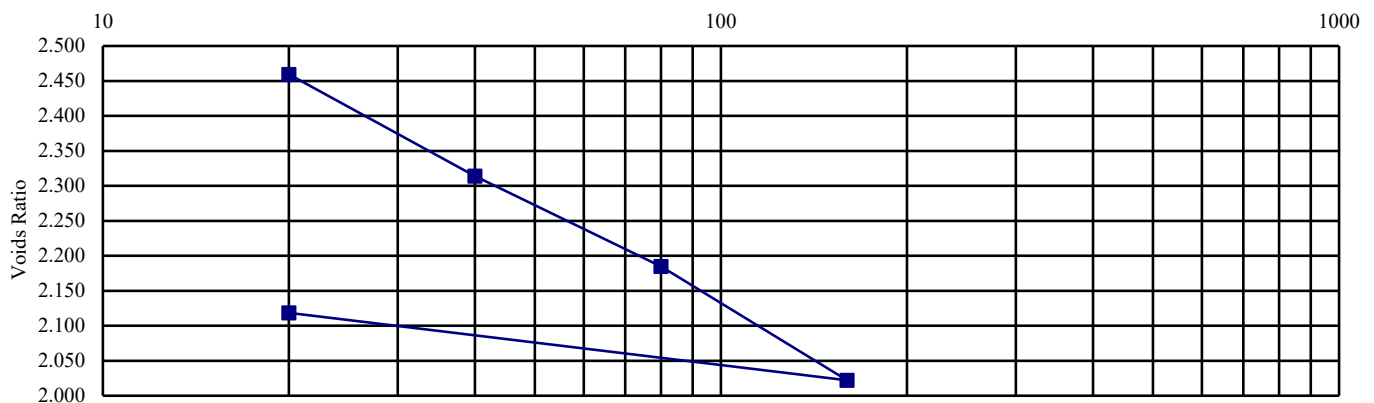
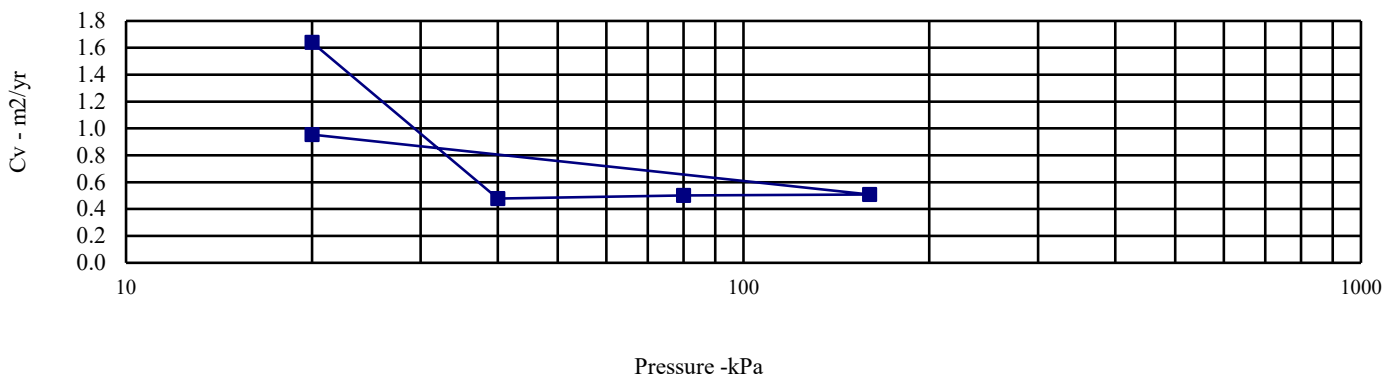
Top Depth (m): 1.00

Sample Number:

Base Depth (m) : 1.27

Sample Type: UT

Initial Conditions		Pressure Range		Mv	Cv	Specimen location	
Moisture Content (%):	80	kPa		m2/MN	m2/yr	within tube:	Top
Bulk Density (Mg/m3):	1.31	0	20	2.298	1.641	Method used to	
Dry Density (Mg/m3):	0.73	20	40	2.103	0.478	determine CV:	T90
Voids Ratio:	2.626	40	80	0.975	0.502	Nominal temperature	
Degree of saturation:	80.4	80	160	0.639	0.509	during test 'C':	20
Height (mm):	20.014	160	20	0.228	0.953	Remarks:	
Diameter (mm)	75.008					See summary of soil descriptions	
Particle Density (Mg/m3):	2.65						
Assumed							



Dyke Road Galway

Contract No:
PSL24/3965
Client Ref:
13614-02-24

ONE DIMENSIONAL CONSOLIDATION TEST

BS 1377: Part 5: 1990: Clause 3

Hole Number: BH02

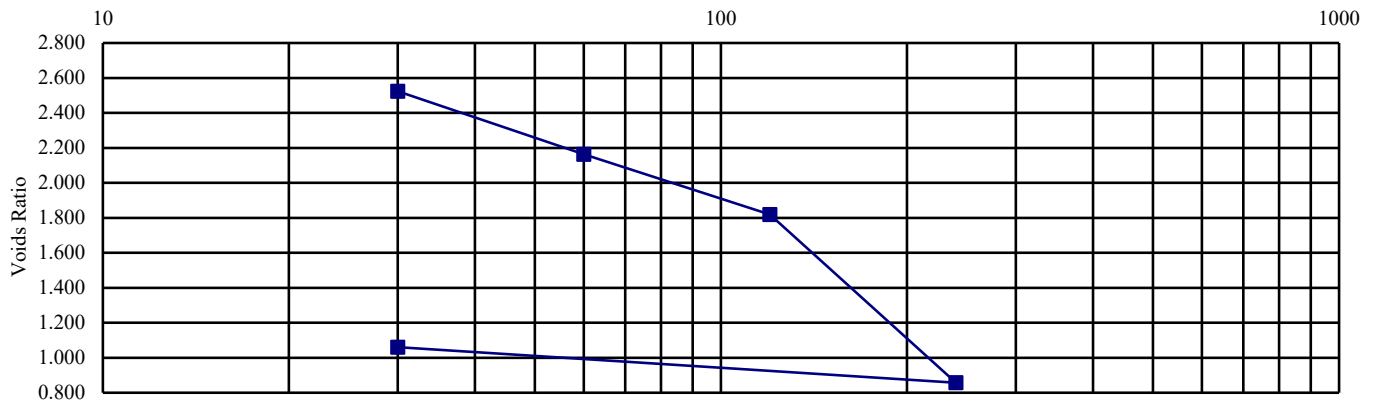
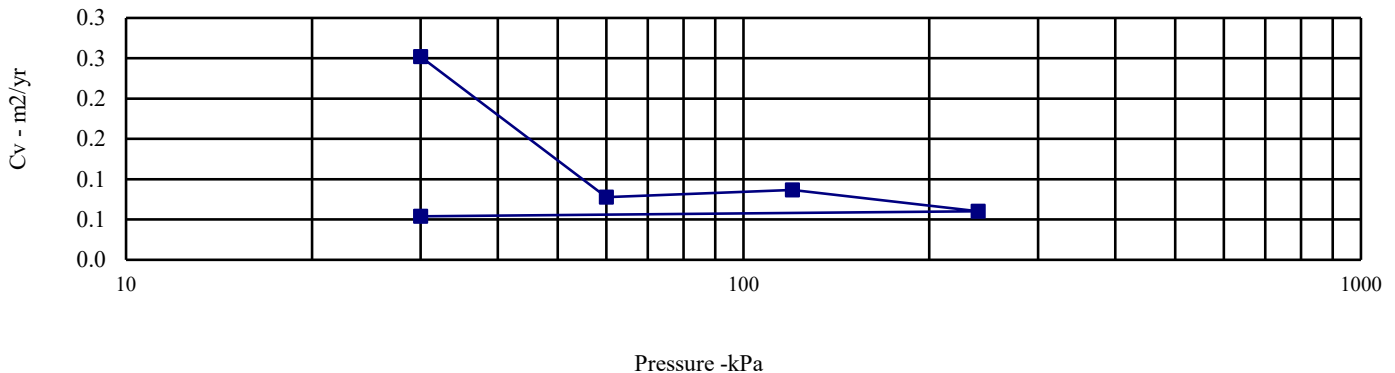
Top Depth (m): 3.00

Sample Number:

Base Depth (m) : 3.45

Sample Type: UT

Initial Conditions		Pressure Range		Mv	Cv	Specimen location	
Moisture Content (%):	104	kPa		m2/MN	m2/yr	within tube:	Top
Bulk Density (Mg/m3):	1.35	0	30	3.883	0.252	Method used to	
Dry Density (Mg/m3):	0.66	30	60	3.400	0.078	determine CV:	T90
Voids Ratio:	2.989	60	120	1.820	0.087	Nominal temperature	
Degree of saturation:	92.2	120	240	2.843	0.060	during test 'C':	20
Height (mm):	20.01	240	30	0.522	0.054	Remarks:	
Diameter (mm)	75.01					See summary of soil descriptions	
Particle Density (Mg/m3):	2.65						
Assumed							



