

Corrib Causeway Phase 1, Dyke Road

Infrastructure Report

Galway City Council

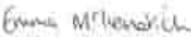
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1. Introduction

1.1 Project Background

AECOM Ireland Ltd (AECOM) has prepared the Infrastructure Report for the development of the Corrib Causeway Phase 1 lands at Dyke Road, Terryland, Galway City (Refer to **Figure 1-1 & Figure 1-2**). The Dyke Road site is located on the edge of Galway City Centre, Galway.

The Infrastructure Report has been prepared to accompany the planning application for a new residential development on the site.



Figure 1-1: Site Location Map

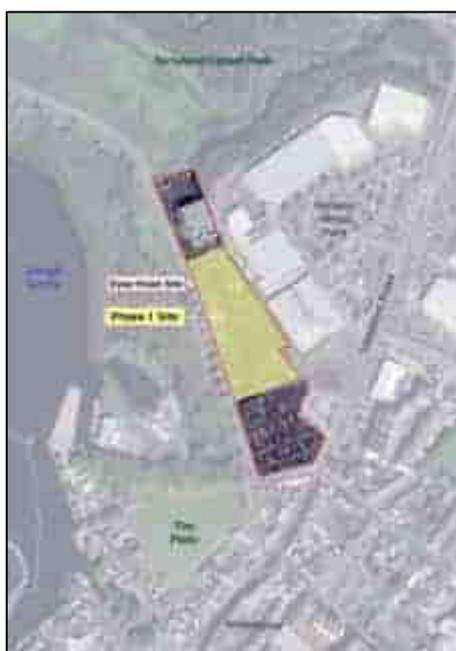


Figure 1-2: Site Location – Phase 1, Tóchar na Coiribe Vision, LDA & MOLA Architecture 2023

The Dyke Road site forms part of a strategic brownfield landbank located on the edge of Galway City Centre which has been identified for comprehensive redevelopment by the Galway Development Plan 2023-2029. The proposed development of the site will be on a phased basis as indicated in **Figure 1-3**. This report pertains to Phase 1 lands only.

The Phase 1 lands are currently used as a public carpark. In the Tóchar na Coiribe Vision the Emerging design proposal explains that “there is also potential to redevelop the south of the car park for civic, commercial, and cultural uses (phase 2). If the Black Box theatre is relocated, there is potential to develop an additional residential block overlooking Terryland Forest Park (Phase 3). In the meantime, the existing uses comprising the Black Box theatre and car parking unaffected by Phase 1 will remain operational as normal, **Figure 1-4**.” (LDA 2024).



Figure 1-3: Project Phasing, Stage 1 Masterplan, MOLA Architecture 2023



Figure 1-4: Emerging Design Proposal, Tóchar na Coiribe Vision, LDA and MOLA 2024

1.2 Existing Development

The existing development consists of a tarmacadam car park which spans over phases 1 & 2 respectively. The most northern part of the existing landholding is home to the Black Box Theatre. The topography of the entire site naturally falls from south to north.

A topographic survey undertaken by Apex Surveys in October 2023 of the overall landholding indicates that ground levels on the site range from 3.84m at the northern end of the site to 7.12m in the southern portion of the site. There is a small retaining wall in the southern portion of the site where the car park levels step up from about 6.0m to around 7.0m. The ground levels on the phase 1 lands typically range from 4.8m to 5.9m with the level in the centre of the site typically being around 5.3m.

It must be noted that the entire site is of hardstanding area. Refer to **Figure 1-5** below.

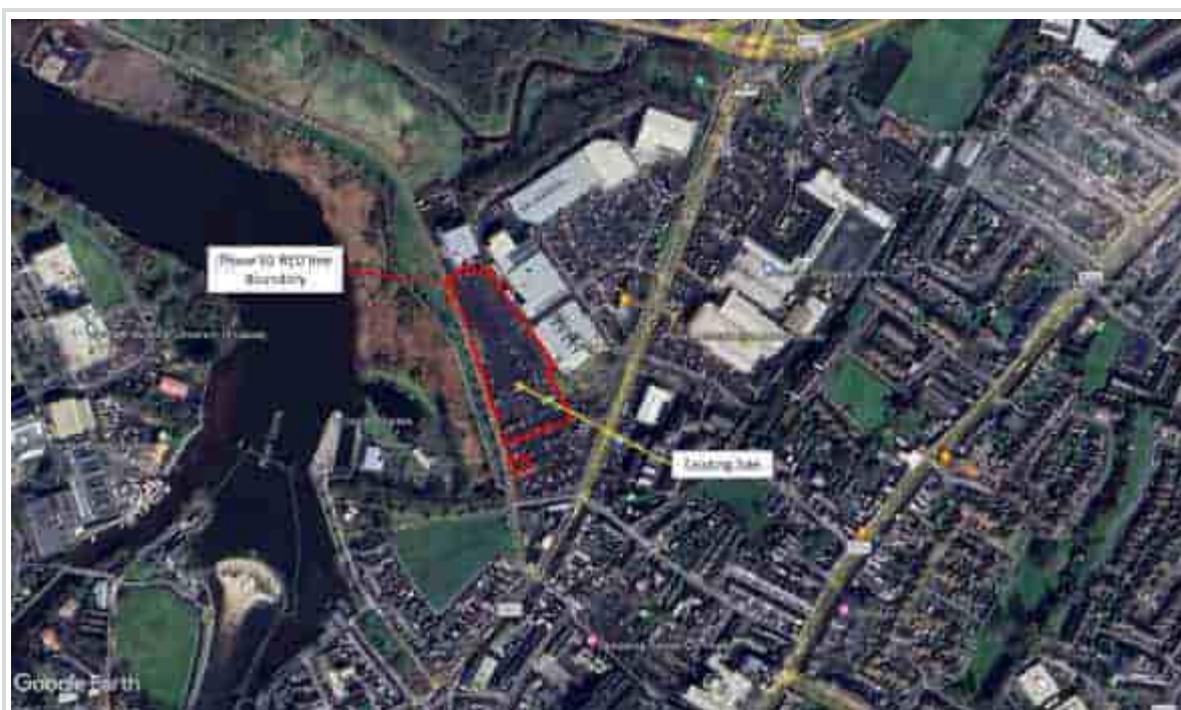


Figure 1-5: Existing Development

1.3 Proposed Development

The proposed development will consist of the construction of a new residential development of 219 no. apartment units and a childcare facility (approx. 241 sq m) in the form of 1 no. new residential block (5 - 9 storeys over lower ground floor level) with associated car parking, bicycle parking, public and communal open spaces, and all ancillary works on a site area of 1.144 ha.

The proposed development will provide for:

- a) 219 no. residential apartment units (109 no. 1-bedroom units, 100 no. 2-bedroom units and 10 no. 3-bedroom units) each with an associated private open space area in the form of a balcony/terrace.
- b) A raised pedestrian boardwalk along the western elevation of the proposed building.
- c) Open Space (approx. 2,778 sq m) is proposed in the form of (a) public open space (approx. 1,183 sq m) to the west of the proposed building fronting on to Dyke Road accommodating outdoor seating, planting, a sunken garden and pedestrian pathways and connections; and (b) communal open space (approx. 1,605 sq m) to the east of the

proposed building in the form of a courtyard including outdoor seating, planting, a children's play area and outdoor sports equipment.

- d) A childcare facility (approx. 241 sq m) at ground floor level with dedicated external play area (approx. 61 sqm) at surface level.
- e) A total of 33 no. new car parking spaces at surface level to serve the proposed residential development (including 2 no. accessible spaces). In addition, 2 no. set down / drop off spaces are proposed to serve the childcare facility.
- f) A total of 465 no. bicycle parking spaces to include 330 no. standard residential spaces, 100 no. visitor spaces, 25 no. cargo bicycle spaces and 10 no. bicycle parking spaces dedicated for the childcare facility staff, all at surface / lower ground floor level.
- g) Vehicular access to serve the development is proposed via Dyke Road at 2 no. new locations along the western site boundary (to the north west and south west of the main development site). Pedestrian and Cyclist access is also proposed throughout the site via Dyke Road and a new pedestrian crossing is also delivered at Dyke Road. The proposed development will extinguish the existing pedestrian connection between Galway Retail Park and the subject site as part of wider proposals for local improvements to permeability.
- h) The removal of 389 no. existing car parking spaces (311 no. from Car Park 1 and 78 no. from Car Park 2) is proposed to provide for the new development. An overall total of 165 no. existing car parking spaces will be maintained in Car Park 2.
- i) The extinguishment of the main existing vehicular entrance serving Car Park 1 and Car Park 2 at Dyke Road with provision made for a new vehicular access point (to the south of the main development site) to facilitate continued access to existing Car Park 2 and the remaining car parking spaces (165 no.).
- j) The removal of existing bring bank facilities including 2 no. clothing banks and 8 no. bottle banks from Dyke Road.
- k) 2 no. telecommunications lattice towers (overall height 6.45 m and 7.67 m) affixed to the rooftop supporting 9 no. 2m 2G/3G/4G antennas; 9 no. 0.8m 5G antennas; 6 no. 0.3m microwave transmission links; together with all associated telecommunications equipment and cabinets. The proposed overall building height including the telecommunications towers is approx. 38.18 m (+43.18 AOD).

The development will also provide for all associated site development works, infrastructure, excavation and clearance works including decommissioning the existing Black Box Theatre waste water pumping station, provision for a new pumping station complete with below ground emergency storage, all boundary treatment/retaining walls, public lighting, internal roads and pathways, ESB substations, switch rooms, water tank rooms, cleaner store and WC, meter rooms, facilities management office, parcel store, comms rooms, plant room, generator room / associated plant space, bin storage, bicycle stores, hard and soft landscaping, play equipment, below ground attenuation tanks, nature based SUDs features, green roofs, roof plant, new and replacement site services and connections for foul drainage, surface water drainage and water supply.

This planning application is accompanied by an Environmental Impact Assessment Report and Natura Impact Statement.

1.4 Galway City Development Plan (2023 - 2029)

In the preparation of this Infrastructure Report, AECOM has considered the Galway City Council Development Plan (2023 - 2029). Chapter 9, which focuses on Environment and Infrastructure, is particularly relevant to the production of this report.

1.4.1 Galway City Development Plan – Flood Risk Assessment Policy

Policy 9.1 of the Galway City Development Plan sets out the following objectives regarding Flood Risk:

1. *Support, in co-operation with the OPW, the implementation of EU Flood Risk Directive (2007/60/EC), the Flood Risk Regulations (SI No, 122 of 2010) and the DECLG and OPW Guidelines for Planning Authorities, the Planning System and Flood Risk Assessment Management (2009), updated/superseding legislation or departmental guidelines and have regard to the findings and relevant identified actions of the Corrib Catchment Flood Risk Management (CFRAM) Study.*
2. *Support and facilitate the implementation of the Coirib go Cósta Galway City Flood Relief Scheme in conjunction with the OPW to support a climate resilient city, protect against flooding and minimise the impact of future climate events. Support in general the associated mitigation and adaptation measures in order to prevent flooding and coastal erosion, subject to appropriate environmental, visual, built heritage and other relevant considerations.*
3. *Ensure the recommendations of the Strategic Flood Risk Assessment (SFRA) for the Galway City Development Plan 2023-2029 are taken into consideration in the assessment of developments in identified areas of flood risk and require site specific Flood Risk Assessment (FRA) and associated design and construction measures appropriate to the scale and nature of the development and the risks arising, in all areas of identified flood risk including on sites where a only small proportion of the site is at risk of flooding and adopt a sequential approach in accordance with the Planning System and Flood Risk Management Guidelines for Planning Authorities (2009).*
4. *Protect and promote sustainable management and uses of water bodies and watercourses from inappropriate development, including rivers, streams, associated undeveloped riparian strips, wetlands, and natural floodplains.*
5. *Ensure flood risk is incorporated into the preparation of any future local area plans, framework plans and masterplans in the city.*
6. *Ensure any proposed measure designed to alleviate flooding/coastal erosion is subject to Appropriate Assessment in accordance with Article 6 of the EU Habitats Directive, where appropriate.*
7. *Continue to protect the coastal area and the foreshore and avoid inappropriate development in areas at risk of coastal erosion and/or would cause and escalate coastal erosion in adjoining areas.*
8. *Protect and maintain, where feasible, undeveloped riparian zones and natural floodplains along the River Corrib and its tributaries.*
8. *Protect and maintain, where feasible, undeveloped riparian zones and natural floodplains along the River Corrib and its tributaries.*

1.4.2 Galway City Development Plan – Water Services Policy

Policy 9.3 of the Galway City Development Plan sets out the following objectives regarding Water Services:

1. *Work in close liaison with Irish Water in the operation of water and wastewater facilities in the city and the upgrade and expansion of the network and the delivery of strategic projects such as the Terryland Water Treatment Plant Intake Works.*
2. *Support the delivery of the objectives of the Irish Water Water Services Strategic Plan (2015) and implementation of the Irish Water Capital Investment Plan 2020 - 2024.*
3. *Work in conjunction with Irish Water to ensure the provision and maintenance, of a high quality and efficient water supply capable of meeting existing and future needs of the city and support any ongoing water mains rehabilitation and water conservation projects.*
4. *Encourage all significant water users to use best practices in water conservation and continue to promote water conservation measures in the design of all new development in the city, such as rainwater harvesting and re-use of grey water, in liaison with Irish Water.*
5. *Support and liaise with Irish Water in the provision of a sustainable and effective wastewater drainage collection and treatment system capable of meeting the existing and future needs of domestic, commercial, and industrial users in the city and MASP area.*
6. *Support the Irish Water ongoing watermain rehabilitation and water leak reduction programme in order to conserve the city's water supply.*
7. *Support the decommissioning of existing individual effluent treatment systems which include septic tanks at locations which include Ballyloughane, where there is a feasible option to connect to the public sewer network. Galway City Council will collaborate with Irish Water in this regard.*
8. *Support the development and implementation of Drinking Water Safety Plans by Irish Water, which seek to protect human health by identifying, assessing, and managing risks to water quality and quantity; taking a holistic approach from source to tap.*
9. *Support the promotion of effective management of trade in discharges to sewers by Irish Water in order to maximise the capacity of existing sewer networks and minimise detrimental impacts on sewage treatment works.*

1.4.3 Galway City Development Plan – Sustainable Urban Drainage Systems (SuDS) Policy

Policy 9.4 of the Galway City Development Plan sets out the following objectives regarding SuDs:

1. *Ensure the use of Sustainable Urban Drainage Systems (SuDS) and sustainable surface water drainage management, wherever practical in the design of development to enable surface water run-off to be managed as near to its source as possible and achieve wider benefits such as sustainable development, water quality, biodiversity local amenity and climate adaptation.*
2. *Promote the use of green infrastructure e.g. green roofs, green walls, bioswales, planting and green spaces for surface water retention purposes as an integrated part of SUDS and to deliver all the ancillary benefits.*

1.5 Terryland River

As per the Galway City – County Geological Site Report (Hennessy et al., 2020. Geological Survey Ireland), the Terryland River exhibits a unique flow pattern; It can either flow out of the River Corrib and disappear underground, acting as a natural sink, or rise and flow toward the Corrib, facilitated by two estavelles akin to springs. The river's general groundwater flow direction originates from the Ballindooley Lough area, flowing southward through an underground conduit system. Within the karst depression, the Eastern and Western estavelles play a crucial role. Although these estavelles connect to Galway Bay or Lough Atalia, the precise discharge locations into these bodies of water remain unidentified. The Terryland River (European Code IE_WE_30T010500) is subject to various sensitivities, including hydro morphology pressures, urban run-off pressures, and being considered a River waterbody risk ('At risk') under the Water Framework Directive.

1.6 Pre-Planning Consultations with Galway City Council

There have been a number of preplanning consultations with GCC and the pre-planning design was shared with GCC in March 2024. All comments raised with respect to the pre-planning design have been addressed in the developed planning stage design through a collaborative and iterative design process.

AECOM presented the surface water strategy during a Teams meeting on 18.06.24, attended by the following GCC representatives Frank Clancy, Padraic MacGoillabhride and Peter Staunton. AECOM outlined the current conditions, the constraints, and the surface water strategy. The strategy which proposes limiting surface water run-off from the site to 25 l/s, by providing two (2) shallow Reinforcement Concrete (RC) attenuation tanks and SuDS measures including Green Roofs, exfiltration permeable paving and lengths of raingarden / swale (also exfiltration systems) was agreed.

1.7 Summary

This report outlines the proposed wastewater drainage, surface water drainage, and water supply network to serve the development. Additionally, it discusses the site constraints that were taken into account during the preliminary design phase.

1.7.1 Surface Water

Current conditions:

The carpark site is nearly 100% impermeable and currently unattenuated flows discharge to the Terryland Stream via an existing drainage network.

Proposal:

It is proposed to restrict the surface water run-off from the site to 25 l/s, by providing two (2) shallow Reinforcement Concrete (RC) attenuation tanks. It is also proposed to incorporate SuDS measures such as an area of Intensive Green Roof, exfiltration permeable paving and lengths of raingarden / swales. The ground conditions are unsuitable for discharge of surface water to ground. Hence, it is simply proposed to attenuate the run-off.

It is also proposed to provide a new separate gravity surface water drainage network to serve the development. The new network will discharge into the existing 525mm Ø concrete pipe and continues northwards to the discharge point into the Terryland Stream.

Further, it is proposed to divert the surface water sewer which serves the Retail development on the Headford Road around the new building and then reconnect it to the 525mm Ø concrete pipe. It is also proposed to relay circa ±155m of the GCC 525mm Ø concrete trunk surface water pipe that runs through the site.

Refer to **Section 2.2** for further details.

1.7.2 Wastewater

Current conditions:

Based on the record drawings from Uisce Éireann and a utility survey, the existing Black Box Theatre is serviced with a foul sewer that runs south to a pumping station (“Black Box WWPS”) located close to the current main entrance to the Dyke Road car park. From there, with a chamber invert level of 2.3m, the sewage is pumped further south along Dyke Road until it joins a combined sewer network on Headford Road at a level of 6.8m.

Proposal:

It is proposed to relay the gravity foul sewer serving the Black Box Theatre and install a new gravity sewer network to serve the development. The existing wastewater pumping station (WWPS) that serves the Black Box Theatre is to be decommissioned and a new WWPS constructed.

The new WWPS has been positioned based on the flood extents within the site and to maximize the separation from buildings. An emergency tank with 24-hour storage capacity at DWF has been provided to serve Phase 1 development and the Black Box Theatre.

The existing 150mm rising main serving the existing WWPS is to be retained and reused. Uisce Éireann have confirmed that a 20m upgrade of a 150mm diameter sewer from Dyke Road to Wood Quay will be required. These works will be undertaken by UÉ.

Refer to **Section 3.2** for further details.

1.7.3 Water Supply

Current Conditions:

A 9” cast-iron watermain runs along Dyke Road. From this watermain, a water connection feeds the Black Box theatre and the Headford Road shopping centre. A 300mm asbestos-cement watermain runs in Headford Road and Bóthar Na Dige Road, while a shorter section of 100mm uPVC water distribution main runs along a short section of Headford Road.

Proposal:

It is proposed to take a connection off the 250mm asbestos watermain on the Dyke Road. The new watermain will pass through the phase 1 lands and loop around 3 sides (south, east, and north) of the development. Refer to **Section 4.2** for further details.

1.7.4 Flood Risk Management

A Stage 3 Site-Specific Flood Risk Assessment has been prepared to accompany this application which addresses the requirements of the Galway City Development Plan 2023 – 2029, the Regional Spatial and Economic Strategy 2020 - 2032 and “The Planning System and Flood Risk Management Guidelines for Planning Authorities”.

2. Surface Water Drainage

2.1 Existing Surface Water Drainage

Based on the record drawings supplied by Uisce Éireann and a utility survey of the site, there are surface water drainage pipes within the proposed Dyke Road development site, collecting surface water from the site and areas to the south of the site (at an invert level of 5.8m), and discharging unattenuated flows into the Terryland wetlands area, north of the existing Black Box Theatre.

The main surface water pipe running south to north along the western boundary of the site is a 450mm Ø concrete pipe. The pipe starts at an invert level of 5.8m on Bóthar Na Dige Road and falls to an invert level of 3.73m around the middle of the subject site, where it increases in size to a 525mm Ø concrete pipe and continues northwards to the discharge point. There is also a surface water pipe running through the site which serves the Retail development on the Headford Road to the east of the proposed development which discharges into this surface water pipe.

Based on the information shown on the record mapping, the existing network runs in a northerly direction to an existing drain within the green area which further discharges to the Terryland Stream. GCC have confirmed the surface water drainage discharges into the Terryland Stream. The approx. bed level of the anticipated discharge point is ~2.9m. The utility records and utility survey are included in **Appendix D**. Refer to **Figure 2-1** for the layout of surface water infrastructure in the vicinity of the site.