Dyke Road Galway

Contract No:
PSL24/3965
Client Ref:
13614-02-24

ONE DIMENSIONAL CONSOLIDATION TEST

BS 1377: Part 5: 1990: Clause 3

Hole Number: BH03

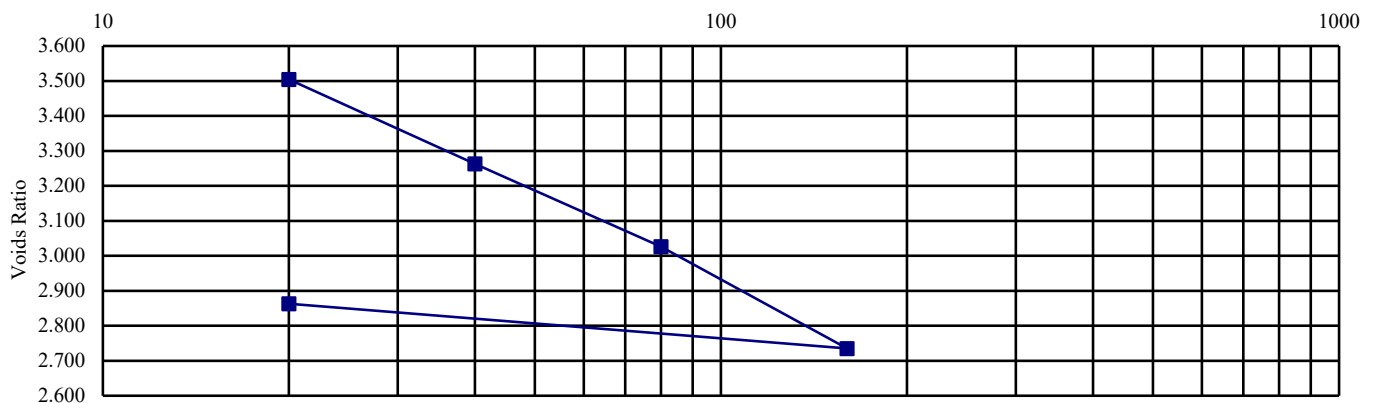
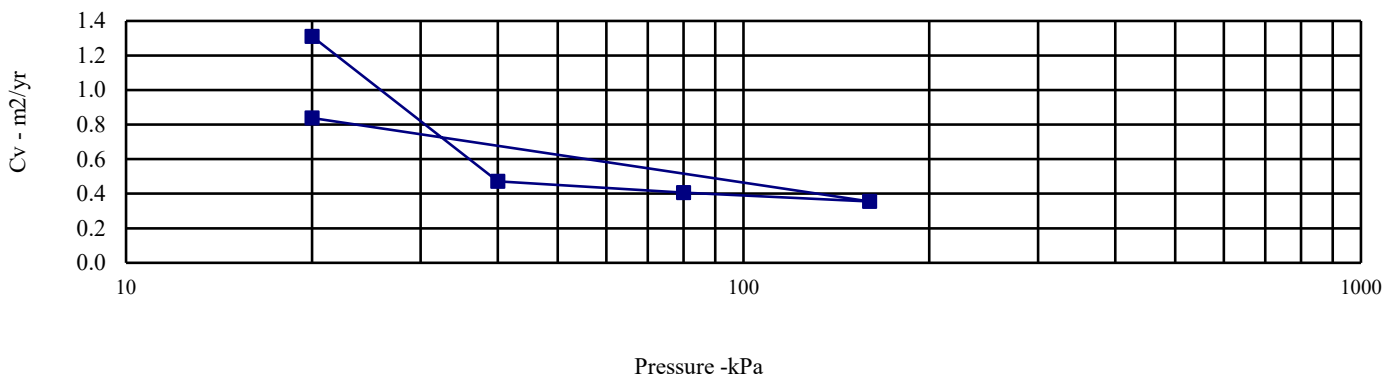
Top Depth (m): 1.00

Sample Number:

Base Depth (m) : 1.45

Sample Type: UT

Initial Conditions		Pressure Range		Mv	Cv	Specimen location	
Moisture Content (%):	147	kPa		m2/MN	m2/yr	within tube:	Top
Bulk Density (Mg/m3):	1.31	0	20	4.867	1.311	Method used to	
Dry Density (Mg/m3):	0.53	20	40	2.677	0.471	determine CV:	T90
Voids Ratio:	3.990	40	80	1.388	0.406	Nominal temperature	
Degree of saturation:	97.7	80	160	0.905	0.355	during test 'C':	20
Height (mm):	20.032	160	20	0.244	0.838	Remarks:	
Diameter (mm)	74.955					See summary of soil descriptions	
Particle Density (Mg/m3):	2.65						
Assumed							



Dyke Road Galway

Contract No:
PSL24/3965
Client Ref:
13614-02-24

ONE DIMENSIONAL CONSOLIDATION TEST

BS 1377: Part 5: 1990: Clause 3

Hole Number: BH03

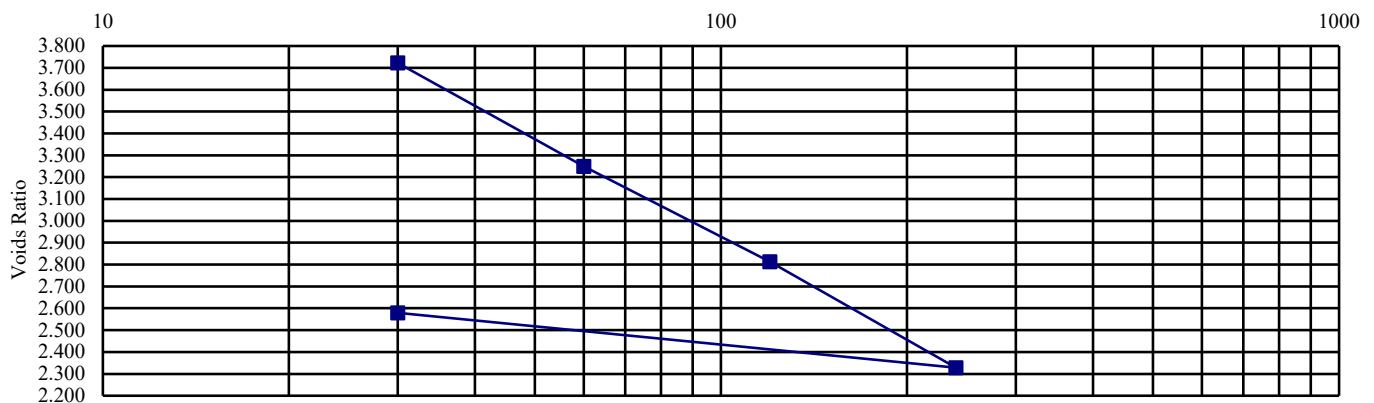
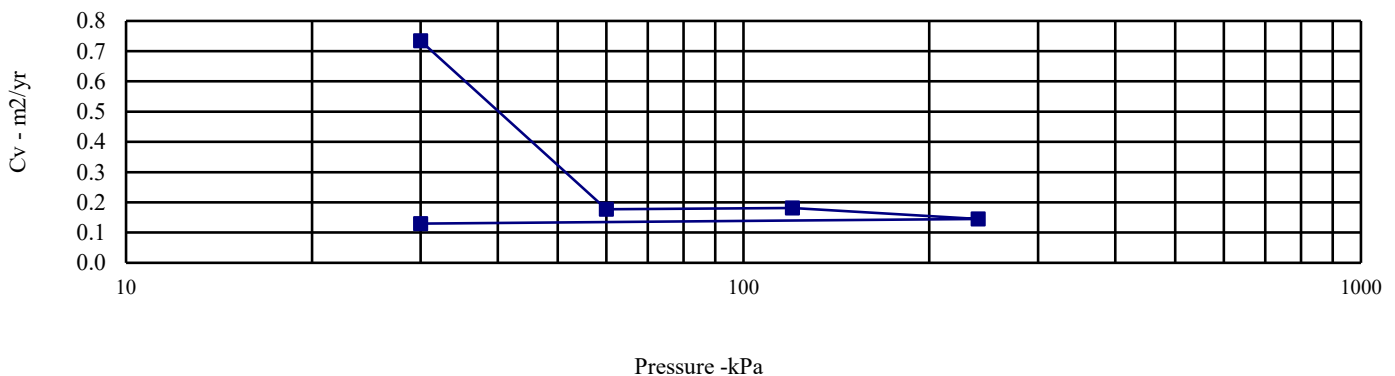
Top Depth (m): 3.00

Sample Number:

Base Depth (m) : 3.45

Sample Type: UT

Initial Conditions		Pressure Range		Mv	Cv	Specimen location	
Moisture Content (%):	151	kPa		m2/MN	m2/yr	within tube:	Top
Bulk Density (Mg/m3):	1.17	0	30	5.672	0.734	Method used to	
Dry Density (Mg/m3):	0.47	30	60	3.341	0.177	determine CV:	T90
Voids Ratio:	4.691	60	120	1.710	0.181	Nominal temperature	
Degree of saturation:	85.5	120	240	1.059	0.145	during test 'C':	20
Height (mm):	20.016	240	30	0.359	0.130	Remarks:	
Diameter (mm)	75.028					See summary of soil descriptions	
Particle Density (Mg/m3):	2.65						
Assumed							



Dyke Road Galway

Contract No:
PSL24/3965
Client Ref:
13614-02-24

ONE DIMENSIONAL CONSOLIDATION TEST

BS 1377: Part 5: 1990: Clause 3

Hole Number: BH06

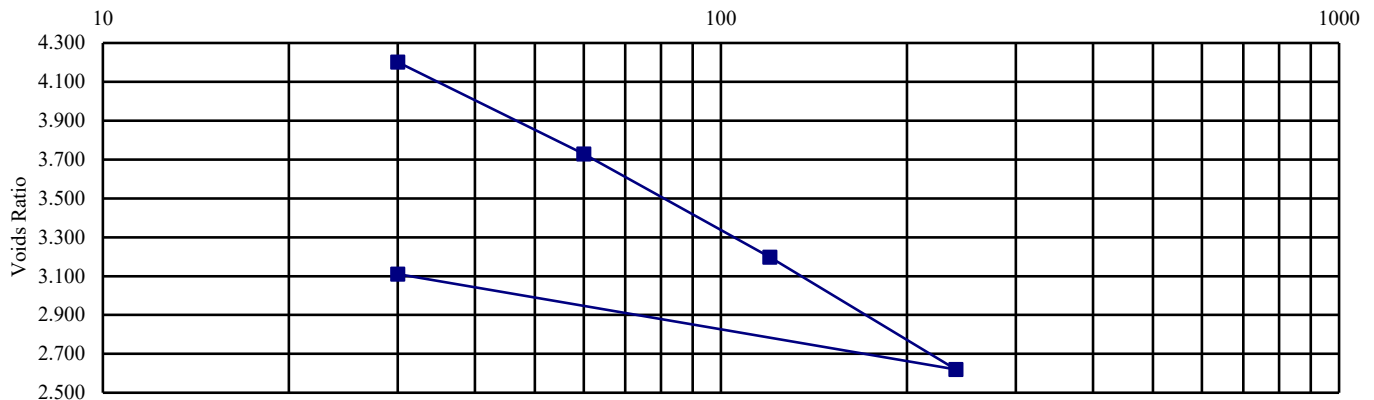
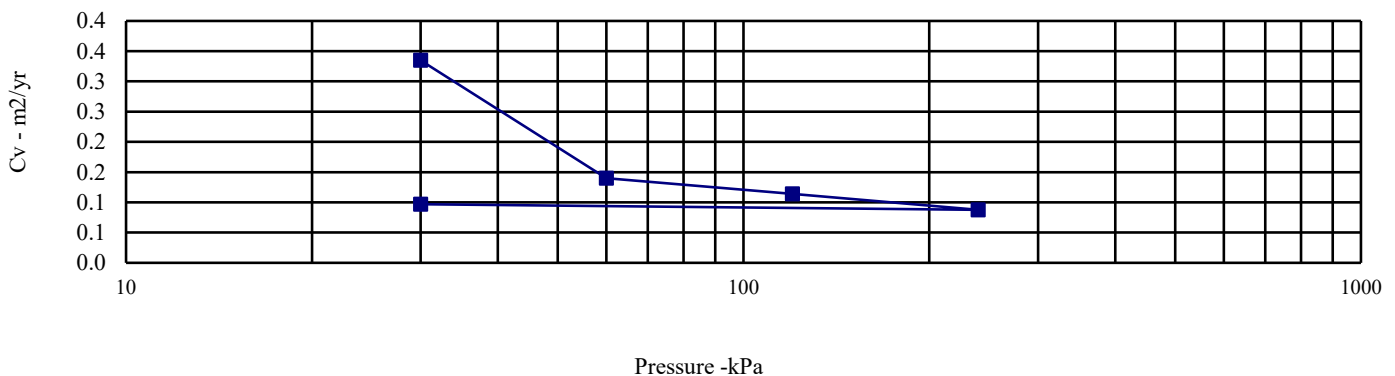
Top Depth (m): 3.00