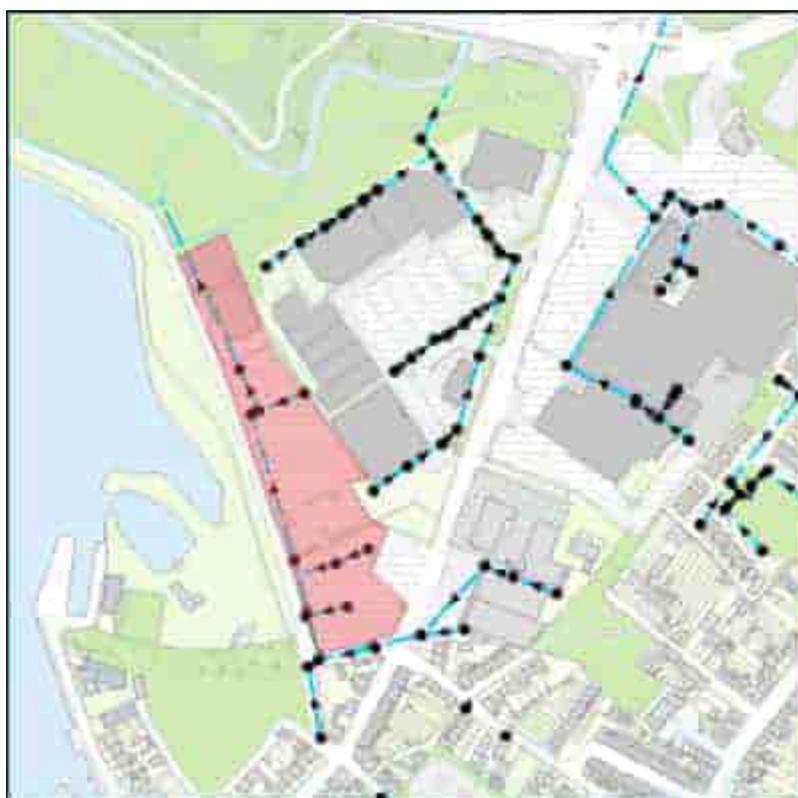


Figure 2-1: Existing Surface Water Infrastructure, UÉ 2024

2.2 Proposed Surface Water Drainage

It is proposed to install a new separate gravity surface water drainage network to serve the development which will discharge to the existing GCC 525mm Ø concrete pipe network which runs from south – north along the western boundary of the site and ultimately discharges to the Terryland Stream. The discharge flow will be limited to 25 l/s by providing two (2) shallow Reinforced Concrete (RC) attenuation tanks with a hydrobrake installed at each outfall manhole. It is also proposed to incorporate SuDS measures such as an area of Intensive Green Roof, exfiltration permeable paving and lengths of raingarden / swales.

Separate connections to the existing surface water network will be provided on the downstream end of each of the attenuation tanks.

Further, it is proposed to divert the surface water sewer which serves the retail development on the Headford Road around the new building and then reconnect it to the 525mm Ø concrete pipe. It is also proposed to relay circa ±155m of the GCC 525mm Ø concrete trunk surface water pipe that runs through the site.

Preliminary investigations undertaken by GCC in 2025 indicate sections of the pipe north of the Phase 1 site (on Phase 3 lands) may require repairs. GCC has confirmed it will carry out any repairs (as may be required) to ensure the Phase 1 development can connect to the existing surface water infrastructure.

Treatment of Run-Off

The quality of the surface water run-off will be much improved from the current situation with the inclusion of the green roofs, exfiltration permeable paving and lengths of raingarden / swales. In addition to these measures, it is proposed to provide a Class I By-Pass hydrocarbon separator upstream of the main development attenuation tanks to remove any hydrocarbons suspended in the site run-off prior to connection to the 525mm Ø concrete pipe.

2.2.1 Design Criteria

The design of residential developments would normally be based on Section 11.3 of the Department of Environment, Heritage and Local Government Recommendations for Site Development Works for Residential Areas and the requirements of Galway City Council.

The following design standards and guidelines have been followed in the design of the proposed surface water drainage network for the site:

- BS EN 752 – Drains and sewer system outside buildings.
- Greater Dublin Strategic Drainage Study (GDSDS) Volume 2 – New Developments.
- BS EN 858-2 – Separator System for Light Liquids (e.g. oil and petrol).
- Pipe network has been designed to ensure no surcharging during a 1 in 5-year return period rainfall event.
- No pipe flooding during a 1 in 100-year return period rainfall event.
- Surface water storage sized based on a 1 in 30-year return period rainfall event.
- An additional 20% has been allowed for climate change in relation to rainfall intensities.
- The following design criteria have been used in the design of the proposed surface water drainage network:

- Carrier pipe network – 1.0m/s to 3.0m/s.
- Colebrook White roughness value of 0.6mm for all pipework.
- Time of entry: 4 minutes.
- Return Period: 5 years.
- Met Eireann rainfall data for site.
- $M5/60 = 17.2$ mm.
- Ratio $r = 0.3$.

The following drawings illustrate the proposed surface water drainage network to serve the site:

- 60710277-ACM-XX-XX-DR-CE-00-0500,
- 60710277-ACM-XX-XX-DR-CE-00-0501, and
- 60710277-ACM-XX-XX-DR-CE-00-0502.

Please refer to **Appendix B** for the proposed surface water network calculations.

2.3 Surface Water Attenuation

'The SuDS Manual' published by CIRIA Document No. C753, was utilised for the management of surface water run-off within the development. This document promotes the use of a variety of alternative measures in the design of sustainable drainage systems, which take into account quantity, quality, and amenity. SuDS encourages the use of soft systems that replicate as far as possible, the natural treatment of surface water and attenuation storage to manage surface water run-off.

Given the existing site is 100% impermeable, the introduction of SuDS measures to manage surface water run-off will result in an improvement in the quantity & quality of run-off discharged from the site. It is proposed to restrict the rate of run-off discharged from the proposed development to 25 l/s. During a 1 in 5-year return period rainfall event, the rate of run-off discharge from the existing site/ car park is approximately 130l/s, resulting in a reduction of ~80% in the proposed scenario.

Several engineering challenges exist with regards the introduction of surface water attenuation.

Ground Conditions;

The ground conditions encountered during the ground investigation are summarised below with reference to in situ and laboratory test results. The full details of the strata encountered during the ground investigation are provided in the exploratory hole logs included in the appendices of the ground investigation report compiled by Ground Investigation Ireland (GII) Limited.

The sequence of strata encountered were generally consistent across the site and generally comprised;

- Surfacing,
- Made Ground,
- Organic Deposits,
- Soft Cohesive Deposits,

- Cohesive Deposits,
- Granular Deposits,
- Bedrock.

While the Ground Investigation (GI) confirmed that the investigation did not come across any karst features, the possibility of karst features was identified in a Geophysical Survey (GS) undertaken by Minerex Geophysics Limited.

The ground conditions are extremely poor, resulting from a combination of high-water table, and very soft silts and peat to depths of circa 6m. Further, soakaway testing took place and confirmed the water level dropped too slowly to allow calculation of 'f', the soil infiltration rate. Therefore, the ground conditions preclude the use of infiltration systems.

Refer to **Appendix E** for the Ground Investigation and Geophysical Survey reports.

Existing Networks;

The invert level of the existing surface water drainage and bed depth of the Terryland Stream are both relatively shallow compared with the subject site. The limited cover precludes the use of crate systems or hydro chamber systems to provide attenuation storage of run-off.

Considering these engineering constraints and as noted above, it is proposed to limit the surface water run-off from the site to 25 l/s by providing two (2) shallow Reinforced Concrete (RC) attenuation tanks with a hydrobrake installed at each outfall manhole. It is also proposed to incorporate SuDS measures such as an area of Intensive Green Roof, exfiltration permeable paving and lengths of raingarden / swales.

2.3.1 Attenuation Storage

The estimated surface water storage was calculated using the "*hr Wallingford Storm Water Storage Estimation Online Tool*". **Table 2-1** below illustrates the design criteria used to complete the calculation.

The total storage required has been sized based on the volume required (**204m³**) to store run-off from the development site during a 1 in 100 – year return period rainfall event. A 20% increase in rainfall intensities as a result of the impacts of climate changes has also been incorporated in this design.

Table 2-1: hr Wallingford Storm Water Storage Estimation Online Tool Design Criteria

Design Criteria

| | |
|--------------------------------------|-----------------------------------|
| Methodology | IH124 |
| Q _{BAR} Estimation Method | Specify Q _{BAR} Manually |
| Q _{BAR} for total Site Area | 25 l/s |
| SOIL Type | 4 |
| SPR | 0.47 |
| Climate Change Allowance Factor | 1.2 |
| Interception Rainfall Depth | 5mm |
| Minimum Flow Rate | 2 l/s |

| | |
|--------------------------|------|
| SAAR | 1281 |
| M5-60 Rainfall Depth | 17mm |
| 'r' Ratio M5-60/M5-2 day | 0.3 |

The proposed intensive green/ blue roof substructure can be seen in **Table 2-2** below. It was agreed that the Intensive Green Roof will be utilized for this development as the available roof space of 1799m² will yield the required storage of 131.2 m³. The remaining 72.8 m³ will be stored in the two (2) RC Attenuation Tanks.

Table 2-2: Proposed Intensive Green/Blue Roof Substructure

| Green Roof | Blue Roof |
|---|---|
| Roof Area Required | Roof Area Required |
| Intensive green roof with 300 mm deep substrate | 100 mm deep substrate for planting and 100 mm deep attenuation cell |
| BauderGREEN intensive landscape system | BauderBLUE STORMCell System |



2.3.2 Rainwater Harvesting

Rainwater harvesting has been considered by the AECOM design team; the concept was further discussed with GCC. It is advised that the small roof footprint to water demand ratio didn't lend itself to rainwater harvesting. RWH is much more effective for developments with a large roof footprint and small demand.

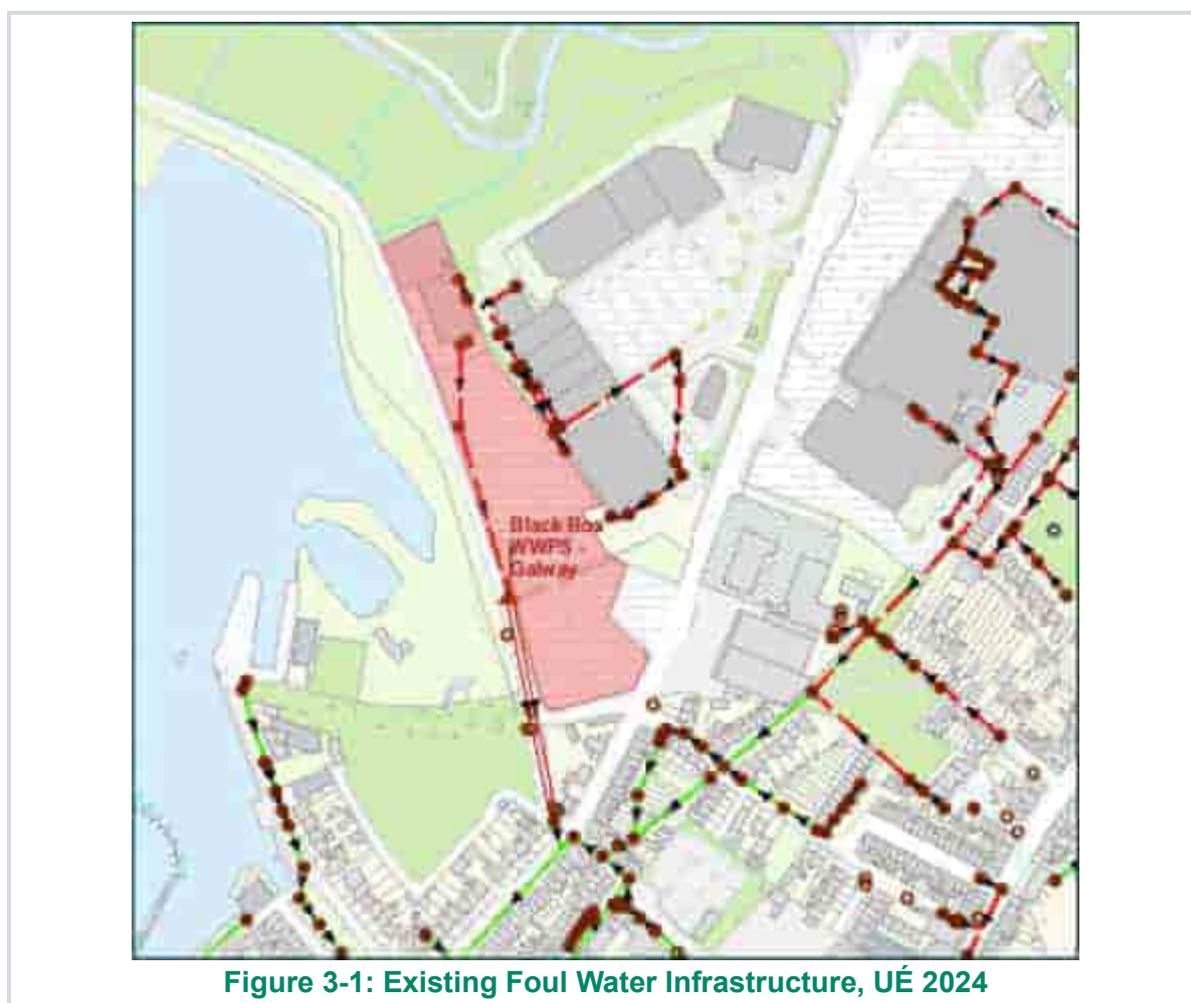
3. Wastewater Drainage

3.1 Existing Wastewater Drainage

Based on the record drawings from Uisce Éireann and a utility survey, the existing Black Box Theatre is serviced with a foul sewer that runs south to a pumping station (“Black Box WWPS”) located close to the current main entrance to the Dyke Road car park. From there, with a chamber invert level of 2.3m, the sewage is pumped further south along Dyke Road until it joins a combined sewer network on Headford Road at a level of 6.8m. The Headford Road retail park to the east of the site has a foul gravity sewer running from north to south to a pumping station.

The utility records and utility survey are included in **Appendix D**.

Refer to **Figure 3-1** for the layout of foul sewer infrastructure in the vicinity of the site.



3.1.1 Uisce Éireann Pre-Connection Enquiry

Pre-Connection Enquiries were submitted to Uisce Éireann on 16th of February 2024 for;

- Phase 1 only,
- Phases 1 & 2, and

- Phases 1 to 3.

Confirmation of Feasibility (CoF) letters for all 3 phases of development have been received and the CoF for Phase 1 is discussed in section 3.2.2 and is included in Appendix A.

Uisce Éireann have confirmed that the existing Dyke Road waste water pumping station was designed to cater only for the Black Box Theatre and that it doesn't have capacity to cater for any additional flows.

3.1.2 CCTV Survey

A CCTV survey of the existing wastewater drainage network within the proposed site was carried out to confirm the pipe routes and confirm the condition of the existing network. The CCTV was issued to UÉ as part of the wastewater diversion application.

3.2 Proposed Wastewater Drainage

Based on the Confirmation of Feasibility and the contents of such noted in **Section 3.2.2**. It is proposed to relay the gravity foul sewer serving the Black Box Theatre and install a new gravity sewer network to serve the development. The existing wastewater pumping station (WWPS) that serves the Black Box Theatre is to be decommissioned and a new WWPS constructed.

The new WWPS has been positioned based on the flood extents within the site and to maximize the separation from buildings. An emergency tank with 24-hour storage capacity at DWF has been provided to serve Phase 1 development and the Black Box Theatre.

The existing 150mm rising main serving the existing WWPS is to be retained and reused. Uisce Éireann have confirmed that a 20m upgrade of a 150mm diameter sewer from Dyke Road to Wood Quay will be required. These works will be undertaken by UÉ.

3.2.1 Foul Pumping Station and Emergency Storage

The foul pumping station has been positioned based on the flood extents within the site. The pumping station is located so that it is above the 1 in 100-year return period event water level and as far away from all buildings as possible. In addition, the above ground elements (kiosk and control room) are located above the 1:200-year return period. IW/UÉ's minimum separation distance to be provided between pumping stations and habitable buildings is 15m which can be achieved within the site.

An emergency tank with 24-hour storage capacity at DWF has been provided to serve Phase 1 development and the Black Box Theatre.

The existing rising main is to be utilised along Dyke Road, and thus a new 25m section is to be laid running from the new WWPS to the existing rising main.

3.2.2 Confirmation of Feasibility

The Confirmation of Feasibility (CoF) was received on the 23rd of May 2024. The wastewater connection is confirmed to be feasible subject to upgrades. This letter noted the following in relation to the wastewater drainage network:

Please note, the existing Black Box pumping station would require upgrading to cater for the wastewater loading and to provide adequate emergency storage for the proposed development. Alternatively, a new Pump Station could be constructed by the customer within their own site, the new pump station (WWPS) should be designed to cater for the proposed development wastewater loading. The existing Black Box pumping station shall be decommissioned with this option, and the existing flows diverted to the new WWPS. The customer will be responsible for designing the new WWPS required for the proposed development and the diverted flows that currently discharge to the existing Black Box Pumping station.

A 20m Approx. foul sewer network upgrade from 150mm diameter to 225mm diameter minimum is required to cater the proposed development at the start of Dyke Road and Wood quay. Any such network upgrade would have to be funded by the customer.

The developer should investigate the separation of storm water and foul on their development lands and that any existing storm water which is entering into the Uisce Éireann (UE) combined system will need to be eliminated. There should be no storm water discharge to the UE network.

Please note that according to our records there is an existing sewer running through this site (see drawing below). Any structures or works over or in close proximity to Uisce Éireann infrastructure that will inhibit access for maintenance or endanger structural or functional integrity of the infrastructure are not allowed. The layout of the development must ensure that this pipe is protected, and adequate separation distances are provided between Uisce Éireann infrastructure and any structures on site. Alternatively, you may enter into a diversion agreement with Uisce Éireann and divert the pipe to accommodate your development. If you wish to proceed with this option, please contact Uisce Éireann at Diversions@water.ie and submit detailed design drawings before submitting your planning application. It will be necessary to provide a wayleave over this pipe to the benefit of Uisce Éireann and ensure that it is accessible for maintenance.

Confirmation of Feasibility for the diversion of the wastewater pipework serving the Black Box Theatre and decommissioning of the existing Dyke Road wastewater pumping station has also been received and is included in Appendix A.

3.2.3 Statement of Design Acceptance

A Statement of Design Acceptance (SoDA) has been issued by UÉ. Refer to Appendix A for details.

3.2.4 Wastewater loading

Three (3) separate PCE applications were submitted to UÉ to determine if there is capacity within the existing Uisce Éireann wastewater infrastructure to cater for the increased load associated within the increase in residents.

3.2.4.1 Phase 1

Phase 1: The Dry Weather Flow (DWF) has been calculated as 1.132 l/s while the peak daily flow (taken as 6 times the Dry Weather Flow) has been calculated as 6.800 l/s. Refer to **Table 3-1** below.

Table 3-1: Phase 1 Wastewater Loading

| Source | Unit | Quantity | Flow (l/day/unit or l/s/ha) | Daily (l/day) | DWF (m ³ /day) | DWF (l/s) | 6xDWF (l/s) | Avg Day / Peak Wk demand (l/s) | Peak Wk Demand (l/s) |
|-----------------------------------|------|----------|-----------------------------------|------------------|------------------------------|--------------|----------------|--|----------------------------|
| Residential Apartments | Pers | 646 | 150 | 96900 | 96.9 | 1.12 | 6.73 | 1.40 | 2.94 |
| Creche | Pers | 20 | 50 | 1000 | 1.00 | 0.012 | 0.069 | 0.014 | 0.030 |
| Total | | | | 97900 | 97.9 | 1.132 | 6.800 | 1.414 | 2.970 |

3.2.4.2 Phase 2

Phase 2: The Dry Weather Flow (DWF) has been calculated as 1.208 l/s while the peak daily flow (taken as 6 times the Dry Weather Flow) has been calculated as 7.248 l/s. Refer to **Table 3-2** below.

Table 3-2: Phase 2 Wastewater Loading

| Source | Unit | Quantity | Flow (l/day/unit or l/s/ha) | Daily (l/day) | DWF (m ³ /day) | DWF (l/s) | 6xDWF (l/s) | Avg Day / Peak Wk demand (l/s) | Peak Wk Demand (l/s) |
|--|-----------|----------|-----------------------------------|------------------|------------------------------|--------------|----------------|--|----------------------------|
| Commercial Area @ 0.6 l/s/ha: | Ha | 0.199 | 0.6 | 10310.96 | 10.31 | 0.119 | 0.72 | 0.149 | 0.313 |
| Community Room (1989 m ³) | | | | | | | | | |
| Hotel | Pers | 360 | 250 | 90000 | 90.00 | 1.042 | 6.25 | 1.302 | 2.734 |
| Hotel FTE | Pers | 60 | 60 | 3600 | 3.60 | 0.042 | 0.25 | 0.052 | 0.109 |
| Retail GIA | Employees | 15 | 30 | 454.50 | 0.455 | 0.005 | 0.03 | 0.007 | 0.014 |
| Total | | | | 105462.98 | 105.46 | 1.208 | 7.248 | 1.510 | 3.171 |

3.2.4.3 Phase 3

Phase 3: The Dry Weather Flow (DWF) has been calculated as 0.33 l/s while the peak daily flow (taken as 6 times the Dry Weather Flow) has been calculated as 1.98 l/s. Refer to **Table 3-3** below.

Table 3-3: Phase 3 Wastewater Loading

| Source | Unit | Quantity | Flow (l/day/unit or l/s/ha) | Daily (l/day) | DWF (m ³ /day) | DWF (l/s) | 6xDWF (l/s) | Avg Day / Peak Wk demand (l/s) | Peak Wk Demand (l/s) |
|-----------------------------------|------|----------|-----------------------------------|------------------|------------------------------|--------------|----------------|--|----------------------------|
| Residential Apartments | Pers | 190 | 150 | 28500 | 28.5 | 0.33 | 1.98 | 0.41 | 0.87 |

The proposed wastewater drainage network has been designed to achieve self-cleansing velocities of 0.75 m/s, as set out in Section 3.6 of the Code of Practice. The proposed network has been designed to convey between 2.5 and 6 times the DWF.

3.2.5 Wastewater Network Design

The following design standards and guidelines have been followed in the design of the proposed wastewater drainage network for the site:

- Uisce Éireann Code of Practice for Wastewater Infrastructure (IW-CDS-5030-03),
- BS EN 752 – Drains and sewer systems outside buildings,
- Sewers for Adoption, 6th Edition,
- Micro Drainage Software Pipeline Design,
- All pipework will be provided at gradients that will provide velocities in the range of 0.75 m/ sec and 2.5 m/sec, when flowing half full,
- Part H Building Regulations.