Sample Number:

Base Depth (m) : 3.45

Sample Type: UT

Initial Conditions		Pressure Range		Mv	Cv	Specimen location	
Moisture Content (%):	167	kPa		m2/MN	m2/yr	within tube:	Top
Bulk Density (Mg/m3):	1.25	0	30	2.584	0.335	Method used to	
Dry Density (Mg/m3):	0.47	30	60	3.028	0.140	determine CV:	T90
Voids Ratio:	4.638	60	120	1.871	0.114	Nominal temperature	
Degree of saturation:	95.2	120	240	1.148	0.088	during test 'C':	20
Height (mm):	20.024	240	30	0.645	0.097	Remarks:	
Diameter (mm)	75.02					See summary of soil descriptions	
Particle Density (Mg/m3):	2.65						
Assumed							



Dyke Road Galway

Contract No:
PSL24/3965
Client Ref:
13614-02-24



7 - 11 Harding Street
Leicester
LE1 4DH

Professional Soils Laboratory

5/7 Hexthorpe Road
Hexthorpe
Doncaster
DN4 0AR

Analytical Test Report: L24/05476/PSL - 24-46441

Your Project Reference:	PSL24/3965 Dyke Road Galway		
Your Order Number:	PSL24/3965	Samples Received / Instructed:	18/06/2024 / 18/06/2024
Report Issue Number:	1	Sample Tested:	18/06 to 24/06/2024
Samples Analysed:	10 soil samples	Report issued:	24/06/2024

Signed

James Gane
Analytical Services Manager
CTS Group

Notes:

General

Please refer to Methodologies page for details pertaining to the analytical methods undertaken.

Samples will be retained for 14 days after issue of this report unless otherwise requested.

Moisture Content was determined in accordance with CTS method statement MS - CL - Sample Prep, oven dried at <30°C.

Moisture Content is reported as a percentage of the dry mass of soil, this calculation is in accordance with BS1377, Part 2, 1990, Clause 3.2

Stone Content was determined in accordance with CTS method statement MS - CL - Sample Prep and refers to the percentage of stones retained on a 10mm BS test sieve.

Where specification limits are included these are for guidance only. Where a measured value has been highlighted this is not implying acceptance or failure and certainty of measurement values have not been taken into account.

Uncertainty of measurement values are available on request.

Samples were supplied by customer, results apply to the samples as received.

Deviating Samples

On receipt samples are compared against our sample holding and handling protocols, where any deviations have been noted these are reported on our deviating sample page (if present)

Accreditation Key

UKAS = UKAS Accreditation, MCERTS = MCERTS Accreditation, u = Unaccredited, subUKAS - Subcontracted to a laboratory UKAS accredited for this test, subMCERTS - Subcontracted to a laboratory MCERTS accredited for this test

MCERTS Accreditation only covers the SAND, CLAY and LOAM matrices

Date of Issue: 29.05.2024

Issued by: J. Gane

Issue No: 4

Rev No: 10



L24/05476/PSL - 24-46441

Project Reference - PSL24/3965 Dyke Road Galway

Analytical Test Results - Solid

Lab Reference			372762	372763	372765	372766	372767	372768
Client Sample ID			-	-	-	-	-	-
Client Sample Location			BH01	BH01	BRC02	BRC02	BRC02	BRC03
Client Sample Type			B	B	B	B	B	B
Client Sample Number			-	-	-	-	-	-
Depth - Top (m)			3.50	6.70	2.00	3.50	7.00	4.00
Depth - Bottom (m)			3.50	6.70	2.00	3.50	7.00	4.00
Date of Sampling			-	-	-	-	-	-
Time of Sampling			-	-	-	-	-	-
Sample Matrix			Clay	Loam	Loam	Clay	Other	Clay
Determinant	Units	Accreditation						
SOM (via TOC)	(%)	UKAS	2.9	5.1	20	2.6	< 0.9	< 0.9



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L24/05476/PSL - 24-46441

Project Reference - PSL24/3965 Dyke Road Galway

Analytical Test Results - Solid

Lab Reference			372769	372770	372771
Client Sample ID			-	-	-
Client Sample Location			BRC03	BRC04	BRC05
Client Sample Type			B	B	B
Client Sample Number			-	-	-
Depth - Top (m)			7.50	4.00	3.50
Depth - Bottom (m)			7.50	4.00	3.50
Date of Sampling			-	-	-
Time of Sampling			-	-	-
Sample Matrix			Clay	Other	Clay
Determinant	Units	Accreditation			
SOM (via TOC)	(%)	UKAS	< 0.9	< 0.9	< 0.9



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L24/05476/PSL - 24-46441

Project Reference - PSL24/3965 Dyke Road Galway

Analytical Test Results - Chemical Analysis

Lab Reference			372762	372763	372764	372765	372766	372767
Client Sample ID			-	-	-	-	-	-
Client Sample Location			BH01	BH01	BH05	BRC02	BRC02	BRC02
Client Sample Type			B	B	B	B	B	B
Client Sample Number			-	-	-	-	-	-
Depth - Top (m)			3.50	6.70	4.00	2.00	3.50	7.00
Depth - Bottom (m)			3.50	6.70	4.00	2.00	3.50	7.00
Date of Sampling			-	-	-	-	-	-
Time of Sampling			-	-	-	-	-	-
Sample Matrix			Clay	Loam	Other	Loam	Clay	Other
Determinant	Units	Accreditation						
Water soluble sulphate (as SO ₄)	(mg/l)	u	1400	1500	1500	820	74	230
pH Value	pH Units	MCERTS	8.1	7.7	9.2	7.8	8.3	8.8
Water Soluble Chloride	(mg/l)	u	27	24	12	44	23	11



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L24/05476/PSL - 24-46441

Project Reference - PSL24/3965 Dyke Road Galway

Analytical Test Results - Chemical Analysis

Lab Reference			372768	372769	372770	372771
Client Sample ID			-	-	-	-
Client Sample Location			BRC03	BRC03	BRC04	BRC05
Client Sample Type			B	B	B	B
Client Sample Number			-	-	-	-
Depth - Top (m)			4.00	7.50	4.00	3.50
Depth - Bottom (m)			4.00	7.50	4.00	3.50
Date of Sampling			-	-	-	-
Time of Sampling			-	-	-	-
Sample Matrix			Clay	Clay	Other	Clay
Determinant	Units	Accreditation				
Water soluble sulphate (as SO ₄)	(mg/l)	u	100	76	89	71
pH Value	pH Units	MCERTS	8.7	9.0	9.3	8.7
Water Soluble Chloride	(mg/l)	u	4.9	7.8	4.6	7.6



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Project Reference - PSL24/3965 Dyke Road Galway

Sample Descriptions

Lab Reference	Client Sample ID	Client Sample Location	Client Sample Type	Client Sample Number	Description	Moisture Content (%)	Stone Content (%)	Passing 2mm test sieve (%)
372762	-	BH01	B	-	Greyish light brown sandy silty clay	130	< 0.1	79
372763	-	BH01	B	-	Brownish grey clayey loam with rare rootlets	110	< 0.1	87
372764	-	BH05	B	-	Grey sandy crushed rock	-	-	100
372765	-	BRC02	B	-	Dark brown slightly gravelly slightly clayey silty loam with rare rootlets	75	49	37
372766	-	BRC02	B	-	Greyish light brown sandy silty clay	100	< 0.1	100
372767	-	BRC02	B	-	Light grey silty crushed rock	3.9	30	21
372768	-	BRC03	B	-	Light grey slightly gravelly slightly sandy silty clay	9.4	9.4	56
372769	-	BRC03	B	-	Light grey slightly gravelly slightly sandy silty clay	10	2.7	57
372770	-	BRC04	B	-	Greenish grey sandy gravel	4.4	29	26
372771	-	BRC05	B	-	Light grey slightly gravelly slightly sandy silty clay	7.7	24	66



L24/05476/PSL - 24-46441

Project Reference - PSL24/3965 Dyke Road Galway

Sample Comments

Lab Reference	Client Sample ID	Client Sample Location	Client Sample Type	Client Sample Number	Comments
372762	-	BH01	B	-	
372763	-	BH01	B	-	
372764	-	BH05	B	-	
372765	-	BRC02	B	-	
372766	-	BRC02	B	-	
372767	-	BRC02	B	-	
372768	-	BRC03	B	-	
372769	-	BRC03	B	-	
372770	-	BRC04	B	-	
372771	-	BRC05	B	-	



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Project Reference - PSL24/3965 Dyke Road Galway

Analysis Methodologies

Test Code	Test Name / Reference	Sample condition for analysis	Sample Preparation	Test Details
ANIONSS	MS - CL - Anions by Aquakem (2:1Extract)	Oven dried	Passing 2mm test sieve	Determination of Anions (inc Sulphate, chloride etc.) in soils by Aquakem. Analysis is based on a 2:1 water to soil extraction ratio
PHS	MS - CL - pH in Soils	As received	Passing 10mm test sieve	Determination of pH in soils using a pH probe (using a 1:3 soil to water extraction)
TOCS	MS - CL - TOC Eltra	Air Dried	Passing 10mm test sieve	Determination of Total Organic Carbon in soils
SAMPLEPREP	MS - CL - Sample Preparation	-	-	Preparation of samples (including determination of moisture content) to allow for subsequent analysis



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Sample Deviations

Deviations are listed below against each sample and associated test method, where deviation(s) are noted it means data may not be representative of the sample at the time of sampling and it is possible that results provided may be compromised.

Observations on receipt

A - No date of sampling provided

C - Received in inappropriate container

H - Contains headspace

T - Temperature on receipt exceeds storage temperature

R - Sample(s) received with less than 96 hours for testing to commence/complete, any result formally classed as deviating will be marked with an X against the applicable test (i.e. RX)

Observations whilst in laboratory

X - Exceeds sampling to extraction or analysis timescales

Lab Reference	Client Sample ID	Client Sample Location	Client Sample Type	Client Sample Number	Test	Deviations
372762	-	BH01	B	-		A
372763	-	BH01	B	-		A
372764	-	BH05	B	-		A
372765	-	BRC02	B	-		A
372766	-	BRC02	B	-		A
372767	-	BRC02	B	-		A
372768	-	BRC03	B	-		A
372769	-	BRC03	B	-		A
372770	-	BRC04	B	-		A
372771	-	BRC05	B	-		A

Laboratory Test Report
Uniaxial Compressive Strength

Project:	Dyke Road Galway	Job Number	13614-02-24
Client:	Ground Investigations Ireland	Lab Ref No	ST 28878
	Catherinestown House, Hazelhatch Road	Date Received	20/06/2024
	Newcastle. Co. Dublin	Date Tested	27/06/2024
Originator:	Mike Sutton	Date Reported	01/07/2024

Sample Reference	Moisture Content	Density (Mg/m ³)	Uniaxial Compressive Strength (N/mm ²)
BRC01 14.65-14.88	0.2	2659	116.4
BRC02 18.37-18.60	0.7	2691	140.3
BRC03 17.17-17.45	0.2	2692	100.2
BRC04 10.11-10.30	0.8	2888	88.1
BRC05 9.65-9.88	0.4	2709	112.7

Remarks: Core BRC01 14.65-14.88 Visible Cracks

Approved Signature
James Ward, Operations Manager
CMTL Ireland Limited

Laboratory Test Report
Point Load Strength Index

Project : Dyke Road Galway	Job Number 13614-02-24
Client : Ground Investigations Ireland	Lab Ref No ST 28879
Catherinestown House, Hazelhatch Road	Date Received 20/06/2024
Newcastle, Co. Dublin	Date Tested 26/06/2024
Originator : Mike Sutton	Date Reported 01/07/2024

Point Load Strength Index

Sample No:-	Depth (m)	Description	Type	Orientation	W (mm)	D (mm)	P (kN)	A	De (mm)	I _s	F	I _{s(50)} MN/m ²
BRC01	12.50-12.60	1	D	⊥	63.0	64.0	25.00	4032	64.0	6.104	1.12	6.82
BRC01	16.05-16.15	1,2	D	⊥	63.0	63.0	28.00	3969	63.0	7.055	1.11	7.83
BRC02	15.37-15.50	1	D	⊥	63.0	65.0	33.00	4095	65.0	7.811	1.13	8.79
BRC02	18.20-18.35	1,3	D	⊥	63.0	63.0	14.00	3969	63.0	3.527	1.11	3.91
BRC03	16.40-16.50	1,2	D	⊥	63.0	63.0	29.00	3969	63.0	7.307	1.11	8.11
BRC03	19.95-20.05	1,2	D	⊥	63.0	64.0	24.00	4032	64.0	5.859	1.12	6.55
BRC04	8.40-8.57	1,2	D	⊥	63.0	65.0	10.00	4095	65.0	2.367	1.13	2.66
BRC04	9.50-9.60	1,2	D	⊥	63.0	63.0	25.00	3969	63.0	6.299	1.11	6.99
BRC05	7.71-7.80	1,2,3	D	⊥	63.0	64.0	22.00	4032	64.0	5.371	1.12	6.00
BRC05	9.25-9.36	1,2	D	⊥	63.0	64.0	35.00	4032	64.0	8.545	1.12	9.55
BRC06	11.00-11.17	1,2	D	⊥	63.0	65.0	11.00	4095	65.0	2.604	1.13	2.93
BRC06	13.34-13.44	1,2	D	⊥	63.0	63.0	14.00	3969	63.0	3.527	1.11	3.91

Description 1 : Blue/ Grey
Description 2 : White Veins
Description 2 : Cracks

I _{s(50)} MN/m ² for	Description 1,2,3
Min	2.66
Mean	6.68
Max	9.55

Test
A = axial
D = diametrical

Relationship to planes of weakness
IL = irregular lump ⊥ = perpendicular
|| = parallel

Mean Value

	I _{s(50)} MN/m ²	U.C.S. MN/m ²
Extremely Weak	<0.05	0.6-1.0
Very Weak	0.05-0.20	1.0-5.0
Weak	0.20-0.50	5.0-25.0
Medium Strong	0.50-2.00	25-50
Strong	2.00-4.50	50-100
Very Strong	4.50-9.00	100-250
Extremely Strong	9.00 +	>250

Approved Signature
James Ward, Operations Manager
CMTL Ireland Limited

APPENDIX 9 – Groundwater Monitoring Records





GROUND INVESTIGATIONS IRELAND
Geotechnical & Environmental

Catharinstown House,
Hazelhatch Road,
Newcastle,
Co. Dublin,
D22 YD52

Tel: 01 601 5175 / 5176
Email: info@gii.ie
Web: www.gii.ie

GROUNDWATER MONITORING

Dyke Road

BOREHOLE	DATE	TIME	GROUNDWATER (m BGL)	Comments
BRC01	26/06/2024	11:10	0.17	
BRC02	26/06/2024	10:32	0.87	
BRC04	26/06/2024	09:25	2.25	
BRC05	26/06/2024	09:55	1.30	

LDA Dyke Road

Galway

Geophysical Survey

Report Status: Final

MGX Project Number: 6756

MGX File Reference: 6756f-005.doc

1st July 2024

Confidential Report To:

Land Development Agency
2nd Floor, Ashford House
Tara Street
Dublin 2
D02 VX67

AECOM

4th Floor, Adelphi Plaza
George's Street Upper
Dun Laoghaire
A96 T927

Report submitted by:
Minerex Geophysics Limited

Issued by:

Unit F4, Maynooth Business Campus
Maynooth, Co. Kildare, W23X7Y5
Ireland
Tel.: 01-6510030
Email: info@mgx.ie

Author: John Connaughton (Geophysicist)

Reviewer: Hartmut Krahn (Senior Geophysicist)



Subsurface Geophysical Investigations

EXECUTIVE SUMMARY

1. Minerex Geophysics Ltd. (MGX) carried out a geophysical survey consisting of 2D-Resistivity (ERT), seismic refraction (p-wave) and MASW (s-wave) surveying for the ground investigation for the proposed LDA development at Dyke Road, County Galway.
2. The main objectives of the survey were to determine the ground conditions under the site, to determine the depth to rock and the overburden thickness, and to detect possible karstified rock in order to help guide the locations of direct ground investigations.
3. The recommended locations were drilled and the results are a zone of Metagabbro (BRC04) within the limestone and a deeper pocket of very stiff to hard clay within the good limestone (BRC03).
4. The clean limestone present is liable to karstification, but it does not have to be karstified.
5. The three different methods allowed for correlations to be identified between them and to improve the interpretation.
6. The urban nature of the site has negative effects on all the surveying methods.
7. The seismic refraction data was interpreted with four layers.
8. Layer 1 is interpreted as road construction material underlain by urban made ground and peat.
9. Layer 2 is described as soft to firm clay and silt or urban made ground or peat and extends to depths of 4 – 8m below ground level (bgl).
10. Layer 3 is only present in the NW of the site and is interpreted as very stiff or very dense overburden but may contain some very weathered rock.
11. Rock is indicated by Layer 4. The depth to the top of this layer ranges from 4 – 9m bgl across most of the site but is 11 to 12m bgl in the NW in BRC01 and BH01.
12. Peat present in layer 1 or 2 would be compressed because of the urban layers.
13. Map 2 indicates three zones reviewed as 'Deep Rock', 'very stiff to hard Clay' and 'Metagabbro/anomalous Rock'. These were interpreted after targeted drilling.
14. The MASW data gives results for 3.6m to 4.5m of soft to firm material across much of the site and to deeper levels in the NW.
15. This report was reviewed and finalised after the complete direct ground investigation data had been received.

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List of Tables, Maps and Figures:

Title	Pages	Document Reference
Table 1: Geophysical Survey Locations and Acquisition Parameters	1 x A4	6756f-Tab1.xlsx
Table 2: Summary of Interpretation	In text	In text
Table 3: MASW S-Wave Velocities and Gmax	4 x A4	6756f-Tab3.xlsx
Table 4: Shear Wave Velocity to Stiffness Relationship	In text	In text
Map 1: Geophysical Survey Location Map	1 x A3	6756f_Drawings.dwg
Map 2: Geophysical Survey Interpretation Map	1 x A3	6756f_Drawings.dwg
Figure 1a: Models of Geophysical Survey	1 x A3	6756f_Drawings.dwg
Figure 1b: Models of Geophysical Survey	1 x A3	6756f_Drawings.dwg
Figure 2a: Interpretation of Geophysical Survey	1 x A3	6756f_Drawings.dwg
Figure 2b: Interpretation of Geophysical Survey	1 x A3	6756f_Drawings.dwg

1. INTRODUCTION

1.1 Background

Minerex Geophysics Ltd. (MGX) carried out a geophysical survey for an LDA site on the Dyke Road, Galway. The survey consisted of 2D-Resistivity (ERT), seismic refraction (p-wave) and MASW (s-wave) measurements. The survey was commissioned by the LDA.

This survey utilized various complementary geophysical methods to improve final interpretations. The role of geophysics as a non-destructive fast method is to provide a geological interpretation over a wide area to complement direct ground investigations at specific locations. The direct ground investigation results can be used to improve the initial geophysical results and interpretation.

The survey was aimed at investigating the ground conditions under the site, the depth to rock and identifying possible karstified rock.

During the tender stage, MGX indicated to the client limitations of the survey in this urban setting and improved the methodology and choice of geophysical methods.

The main purpose of a geophysical survey in this setting is to guide the locations for boreholes, rather than creating a ground model with an accuracy like on a greenfield site. The geophysics will rather have an outcome in a relative scale, like the rock is deeper here than there, and there is rather karst here than there. Then such comparisons can be use to target boreholes.

1.2 Objectives

The main objectives of the geophysical survey were:

- To determine the ground conditions under the site
- To determine the depth to rock and the overburden thickness
- To estimate the strength or stiffness or compaction of overburden materials and the rock quality
- To determine the type of overburden and rock
- To detect lateral changes within the geological layers
- To detect possible karstified zones within the rock or karst features
- To determine the s-wave velocity and to calculate the small strain shear modulus G_{\max}

1.3 Site Description

The site is located in a number of car parks along the Dyke Road just east of the Corrib River in Galway City. The site is bordered by roads to the west, south and southeast, a Retail Park to the east and a greenfield site to the north. The Black Box Theatre is in the north of the site. Access was from the Dyke Road in the west.

The whole site is underlain by tarmac. The total survey area covered an area of approx. 18,000m².

1.4 Geology

Online geological maps of Ireland (GSI, 2024) give the following information:

The quaternary sediments are described as urban while there is fen peat noted on the opposite side of the road to the west.

In terms of rock, the survey area is underlain by Visian Limestones, described as undifferentiated limestones.

Visian Limestones are karstifiable and there are many karst features noted in the Galway area, however the nearest features noted to this site are over 2km to the east.

There is an unconformity approx. 200m to the west with metamorphic rock to the west of it.