The following drawings illustrate the proposed wastewater drainage network and diversions associated with the proposals:

- 60710277-ACM-XX-XX-DR-CE-00-0500,
- 60710277-ACM-XX-XX-DR-CE-00-0501,
- 60710277-ACM-XX-XX-DR-CE-00-0502 and
- 60710277-ACM-XX-XX-DR-CE-00-0503.

Please refer to Appendix C for more information on the wastewater drainage network design calculations.

4. Water Supply

4.1 Existing Water Supply Infrastructure

Utility records were obtained from Uisce Éireann (UÉ) to indicate existing watermains in the vicinity of the site. The location of the watermain was verified with a utility survey.

A 9" cast-iron watermain runs along Dyke Road. From this watermain, a water connection feeds the Black Box theatre and the Headford Road shopping centre. A 300mm asbestos-cement watermain runs in Headford Road and Bóthar Na Dige Road, while a shorter section of 100mm uPVC water distribution main runs along a short section of Headford Road.

The utility records and utility survey are included in **Appendix D**.

Refer to **Figure 4-1** for the layout of potable water infrastructure in the vicinity of the site.

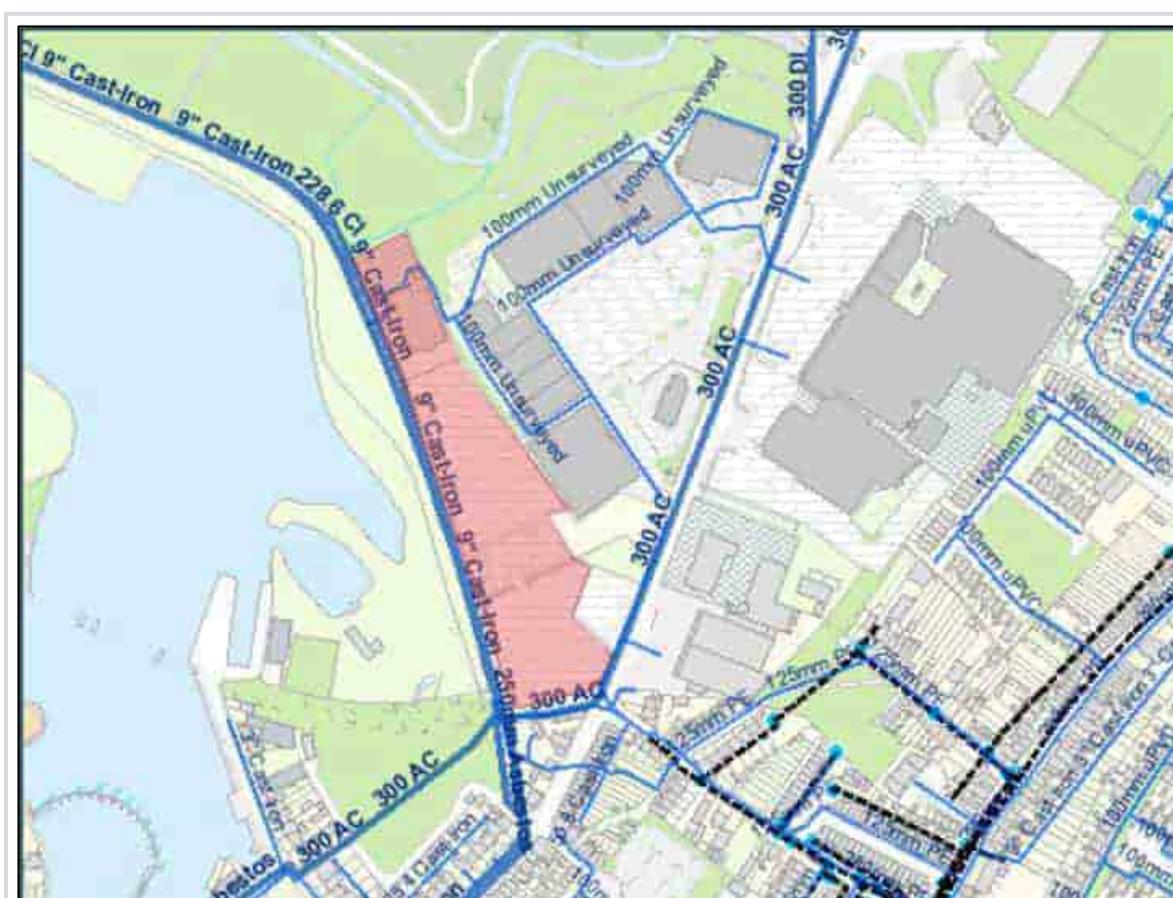


Figure 4-1: Existing Watermain Infrastructure, UÉ 2024

4.1.1 Uisce Éireann Pre-Connection Enquiry

Pre-Connection Enquiries have been submitted to Uisce Éireann for

- Phase 1 only,
- Phases 1 & 2, and
- Phases 1 to 3.

On-site water storage for firefighting may be required by the Galway City Fire Service.

4.2 Proposed Water Supply Network

It is proposed to take a connection off the 250mm asbestos watermain on the Dyke Road. The new watermain will pass through the phase 1 lands and loop around 3 sides (south, east, and north) of the development.

The following drawings illustrate the proposed watermain network within the site:

- 60710277-ACM-XX-XX-DR-CE-00-2700,
- 60710277-ACM-XX-XX-DR-CE-00-2701, and
- 60710277-ACM-XX-XX-DR-CE-00-2702.

The internal water supply network is based on the requirements of the Uisce Éireann Code of Practice for Water Supply and the Technical Guidance Document – Part B of the Building Regulations 2006:

- Hydrants are positioned within 46m of the proposed buildings.
- A bulk water meter has been provided at the connection to the existing Uisce Éireann distribution watermain at the site entrance.
- No new watermain up to and including 150mm in diameter shall be laid within 3m of an existing or proposed structure. See Section 3.5.9 of Water Code of Practice.

Uisce Éireann will be consulted with regard network upgrades to accommodate future demand and consolidate the resilience of the local network.

Firefighting water supplies and fire hydrants will be provided as required in accordance with the Building Regulations and the requirement of Galway City Fire Service.

4.2.1 Confirmation of Feasibility

The Confirmation of Feasibility (CoF) was received on the 23rd of May 2024. The water connection is confirmed to be feasible without infrastructure upgrade by Uisce Éireann (UÉ). This letter noted the following in relation to the water network:

The water main along Dyke Road is the preferred connection point for the proposed development.

4.2.2 Statement of Design Acceptance

A Statement of Design Acceptance (SoDA) has been issued by UÉ. Refer to Appendix A for details.

5. Access and Traffic

The existing access to the development and car park will be decommissioned and a new access to the proposed development will form as part of the proposed works. Please refer to AECOM Drawing **60710277-ACM-XX-XX-DR-CE-00-0002 & 60710277-ACM-XX-XX-DR-CE-00-0003** for the location of the access.

Please refer to the project Traffic and Transport Assessment and Mobility Management Plan reports compiled by PUNCH for further information on site accessibility.

6. Flood Risk Assessment

As part of the preliminary design of the proposed development, AECOM prepared a site-specific Stage 3 Flood Risk Assessment which describes the flood risks to the site and addresses the requirements of Galway City Development Plan (2023– 2029). This FRA has been prepared in accordance with the *Planning System and Flood Risk Management Guidelines*, (OPW, 2009).

Appendix A Uisce Éireann

- 7. Confirmation of Feasibility for Phase 1 Development**
- 8. Statement of Design Compliance (SoDA)**
- 9. Confirmation of Feasibility for Wastewater Diversion**

CONFIRMATION OF FEASIBILITY

Sumana Jain Kelly
Aecom
1st Floor Montrose
Carrigaline Road
Douglas
Cork
T12H90H

11 March 2025

Uisce Éireann
Bosca OP 448
Oifig Sheachadta na
Cathrach Theas
Gathair Chorcaí

Uisce Éireann
PO Box 448
South City
Delivery Office
Cork City

www.water.ie

Our Ref: CDS24001446 Pre-Connection Enquiry
Site at, Dyke Road Car Park, Galway City, Galway

Dear Applicant/Agent,

We have completed the review of the Pre-Connection Enquiry.

Uisce Éireann has reviewed the pre-connection enquiry in relation to a Water & Wastewater connection for a Housing Development of 219no. unit(s) and 1no. Creche unit at Site at, Dyke Road Car Park, Galway City, Galway, (the **Development**).

Based upon the details provided we can advise the following regarding connecting to the networks;

- **Water Connection**
 - Feasible without infrastructure upgrade by Irish Water
 - **Water Treatment Plant**
There is sufficient capacity for the proposed development.
- **Wastewater Connection**
 - Feasible Subject to upgrades
 - **Wastewater Treatment Plant**
There is sufficient capacity for the proposed development.

Water Network

The water main along Dyke Road is the preferred connection point for the proposed development.

Sumana Jain Kelly
1st Floor Montrose
Carrigaline Road
Douglas, Cork T12H90H

Uisce Éireann
Bosca OP-448
Oifig Sheachadta na
Cathrach Theas
Cathair Chorcaí

9 October 2024

**Re: Design Submission for Site at, Dyke Road Car Park, Galway City, Galway (the “Development”)
(the “Design Submission”) / Connection Reference No: CDS24001446**

Uisce Éireann
PO Box 448
South City
Delivery Office
Cork City
www.water.ie

Dear Sumana Jain Kelly,

Many thanks for your recent Design Submission.

We have reviewed your proposal for the connection(s) at the Development. Based on the information provided, which included the documents outlined in Appendix A to this letter, Uisce Éireann has no objection to your proposals.

This letter does not constitute an offer, in whole or in part, to provide a connection to any Uisce Éireann infrastructure. Before you can connect to our network you must sign a connection agreement with Uisce Éireann. This can be applied for by completing the connection application form at www.water.ie/connections. Uisce Éireann’s current charges for water and wastewater connections are set out in the Water Charges Plan as approved by the Commission for Regulation of Utilities (CRU) (https://www.cru.ie/document_group/irish-waters-water-charges-plan-2018/).

You the Customer (including any designers/contractors or other related parties appointed by you) is entirely responsible for the design and construction of all water and/or wastewater infrastructure within the Development which is necessary to facilitate connection(s) from the boundary of the Development to Uisce Éireann’s network(s) (the “**Self-Lay Works**”), as reflected in your Design Submission. Acceptance of the Design Submission by Uisce Éireann does not, in any way, render Uisce Éireann liable for any elements of the design and/or construction of the Self-Lay Works.

If you have any further questions, please contact your Uisce Éireann representative:

Name: Kyle Jackson

Email: kyle.jackson@water.ie

Yours sincerely,



Dermot Phelan
Connections Delivery Manager

Stiúrthóirí / Directors: Tony Keohane (Cathaoirleach / Chairman), Niall Gleeson (POF / CEO), Christopher Banks, Fred Barry, Gerard Britchfield, Liz Joyce, Patricia King, Eileen Maher, Cathy Mannion, Michael Walsh.

Oifig Chláraithe / Registered Office: Teach Colvill, 24-26 Sráid Thalbóid, Baile Átha Cliath 1, D01 NP86 / Colvill House, 24-26 Talbot Street, Dublin, Ireland D01NP86

Is cuideachta ghníomhaíochta ainmnithe atá faoi theorainn scaireanna é Uisce Éireann / Uisce Éireann is a design activity company, limited by shares.
Cláraithe in Éirinn Uimh.: 530363 / Registered in Ireland No.: 530363.

Appendix A

Document Title & Revision

- [60710277-ACM-XX-XX-DR-CE-00-0500_P3_01_ProposedDrainage]
- [60710277-ACM-XX-XX-DR-CE-00-0501_P3_01_ProposedDrainage]
- [60710277-ACM-XX-XX-DR-CE-00-0502_P3_01_ProposedDrainage]
- [60710277-ACM-XX-XX-DR-CE-00-0503_P3_01_ProsposedWWPS]
- [60710277-ACM-XX-XX-DR-CE-00-0500_P1_01_ ProsposedWWPS]
- [60710277-ACM-XX-XX-DR-CE-00-0501_P3_01_ProposedWater]
- [60710277-ACM-XX-XX-DR-CE-00-0500_P3_01_ProposedWater01]
- [60710277-ACM-XX-XX-DR-CE-00-0501_P3_01_ProposedWater02]

Standard Details/Code of Practice Exemption:

Not applicable

For further information, visit www.water.ie/connections

Notwithstanding any matters listed above, the Customer (including any appointed designers/contractors, etc.) is entirely responsible for the design and construction of the Self-Lay Works. Acceptance of the Design Submission by Uisce Éireann will not, in any way, render Uisce Éireann liable for any elements of the design and/or construction of the Self-Lay Works.

- THIS DRAWING IS TO BE READ IN CONJUNCTION WITH ALL OTHER RELEVANT ARCHITECTURAL AND ENGINEERING DRAWINGS. ANY DISCREPANCIES, ERRORS OR OMISSIONS TO BE BROUGHT TO THE ATTENTION OF THE DESIGNER.
- ALL DIMENSIONS TO BE CHECKED BY THE CONTRACTOR ON SITE PRIOR TO COMMENCEMENT OF WORKS.
- AECOM LIMITED TO BE INFORMED BY THE CONTRACTOR OF ANY DISCREPANCIES PRIOR TO THE COMMENCEMENT OF WORKS ON SITE.
- DIMENSIONS OF ALL BOUNDARIES AND ADJOINING ROADS TO BE CHECKED ON SITE PRIOR TO COMMENCEMENT OF WORKS.
- DO NOT SCALE. ALL MEASUREMENTS AND COORDINATES TO BE CHECKED ON SITE.
- THE LOCATION & DEPTH OF SERVICES TO BE CHECKED ON SITE PRIOR TO COMMENCING ANY WORKS.
- MANHOLE COVERS AND FRAMES IN PUBLICLY ACCESSIBLE AREAS SHALL BE HEAVY DUTY CAST IRON, CLASS D400, DOUBLE SEALED AND LOCKABLE TYPE COMPLYING WITH BS EN 124-2:2015.
- GULLY GRATINGS & FRAMES SHALL COMPLY WITH BS EN 124-2:2015.
- EXISTING INVERT LEVELS TO BE VERIFIED ON SITE BEFORE COMMENCING CONSTRUCTION.
- SURFACE WATER & FOUL SEWER PIPES LESS THAN 1.2m BELOW THE ROAD SURFACE OR LESS THAN 0.9m IN NON-TRAFFICKED FOOTPATHS AND LANDSCAPE AREAS (WITH AN ABSOLUTE MINIMUM DEPTH OF COVER ABOVE THE EXTERNAL CROWN OF THE PIPE Ø750mm) SHALL BE PROTECTED FROM DAMAGE BY PROVIDING MINIMUM 150mm THICK CONCRETE C16/20 HAUNCH IN ACCORDANCE WITH IS EN 12620.
- CCTV SURVEY TO BE CONDUCTED PRIOR TO COMMENCEMENT OF ANY WORKS TO DETERMINE THE CONDITION AND VERIFY LEVELS OF THE EXISTING FOUL AND SURFACE WATER PIPES/ MANHOLES. ANY SUB-STANDARD OR DEFECTIVE ELEMENTS OF THE EXISTING PIPES/MANHOLES TO BE REPORTED AND CORRECTED.
- ALL SURFACE WATER DRAINAGE DETAILS TO BE IN ACCORDANCE WITH TI SPECIFICATIONS FOR ROAD WORKS.
- ALL FOUL DRAINAGE TO BE IN ACCORDANCE WITH THE IRISH WATER STANDARD DETAILS AND CODES OF PRACTICE.
- CONNECTIONS TO THE EXISTING DRAINAGE NETWORK REQUIRE APPROVAL FROM IRISH WATER & GALWAY CITY COUNCIL.
- REFER TO ARCHITECT DRAWINGS FOR ARCHITECTURAL DETAILS.
- REFER TO LANDSCAPE ARCHITECT DRAWINGS FOR LANDSCAPING DETAILS.

LEGEND

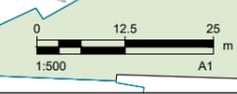
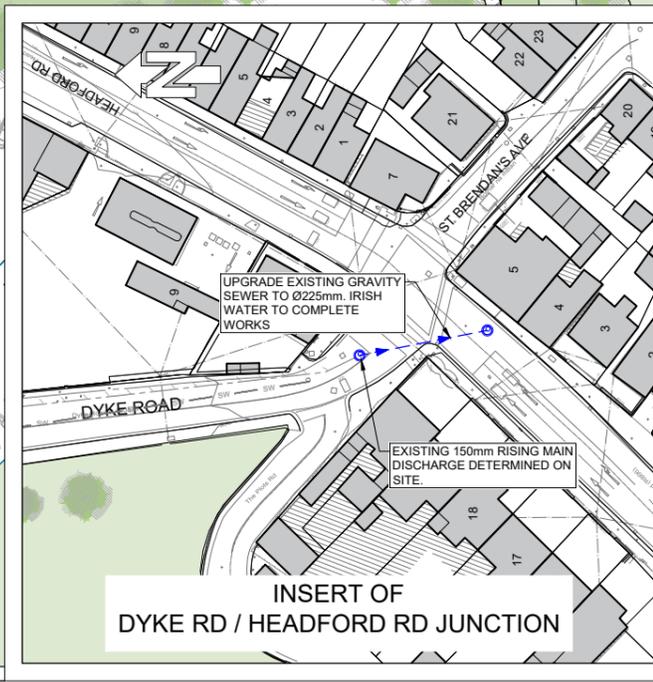
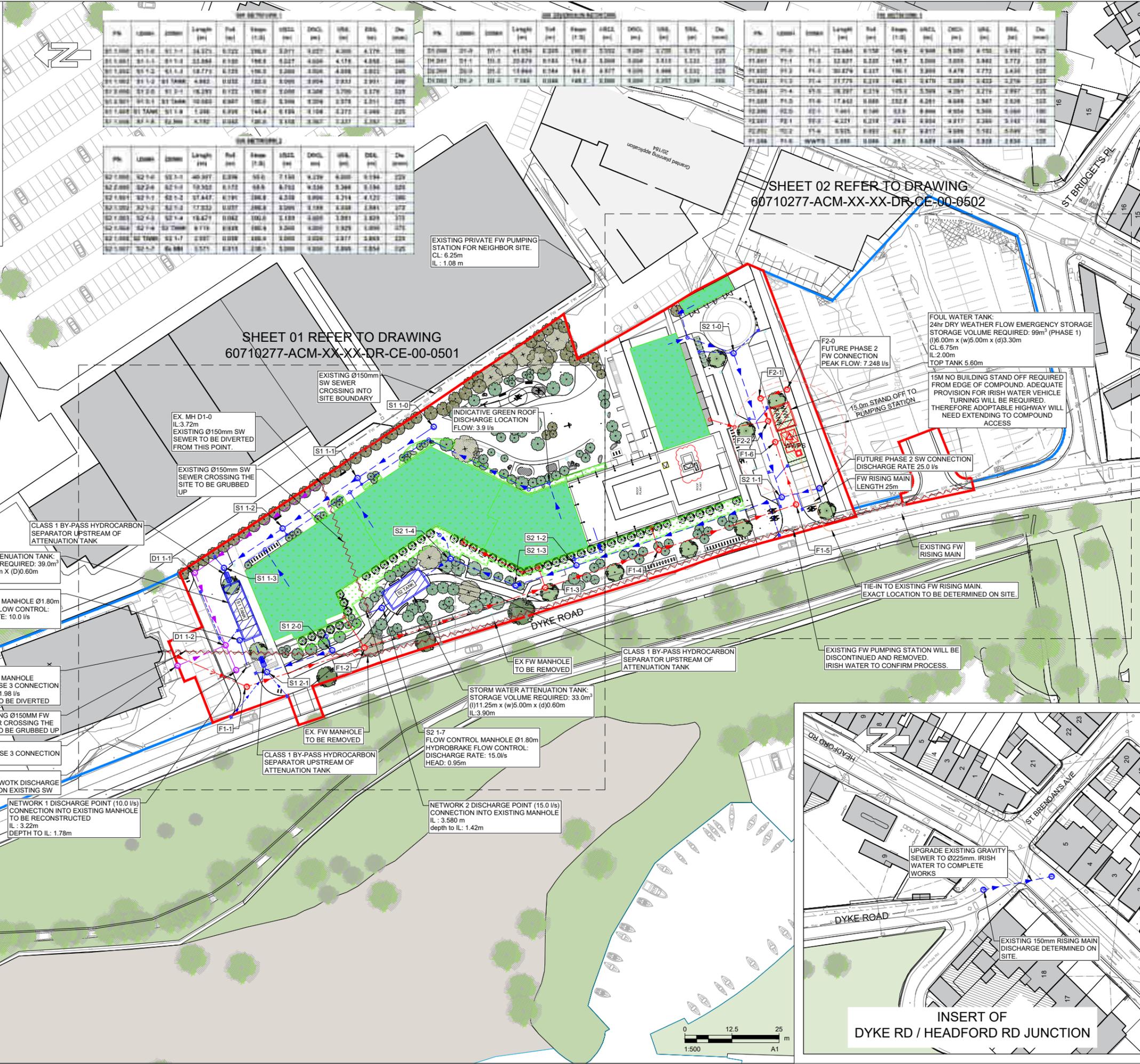
- PHASE 1 PLANNING APPLICATION BOUNDARY
- GCC OWNED LANDS
- EXISTING FW PIPE FROM RECORDS
- EXISTING SW PIPE FROM RECORDS
- EXISTING SW GULLY
- PROPOSED SW PIPE & MH
- PROPOSED SW PIPE & INSPECTION CHAMBER
- PROPOSED SW PIPE & MH DIVERSION
- PROPOSED GREEN ROOF
- PROPOSED RAIN GARDEN
- PROPOSED SW ATTENUATION
- PROPOSED SW GULLY
- PROPOSED FW PIPE & MH
- PROPOSED FW RISING MAIN
- PROPOSED FW ATTENUATION
- PROPOSED PUMPING STATION

| PK | LINE | START | END | LENGTH (M) | INVERT (M) | OUTLET (M) | DIAMETER (MM) | AREA (M ²) | PERIMETER (M) | DEPTH (M) | CL | IL |
|----------|---------|---------|---------|------------|------------|------------|---------------|------------------------|---------------|-----------|------|------|
| 01.1.001 | S1-1-1 | 01.1.00 | 01.1.01 | 1.00 | 1.00 | 1.00 | 150 | 0.00 | 0.00 | 0.00 | 6.25 | 5.25 |
| 01.1.002 | S1-1-2 | 01.1.01 | 01.1.02 | 1.00 | 1.00 | 1.00 | 150 | 0.00 | 0.00 | 0.00 | 6.25 | 5.25 |
| 01.1.003 | S1-1-3 | 01.1.02 | 01.1.03 | 1.00 | 1.00 | 1.00 | 150 | 0.00 | 0.00 | 0.00 | 6.25 | 5.25 |
| 01.1.004 | S1-1-4 | 01.1.03 | 01.1.04 | 1.00 | 1.00 | 1.00 | 150 | 0.00 | 0.00 | 0.00 | 6.25 | 5.25 |
| 01.1.005 | S1-1-5 | 01.1.04 | 01.1.05 | 1.00 | 1.00 | 1.00 | 150 | 0.00 | 0.00 | 0.00 | 6.25 | 5.25 |
| 01.1.006 | S1-1-6 | 01.1.05 | 01.1.06 | 1.00 | 1.00 | 1.00 | 150 | 0.00 | 0.00 | 0.00 | 6.25 | 5.25 |
| 01.1.007 | S1-1-7 | 01.1.06 | 01.1.07 | 1.00 | 1.00 | 1.00 | 150 | 0.00 | 0.00 | 0.00 | 6.25 | 5.25 |
| 01.1.008 | S1-1-8 | 01.1.07 | 01.1.08 | 1.00 | 1.00 | 1.00 | 150 | 0.00 | 0.00 | 0.00 | 6.25 | 5.25 |
| 01.1.009 | S1-1-9 | 01.1.08 | 01.1.09 | 1.00 | 1.00 | 1.00 | 150 | 0.00 | 0.00 | 0.00 | 6.25 | 5.25 |
| 01.1.010 | S1-1-10 | 01.1.09 | 01.1.10 | 1.00 | 1.00 | 1.00 | 150 | 0.00 | 0.00 | 0.00 | 6.25 | 5.25 |

| PK | LINE | START | END | LENGTH (M) | INVERT (M) | OUTLET (M) | DIAMETER (MM) | AREA (M ²) | PERIMETER (M) | DEPTH (M) | CL | IL |
|----------|---------|---------|---------|------------|------------|------------|---------------|------------------------|---------------|-----------|------|------|
| 02.1.001 | S2-1-1 | 02.1.00 | 02.1.01 | 1.00 | 1.00 | 1.00 | 150 | 0.00 | 0.00 | 0.00 | 6.25 | 5.25 |
| 02.1.002 | S2-1-2 | 02.1.01 | 02.1.02 | 1.00 | 1.00 | 1.00 | 150 | 0.00 | 0.00 | 0.00 | 6.25 | 5.25 |
| 02.1.003 | S2-1-3 | 02.1.02 | 02.1.03 | 1.00 | 1.00 | 1.00 | 150 | 0.00 | 0.00 | 0.00 | 6.25 | 5.25 |
| 02.1.004 | S2-1-4 | 02.1.03 | 02.1.04 | 1.00 | 1.00 | 1.00 | 150 | 0.00 | 0.00 | 0.00 | 6.25 | 5.25 |
| 02.1.005 | S2-1-5 | 02.1.04 | 02.1.05 | 1.00 | 1.00 | 1.00 | 150 | 0.00 | 0.00 | 0.00 | 6.25 | 5.25 |
| 02.1.006 | S2-1-6 | 02.1.05 | 02.1.06 | 1.00 | 1.00 | 1.00 | 150 | 0.00 | 0.00 | 0.00 | 6.25 | 5.25 |
| 02.1.007 | S2-1-7 | 02.1.06 | 02.1.07 | 1.00 | 1.00 | 1.00 | 150 | 0.00 | 0.00 | 0.00 | 6.25 | 5.25 |
| 02.1.008 | S2-1-8 | 02.1.07 | 02.1.08 | 1.00 | 1.00 | 1.00 | 150 | 0.00 | 0.00 | 0.00 | 6.25 | 5.25 |
| 02.1.009 | S2-1-9 | 02.1.08 | 02.1.09 | 1.00 | 1.00 | 1.00 | 150 | 0.00 | 0.00 | 0.00 | 6.25 | 5.25 |
| 02.1.010 | S2-1-10 | 02.1.09 | 02.1.10 | 1.00 | 1.00 | 1.00 | 150 | 0.00 | 0.00 | 0.00 | 6.25 | 5.25 |

| PK | LINE | START | END | LENGTH (M) | INVERT (M) | OUTLET (M) | DIAMETER (MM) | AREA (M ²) | PERIMETER (M) | DEPTH (M) | CL | IL |
|----------|---------|---------|---------|------------|------------|------------|---------------|------------------------|---------------|-----------|------|------|
| 03.1.001 | S3-1-1 | 03.1.00 | 03.1.01 | 1.00 | 1.00 | 1.00 | 150 | 0.00 | 0.00 | 0.00 | 6.25 | 5.25 |
| 03.1.002 | S3-1-2 | 03.1.01 | 03.1.02 | 1.00 | 1.00 | 1.00 | 150 | 0.00 | 0.00 | 0.00 | 6.25 | 5.25 |
| 03.1.003 | S3-1-3 | 03.1.02 | 03.1.03 | 1.00 | 1.00 | 1.00 | 150 | 0.00 | 0.00 | 0.00 | 6.25 | 5.25 |
| 03.1.004 | S3-1-4 | 03.1.03 | 03.1.04 | 1.00 | 1.00 | 1.00 | 150 | 0.00 | 0.00 | 0.00 | 6.25 | 5.25 |
| 03.1.005 | S3-1-5 | 03.1.04 | 03.1.05 | 1.00 | 1.00 | 1.00 | 150 | 0.00 | 0.00 | 0.00 | 6.25 | 5.25 |
| 03.1.006 | S3-1-6 | 03.1.05 | 03.1.06 | 1.00 | 1.00 | 1.00 | 150 | 0.00 | 0.00 | 0.00 | 6.25 | 5.25 |
| 03.1.007 | S3-1-7 | 03.1.06 | 03.1.07 | 1.00 | 1.00 | 1.00 | 150 | 0.00 | 0.00 | 0.00 | 6.25 | 5.25 |
| 03.1.008 | S3-1-8 | 03.1.07 | 03.1.08 | 1.00 | 1.00 | 1.00 | 150 | 0.00 | 0.00 | 0.00 | 6.25 | 5.25 |
| 03.1.009 | S3-1-9 | 03.1.08 | 03.1.09 | 1.00 | 1.00 | 1.00 | 150 | 0.00 | 0.00 | 0.00 | 6.25 | 5.25 |
| 03.1.010 | S3-1-10 | 03.1.09 | 03.1.10 | 1.00 | 1.00 | 1.00 | 150 | 0.00 | 0.00 | 0.00 | 6.25 | 5.25 |

ISO A1 594mm x 841mm
Approved: _____
Checked: _____
Designer: _____
Project Management Initials: _____
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Last saved by: PETER.SMITH (2024-06-28) Last Plotted: 2024-07-03
ORDNANCE SURVEY IRELAND LICENCE No. CYAL50405553
ORDNANCE SURVEY IRELAND / GOVERNMENT OF IRELAND

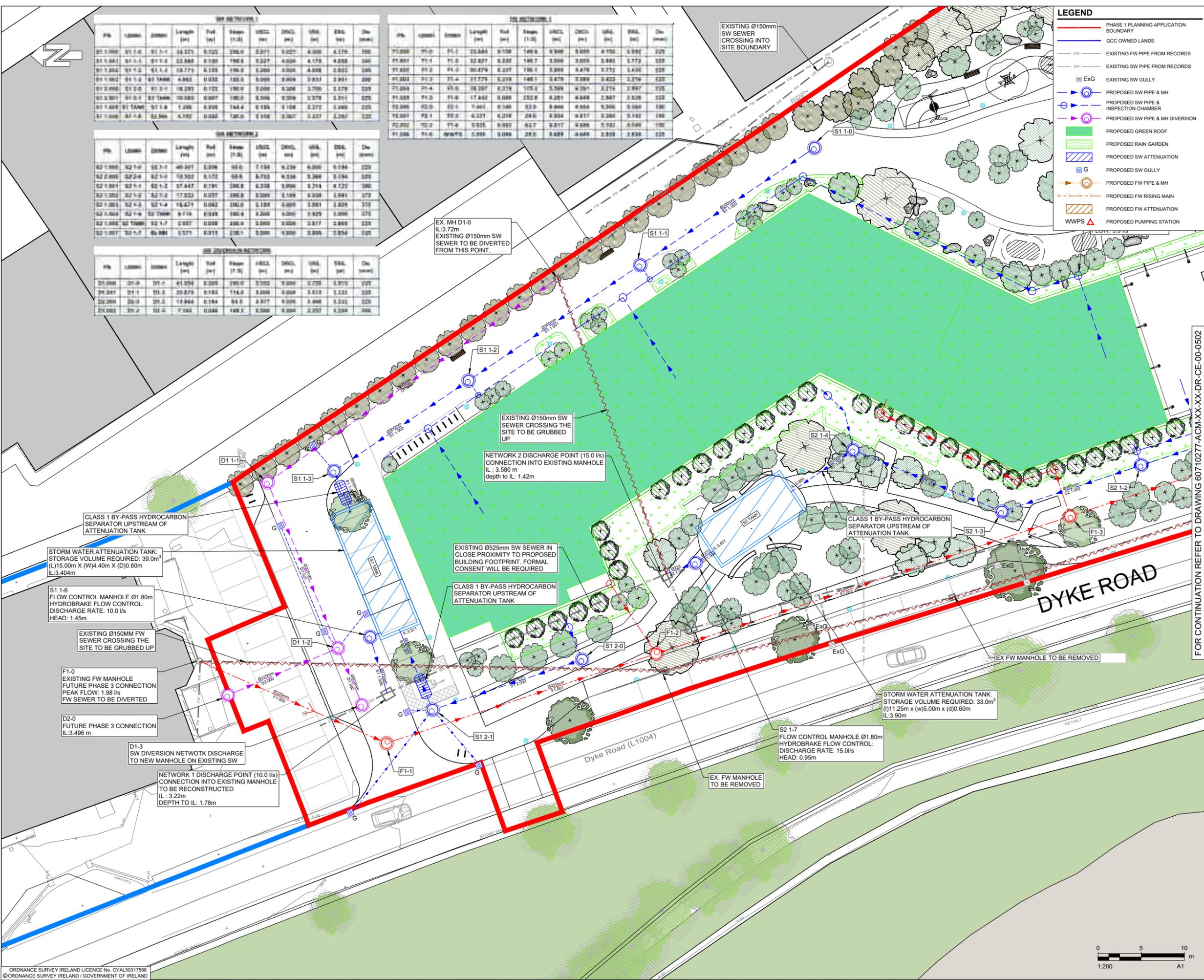


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 - CCTV SURVEY TO BE CONDUCTED PRIOR TO COMMENCEMENT OF ANY WORKS TO DETERMINE THE CONDITION AND VERIFY LEVELS OF THE EXISTING FOUL AND SURFACE WATER PIPES/ MANHOLES. ANY SUB-STANDARD OR DEFECTIVE ELEMENTS OF THE EXISTING PIPES/MANHOLES TO BE REPORTED AND CORRECTED.
 - ALL SURFACE WATER DRAINAGE DETAILS TO BE IN ACCORDANCE WITH TI SPECIFICATIONS FOR ROAD WORKS.
 - ALL FOUL DRAINAGE TO BE IN ACCORDANCE WITH THE IRISH WATER STANDARD DETAILS AND CODES OF PRACTICE.
 - CONNECTIONS TO THE EXISTING DRAINAGE NETWORK REQUIRE APPROVAL FROM IRISH WATER & GALWAY CITY COUNCIL.
 - REFER TO ARCHITECT DRAWINGS FOR ARCHITECTURAL DETAILS.
 - REFER TO LANDSCAPE ARCHITECT DRAWINGS FOR LANDSCAPING DETAILS.

ISSUE/REVISION

| IR | DATE | DESCRIPTION |
|----|------------|--------------|
| 01 | 05.07.2024 | FOR PLANNING |



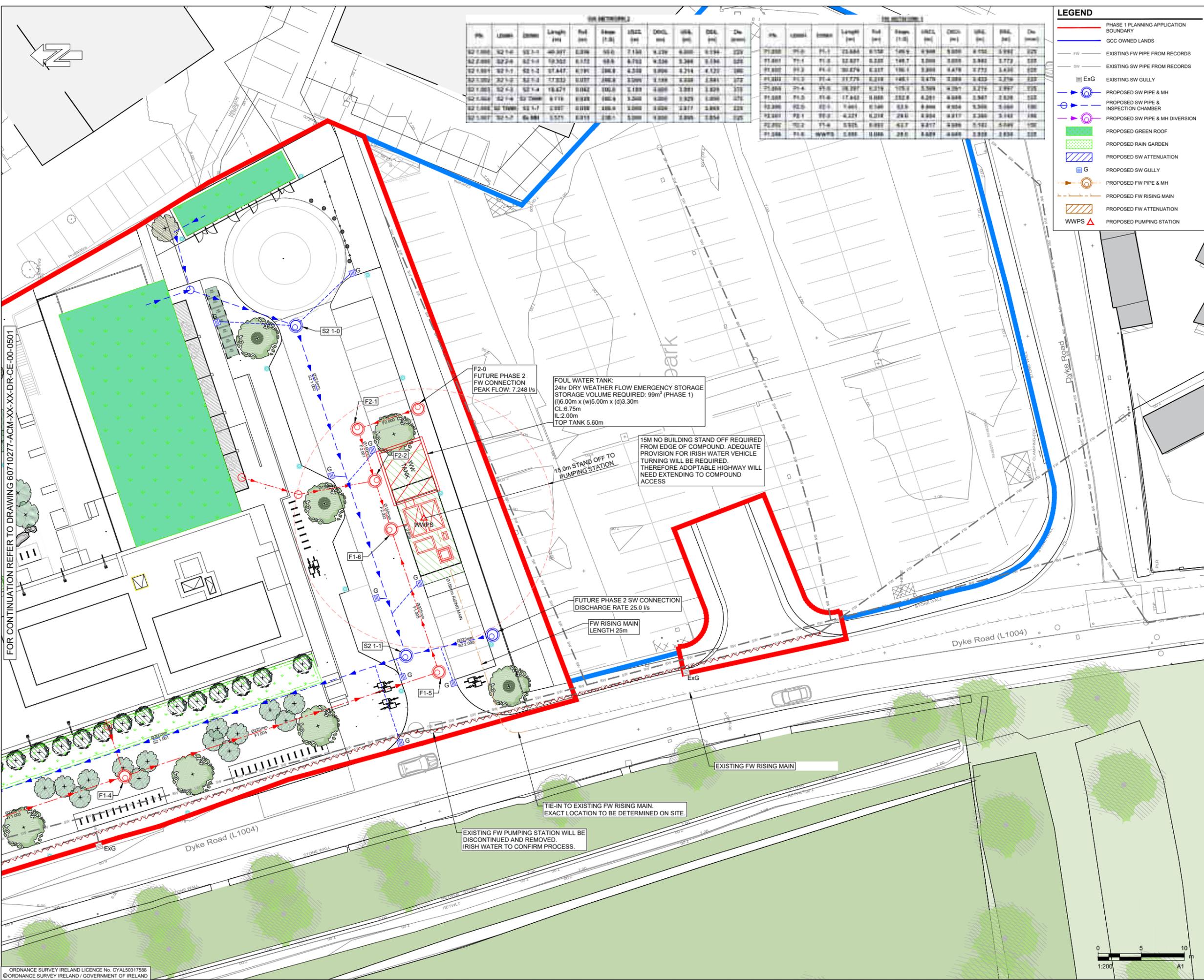
LEGEND

- PHASE 1 PLANNING APPLICATION BOUNDARY
- GCC OWNED LANDS
- EXISTING FW PIPE FROM RECORDS
- EXISTING SW PIPE FROM RECORDS
- EXISTING SW GULLY
- PROPOSED SW PIPE & MH
- PROPOSED SW PIPE & INSPECTION CHAMBER
- PROPOSED SW PIPE & MH DIVERSION
- PROPOSED GREEN ROOF
- PROPOSED RAIN GARDEN
- PROPOSED SW ATTENUATION
- PROPOSED SW GULLY
- PROPOSED FW PIPE & MH
- PROPOSED FW RISING MAIN
- PROPOSED FW ATTENUATION
- PROPOSED PUMPING STATION

FOR CONTINUATION REFER TO DRAWING 60710277-ACM-XX-XX-DR-CE-00-0502



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 Last saved by: PETER.SMITH (2024-06-28) Last Potted: 2024-07-03
 ISO A1 594mm x 841mm
 Designer: _____ Checked: _____ Approved: _____
 Project Management Initials: _____



PROPOSED MAINS

| PK | CHAIN | START | LENGTH (M) | PIPE DIA (MM) | DEPTH (M) | INVERT (M) | OUTLET (M) | VELOCITY (M/S) | Q (L/S) | Q (M³/S) | Q (M³/D) |
|---------|--------|--------|------------|---------------|-----------|------------|------------|----------------|---------|----------|----------|
| 62.7000 | 62.700 | 62.700 | 40.307 | 300 | 0.8 | 7.190 | 6.270 | 0.800 | 0.194 | 0.229 | 2.00 |
| 62.7000 | 62.700 | 62.700 | 19.302 | 150 | 0.8 | 6.702 | 6.240 | 0.300 | 0.104 | 0.120 | 1.00 |
| 62.7000 | 62.700 | 62.700 | 57.447 | 150 | 0.8 | 6.702 | 6.240 | 0.300 | 0.104 | 0.120 | 1.00 |
| 62.7000 | 62.700 | 62.700 | 77.323 | 150 | 0.8 | 6.702 | 6.240 | 0.300 | 0.104 | 0.120 | 1.00 |
| 62.7000 | 62.700 | 62.700 | 18.429 | 150 | 0.8 | 6.702 | 6.240 | 0.300 | 0.104 | 0.120 | 1.00 |
| 62.7000 | 62.700 | 62.700 | 6.719 | 150 | 0.8 | 6.702 | 6.240 | 0.300 | 0.104 | 0.120 | 1.00 |
| 62.7000 | 62.700 | 62.700 | 1.000 | 150 | 0.8 | 6.702 | 6.240 | 0.300 | 0.104 | 0.120 | 1.00 |
| 62.7000 | 62.700 | 62.700 | 1.000 | 150 | 0.8 | 6.702 | 6.240 | 0.300 | 0.104 | 0.120 | 1.00 |
| 62.7000 | 62.700 | 62.700 | 1.000 | 150 | 0.8 | 6.702 | 6.240 | 0.300 | 0.104 | 0.120 | 1.00 |
| 62.7000 | 62.700 | 62.700 | 1.000 | 150 | 0.8 | 6.702 | 6.240 | 0.300 | 0.104 | 0.120 | 1.00 |

EXISTING MAINS

| PK | CHAIN | START | LENGTH (M) | PIPE DIA (MM) | DEPTH (M) | INVERT (M) | OUTLET (M) | VELOCITY (M/S) | Q (L/S) | Q (M³/S) | Q (M³/D) |
|--------|-------|-------|------------|---------------|-----------|------------|------------|----------------|---------|----------|----------|
| 71.000 | 71.0 | 71.0 | 22.866 | 150 | 0.8 | 6.200 | 6.200 | 0.300 | 0.104 | 0.120 | 1.00 |
| 71.000 | 71.0 | 71.0 | 22.867 | 150 | 0.8 | 6.200 | 6.200 | 0.300 | 0.104 | 0.120 | 1.00 |
| 71.000 | 71.0 | 71.0 | 22.868 | 150 | 0.8 | 6.200 | 6.200 | 0.300 | 0.104 | 0.120 | 1.00 |
| 71.000 | 71.0 | 71.0 | 21.779 | 150 | 0.8 | 6.200 | 6.200 | 0.300 | 0.104 | 0.120 | 1.00 |
| 71.000 | 71.0 | 71.0 | 18.207 | 150 | 0.8 | 6.200 | 6.200 | 0.300 | 0.104 | 0.120 | 1.00 |
| 71.000 | 71.0 | 71.0 | 17.842 | 150 | 0.8 | 6.200 | 6.200 | 0.300 | 0.104 | 0.120 | 1.00 |
| 71.000 | 71.0 | 71.0 | 1.000 | 150 | 0.8 | 6.200 | 6.200 | 0.300 | 0.104 | 0.120 | 1.00 |
| 71.000 | 71.0 | 71.0 | 4.221 | 150 | 0.8 | 6.200 | 6.200 | 0.300 | 0.104 | 0.120 | 1.00 |
| 71.000 | 71.0 | 71.0 | 5.915 | 150 | 0.8 | 6.200 | 6.200 | 0.300 | 0.104 | 0.120 | 1.00 |
| 71.000 | 71.0 | 71.0 | 1.000 | 150 | 0.8 | 6.200 | 6.200 | 0.300 | 0.104 | 0.120 | 1.00 |

LEGEND

- PHASE 1 PLANNING APPLICATION BOUNDARY
- GCC OWNED LANDS
- EXISTING FW PIPE FROM RECORDS
- EXISTING SW PIPE FROM RECORDS
- EXISTING SW GULLY
- PROPOSED SW PIPE & MH
- PROPOSED SW PIPE & INSPECTION CHAMBER
- PROPOSED SW PIPE & MH DIVERSION
- PROPOSED GREEN ROOF
- PROPOSED RAIN GARDEN
- PROPOSED SW ATTENUATION
- PROPOSED SW GULLY
- PROPOSED FW PIPE & MH
- PROPOSED FW RISING MAIN
- PROPOSED FW ATTENUATION
- PROPOSED PUMPING STATION

AECOM

PROJECT
 Phase 1
 Corrib Causeway
 Dyke Road, Galway

CLIENT
 Land Development
 Agency (LDA) / GCC

CONSULTANT
 AECOM
 BLOCK 6, GALWAY TECHNOLOGY PARK
 PARKMORE
 GALWAY, H91 W30F, IRELAND
www.aecom.com

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 - REFER TO LANDSCAPE ARCHITECT DRAWINGS FOR LANDSCAPING DETAILS.