1.5 Report

This report includes the results and interpretation of the geophysical survey. Maps, figures and tables are included to illustrate the results of the survey. More detailed descriptions of geophysical methods and measurements can be found in GSEG (2002), Milsom (1989) and Reynolds (1997).

The description of soil, rock and the use of geotechnical terms (soft, stiff, dense etc) follows Eurocode (2007) and BSI (2020) standards. The terms are defined in the standards and the physical parameters are related from experience. This geophysical survey has been acquired, processed, interpreted and reported in accordance with these guidelines.

The client provided maps of the site and the digital version was used as the background map in this report. Elevations were surveyed on site and are used in the vertical sections.

The interpretative nature and the non-invasive survey methods must be taken into account when considering the results of this survey and Minerex Geophysics Limited, while using appropriate practice to execute, interpret and present the data, give no guarantees in relation to the existing subsurface.

2. GEOPHYSICAL SURVEY

2.1 Methodology

The methodology consisted of using 2D-Resistivity (ERT), Seismic Refraction and MASW (Multi-Channel Analysis of Surface Waves) surveying.

The 2D-Resistivity survey was carried out first, the data was analysed on site and additional surveying was carried out targeting certain features and ensuring a good spread of lines across the site.

The survey locations are indicated on Map 1. The lines and parameters are tabulated in Table 1.

2.2 2D-Resistivity (ERT)

2D-Resistivity lines were surveyed with electrode spacing of 3m, up to 64 electrodes per set-up and a maximum length of 189m per set-up. The readings were taken with a Tigre Resistivity Meter, Imager Cables, stainless steel electrodes and a laptop with ImagerPro acquisition software.

Lines R2 and R3 were surveyed using the roll-along mode to lengths longer than 189m to achieve continuous coverage to a depth of 15m along the ends of the lines.

The electrodes were placed inside small drilled holes (12mm), and saline water was added to make a good electrical connection. The holes were subsequently re-sealed upon completion of the lines.

During 2D-Resistivity surveying, data is acquired in the form of linear arrays using a suite of metal electrodes. A current is induced into the ground via a pair of electrodes whilst a potential difference is measured across a second pair of electrodes. This allows for the recording of the apparent resistivity in a two-dimensional arrangement below the line. The data is inverted after the survey to obtain a model of subsurface resistivities. The generated model resistivity values and their spatial distribution can then be related to typical values for different geological materials.

The penetration depth of a resistivity set-up increases towards the centre where it reaches an approx. depth of 15m below ground level (bgl).

The presence of metal underground services like water pipes may influence the results of the survey. There are large diameter metal water mains along the dyke road and the resistivity lines were kept away from these in as much as possible.

2D-Resistivity has previously proven zones of anomalous or karstified rock with lateral extents of 10m and more.

2.3 Seismic Refraction

Seismic refraction lines were surveyed with geophone spacing of 3m and 24 geophones per set-up resulting in a 69m length per set-up. The recording equipment consisted of a 24 Channel GEOMETRICS ES-3000 engineering seismograph with 4.5Hz vertical geophones. The seismic energy source consisted of a hammer and plate. A zero-delay trigger was used to start the recording. Normally 7 shot points per p-wave set-up were used.

The seismic refraction survey method focuses on propagating p-waves travelling through the subsurface, which are generated by a seismic source. As the wave propagates through the subsurface, its velocity varies as it travels through overburden, rock with different elastic properties, and along geological boundaries. Velocity data is recorded via the surveying equipment, which is then processed, allowing geological layer thicknesses and boundaries to be established.

Seismic Refraction generally determines the depth to horizontal or near horizontal layers where the compaction or strength or rock quality changes with an accuracy of around 20% of the depth to that layer. Where the layers are shallower than the geophone spacing depth deviations of ± 1 m to top of layers can occur. Where low velocity layers or shadow zones are present (e.g., below solid ground surface) or where layers dip with more than 20 degree angle the accuracy becomes much less. This lower accuracy is the case here on this project.

A low velocity layer exists for the seismic waves below the solid surface layer. This makes it less certain or impossible to pick first breaks from geophones near the source and therefore no velocity determination for the shallow subsurface is possible. This results in larger deviations in the modelling and borehole results are required for a final calibration of the results.

The seismic refraction set-ups with 69m individual length have a reasonable penetration depth of around 15m. An internationally accepted maximum depth estimate for a seismic refraction set-up is 1/6 of the set-up length including offshots. The depth penetration varies according to the velocity structure of the subsurface. In this report we used a depth of 15m bgl. where the seismic modelling was ended as deeper modelling becomes less meaningful.

2.4 MASW (Multichannel Analysis of Surface Waves)

The seismic shear wave velocity was determined by active MASW surveying. MASW (Multi-Channel Analysis of Surface Waves) determines the bulk seismic shear wave velocity versus depth. The velocities are used to determine the small strain shear modulus.

The MASW method was acquired along with the seismic refraction survey though the shots were done individually with a larger time window. The recording equipment consisted of a 24 Channel GEOMETRICS ES-3000 engineering seismograph with 4.5Hz vertical geophones. The seismic energy source consisted of a

hammer and plate. A zero-delay trigger was used to start the recording. The shot points were located at the ends of the set-ups.

Many constraints exist for the MASW method and the main factors on this site that affect the methods are strong vertical velocity gradients, particularly between the overburden and rock, and changing velocity structure and layer thicknesses along the lines.

2.5 Site Work

The data acquisition was carried out on the nights of the 28th and 29th of March 2024. The weather conditions were fair throughout the acquisition period. Health and safety standards were adhered to at all times. A traffic management system was in place, clearly highlighted by the use of warning signs and cones.

The locations and elevations were surveyed with a Carlson NR3 RTK-GPS to accuracy < 0.05m.

3. RESULTS AND INTERPRETATION

The interpretation of geophysical data was executed utilizing the known response of geophysical measurements, typical physical parameters for subsurface features that may underlay the site, and the experience of the authors.

Ground investigation results were available after the survey and the abbreviated borehole logs are indicated on the sections. The overburden was abbreviated as clay, silt, sand and gravel. The rock was generally divided into weathered limestone (based on RQD value < 50%) and limestone or metagabbro rock (> 50%). This can be done only to a certain extent as the rock can be very variable. RQD values, fracture indices and non-intact zones often change rapidly with depth. The small size of a borehole only represents a very small volume of ground while the geophysical survey on the other end of the scale averages over a large volume of ground.

3.1 2D-Resistivity (ERT)

The 2D-Resistivity data was positioned and inverted with the RES2DINV inversion package. Lines using the roll-along method were concatenated for a joint inversion. The programme uses a smoothness constrained least-squares inversion method to produce a 2D model of the subsurface resistivities from the recorded apparent resistivity values. Three variations of the least squares method are available and for this project the Jacobian Matrix was recalculated for the first three iterations, then a Quasi-Newton approximation was used for subsequent iterations. Each dataset was inverted using seven iterations resulting in a typical RMS error of <3.0%. The resulting models were colour contoured with the same resistivity scale for all lines and they are displayed as cross sections (Figures 1a – 1b).

The data shows interference from urban ground and conditions. High resistivities near the surface are likely due to road construction material while rapid changes along some lines may be due to interference from surrounding metal.

Resistivities are characteristic for certain overburden and rock types. If there is a high content of clay minerals (which are electrically conductive) then the overburden resistivity will be lower than as if there is a high content of clastic grains like sand or gravel. The purer the clay and the lower the sand and gravel content, the lower the resistivity. Water content in overburden layers can influence the resistivities, but generally clay content has a more dominating effect.

Karstified rock is defined in this report as a formerly intact clean limestone rock, liable to karstification, that has been partially dissolved by water over long geological time scales and where the cavities and voids have either remained empty (filled by air) or became filled by overburden sediment (clay, silt, sand), weathering product of the broken rock itself or water. This process would lead to a reduction of the resistivity of the overall rock and therefore karstified rock has a lower resistivity than intact clean limestone rock. This is generally indicated by lower resistivities embedded within high resistivity at depth.

High resistivities near the surface are likely an effect of the material used for the construction of the car parks. The low resistivities underlying this are interpreted as peat or clay and silts. High resistivities at depth are interpreted as clean limestone while lower resistivities within the rock layer are described as possible karst features. Due to the built-up nature of the site, the data may be disturbed by non-geological features and any possible karst features would require direct ground investigations in order to determine the rock quality in these areas. Between 130 – 160m along line R4, the high resistivities near the surface are likely caused by some disturbance.

Some features within the 2D-Resistivity models may indicate possible karst zones, though disturbance from metal can never be ruled out. Three deeper areas with different resistivities are indicated Map 2. A linear feature within relatively shallow rock in the SE of the site stretching SW to NE is indicated on all four resistivity lines and in comparison with the rotary core logs was interpreted as Metagabbro or anomalous rock. In the NW of the site at the black box theatre the results indicate deeper rock and the modelling was only working to a depth of around 10m bgl. A third area in the central site area relates to a low resistivity feature within the rock layer along line R2 which was found in the drilling to be very stiff to hard Clay.

3.2 Seismic Refraction

The seismic refraction data was positioned and processed with the SEISMAGER software package to give a layered model of the subsurface. The number of layers has been determined by analysing the seismic traces and a total of 4 layers were used in the models. All seismic lines were subject to a standardised processing sequence which consisted of a topographic correction which was based on integrated elevation data, first break picking, tomographic inversion, travel-time computation via ray-tracing and velocity modelling. Residual deviations of typically 0.4 to 1.8 msec RMS have been obtained for each line. Following each processing stage QC procedures were adhered to. The resulting layer boundaries are shown as thick lines overlaid on the 2D-Resistivity cross sections (Figure 1a – 1b). The average seismic velocities obtained within the layers are annotated on the sections as bold black numbers.

The p-wave seismic velocity is closely linked to the density of subsurface materials and to parameters like compaction, stiffness, strength and rock quality. The higher the density of the subsurface materials the higher the seismic velocity. More compacted, stiffer, denser and stronger material will have a higher seismic velocity. For rock, the seismic velocity is higher when the rock is stronger, less weathered and has a higher quality. If the rock is more weathered, broken, fractured, fissured or karstified then the seismic velocity will be reduced compared to that of intact fresh rock.

Because of the above relationship, the seismic refraction method and seismic velocities are suitable to investigate ground where the layers get denser, more compacted and stronger with depth. A disadvantage is that some materials may have the same seismic velocity: Stiff to very stiff highly consolidated overburden and a weathered rock can have the same seismic velocity range (as could be the case in the layer 3 below).

The modelled seismic data has created the following layered ground model:

Layer 1 has seismic velocities of 500m/s. The velocity is a mix of the tarmac and underlying material that represents a 'shadow zone' as described in Ch2.3.

Layer 2 was modelled with a velocity of 1200m/s and is interpreted as overburden material with soft to firm strength or compaction. This depth of the layer extends to elevation 0mOD across much of the site but is deeper to the NW where the deep rock is interpreted.