AECOM
PLANNING

PURPOSE
 P3 PLANNING

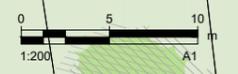
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| IR | DATE | DESCRIPTION |
|----|------------|--------------|
| 01 | 05.07.2024 | FOR PLANNING |

PROJECT NUMBER
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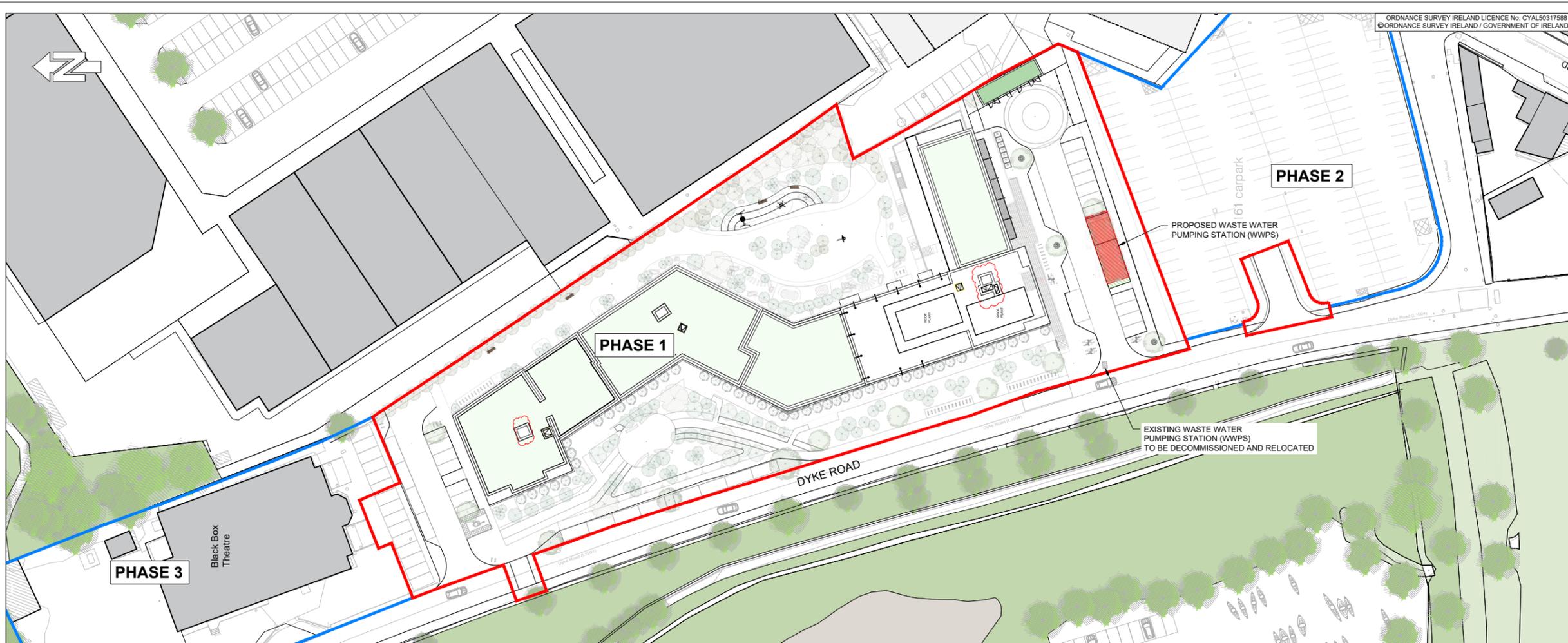
SHEET TITLE
 PROPOSED DRAINAGE
 SHEET 02

SHEET NUMBER
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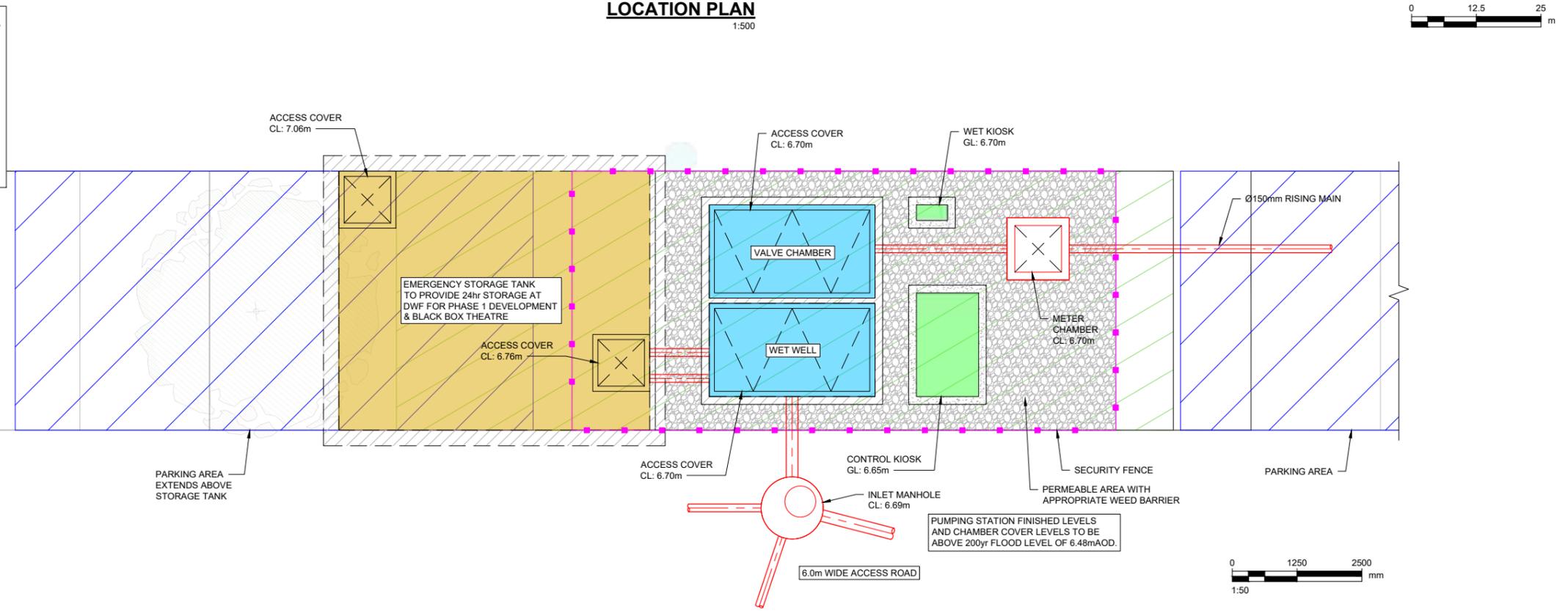
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 Last saved by: PETER.SMITH(2024-07-04) Last Plotted: 2024-07-04
 Project Management Initials: _____ Designer: _____ Checked: _____ Approved: _____
 ISO A1 594mm x 841mm



LEGEND

- PHASE 1 PROPOSED BOUNDARY
- CLIENT OWNED LAND BOUNDARY
- PROPOSED SECURITY FENCE
- PROPOSED FW PIPE
- PROPOSED PUMPING STATION
- PROPOSED KIOSKS
- PROPOSED STORAGE
- ▨ PROPOSED PUBLIC PARKING



PROJECT
Phase 1
Corrib Causeway
Dyke Road, Galway

CLIENT
Land Development
Agency (LDA) / GCC

CONSULTANT
AECOM
BLOCK 6, GALWAY TECHNOLOGY PARK
PARKMORE
GALWAY, H91 W30F, IRELAND
www.aecom.com

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 17. REFER TO LANDSCAPE ARCHITECT DRAWINGS FOR LANDSCAPING DETAILS.



PURPOSE
P3 PLANNING

ISSUE/REVISION

| IR | DATE | DESCRIPTION |
|----|------------|--------------|
| 01 | 05.07.2024 | FOR PLANNING |

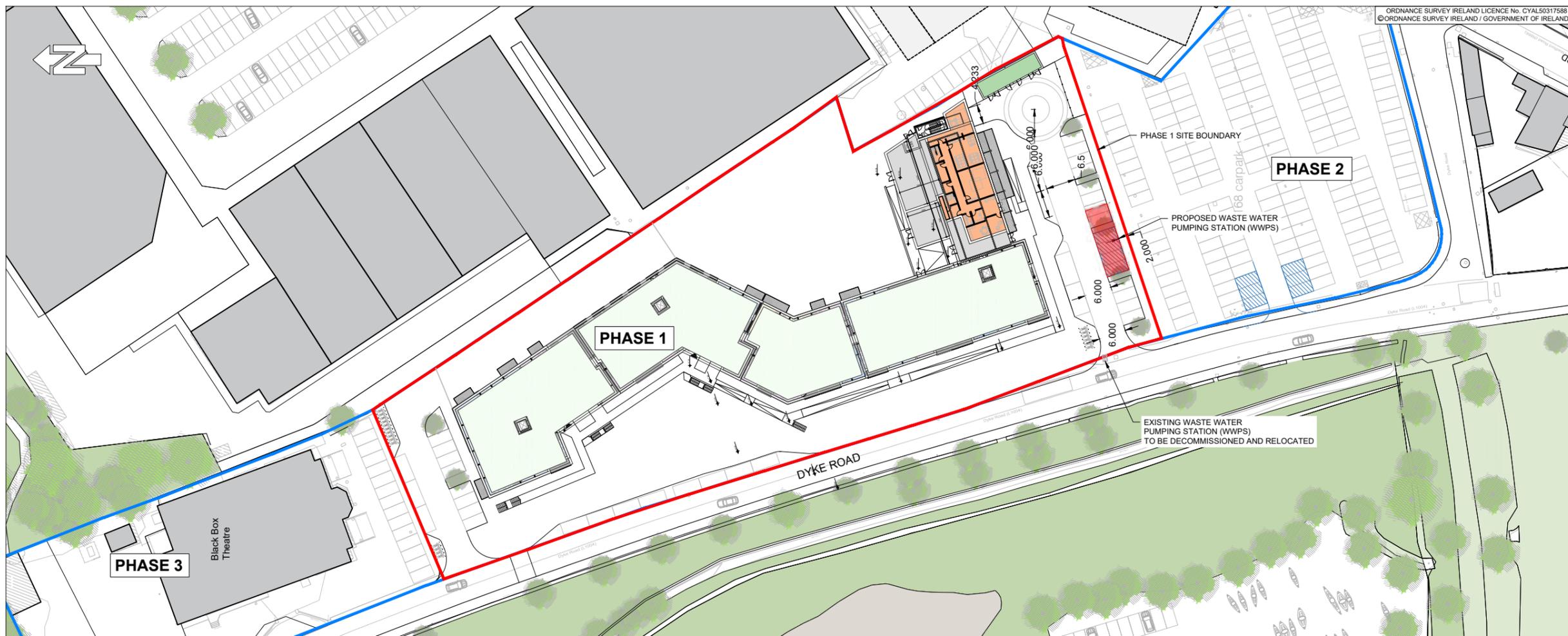
PROJECT NUMBER
60710277

SHEET TITLE
PROPOSED PHASE 1
WASTE WATER PUMPING STATION
(WWPS)

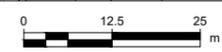
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60710277-ACM-XX-XX-DR-CE-00-0503

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 Last saved by: PETER.SMITH(2024-06-06) Last Plotted: 2024-06-06
 ISO A1 594mm x 841mm
 Approved: _____
 Checked: _____
 Designer: _____
 Project Management Initials: _____



LOCATION PLAN
1:500



PROJECT
Dyke Road,
Galway

CLIENT
Land Development
Agency

CONSULTANT
AECOM
BLOCK 6, GALWAY TECHNOLOGY PARK
PARKMORE
GALWAY, H91 W30F, IRELAND
www.aecom.com

- NOTES**
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 - DIMENSIONS IN MILLIMETRES UNLESS NOTED OTHERWISE.
 - FOR PROPOSED DRAINAGE REFER TO DRAWING 60710277-ACM-XX-XX-DR-CE-00-0501.

LEGEND

- PHASE 1 PROPOSED BOUNDARY
- CLIENT OWNED LAND BOUNDARY
- PROPOSED SECURITY FENCE
- PROPOSED FW PIPE
- PROPOSED PUMPING STATION
- PROPOSED KIOSKS
- PROPOSED STORAGE
- PROPOSED PUBLIC PARKING

PURPOSE
P1 INFORMATION

ISSUE/REVISION

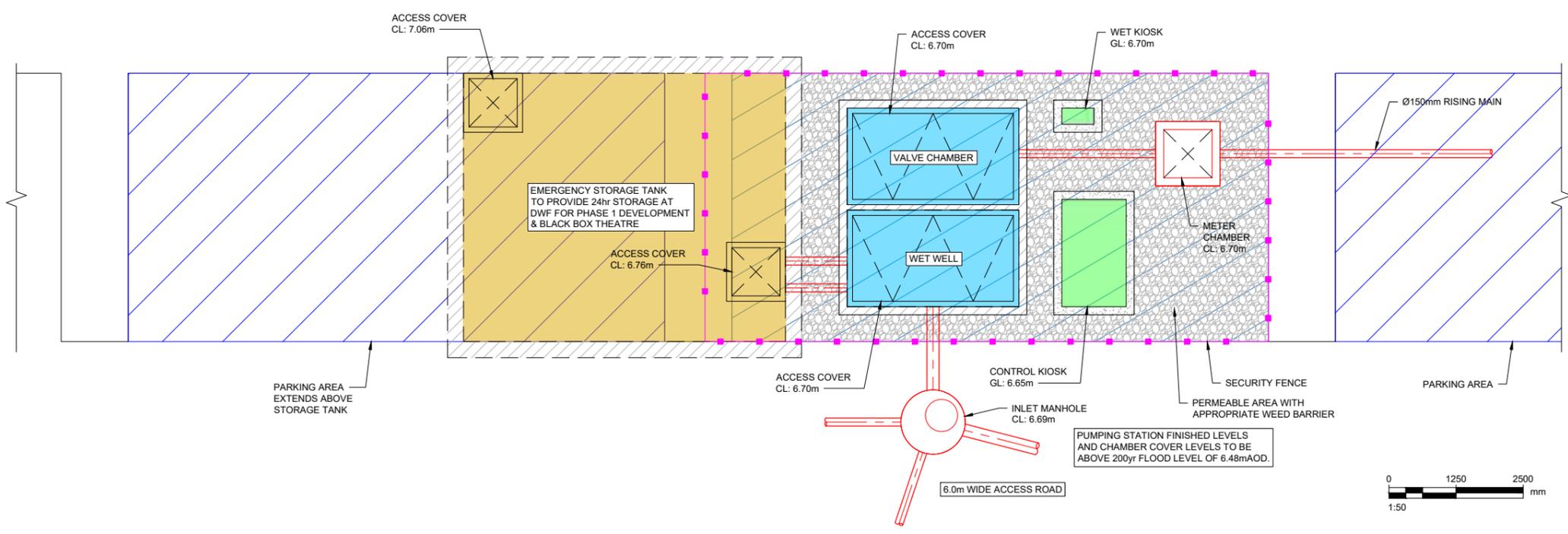
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|----|------------|-----------------------------|
| 01 | 05.06.2024 | FIRST ISSUE FOR INFORMATION |

DRAFT COPY

PROJECT NUMBER
60710277

SHEET TITLE
PROPOSED PHASE 1
WASTE WATER PUMPING STATION
(WWPS)

SHEET NUMBER
60710277-ACM-XX-XX-DR-CE-00-0506



WWPS DETAILS
1:500

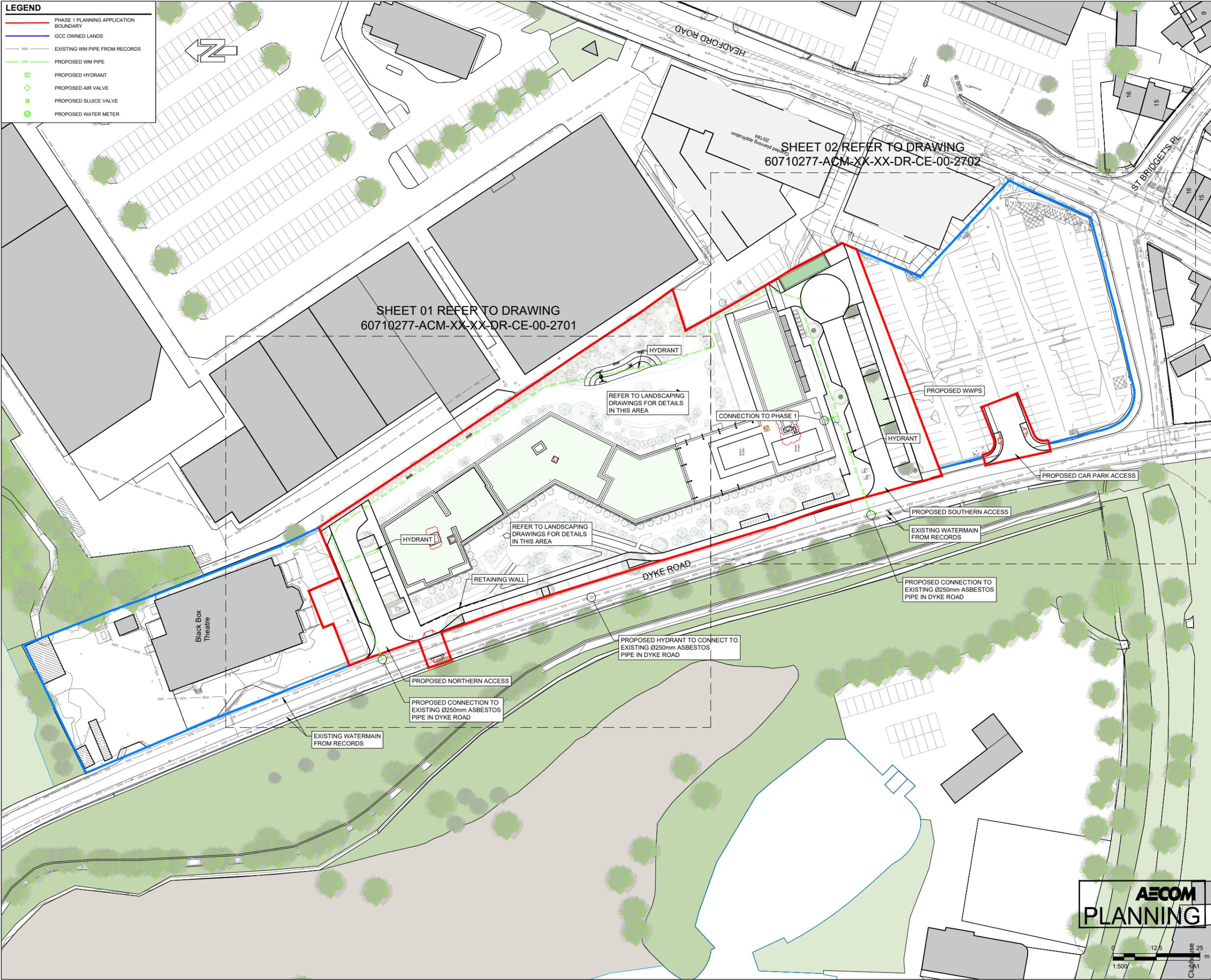


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ISO A1 594mm x 841mm
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 Last saved by: PROPHETT (2024-07-25) Last Plotted: 2024-07-25

LEGEND

- PHASE 1 PLANNING APPLICATION BOUNDARY
- GCC OWNED LANDS
- EXISTING WM PIPE FROM RECORDS
- PROPOSED WM PIPE
- PROPOSED HYDRANT
- ◇ PROPOSED AIR VALVE
- PROPOSED SLUICE VALVE
- PROPOSED WATER METER



SHEET 01 REFER TO DRAWING
 60710277-ACM-XX-XX-DR-CE-00-2701

SHEET 02 REFER TO DRAWING
 60710277-ACM-XX-XX-DR-CE-00-2702



PROJECT
 Phase 1
 Corrib Causeway
 Dyke Road, Galway

CLIENT
 Land Development
 Agency (LDA) / GCC

CONSULTANT
 AECOM
 BLOCK 6, GALWAY TECHNOLOGY PARK
 PARKMORE
 GALWAY, H91 W30F, IRELAND
www.aecom.com

- NOTES**
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 9. ALL PROPOSED WATERMAIN PIPEWORK TO BE HDPE WITH MINIMUM PE80 RATING IN ACCORDANCE WITH UISCE EIREANN CODE OF PRACTICE FOR WATER INFRASTRUCTURE SECTION 3.15.2.
 10. THE MINIMUM PIPE DIAMETER PROPOSED IS 150mm.
 11. ALL WATERMANS WILL HAVE A MINIMUM COVER OF 900mm. ALL SERVICE PIPES CONNECTING PROPERTIES WILL HAVE MINIMUM COVER OF 750mm.
 12. HYDRANTS SHALL BE DOUBLE FLANGED DRILLED TO PN16 AND SHALL COMPLY WITH BS EN 14399, IS EN 1074 PART 6 AND 135750 REFER TO UISCE EIREANN CODE OF PRACTICE FOR WATER INFRASTRUCTURE SECTION 3.16.5.
 13. SLUICE VALVES HAVE BEEN PROVIDED SO THAT INDIVIDUAL SECTIONS CAN BE ISOLATED & WILL COMPLY TO BS 5153. THE DEPTH OF THE SLUICE VALVE SPINDLE CAP BELOW FINISHED GROUND WILL NOT EXCEED 300mm.
 14. ALL WATERMAIN DETAILS TO BE IN ACCORDANCE WITH THE UISCE EIREANN INFRASTRUCTURE STANDARD DETAILS AND CODE OF PRACTICE FOR WATER INFRASTRUCTURE.
 15. ALL WATERMAIN WORKS SHALL BE CARRIED OUT IN ACCORDANCE WITH UISCE EIREANN STANDARD DETAIL AND THE CODE OF PRACTICE FOR WATER INFRASTRUCTURE DOCUMENT CDS-5030-03.
 16. ALL PIPE MATERIALS SHALL BE IN COMPLIANCE WITH SECTION 3.9 OF UISCE EIREANN CODE OF PRACTICE DOCUMENT CDS-5020-03.
 17. INDIVIDUAL WATER SERVICE CONNECTIONS AND BOUNDARY BOXES TO EACH DWELLING SHALL BE IN COMPLIANCE WITH UISCE EIREANN STANDARD DETAIL STD-W-02.
 18. SEPARATION DISTANCES FROM OTHER SERVICES, BOUNDARY WALLS ETC. SHALL BE IN ACCORDANCE WITH UISCE EIREANN STANDARD DETAIL STD-W-11.
 19. SEPARATION DISTANCES FROM TREES, SHRUBS ETC. SHALL BE IN ACCORDANCE WITH UISCE EIREANN STANDARD DETAIL STD-W-12 & 12A.
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 24. SCOUR VALVE AND CHAMBER DETAILS SHALL BE IN ACCORDANCE WITH UISCE EIREANN STANDARD DETAIL STD-W-30.

PURPOSE
 P3 PLANNING

ISSUE/REVISION

| IR | DATE | DESCRIPTION |
|----|------------|--------------|
| 01 | 05.07.2024 | FOR PLANNING |

PROJECT NUMBER
 60710277

SHEET TITLE
 PROPOSED WATERMAIN
 KEYPLAN

SHEET NUMBER
 60710277-ACM-XX-XX-DR-CE-00-2700

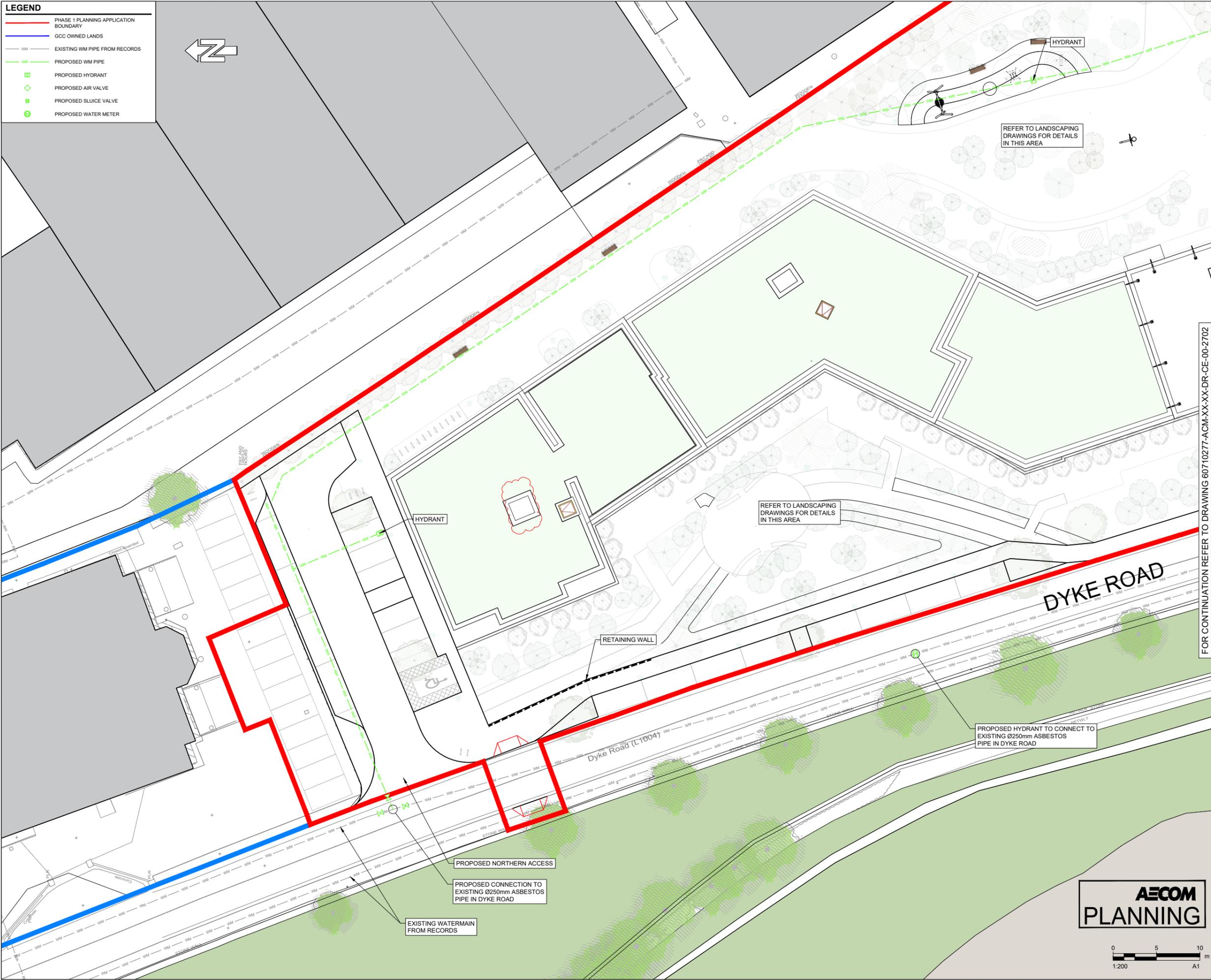


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 Last saved by: PROPHETT (2024-07-25) Last Plotted: 2024-07-25
 Project Management Initials: Designer: Checked: Approved: ISO A1 594mm x 841mm

LEGEND

| | |
|--|---------------------------------------|
| | PHASE 1 PLANNING APPLICATION BOUNDARY |
| | GCC OWNED LANDS |
| | EXISTING WM PIPE FROM RECORDS |
| | PROPOSED WM PIPE |
| | PROPOSED HYDRANT |
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FOR CONTINUATION REFER TO DRAWING 60710277-ACM-XX-DR-CE-00-2702



PURPOSE
 P3 PLANNING

ISSUE/REVISION

| IR | DATE | DESCRIPTION |
|----|------------|--------------|
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PROJECT NUMBER
 60710277

SHEET TITLE
 PROPOSED WATERMAIN
 (SHEET 01)

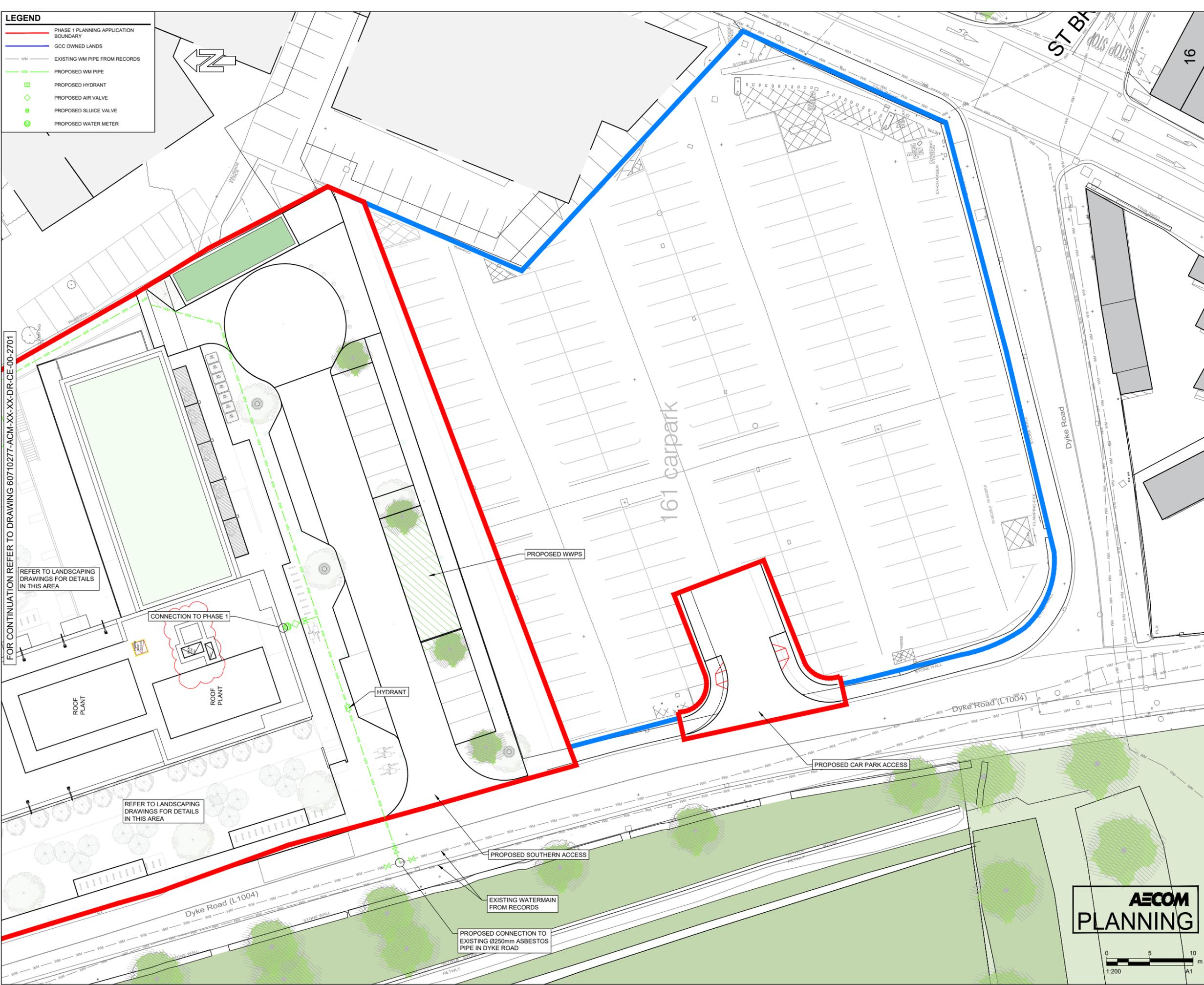
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LEGEND

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PURPOSE
 P3 PLANNING

ISSUE/REVISION

| IR | DATE | DESCRIPTION |
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| 01 | 05.07.2024 | FOR PLANNING |

PROJECT NUMBER
 60710277

SHEET TITLE
 PROPOSED WATERMAIN
 (SHEET 02)

SHEET NUMBER
 60710277-ACM-XX-XX-DR-CE-00-2702

AECOM
PLANNING

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Wastewater Network

Please note, the existing Black Box pumping station would require upgrading to cater for the wastewater loading and to provide adequate emergency storage for the proposed development.

Alternatively, a new Pump Station could be constructed by the customer within their own site, the new pump station (WWPS) should be designed to cater for the proposed development wastewater loading. The existing Black Box pumping station shall be decommissioned with this option, and the existing flows diverted to the new WWPS. The customer will be responsible for designing the new WWPS required for the proposed development and the diverted flows that currently discharge to the existing Black Box Pumping station.

A 20m Approx. foul sewer network upgrade from 150mm diameter to 225mm diameter minimum is required to cater the proposed development at the start of Dyke Road and Wood quay. Any such network upgrade would have to be funded by the customer.

The developer should investigate the separation of storm water and foul on their development lands and that any existing storm water which is entering into the Uisce Eireann (UE) combined system will need to be eliminated. There should be no storm water discharge to the UE network.

Please note that according to our records there is an existing sewer running through this site (see drawing below). Any structures or works over or in close proximity to Uisce Eireann infrastructure that will inhibit access for maintenance or endanger structural or functional integrity of the infrastructure are not allowed. The layout of the development must ensure that this pipe is protected and adequate separation distances are provided between Uisce Eireann infrastructure and any structures on site. Alternatively you may enter into a diversion agreement with Uisce Eireann and divert the pipe to

accommodate your development. If you wish to proceed with this option please contact Uisce Éireann at Diversions@water.ie and submit detailed design drawings before submitting your planning application. It will be necessary to provide a wayleave over this pipe to the benefit of Uisce Éireann and ensure that it is accessible for maintenance. For more information, please see go to the link below: <https://www.water.ie/connections/developer-services/diversions/>

This letter does not constitute an offer, in whole or in part, to provide a connection to any Uisce Éireann infrastructure. Before the Development can be connected to our network(s) you must submit a connection application and be granted and sign a connection agreement with Uisce Éireann.

As the network capacity changes constantly, this review is only valid at the time of its completion. As soon as planning permission has been granted for the Development, a completed connection application should be submitted. The connection application is available at www.water.ie/connections/get-connected/

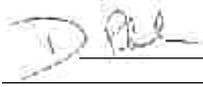
Where can you find more information?

- **Section A** - What is important to know?
- **Section B** - Details of Uisce Éireann's Network(s)

This letter is issued to provide information about the current feasibility of the proposed connection(s) to Uisce Éireann's network(s). This is not a connection offer and capacity in Uisce Éireann's network(s) may only be secured by entering into a connection agreement with Uisce Éireann.

For any further information, visit www.water.ie/connections, email newconnections@water.ie or contact 1800 278 278.

Yours sincerely,



Dermot Phelan
Connections Delivery Manager

Section A - What is important to know?

| What is important to know? | Why is this important? |
|---|---|
| Do you need a contract to connect? | <ul style="list-style-type: none"> • Yes, a contract is required to connect. This letter does not constitute a contract or an offer in whole or in part to provide a connection to Uisce Éireann's network(s). • Before the Development can connect to Uisce Éireann's network(s), you must submit a connection application <u>and be granted and sign</u> a connection agreement with Uisce Éireann. |
| When should I submit a Connection Application? | <ul style="list-style-type: none"> • A connection application should only be submitted after planning permission has been granted. |
| Where can I find information on connection charges? | <ul style="list-style-type: none"> • Uisce Éireann connection charges can be found at: https://www.water.ie/connections/information/charges/ |
| Who will carry out the connection work? | <ul style="list-style-type: none"> • All works to Uisce Éireann's network(s), including works in the public space, must be carried out by Uisce Éireann*. <p>*Where a Developer has been granted specific permission and has been issued a connection offer for Self-Lay in the Public Road/Area, they may complete the relevant connection works</p> |
| Fire flow Requirements | <ul style="list-style-type: none"> • The Confirmation of Feasibility does not extend to fire flow requirements for the Development. Fire flow requirements are a matter for the Developer to determine. • What to do? - Contact the relevant Local Fire Authority |
| Plan for disposal of storm water | <ul style="list-style-type: none"> • The Confirmation of Feasibility does not extend to the management or disposal of storm water or ground waters. • What to do? - Contact the relevant Local Authority to discuss the management or disposal of proposed storm water or ground water discharges. |
| Where do I find details of Uisce Éireann's network(s)? | <ul style="list-style-type: none"> • Requests for maps showing Uisce Éireann's network(s) can be submitted to: datarequests@water.ie |

| | |
|---|---|
| <p>What are the design requirements for the connection(s)?</p> | <ul style="list-style-type: none"> The design and construction of the Water & Wastewater pipes and related infrastructure to be installed in this Development shall comply with <i>the Uisce Éireann Connections and Developer Services Standard Details and Codes of Practice</i>, available at www.water.ie/connections |
| <p>Trade Effluent Licensing</p> | <ul style="list-style-type: none"> Any person discharging trade effluent** to a sewer, must have a Trade Effluent Licence issued pursuant to section 16 of the Local Government (Water Pollution) Act, 1977 (as amended). More information and an application form for a Trade Effluent License can be found at the following link: https://www.water.ie/business/trade-effluent/about/ <p>**trade effluent is defined in the Local Government (Water Pollution) Act, 1977 (as amended)</p> |

Section B – Details of Uisce Éireann’s Network(s)

The map included below outlines the current Uisce Éireann infrastructure adjacent the Development: To access Uisce Éireann Maps email

datarequests@water.ie



Reproduced from the Ordnance Survey of Ireland by Permission of the Government. License No. 3-3-34

Note: The information provided on the included maps as to the position of Uisce Éireann’s underground network(s) is provided as a general guide only. The information is based on the best available information provided by each Local Authority in Ireland to Uisce Éireann.

Whilst every care has been taken in respect of the information on Uisce Éireann’s network(s), Uisce Éireann assumes no responsibility for and gives no guarantees, undertakings or warranties concerning the accuracy, completeness or up to date nature of the information provided, nor does it accept any liability whatsoever arising from or out of any errors or omissions. This information

should not be solely relied upon in the event of excavations or any other works being carried out in the vicinity of Uisce Éireann's underground network(s). The onus is on the parties carrying out excavations or any other works to ensure the exact location of Uisce Éireann's underground network(s) is identified prior to excavations or any other works being carried out. Service connection pipes are not generally shown but their presence should be anticipated.

Appendix B Surface Water Drainage Calculations

| | | |
|------------------------------|-------------------------------|---|
| AECOM | | Page 1 |
| Midpoint | 60710277 LDA Dyke Road Galway |  |
| Alencon Link | Surafce Water Network 1 | |
| Basingstoke, RG21 7PP | Hydraulic Design | |
| Date 16/12/2024 15:42 | Designed by Thorne Prophet | |
| File Storm Water Network.MDX | Checked by Emma McKendrick | |
| Innovyze | Network 2020.1.3 | |

STORM SEWER DESIGN by the Modified Rational Method

Design Criteria for Storm 1

Pipe Sizes STANDARD Manhole Sizes STANDARD

FSR Rainfall Model - Scotland and Ireland

| | | | |
|--------------------------------------|--------|---------------------------------------|-------|
| Return Period (years) | 5 | PIMP (%) | 100 |
| M5-60 (mm) | 17.200 | Add Flow / Climate Change (%) | 20 |
| Ratio R | 0.300 | Minimum Backdrop Height (m) | 0.000 |
| Maximum Rainfall (mm/hr) | 50 | Maximum Backdrop Height (m) | 0.000 |
| Maximum Time of Concentration (mins) | 30 | Min Design Depth for Optimisation (m) | 0.000 |
| Foul Sewage (l/s/ha) | 0.000 | Min Vel for Auto Design only (m/s) | 1.00 |
| Volumetric Runoff Coeff. | 0.750 | Min Slope for Optimisation (1:X) | 200 |

Designed with Level Soffits

Network Design Table for Storm 1

« - Indicates pipe capacity < flow

| PN | Length (m) | Fall (m) | Slope (1:X) | I.Area (ha) | T.E. (mins) | Base Flow (l/s) | k (mm) | HYD SECT | DIA (mm) | Section Type | Auto Design |
|----------|------------|----------|-------------|-------------|-------------|-----------------|--------|----------|----------|--------------|-------------|
| S1 1.000 | 24.373 | 0.122 | 200.0 | 0.000 | 4.00 | 0.0 | 0.600 | o | 300 | Pipe/Conduit | 🚫 |
| S1 1.001 | 23.988 | 0.120 | 199.9 | 0.285 | 0.00 | 0.0 | 0.600 | o | 300 | Pipe/Conduit | 🟢 |
| S1 1.002 | 18.773 | 0.125 | 150.0 | 0.035 | 0.00 | 0.0 | 0.600 | o | 300 | Pipe/Conduit | 🟢 |
| S1 1.003 | 4.863 | 0.032 | 152.0 | 0.036 | 0.00 | 0.0 | 0.600 | o | 300 | Pipe/Conduit | 🟢 |
| S1 1.004 | 14.666 | 0.029 | 500.0 | 0.000 | 0.00 | 0.0 | 0.600 | [] | -1 | Pipe/Conduit | 🚫 |
| S1 2.000 | 18.293 | 0.122 | 150.0 | 0.020 | 4.00 | 0.0 | 0.600 | o | 225 | Pipe/Conduit | 🚫 |
| S1 2.001 | 10.065 | 0.067 | 150.0 | 0.033 | 0.00 | 0.0 | 0.600 | o | 225 | Pipe/Conduit | 🟢 |
| S1 2.002 | 2.237 | 0.015 | 150.0 | 0.000 | 0.00 | 0.0 | 0.600 | o | 225 | Pipe/Conduit | 🟢 |
| S1 1.005 | 1.300 | 0.009 | 144.4 | 0.000 | 0.00 | 0.0 | 0.600 | o | 225 | Pipe/Conduit | 🚫 |
| S1 1.006 | 6.782 | 0.045 | 150.0 | 0.000 | 0.00 | 0.0 | 0.600 | o | 225 | Pipe/Conduit | 🚫 |