Layer 3 velocities of 2000m/s indicate a very stiff or very dense overburden. This layer is only present in the NW of the site. This layer may also contain some very weathered limestone.

Good Rock (Layer 4) is indicated by seismic velocities of 3500 - 4000m/s. The depth to the top of the good rock ranges from 4 – 9m below ground level (bgl) across much of the site but falls to 11 to 12m bgl in the NW in RC BRC01 and BH01. There is a pocket of very stiff to hard clay included in this layer, as drilled in BRC03.

3.3 Interpretation of Resistivity and Seismic Refraction

Table 2 summarises the interpretation. The stiffness or compaction of overburden and the rock strength or quality have been estimated from the seismic velocity. The estimation of the excavatability for the bedrock has been made according to the caterpillar chart published in Reynolds (1997). The geotechnical assessment for rippability will have to take factors like rock type and jointing into account and the estimation in this report is solely based on the seismic velocities.

Interpreted cross sections are shown in Figure 2a – 2b as well as areas highlighted as containing possible karstified limestone or deep rock. Possible Karst zones or deep rock are shown on Map 2. The interpretation has been made from all available information. For overburden layers and the top of the rock the seismic refraction data has been used as seismic refraction is the best method to delineate layer boundaries. The resistivity model values have been used in a general sense to determine overburden materials and rock type as well as identifying possible karst zones within the rock.

Resistivity data is better suited to show overburden material, rock types and features within the rock while seismic refraction velocities are indicating the change of compaction, stiffness or rock quality with depth. Along short sections where only one data type is available an interpolation for the interpreted layers was made.

Table 2: Summary of Interpretation

Layer	General Seismic Velocity Range (m/sec)	General Resistivity Range (Ohmm)	Interpretation	Estimated Excavation Method
1	500	Any	Road Construction Material over urban Made Ground or Peat	Diggable

2	1200	<125	Soft to firm Clay and Silt or urban Made Ground or Peat	Diggable
3	2000	<250	Very stiff or very dense Overburden	Diggable
4a	N/A	< 125	Very stiff to hard Clay	Digging and ripping
4b	3500 - 4000	Any	Limestone	Breaking & Blasting
4c	3500 - 4000	Any	Metagabbro/anomalous Rock	Breaking & Blasting

3.4 MASW (Multichannel Analysis of Surface Waves)

The MASW lines were positioned, processed, analysed and modelled with the SEISIMAGER/SW and the SURFSEIS6 software packages. The objective is to obtain a model of shear wave velocity versus depth.

All shot points were analysed in order to extract the best possible dispersion curve for the modelling stage.

Following processing steps are done to achieve this:

1. Edit the shot point geometry and display the shot points for each array
2. Edit traces and/or apply filters to improve the shot record for the next step
3. A dispersion curve (phase velocity versus frequency plot or dispersion image) is computed
4. For each shot the maximum amplitude at each frequency of the dispersion image is selected and the picks for the dispersion curve are truncated (frequency gate), smoothed and brought forward into the modelling process
5. An initial model of shear-wave velocity, V_s , versus depth is computed
6. An inversion is carried out to create the final V_s curve (Shear wave versus depth). The valid useful depth range is noted and the data saved in a file
7. For stable repeatable results the shear wave velocity versus depth is extracted and the depth range covered by the real survey data is then listed in Table 3
8. The results for the two opposed shot points at the end of each array are compared and an average shear wave velocity is computed
9. The small strain shear modulus (also named G_{max}) for each shot point and depth is computed by using a density of 1800 kg/m³ typical for consolidated overburden (Eq. 1)

(Eq. 1) $G = V_s^2 * \rho * 10^{-6}$

where G = Shear Modulus (MPa)

V_s = Seismic Shear Wave Velocity (m/s)

ρ = Density (kg/m³)

The results are displayed in tabular format in Table 3. The results show the seismic shear wave velocity and the small strain shear modulus vs. depth. S-Wave Velocities across all the lines range from 74 – 292m/s and the small strain shear moduli from 10 – 153MPa.

The relationship between shear wave velocities and material stiffness is summarised below:

Table 4: Shear Wave Velocity to Stiffness Relationship

Shear Wave Velocity Vs Range in m/s	Material Stiffness
< 150	Soft
150 to 300	Firm
300 to 500	Stiff
> 500 m/s	Very Stiff

Intensive efforts have been made to extract the best dispersion curves by time gating, selecting and test processing various source versus receiver trace distances and trace ranges and by directional selection of traces.

The depth of surveying was generally restricted by the strong vertical velocity gradient in particular between the overburden and rock layers. The MASW data can survey the softer layers below the construction material which is not possible by the seismic refraction method. The data shows soft to firm material across the site. The depth of the survey ranges from 3.6m to 4.5m across much of the site but increases to 7.2 – 9m in the NW which also indicating a change in the depth to rock.

4. CONCLUSIONS

The following conclusions are made:

- The geophysical surveys carried out at the LDA Dyke Road site consisted of 2D-Resistivity (ERT), seismic refraction and MASW surveying.
- The purpose of this survey was to provide some guidance for borehole locations by indicating general geological changes across the site and highlighting possible areas of karstified limestone.
- The recommended locations were drilled and the results are incorporated into this interpretation.
- The drilling found a zone of Metagabbro (BRC04) within the limestone and a deeper pocket of very stiff to hard clay within the good limestone (BRC03).
- The urban environment has interfered with the data, namely the tarmac and low velocity layer for the seismic refraction and underground metal for the resistivity.
- At all locations there was a correlation between all three geophysical survey methods, high resistivities generally match with increases in seismic refraction velocities while the depth of the MASW survey showing soft to firm materials ties in with layers 1 and 2 from the seismic refraction survey and extends to greater depths in the NW where these layers are thicker.
- Some high resistivities at depth indicate that there is clean limestone present that is liable to karstification, but it does not have to be karstified.
- The seismic refraction survey was modelled with a total of four layers.
- Layer 1 is mainly affected by the road construction. High resistivities near the surface indicate road construction material such as gravel and tarmac. This layer would also contain urban made ground and peat.
- Layer 2 is interpreted as soft to firm clay and silt or urban made ground or peat. This layer extends down to an elevation of approx. 0mOD across much of the site but extends deeper in the NW.
- Layer 3 is described as very stiff or very dense overburden. This layer is only present in the NW of the site. It may contain some very weathered rock.
- Layer 4 is interpreted as rock. The depth to the top of this layer is between 4 – 9m bgl across most of the site but 11 to 12m bgl in the NW in RC BRC01 and BH01. Due to the interference the seismic modelling depth was limited here to around 10m.
- Resistivities and rotary coring lead to a subdivision of Layer 4 into 3 zones with 4a as very stiff to hard clay, 4b limestone and 4c metagabbro/anomalous rock. These are indicated in Figures 2a and 2b.

- The resistivities give general indications of overburden types and possible features within the rock but the built-up nature of the site can have a non-geological effect on the data as well and care must be taken when using the interpretation.
- There are generally high resistivities near the surface. This would be typical for road construction materials such as tarmac and gravel fill.
- Peat present in layer 1 or 2 would be compressed because of the urban layers.
- Map 2 indicates three zones reviewed as 'Deep Rock' in the NW, very stiff to hard Clay and Metagabbro/anomalous Rock. These were interpreted after targeted drilling.
- Table 3 gives the results from the MASW survey and shows shear wave velocities of 74 – 292m/s across the site which indicates soft to firm material. The small strain shear moduli range from 10 to 153MPa. The depth of these layers is shown to increase towards the NW.
- The interpretation presented here was reviewed after the geotechnical data became available.

5. REFERENCES

1. **BSI, 2020.** BS5930: 2015+A1:2020, Code of Practice for Ground Investigations, British Standards Institute 2020
2. **Eurocode, 2007.** EN 1997-2:2007. Eurocode 7. Part 2 Ground Investigation and Testing 2007
3. **GSEG, 2002.** Geophysics in Engineering Investigations. Geological Society Engineering Geology Special Publication 19, London, 2002
4. **GSI, 2024.** Online Geological Map of Ireland. Geological Survey of Ireland 2024
5. **Milsom, 1989.** Field Geophysics. John Wiley and Sons, 1989
6. **Reynolds, 1997.** An Introduction to Applied and Environmental Geophysics. John Wiley and Son, 1997
7. **Weaver, 1975.** Geological Factors significant in the Assessment of Rippability, 1975

Table 1: Geophysical Survey Locations and Acquisition Parameters

2D-Resistivity (ERT) Survey					
Site	Line	Electrode Spacing (m)	Length (m)	Display direction	Survey Type/Mode
Car Park	R1	3	189	NW-SE	Traverse
Car Park	R2	3	333	N-S	Roll Along
Car Park	R3	3	237	NW-SE	Roll Along
Car Park	R4	3	189	NW-SE	Traverse
		SUM	948		
Seismic Refraction Survey					
Site	Line	Geophone Spacing (m)	Length (m)	Display direction	Survey Type/Mode
Car Park	S1	3	69	NW-SE	Single Setup
Car Park	S2	3	69	N-S	Single Setup
Car Park	S3	3	69	N-S	Single Setup
Car Park	S4	3	69	N-S	Single Setup
Car Park	S5	3	69	N-S	Single Setup
Car Park	S6	3	69	N-S	Single Setup
Car Park	S7	3	69	NW-SE	Single Setup
Car Park	S8	3	69	NW-SE	Single Setup
Car Park	S9	3	69	NW-SE	Single Setup
Car Park	S10	3	69	NW-SE	Single Setup
		SUM	690		
MASW Survey					
Site	Line	Geophone Spacing (m)	Length (m)	Display direction	Survey Type/Mode
Car Park	M1	3	69	NW-SE	1D-MASW
Car Park	M2	3	69	N-S	1D-MASW
Car Park	M3	3	69	N-S	1D-MASW
Car Park	M4	3	69	N-S	1D-MASW
Car Park	M5	3	69	N-S	1D-MASW
Car Park	M6	3	69	N-S	1D-MASW
Car Park	M7	3	69	NW-SE	1D-MASW
Car Park	M8	3	69	NW-SE	1D-MASW
Car Park	M9	3	69	NW-SE	1D-MASW
Car Park	M10	3	69	NW-SE	1D-MASW
		SUM	690		