Network Results Table

| PN | Rain (mm/hr) | T.C. (mins) | US/IL (m) | Σ I.Area (ha) | Σ Base Flow (l/s) | Foul (l/s) | Add Flow (l/s) | Vel (m/s) | Cap (l/s) | Flow (l/s) |
|----------|--------------|-------------|-----------|---------------|-------------------|------------|----------------|-----------|-----------|------------|
| S1 1.000 | 50.00 | 4.37 | 4.300 | 0.000 | 0.0 | 0.0 | 0.0 | 1.11 | 78.3 | 0.0 |
| S1 1.001 | 50.00 | 4.73 | 4.178 | 0.285 | 0.0 | 0.0 | 7.7 | 1.11 | 78.3 | 46.3 |
| S1 1.002 | 50.00 | 4.97 | 4.058 | 0.320 | 0.0 | 0.0 | 8.7 | 1.28 | 90.6 | 52.0 |
| S1 1.003 | 50.00 | 5.04 | 3.933 | 0.356 | 0.0 | 0.0 | 9.6 | 1.27 | 90.0 | 57.8 |
| S1 1.004 | 50.00 | 5.16 | 3.404 | 0.356 | 0.0 | 0.0 | 9.6 | 1.90 | 15175.3 | 57.8 |
| S1 2.000 | 50.00 | 4.29 | 3.700 | 0.020 | 0.0 | 0.0 | 0.5 | 1.07 | 42.4 | 3.2 |
| S1 2.001 | 50.00 | 4.44 | 3.578 | 0.053 | 0.0 | 0.0 | 1.4 | 1.07 | 42.4 | 8.6 |
| S1 2.002 | 50.00 | 4.48 | 3.511 | 0.053 | 0.0 | 0.0 | 1.4 | 1.07 | 42.4 | 8.6 |
| S1 1.005 | 50.00 | 5.18 | 3.375 | 0.409 | 0.0 | 0.0 | 11.1 | 1.09 | 43.2« | 66.5 |
| S1 1.006 | 50.00 | 4.11 | 3.337 | 0.000 | 10.0 | 0.0 | 1.7 | 1.07 | 42.4 | 10.0 |

Midpoint
Alencon Link
Basingstoke, RG21 7PP

60710277 LDA Dyke Road Galway
Surface Water Network 1
Hydraulic Design



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Designed by Thorne Prophet
Checked by Emma McKendrick

Innovyze

Network 2020.1.3

Manhole Schedules for Storm 1

| MH Name | MH CL (m) | MH Depth (m) | MH Connection | MH Diam., L*W (mm) | PN | Pipe Out Invert Level (m) | Diameter (mm) | PN | Pipes In Invert Level (m) | Diameter (mm) | Backdrop (mm) |
|----------|-----------|--------------|---------------|--------------------|----------|---------------------------|---------------|----------|---------------------------|---------------|---------------|
| S1 1-0 | 5.071 | 0.771 | Open Manhole | 1500 | S1 1.000 | 4.300 | 300 | | | | |
| S1 1-1 | 5.027 | 0.849 | Open Manhole | 1350 | S1 1.001 | 4.178 | 300 | S1 1.000 | 4.178 | 300 | |
| S1 1-2 | 5.000 | 0.942 | Open Manhole | 1350 | S1 1.002 | 4.058 | 300 | S1 1.001 | 4.058 | 300 | |
| S1 1-3 | 5.000 | 1.067 | Open Manhole | 1350 | S1 1.003 | 3.933 | 300 | S1 1.002 | 3.933 | 300 | |
| S1 1-4 | 5.004 | 1.600 | Open Manhole | 1500 | S1 1.004 | 3.404 | -1 | S1 1.003 | 3.901 | 300 | |
| S1 2-0 | 5.000 | 1.300 | Open Manhole | 1200 | S1 2.000 | 3.700 | 225 | | | | |
| S1 2-1 | 5.306 | 1.728 | Open Manhole | 1200 | S1 2.001 | 3.578 | 225 | S1 2.000 | 3.578 | 225 | |
| S1 2-2 | 5.209 | 1.698 | Open Manhole | 1200 | S1 2.002 | 3.511 | 225 | S1 2.001 | 3.511 | 225 | |
| S1 1-5 | 5.186 | 1.811 | Open Manhole | 1500 | S1 1.005 | 3.375 | 225 | S1 1.004 | 3.375 | -1 | |
| | | | | | | | | S1 2.002 | 3.496 | 225 | 121 |
| S1 1-6 | 5.108 | 1.771 | Open Manhole | 1800 | S1 1.006 | 3.337 | 225 | S1 1.005 | 3.366 | 225 | 29 |
| S1 Ex MH | 5.067 | 1.775 | Open Manhole | 525 | | OUTFALL | | S1 1.006 | 3.292 | 225 | |

| MH Name | Manhole Easting (m) | Manhole Northing (m) | Intersection Easting (m) | Intersection Northing (m) | Manhole Access | Layout (North) |
|----------|---------------------|----------------------|--------------------------|---------------------------|----------------|----------------|
| S1 1-0 | 529879.590 | 725989.868 | 529879.590 | 725989.868 | Required | |
| S1 1-1 | 529865.991 | 726010.095 | 529865.991 | 726010.095 | Required | |
| S1 1-2 | 529852.608 | 726030.003 | 529852.608 | 726030.003 | Required | |
| S1 1-3 | 529842.136 | 726045.583 | 529842.136 | 726045.583 | Required | |
| S1 1-4 | 529837.643 | 726043.722 | 529837.643 | 726043.722 | Required | |
| S1 2-0 | 529820.276 | 726016.828 | 529820.276 | 726016.828 | Required | |
| S1 2-1 | 529814.467 | 726034.174 | 529814.467 | 726034.174 | Required | |
| S1 2-2 | 529823.777 | 726038.000 | 529823.777 | 726038.000 | Required | |
| S1 1-5 | 529823.405 | 726040.206 | 529823.405 | 726040.206 | Required | |
| S1 1-6 | 529822.907 | 726041.407 | 529822.907 | 726041.407 | Required | |
| S1 Ex MH | 529816.410 | 726039.462 | | | No Entry | |

Midpoint
Alencon Link
Basingstoke, RG21 7PP

60710277 LDA Dyke Road Galway
Surface Water Network 1
Hydraulic Design



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PIPELINE SCHEDULES for Storm 1

Upstream Manhole

| PN | Hyd Sect | Diam (mm) | MH Name | C.Level (m) | I.Level (m) | D.Depth (m) | MH Connection | MH DIAM., L*W (mm) |
|----------|----------|-----------|---------|-------------|-------------|-------------|---------------|--------------------|
| S1 1.000 | o | 300 | S1 1-0 | 5.071 | 4.300 | 0.471 | Open Manhole | 1500 |
| S1 1.001 | o | 300 | S1 1-1 | 5.027 | 4.178 | 0.549 | Open Manhole | 1350 |
| S1 1.002 | o | 300 | S1 1-2 | 5.000 | 4.058 | 0.642 | Open Manhole | 1350 |
| S1 1.003 | o | 300 | S1 1-3 | 5.000 | 3.933 | 0.767 | Open Manhole | 1350 |
| S1 1.004 | [] | -1 | S1 1-4 | 5.004 | 3.404 | 0.800 | Open Manhole | 1500 |
| S1 2.000 | o | 225 | S1 2-0 | 5.000 | 3.700 | 1.075 | Open Manhole | 1200 |
| S1 2.001 | o | 225 | S1 2-1 | 5.306 | 3.578 | 1.503 | Open Manhole | 1200 |
| S1 2.002 | o | 225 | S1 2-2 | 5.209 | 3.511 | 1.473 | Open Manhole | 1200 |
| S1 1.005 | o | 225 | S1 1-5 | 5.186 | 3.375 | 1.586 | Open Manhole | 1500 |
| S1 1.006 | o | 225 | S1 1-6 | 5.108 | 3.337 | 1.546 | Open Manhole | 1800 |

Downstream Manhole

| PN | Length (m) | Slope (1:X) | MH Name | C.Level (m) | I.Level (m) | D.Depth (m) | MH Connection | MH DIAM., L*W (mm) |
|----------|------------|-------------|----------|-------------|-------------|-------------|---------------|--------------------|
| S1 1.000 | 24.373 | 200.0 | S1 1-1 | 5.027 | 4.178 | 0.549 | Open Manhole | 1350 |
| S1 1.001 | 23.988 | 199.9 | S1 1-2 | 5.000 | 4.058 | 0.642 | Open Manhole | 1350 |
| S1 1.002 | 18.773 | 150.0 | S1 1-3 | 5.000 | 3.933 | 0.767 | Open Manhole | 1350 |
| S1 1.003 | 4.863 | 152.0 | S1 1-4 | 5.004 | 3.901 | 0.803 | Open Manhole | 1500 |
| S1 1.004 | 14.666 | 500.0 | S1 1-5 | 5.186 | 3.375 | 1.011 | Open Manhole | 1500 |
| S1 2.000 | 18.293 | 150.0 | S1 2-1 | 5.306 | 3.578 | 1.503 | Open Manhole | 1200 |
| S1 2.001 | 10.065 | 150.0 | S1 2-2 | 5.209 | 3.511 | 1.473 | Open Manhole | 1200 |
| S1 2.002 | 2.237 | 150.0 | S1 1-5 | 5.186 | 3.496 | 1.465 | Open Manhole | 1500 |
| S1 1.005 | 1.300 | 144.4 | S1 1-6 | 5.108 | 3.366 | 1.517 | Open Manhole | 1800 |
| S1 1.006 | 6.782 | 150.0 | S1 Ex MH | 5.067 | 3.292 | 1.550 | Open Manhole | 525 |

Free Flowing Outfall Details for Storm 1

| Outfall Pipe Number | Outfall Name | C. Level (m) | I. Level (m) | Min I. Level (m) | D,L (mm) | W (mm) |
|---------------------|--------------|--------------|--------------|------------------|----------|--------|
| S1 1.006 | S1 Ex MH | 5.067 | 3.292 | 3.220 | 525 | 0 |

Simulation Criteria for Storm 1

| | | | |
|---------------------------------|-------|--|-------|
| Volumetric Runoff Coeff | 0.750 | Additional Flow - % of Total Flow | 0.000 |
| Areal Reduction Factor | 1.000 | MADD Factor * 10m ³ /ha Storage | 2.000 |
| Hot Start (mins) | 0 | Inlet Coefficient | 0.800 |
| Hot Start Level (mm) | 0 | Flow per Person per Day (l/per/day) | 0.000 |
| Manhole Headloss Coeff (Global) | 0.500 | Run Time (mins) | 60 |
| Foul Sewage per hectare (l/s) | 0.000 | Output Interval (mins) | 1 |
| Number of Input Hydrographs | 0 | Number of Offline Controls | 0 |
| Number of Online Controls | 1 | Number of Storage Structures | 1 |
| | | Number of Time/Area Diagrams | 2 |
| | | Number of Real Time Controls | 0 |

Synthetic Rainfall Details

| | | | |
|-----------------------|----------------------|-----------------------|--------|
| Rainfall Model | FSR | Profile Type | Summer |
| Return Period (years) | 5 | Cv (Summer) | 0.750 |
| Region | Scotland and Ireland | Cv (Winter) | 0.840 |
| M5-60 (mm) | 17.100 | Storm Duration (mins) | 30 |
| Ratio R | 0.300 | | |

| | | |
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| Innovyze | Network 2020.1.3 | |

Online Controls for Storm 1

Hydro-Brake® Optimum Manhole: S1 1-6, DS/PN: S1 1.006, Volume (m³): 4.5

| | | | |
|-------------------|----------------------------|-----------------------------------|-------|
| Unit Reference | MD-SHE-0139-1000-1450-1000 | Sump Available | Yes |
| Design Head (m) | 1.450 | Diameter (mm) | 139 |
| Design Flow (l/s) | 10.0 | Invert Level (m) | 3.337 |
| Flush-Flo™ | Calculated | Minimum Outlet Pipe Diameter (mm) | 225 |
| Objective | Minimise upstream storage | Suggested Manhole Diameter (mm) | 1200 |
| Application | Surface | | |

| Control Points | Head (m) | Flow (l/s) | Control Points | Head (m) | Flow (l/s) |
|---------------------------|----------|------------|---------------------------|----------|------------|
| Design Point (Calculated) | 1.450 | 10.0 | Kick-Flo® | 0.911 | 8.0 |
| Flush-Flo™ | 0.430 | 10.0 | Mean Flow over Head Range | - | 8.7 |

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

| Depth (m) | Flow (l/s) |
|-----------|------------|-----------|------------|-----------|------------|-----------|------------|-----------|------------|
| 0.100 | 5.0 | 0.800 | 9.1 | 2.000 | 11.6 | 4.000 | 16.2 | 7.000 | 21.2 |
| 0.200 | 9.1 | 1.000 | 8.4 | 2.200 | 12.2 | 4.500 | 17.1 | 7.500 | 21.9 |
| 0.300 | 9.8 | 1.200 | 9.1 | 2.400 | 12.7 | 5.000 | 18.0 | 8.000 | 22.6 |
| 0.400 | 10.0 | 1.400 | 9.8 | 2.600 | 13.2 | 5.500 | 18.8 | 8.500 | 23.2 |
| 0.500 | 10.0 | 1.600 | 10.5 | 3.000 | 14.1 | 6.000 | 19.6 | 9.000 | 23.9 |
| 0.600 | 9.8 | 1.800 | 11.1 | 3.500 | 15.2 | 6.500 | 20.4 | 9.500 | 24.5 |

| | | |
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Storage Structures for Storm 1

Bio-Retention Area Manhole: S1 2-0, DS/PN: S1 2.000

| | | | |
|--------------------------------------|---------|--------------------------------------|---------|
| Invert Level (m) | 4.700 | Infiltration Coefficient Side (m/hr) | 0.00000 |
| Porosity | 0.30 | Safety Factor | 2.0 |
| Infiltration Coefficient Base (m/hr) | 0.00000 | | |

| Depth (m) | Area (m ²) | Perimeter (m) | Depth (m) | Area (m ²) | Perimeter (m) |
|-----------|------------------------|---------------|-----------|------------------------|---------------|
| 0.000 | 117.0 | 81.000 | 0.300 | 142.0 | 84.000 |

| | | |
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| Innovyze | Network 2020.1.3 | |

1 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for Storm 1

Simulation Criteria

| | | | |
|---------------------------------|-------|--|-------|
| Areal Reduction Factor | 1.000 | Additional Flow - % of Total Flow | 0.000 |
| Hot Start (mins) | 0 | MADD Factor * 10m ³ /ha Storage | 2.000 |
| Hot Start Level (mm) | 0 | Inlet Coefficient | 0.800 |
| Manhole Headloss Coeff (Global) | 0.500 | Flow per Person per Day (l/per/day) | 0.000 |
| Foul Sewage per hectare (l/s) | 0.000 | | |

| | | | | | |
|-----------------------------|---|------------------------------|---|------------------------------|---|
| Number of Input Hydrographs | 0 | Number of Offline Controls | 0 | Number of Time/Area Diagrams | 2 |
| Number of Online Controls | 1 | Number of Storage Structures | 1 | Number of Real Time Controls | 0 |

Synthetic Rainfall Details

| | | | | | |
|----------------|----------------------|---------|-------------|-------------|-------|
| Rainfall Model | FSR M5-60 (mm) | 17.200 | Cv (Summer) | 0.750 | |
| Region | Scotland and Ireland | Ratio R | 0.300 | Cv (Winter) | 0.840 |

| | |
|------------------------------------|---------------------------------|
| Margin for Flood Risk Warning (mm) | 300.0 |
| Analysis Timestep | 2.5 Second Increment (Extended) |
| DTS Status | ON |
| DVD Status | OFF |
| Inertia Status | OFF |

| | |
|--------------------------|---|
| Profile(s) | Summer and Winter |
| Duration(s) (mins) | 15, 30, 60, 120, 180, 240, 360, 480, 600, 720, 960, 1440, 2160, 2880, 4320, 5760, 7200, 8640, 10080 |
| Return Period(s) (years) | 1, 30, 100 |
| Climate Change (%) | 20, 20, 20 |

| PN | US/MH Name | Storm | Return Period | Climate Change | First (X) Surge | First (Y) Flood | First (Z) Overflow | Overflow Act. | Water Level (m) | Surcharged Depth (m) | Flooded Volume (m ³) |
|----------|------------|------------|---------------|----------------|-----------------|-----------------|--------------------|---------------|-----------------|----------------------|----------------------------------|
| S1 1.000 | S1 1-0 | 15 Summer | 1 | +20% | | | | | 4.300 | -0.300 | 0.000 |
| S1 1.001 | S1 1-1 | 360 Winter | 1 | +20% | | | | | 4.212 | -0.266 | 0.000 |
| S1 1.002 | S1 1-2 | 360 Winter | 1 | +20% | | | | | 4.098 | -0.260 | 0.000 |
| S1 1.003 | S1 1-3 | 15 Winter | 1 | +20% | | | | | 3.987 | -0.246 | 0.000 |
| S1 1.004 | S1 1-4 | 30 Winter | 1 | +20% | | | | | 3.456 | -0.748 | 0.000 |
| S1 2.000 | S1 2-0 | 15 Winter | 1 | +20% | | | | | 3.743 | -0.182 | 0.000 |
| S1 2.001 | S1 2-1 | 15 Winter | 1 | +20% | | | | | 3.647 | -0.156 | 0.000 |
| S1 2.002 | S1 2-2 | 15 Winter | 1 | +20% | | | | | 3.588 | -0.148 | 0.000 |
| S1 1.005 | S1 1-5 | 30 Winter | 1 | +20% | 100/60 Winter | | | | 3.456 | -0.144 | 0.000 |
| S1 1.006 | S1 1-6 | 30 Winter | 1 | +20% | 30/120 Winter | | | | 3.454 | -0.108 | 0.000 |

| PN | US/MH Name | Flow / Cap. | Overflow (l/s) | Half Drain Time (mins) | Pipe Flow (l/s) | Status | Level Exceeded |
|----------|------------|-------------|----------------|------------------------|-----------------|--------|----------------|
| S1 1.000 | S1 1-0 | 0.00 | | | 0.0 | OK | |
| S1 1.001 | S1 1-1 | 0.03 | | | 2.1 | OK | |
| S1 1.002 | S1 1-2 | 0.04 | | | 3.4 | OK | |
| S1 1.003 | S1 1-3 | 0.07 | | | 4.4 | OK | |
| S1 1.004 | S1 1-4 | 0.00 | | | 3.7 | OK | |
| S1 2.000 | S1 2-0 | 0.08 | | 5 | 3.1 | OK | |
| S1 2.001 | S1 2-1 | 0.20 | | | 7.1 | OK | |
| S1 2.002 | S1 2-2 | 0.25 | | | 7.1 | OK | |
| S1 1.005 | S1 1-5 | 0.21 | | | 6.2 | OK | |
| S1 1.006 | S1 1-6 | 0.21 | | | 6.2 | OK | |

| | | |
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| Innovyze | Network 2020.1.3 | |

30 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for Storm 1

Simulation Criteria

| | | | |
|---------------------------------|-------|--|-------|
| Areal Reduction Factor | 1.000 | Additional Flow - % of Total Flow | 0.000 |
| Hot Start (mins) | 0 | MADD Factor * 10m ³ /ha Storage | 2.000 |
| Hot Start Level (mm) | 0 | Inlet Coefficient | 0.800 |
| Manhole Headloss Coeff (Global) | 0.500 | Flow per Person per Day (l/per/day) | 0.000 |
| Foul Sewage per hectare (l/s) | 0.000 | | |

| | | | | | |
|-----------------------------|---|------------------------------|---|------------------------------|---|
| Number of Input Hydrographs | 0 | Number of Offline Controls | 0 | Number of Time/Area Diagrams | 2 |
| Number of Online Controls | 1 | Number of Storage Structures | 1 | Number of Real Time Controls | 0 |

Synthetic Rainfall Details

| | | | | | |
|----------------|----------------------|---------|-------------|-------------|-------|
| Rainfall Model | FSR M5-60 (mm) | 17.200 | Cv (Summer) | 0.750 | |
| Region | Scotland and Ireland | Ratio R | 0.300 | Cv (Winter) | 0.840 |

| | |
|------------------------------------|---------------------------------|
| Margin for Flood Risk Warning (mm) | 300.0 |
| Analysis Timestep | 2.5 Second Increment (Extended) |
| DTS Status | ON |
| DVD Status | OFF |
| Inertia Status | OFF |

| | |
|--------------------------|---|
| Profile(s) | Summer and Winter |
| Duration(s) (mins) | 15, 30, 60, 120, 180, 240, 360, 480, 600, 720, 960, 1440, 2160, 2880, 4320, 5760, 7200, 8640, 10080 |
| Return Period(s) (years) | 1, 30, 100 |
| Climate Change (%) | 20, 20, 20 |

| PN | US/MH Name | Storm | Return Period | Climate Change | First (X) Surge | First (Y) Flood | First (Z) Overflow | Overflow Act. | Water Level (m) | Surcharged Depth (m) | Flooded Volume (m ³) |
|----------|------------|------------|---------------|----------------|-----------------|-----------------|--------------------|---------------|-----------------|----------------------|----------------------------------|
| S1 1.000 | S1 1-0 | 15 Summer | 30 | +20% | | | | | 4.300 | -0.300 | 0.000 |
| S1 1.001 | S1 1-1 | 120 Winter | 30 | +20% | | | | | 4.250 | -0.228 | 0.000 |
| S1 1.002 | S1 1-2 | 120 Winter | 30 | +20% | | | | | 4.146 | -0.212 | 0.000 |
| S1 1.003 | S1 1-3 | 120 Winter | 30 | +20% | | | | | 4.043 | -0.190 | 0.000 |
| S1 1.004 | S1 1-4 | 240 Winter | 30 | +20% | | | | | 3.588 | -0.616 | 0.000 |
| S1 2.000 | S1 2-0 | 15 Winter | 30 | +20% | | | | | 3.764 | -0.161 | 0.000 |
| S1 2.001 | S1 2-1 | 15 Winter | 30 | +20% | | | | | 3.693 | -0.110 | 0.000 |
| S1 2.002 | S1 2-2 | 15 Winter | 30 | +20% | | | | | 3.643 | -0.093 | 0.000 |
| S1 1.005 | S1 1-5 | 240 Winter | 30 | +20% | 100/60 Winter | | | | 3.588 | -0.012 | 0.000 |
| S1 1.006 | S1 1-6 | 240 Winter | 30 | +20% | 30/120 Winter | | | | 3.590 | 0.028 | 0.000 |

| PN | US/MH Name | Flow / Cap. | Overflow (l/s) | Half Drain Time (mins) | Pipe Flow (l/s) | Status | Level Exceeded |
|----------|------------|-------------|----------------|------------------------|-----------------|------------|----------------|
| S1 1.000 | S1 1-0 | 0.00 | | | 0.0 | OK | |
| S1 1.001 | S1 1-1 | 0.13 | | | 9.1 | OK | |
| S1 1.002 | S1 1-2 | 0.19 | | | 14.7 | OK | |
| S1 1.003 | S1 1-3 | 0.29 | | | 17.2 | OK | |
| S1 1.004 | S1 1-4 | 0.00 | | | 13.5 | OK | |
| S1 2.000 | S1 2-0 | 0.18 | | 6 | 6.9 | OK | |
| S1 2.001 | S1 2-1 | 0.51 | | | 18.2 | OK | |
| S1 2.002 | S1 2-2 | 0.63 | | | 17.9 | OK | |
| S1 1.005 | S1 1-5 | 0.33 | | | 9.9 | OK | |
| S1 1.006 | S1 1-6 | 0.32 | | | 9.5 | SURCHARGED | |

| | | |
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| Innovyze | Network 2020.1.3 | |