Table 3: MASW S-Wave Velocities and Gmax

Line	Depth (m)	Left S-Wave Velocity (m/s)	Right S-Wave Velocity (m/s)	Average S-Wave Velocity (m/s)	Average Gmax - Shear Modulus (Mpa)
S1					
	0.8	160	147	153	42
	1.6	178	165	171	53
	2.4	122	117	119	26
	3.2	107	53	80	11
	4.0	151	129	140	35
	4.8	143	192	168	51
	5.6	146	96	121	26
	6.4	141	92	117	25
	7.2	181	92	136	33
S2					
	0.4	66	82	74	10
	0.8	131	111	121	27
	1.2	168	138	153	42
	1.6	147	114	130	31
	2.0	103	114	109	21
	2.4	66	113	90	14
	2.8	80	116	98	17
	3.2	120	106	113	23
	3.6	112	105	108	21
S3					
	1.0	247	217	232	97
	2.0	234	278	256	118
	3.0	269	232	250	113
	4.0	108	147	128	29

Table 3: MASW S-Wave Velocities and Gmax

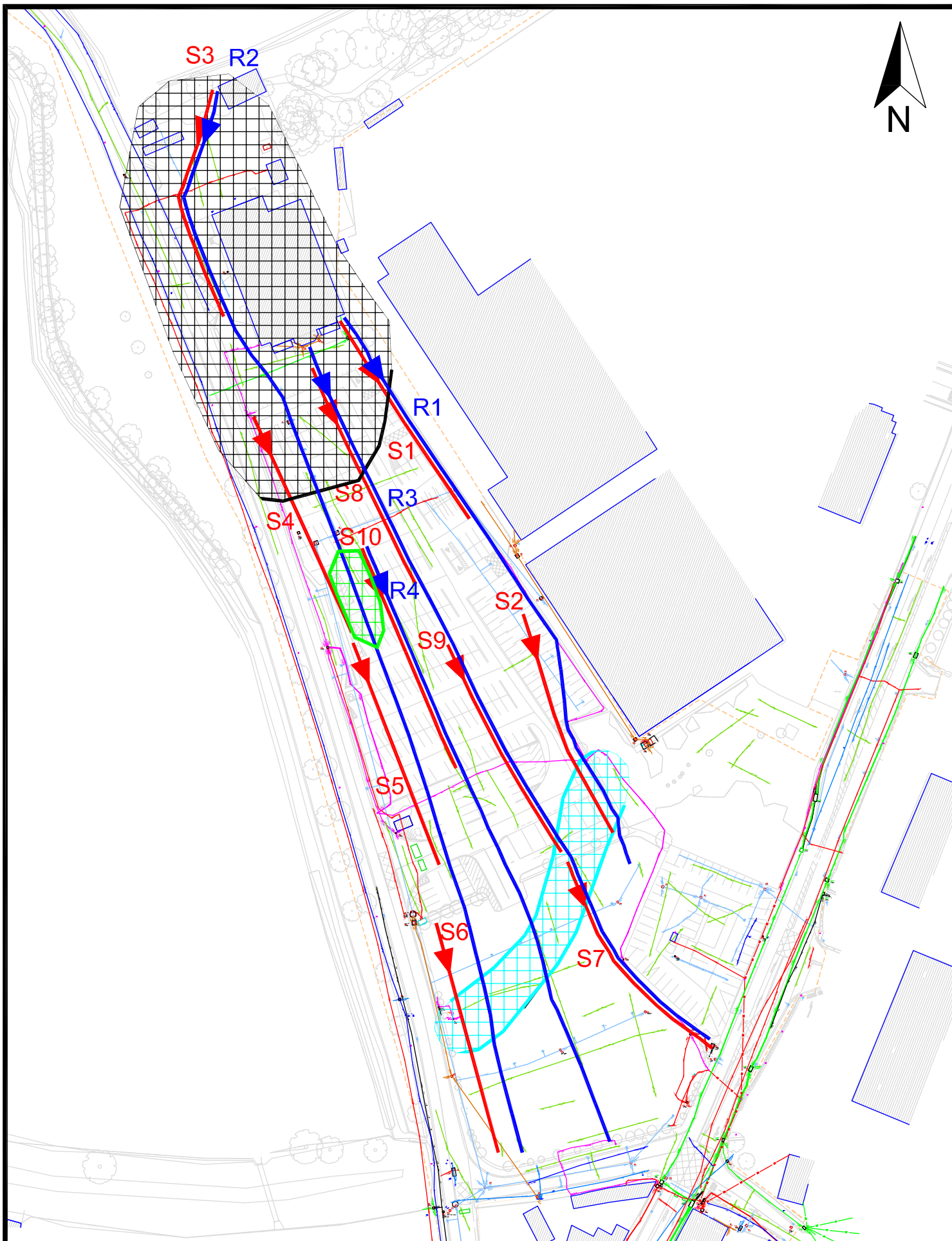
Line	Depth (m)	Left S-Wave Velocity (m/s)	Right S-Wave Velocity (m/s)	Average S-Wave Velocity (m/s)	Average Gmax - Shear Modulus (Mpa)
	5.0	79	110	95	16
	6.0	145	173	159	46
	7.0	279	247	263	124
	8.0	293	259	276	137
	9.0	135	189	162	47
S4					
	1.0	213	240	226	92
	2.0	286	248	267	129
	3.0	263	133	198	71
	4.0	173	98	135	33
	5.0	124	170	147	39
	6.0	180	219	200	72
	7.0	240	228	234	99
	8.0	304	208	256	118
	9.0	300	186	243	106
S5					
	0.4	101	152	126	29
	0.8	81	161	121	26
	1.2	111	166	139	35
	1.6	157	159	158	45
	2.0	175	144	159	46
	2.4	162	128	145	38
	2.8	138	116	127	29
	3.2	115	110	113	23
	3.6	104	107	105	20

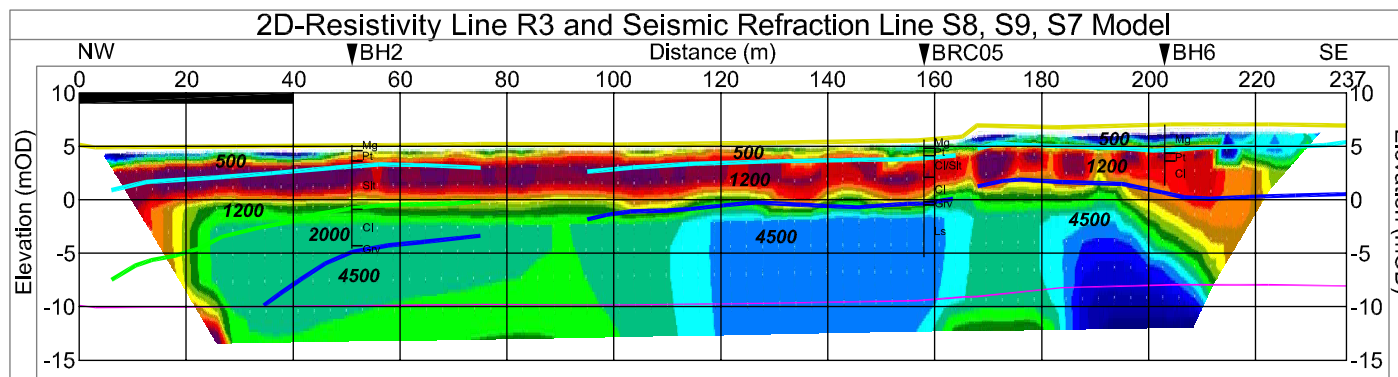
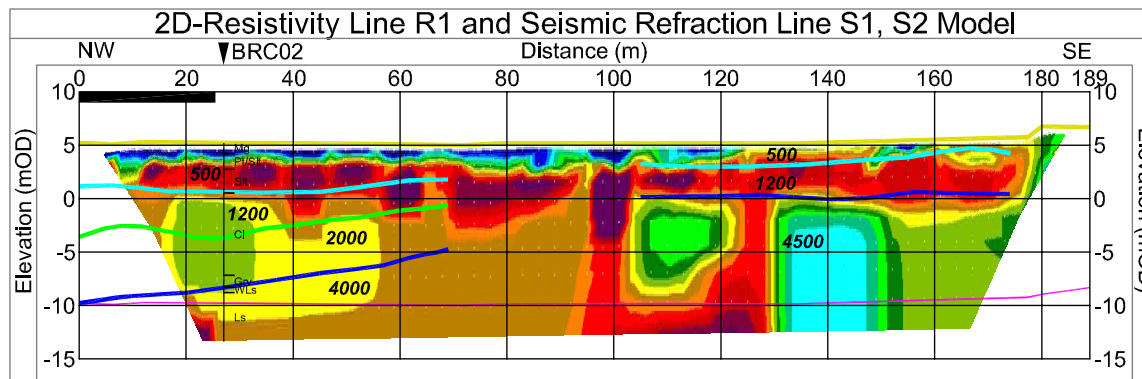
Table 3: MASW S-Wave Velocities and Gmax

Line	Depth (m)	Left S-Wave Velocity (m/s)	Right S-Wave Velocity (m/s)	Average S-Wave Velocity (m/s)	Average Gmax - Shear Modulus (Mpa)
S6					
	0.4	210	227	219	86
	0.8	234	253	244	107
	1.2	248	288	268	129
	1.6	231	311	271	132
	2.0	190	316	253	115
	2.4	135	300	217	85
	2.8	108	257	183	60
	3.2	86	216	151	41
	3.6	77	120	99	18
S7					
	0.5	214	242	228	94
	1.0	281	250	266	127
	1.5	319	265	292	153
	2.0	289	256	273	134
	2.5	215	242	229	94
	3.0	139	217	178	57
	3.5	117	204	161	46
	4.0	142	198	170	52
	4.5	164	193	178	57
S8					
	1.0	118	136	127	29
	2.0	192	171	182	59
	3.0	157	125	141	36
	4.0	86	94	90	15

Table 3: MASW S-Wave Velocities and Gmax

Line	Depth (m)	Left S-Wave Velocity (m/s)	Right S-Wave Velocity (m/s)	Average S-Wave Velocity (m/s)	Average Gmax - Shear Modulus (Mpa)
	5.0	61	153	107	21
	6.0	80	126	103	19
	7.0	122	124	123	27
	8.0	149	179	164	49
	9.0	136	156	146	38
S9					
	0.4	110	126	118	25
	0.8	154	187	171	52
	1.2	192	233	212	81
	1.6	215	232	223	90
	2.0	221	184	203	74
	2.4	220	117	168	51
	2.8	197	80	138	34
	3.2	140	65	102	19
	3.6	92	59	75	10
S10					
	0.5	80	88	84	13
	1.0	180	173	176	56
	1.5	266	235	250	113
	2.0	235	215	225	91
	2.5	158	165	161	47
	3.0	199	154	177	56
	3.5	245	166	206	76
	4.0	190	182	186	62
	4.5	169	169	169	51

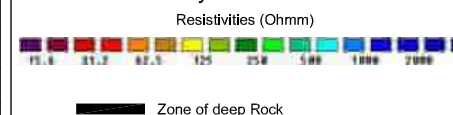




Abbreviated GI Logs:

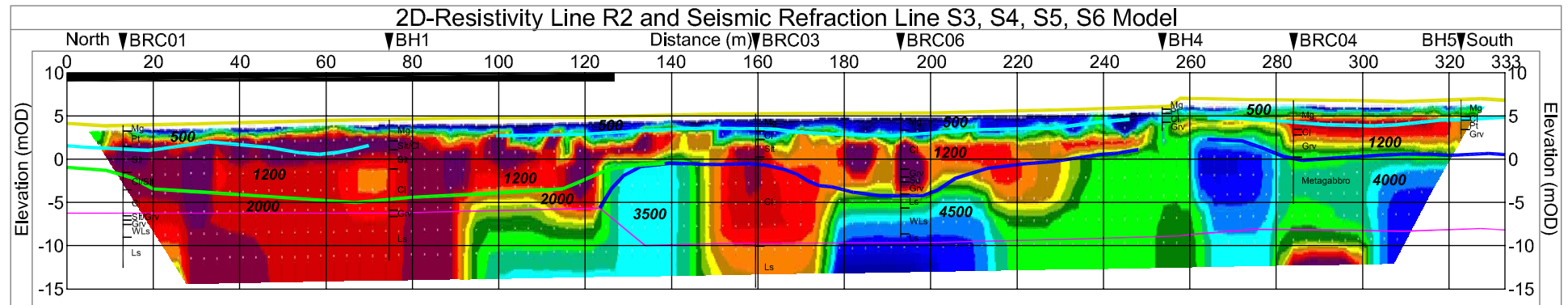
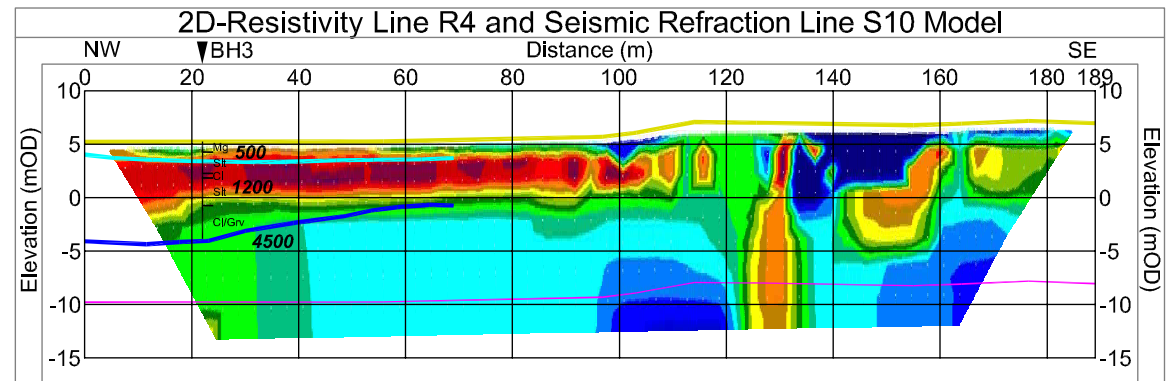
▼ BRC06	Borehole Name and Location	
Pt	Peat	Mg Made Ground
Cl	Clay	Slt Silt
Grv	Gravel	WLS Weathered Limestone
Sd	Sand	Ls Limestone

2D-Resistivity Model Values:



Layers from Seismic Refraction Model:

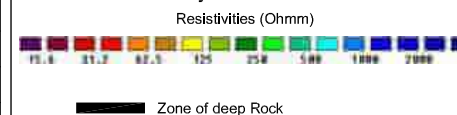
- Ground Surface/Top of Layer 1 (500 m/s)
- Top of Layer 2 (1200 m/s)
- Top of Layer 3 (2000 m/s)
- Top of Layer 4 (3500 - 4500 m/s)
- Seismic Modelling Depth
- 1800** Seismic Velocity in m/s



Abbreviated GI Logs:

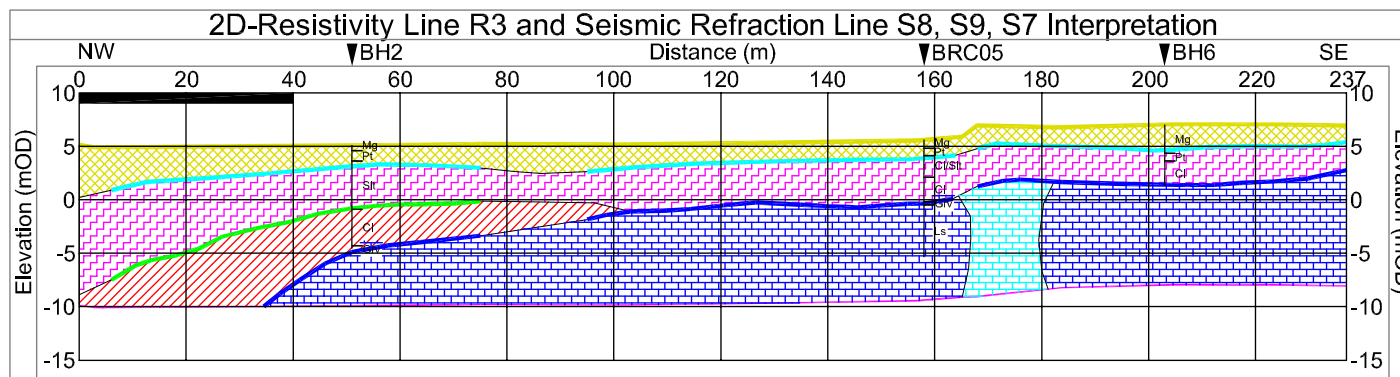
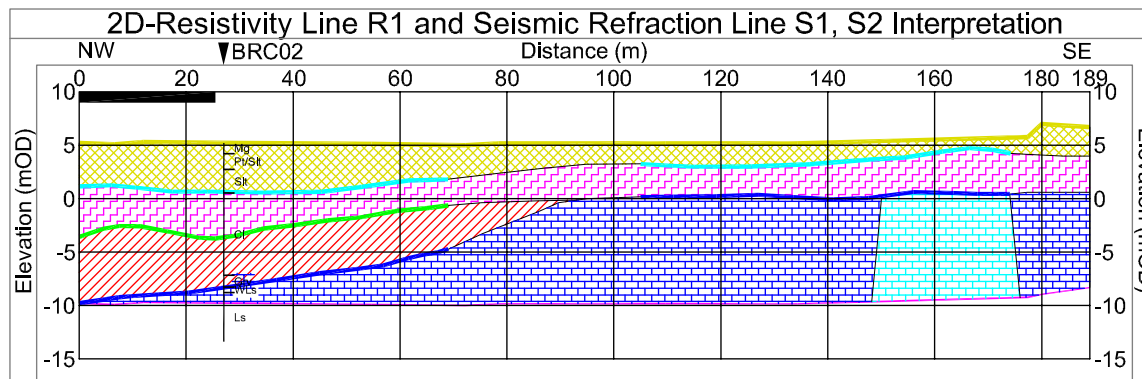
▼ BRC06		Borehole Name and Location	
Pt	Peat	Mg	Made Ground
Cl	Clay	Slt	Silt
Grv	Gravel	WLs	Weathered Limestone
Sd	Sand	Ls	Limestone

2D-Resistivity Model Values:



Layers from Seismic Refraction Model:

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- Top of Layer 2 (1200 m/s)
- Top of Layer 3 (2000 m/s)
- Top of Layer 4 (3500 - 4500 m/s)
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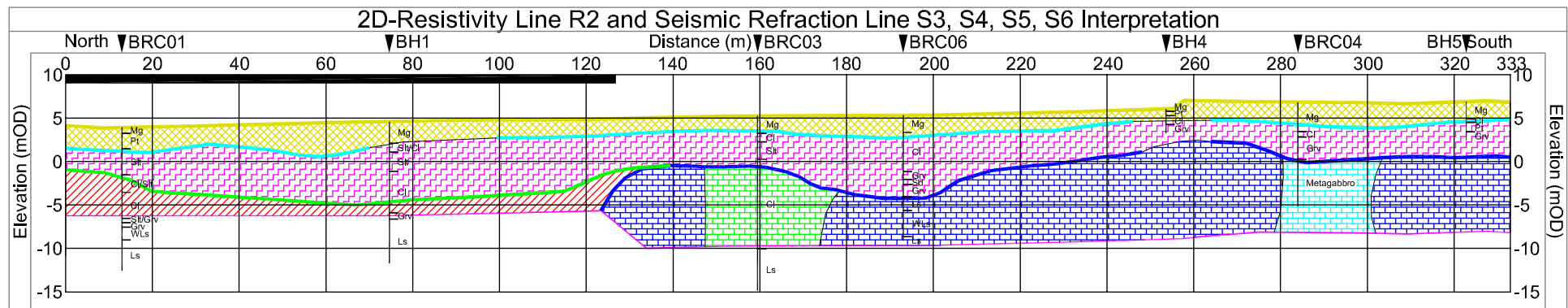
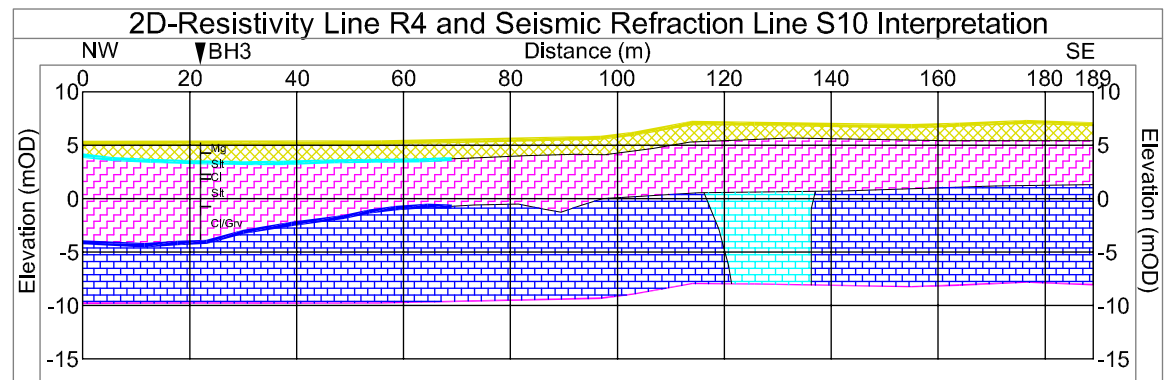


Interpretation:

- 1 Road Construction Material over Urban Made Ground or Peat
- 2 Soft to firm Clay and Silt or urban Made Ground or Peat
- 3 Very stiff or very dense Overburden
- 4a Very stiff to hard Clay
- 4b Limestone
- 4c Metagabbro/anomalous Rock
- Zone of deep Rock

Abbreviated GI Logs:

▼ BRC06 Borehole Name and Location			
Pt	Peat	Mg	Made Ground
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Borehole Name and Location			
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