100 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for Storm 1

Simulation Criteria

| | | | |
|---------------------------------|-------|--|-------|
| Areal Reduction Factor | 1.000 | Additional Flow - % of Total Flow | 0.000 |
| Hot Start (mins) | 0 | MADD Factor * 10m ³ /ha Storage | 2.000 |
| Hot Start Level (mm) | 0 | Inlet Coefficient | 0.800 |
| Manhole Headloss Coeff (Global) | 0.500 | Flow per Person per Day (l/per/day) | 0.000 |
| Foul Sewage per hectare (l/s) | 0.000 | | |

| | | | | | |
|-----------------------------|---|------------------------------|---|------------------------------|---|
| Number of Input Hydrographs | 0 | Number of Offline Controls | 0 | Number of Time/Area Diagrams | 2 |
| Number of Online Controls | 1 | Number of Storage Structures | 1 | Number of Real Time Controls | 0 |

Synthetic Rainfall Details

| | | | | | |
|----------------|----------------------|---------|-------------|-------------|-------|
| Rainfall Model | FSR M5-60 (mm) | 17.200 | Cv (Summer) | 0.750 | |
| Region | Scotland and Ireland | Ratio R | 0.300 | Cv (Winter) | 0.840 |

| | |
|------------------------------------|---------------------------------|
| Margin for Flood Risk Warning (mm) | 300.0 |
| Analysis Timestep | 2.5 Second Increment (Extended) |
| DTS Status | ON |
| DVD Status | OFF |
| Inertia Status | OFF |

| | |
|--------------------------|---|
| Profile(s) | Summer and Winter |
| Duration(s) (mins) | 15, 30, 60, 120, 180, 240, 360, 480, 600, 720, 960, 1440, 2160, 2880, 4320, 5760, 7200, 8640, 10080 |
| Return Period(s) (years) | 1, 30, 100 |
| Climate Change (%) | 20, 20, 20 |

| PN | US/MH Name | Storm | Return Period | Climate Change | First (X) Surge | First (Y) Flood | First (Z) Overflow | Overflow Act. | Water Level (m) | Surcharged Depth (m) | Flooded Volume (m ³) |
|----------|------------|------------|---------------|----------------|-----------------|-----------------|--------------------|---------------|-----------------|----------------------|----------------------------------|
| S1 1.000 | S1 1-0 | 15 Summer | 100 | +20% | | | | | 4.300 | -0.300 | 0.000 |
| S1 1.001 | S1 1-1 | 60 Winter | 100 | +20% | | | | | 4.266 | -0.212 | 0.000 |
| S1 1.002 | S1 1-2 | 60 Winter | 100 | +20% | | | | | 4.164 | -0.194 | 0.000 |
| S1 1.003 | S1 1-3 | 120 Winter | 100 | +20% | | | | | 4.065 | -0.168 | 0.000 |
| S1 1.004 | S1 1-4 | 240 Winter | 100 | +20% | | | | | 3.709 | -0.495 | 0.000 |
| S1 2.000 | S1 2-0 | 15 Winter | 100 | +20% | | | | | 3.774 | -0.151 | 0.000 |
| S1 2.001 | S1 2-1 | 15 Winter | 100 | +20% | | | | | 3.713 | -0.090 | 0.000 |
| S1 2.002 | S1 2-2 | 240 Winter | 100 | +20% | | | | | 3.710 | -0.026 | 0.000 |
| S1 1.005 | S1 1-5 | 240 Winter | 100 | +20% | 100/60 Winter | | | | 3.709 | 0.109 | 0.000 |
| S1 1.006 | S1 1-6 | 240 Winter | 100 | +20% | 30/120 Winter | | | | 3.710 | 0.148 | 0.000 |

| PN | US/MH Name | Flow / Cap. | Overflow (l/s) | Half Drain Time (mins) | Pipe Flow (l/s) | Status | Level Exceeded |
|----------|------------|-------------|----------------|------------------------|-----------------|------------|----------------|
| S1 1.000 | S1 1-0 | 0.00 | | | 0.0 | OK | |
| S1 1.001 | S1 1-1 | 0.19 | | | 13.1 | OK | |
| S1 1.002 | S1 1-2 | 0.27 | | | 21.0 | OK | |
| S1 1.003 | S1 1-3 | 0.40 | | | 23.9 | OK | |
| S1 1.004 | S1 1-4 | 0.00 | | | 17.2 | OK | |
| S1 2.000 | S1 2-0 | 0.24 | | 5 | 8.9 | OK | |
| S1 2.001 | S1 2-1 | 0.67 | | | 23.7 | OK | |
| S1 2.002 | S1 2-2 | 0.17 | | | 4.8 | OK | |
| S1 1.005 | S1 1-5 | 0.35 | | | 10.4 | SURCHARGED | |
| S1 1.006 | S1 1-6 | 0.33 | | | 10.0 | SURCHARGED | |

| | | |
|---|--|---|
| AECOM | | Page 1 |
| Midpoint Alencon Link Basingstoke, RG21 7PP | 60710277 LDA Dyke Road Galway Surface Water Network 2 Hydraulic Design |  |
| Date 16/12/2024 15:57 File Storm Water Network.MDX | Designed by Thorne Prophet Checked by Emma McKendrick | |
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STORM SEWER DESIGN by the Modified Rational Method

Design Criteria for Storm 2

Pipe Sizes STANDARD Manhole Sizes STANDARD

FSR Rainfall Model - Scotland and Ireland

| | | | |
|--------------------------------------|--------|---------------------------------------|-------|
| Return Period (years) | 5 | PIMP (%) | 100 |
| M5-60 (mm) | 17.200 | Add Flow / Climate Change (%) | 20 |
| Ratio R | 0.300 | Minimum Backdrop Height (m) | 0.000 |
| Maximum Rainfall (mm/hr) | 50 | Maximum Backdrop Height (m) | 0.000 |
| Maximum Time of Concentration (mins) | 30 | Min Design Depth for Optimisation (m) | 0.000 |
| Foul Sewage (l/s/ha) | 0.000 | Min Vel for Auto Design only (m/s) | 1.00 |
| Volumetric Runoff Coeff. | 0.750 | Min Slope for Optimisation (1:X) | 200 |

Designed with Level Soffits

Network Design Table for Storm 2

« - Indicates pipe capacity < flow

| PN | Length (m) | Fall (m) | Slope (1:X) | I.Area (ha) | T.E. (mins) | Base Flow (l/s) | k (mm) | HYD SECT | DIA (mm) | Section Type | Auto Design |
|----------|---------------|-------------|----------------|----------------|----------------|--------------------|-----------|-------------|-------------|--------------|----------------|
| S2 1.000 | 40.307 | 0.806 | 50.0 | 0.138 | 4.00 | 0.0 | 0.600 | o | 225 | Pipe/Conduit | 🚫 |
| S2 2.000 | 10.303 | 0.172 | 59.9 | 0.000 | 4.00 | 0.0 | 0.600 | o | 225 | Pipe/Conduit | ✅ |
| S2 1.001 | 57.447 | 0.191 | 300.8 | 0.047 | 0.00 | 0.0 | 0.600 | o | 300 | Pipe/Conduit | ✅ |
| S2 1.002 | 17.032 | 0.057 | 298.8 | 0.027 | 0.00 | 0.0 | 0.600 | o | 375 | Pipe/Conduit | ✅ |
| S2 1.003 | 18.671 | 0.062 | 300.0 | 0.000 | 0.00 | 0.0 | 0.600 | o | 375 | Pipe/Conduit | ✅ |
| S2 1.004 | 8.710 | 0.029 | 300.4 | 0.022 | 0.00 | 0.0 | 0.600 | o | 375 | Pipe/Conduit | ✅ |
| S2 1.005 | 11.269 | 0.023 | 500.0 | 0.000 | 0.00 | 0.0 | 0.600 | [] | -2 | Pipe/Conduit | 🚫 |
| S2 1.006 | 2.507 | 0.008 | 300.0 | 0.000 | 0.00 | 0.0 | 0.600 | o | 225 | Pipe/Conduit | 🚫 |
| S2 1.007 | 7.444 | 0.050 | 150.0 | 0.000 | 0.00 | 0.0 | 0.600 | o | 225 | Pipe/Conduit | 🚫 |

Network Results Table

| PN | Rain (mm/hr) | T.C. (mins) | US/IL (m) | E I.Area (ha) | E Base Flow (l/s) | Foul (l/s) | Add Flow (l/s) | Vel (m/s) | Cap (l/s) | Flow (l/s) |
|----------|-----------------|----------------|--------------|------------------|----------------------|---------------|-------------------|--------------|--------------|---------------|
| S2 1.000 | 50.00 | 4.36 | 6.000 | 0.138 | 0.0 | 0.0 | 3.7 | 1.85 | 73.7 | 22.4 |
| S2 2.000 | 50.00 | 4.10 | 5.366 | 0.000 | 25.0 | 0.0 | 4.2 | 1.69 | 67.3 | 25.0 |
| S2 1.001 | 50.00 | 5.42 | 4.314 | 0.185 | 25.0 | 0.0 | 10.0 | 0.90 | 63.7 | 60.1 |
| S2 1.002 | 50.00 | 5.70 | 4.048 | 0.212 | 25.0 | 0.0 | 10.7 | 1.04 | 115.2 | 64.4 |
| S2 1.003 | 50.00 | 6.00 | 3.991 | 0.212 | 25.0 | 0.0 | 10.7 | 1.04 | 115.0 | 64.4 |
| S2 1.004 | 50.00 | 6.14 | 3.929 | 0.234 | 25.0 | 0.0 | 11.3 | 1.04 | 114.9 | 68.0 |
| S2 1.005 | 50.00 | 6.25 | 3.900 | 0.234 | 25.0 | 0.0 | 11.3 | 1.60 | 9145.2 | 68.0 |
| S2 1.006 | 50.00 | 6.31 | 3.877 | 0.234 | 25.0 | 0.0 | 11.3 | 0.75 | 29.8« | 68.0 |
| S2 1.007 | 50.00 | 4.12 | 3.869 | 0.000 | 15.0 | 0.0 | 2.5 | 1.07 | 42.4 | 15.0 |

Midpoint
Alencon Link
Basingstoke, RG21 7PP

60710277 LDA Dyke Road Galway
Surface Water Network 2
Hydraulic Design



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Checked by Emma McKendrick

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Manhole Schedules for Storm 2

| MH Name | MH CL (m) | MH Depth (m) | MH Connection | MH Diam., L*W (mm) | PN | Pipe Out Invert Level (m) | Diameter (mm) | PN | Pipes In Invert Level (m) | Diameter (mm) | Backdrop (mm) |
|----------|-----------|--------------|---------------|--------------------|----------|---------------------------|---------------|----------|---------------------------|---------------|---------------|
| S2 1-0 | 7.150 | 1.150 | Open Manhole | 1200 | S2 1.000 | 6.000 | 225 | | | | |
| S2 2-0 | 6.702 | 1.336 | Open Manhole | 1200 | S2 2.000 | 5.366 | 225 | | | | |
| S2 1-1 | 6.336 | 2.022 | Open Manhole | 1200 | S2 1.001 | 4.314 | 300 | S2 1.000 | 5.194 | 225 | 805 |
| | | | | | | | | S2 2.000 | 5.194 | 225 | 805 |
| S2 1-2 | 5.000 | 0.952 | Open Manhole | 1350 | S2 1.002 | 4.048 | 375 | S2 1.001 | 4.123 | 300 | |
| S2 1-3 | 5.189 | 1.198 | Open Manhole | 1350 | S2 1.003 | 3.991 | 375 | S2 1.002 | 3.991 | 375 | |
| S2 1-4 | 5.000 | 1.071 | Open Manhole | 1350 | S2 1.004 | 3.929 | 375 | S2 1.003 | 3.929 | 375 | |
| S2 1-5 | 5.000 | 1.100 | Open Manhole | 3000 | S2 1.005 | 3.900 | -2 | S2 1.004 | 3.900 | 375 | |
| S2 1-6 | 5.000 | 1.123 | Open Manhole | 3000 | S2 1.006 | 3.877 | 225 | S2 1.005 | 3.877 | -2 | |
| S2 1-7 | 5.000 | 1.131 | Open Manhole | 1200 | S2 1.007 | 3.869 | 225 | S2 1.006 | 3.869 | 225 | |
| S2 Ex MH | 5.299 | 1.480 | Open Manhole | 600 | | OUTFALL | | S2 1.007 | 3.819 | 225 | |

| MH Name | Manhole Easting (m) | Manhole Northing (m) | Intersection Easting (m) | Intersection Northing (m) | Manhole Access | Layout (North) |
|----------|---------------------|----------------------|--------------------------|---------------------------|----------------|----------------|
| S2 1-0 | 529899.110 | 725910.236 | 529899.110 | 725910.236 | Required | |
| S2 2-0 | 529863.275 | 725887.424 | 529863.275 | 725887.424 | Required | |
| S2 1-1 | 529860.886 | 725897.446 | 529860.886 | 725897.446 | Required | |
| S2 1-2 | 529842.822 | 725951.978 | 529842.822 | 725951.978 | Required | |
| S2 1-3 | 529837.432 | 725968.135 | 529837.432 | 725968.135 | Required | |
| S2 1-4 | 529844.564 | 725985.390 | 529844.564 | 725985.390 | Required | |
| S2 1-5 | 529839.705 | 725992.619 | 529839.705 | 725992.619 | Required | |
| S2 1-6 | 529832.887 | 726001.592 | 529832.887 | 726001.592 | Required | |
| S2 1-7 | 529831.488 | 726003.672 | 529831.488 | 726003.672 | Required | |
| S2 Ex MH | 529824.069 | 726004.278 | | | No Entry | |

Midpoint
Alencon Link
Basingstoke, RG21 7PP

60710277 LDA Dyke Road Galway
Surafce Water Network 2
Hydraulic Design



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PIPELINE SCHEDULES for Storm 2

Upstream Manhole

| PN | Hyd Sect | Diam (mm) | MH Name | C.Level (m) | I.Level (m) | D.Depth (m) | MH Connection | MH DIAM., L*W (mm) |
|----------|----------|-----------|---------|-------------|-------------|-------------|---------------|--------------------|
| S2 1.000 | o | 225 | S2 1-0 | 7.150 | 6.000 | 0.925 | Open Manhole | 1200 |
| S2 2.000 | o | 225 | S2 2-0 | 6.702 | 5.366 | 1.111 | Open Manhole | 1200 |
| S2 1.001 | o | 300 | S2 1-1 | 6.336 | 4.314 | 1.722 | Open Manhole | 1200 |
| S2 1.002 | o | 375 | S2 1-2 | 5.000 | 4.048 | 0.577 | Open Manhole | 1350 |
| S2 1.003 | o | 375 | S2 1-3 | 5.189 | 3.991 | 0.823 | Open Manhole | 1350 |
| S2 1.004 | o | 375 | S2 1-4 | 5.000 | 3.929 | 0.696 | Open Manhole | 1350 |
| S2 1.005 | [] | -2 | S2 1-5 | 5.000 | 3.900 | 0.500 | Open Manhole | 3000 |
| S2 1.006 | o | 225 | S2 1-6 | 5.000 | 3.877 | 0.898 | Open Manhole | 3000 |
| S2 1.007 | o | 225 | S2 1-7 | 5.000 | 3.869 | 0.906 | Open Manhole | 1200 |

Downstream Manhole

| PN | Length (m) | Slope (1:X) | MH Name | C.Level (m) | I.Level (m) | D.Depth (m) | MH Connection | MH DIAM., L*W (mm) |
|----------|------------|-------------|----------|-------------|-------------|-------------|---------------|--------------------|
| S2 1.000 | 40.307 | 50.0 | S2 1-1 | 6.336 | 5.194 | 0.917 | Open Manhole | 1200 |
| S2 2.000 | 10.303 | 59.9 | S2 1-1 | 6.336 | 5.194 | 0.917 | Open Manhole | 1200 |
| S2 1.001 | 57.447 | 300.8 | S2 1-2 | 5.000 | 4.123 | 0.577 | Open Manhole | 1350 |
| S2 1.002 | 17.032 | 298.8 | S2 1-3 | 5.189 | 3.991 | 0.823 | Open Manhole | 1350 |
| S2 1.003 | 18.671 | 300.0 | S2 1-4 | 5.000 | 3.929 | 0.696 | Open Manhole | 1350 |
| S2 1.004 | 8.710 | 300.4 | S2 1-5 | 5.000 | 3.900 | 0.725 | Open Manhole | 3000 |
| S2 1.005 | 11.269 | 500.0 | S2 1-6 | 5.000 | 3.877 | 0.523 | Open Manhole | 3000 |
| S2 1.006 | 2.507 | 300.0 | S2 1-7 | 5.000 | 3.869 | 0.906 | Open Manhole | 1200 |
| S2 1.007 | 7.444 | 150.0 | S2 Ex MH | 5.299 | 3.819 | 1.255 | Open Manhole | 600 |

Free Flowing Outfall Details for Storm 2

| Outfall Pipe Number | Outfall Name | C. Level (m) | I. Level (m) | Min I. Level (m) | D,L (mm) | W (mm) |
|---------------------|--------------|--------------|--------------|------------------|----------|--------|
| S2 1.007 | S2 Ex MH | 5.299 | 3.819 | 0.000 | 600 | 0 |

Simulation Criteria for Storm 2

| | | | |
|---------------------------------|-------|--|-------|
| Volumetric Runoff Coeff | 0.750 | Additional Flow - % of Total Flow | 0.000 |
| Areal Reduction Factor | 1.000 | MADD Factor * 10m ³ /ha Storage | 2.000 |
| Hot Start (mins) | 0 | Inlet Coefficient | 0.800 |
| Hot Start Level (mm) | 0 | Flow per Person per Day (l/per/day) | 0.000 |
| Manhole Headloss Coeff (Global) | 0.500 | Run Time (mins) | 60 |
| Foul Sewage per hectare (l/s) | 0.000 | Output Interval (mins) | 1 |
| Number of Input Hydrographs | 0 | Number of Offline Controls | 0 |
| Number of Online Controls | 1 | Number of Storage Structures | 2 |
| | | Number of Time/Area Diagrams | 1 |
| | | Number of Real Time Controls | 0 |

Synthetic Rainfall Details

| | | | |
|-----------------------|----------------------|-----------------------|--------|
| Rainfall Model | FSR | Profile Type | Summer |
| Return Period (years) | 5 | Cv (Summer) | 0.750 |
| Region | Scotland and Ireland | Cv (Winter) | 0.840 |
| M5-60 (mm) | 17.100 | Storm Duration (mins) | 30 |
| Ratio R | 0.300 | | |

| | | |
|---|--|---|
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Online Controls for Storm 2

Hydro-Brake® Optimum Manhole: S2 1-7, DS/PN: S2 1.007, Volume (m³): 1.3

| | | | |
|-------------------|----------------------------|-----------------------------------|-------|
| Unit Reference | MD-SHE-0176-1500-0950-1500 | Sump Available | Yes |
| Design Head (m) | 0.950 | Diameter (mm) | 176 |
| Design Flow (l/s) | 15.0 | Invert Level (m) | 3.869 |
| Flush-Flo™ | Calculated | Minimum Outlet Pipe Diameter (mm) | 225 |
| Objective | Minimise upstream storage | Suggested Manhole Diameter (mm) | 1200 |
| Application | Surface | | |

| Control Points | Head (m) | Flow (l/s) | Control Points | Head (m) | Flow (l/s) |
|---------------------------|----------|------------|---------------------------|----------|------------|
| Design Point (Calculated) | 0.950 | 15.0 | Kick-Flo® | 0.676 | 12.8 |
| Flush-Flo™ | 0.313 | 15.0 | Mean Flow over Head Range | - | 12.7 |

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

| Depth (m) | Flow (l/s) |
|-----------|------------|-----------|------------|-----------|------------|-----------|------------|-----------|------------|
| 0.100 | 6.2 | 0.800 | 13.8 | 2.000 | 21.3 | 4.000 | 29.7 | 7.000 | 39.0 |
| 0.200 | 14.5 | 1.000 | 15.3 | 2.200 | 22.3 | 4.500 | 31.5 | 7.500 | 40.3 |
| 0.300 | 15.0 | 1.200 | 16.7 | 2.400 | 23.3 | 5.000 | 33.1 | 8.000 | 41.6 |
| 0.400 | 14.8 | 1.400 | 18.0 | 2.600 | 24.2 | 5.500 | 34.7 | 8.500 | 42.8 |
| 0.500 | 14.5 | 1.600 | 19.2 | 3.000 | 25.9 | 6.000 | 36.2 | 9.000 | 44.0 |
| 0.600 | 13.9 | 1.800 | 20.3 | 3.500 | 27.9 | 6.500 | 37.6 | 9.500 | 45.2 |

| | | |
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1 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for Storm 2

Simulation Criteria

| | | | |
|---------------------------------|-------|--|-------|
| Areal Reduction Factor | 1.000 | Additional Flow - % of Total Flow | 0.000 |
| Hot Start (mins) | 0 | MADD Factor * 10m ³ /ha Storage | 2.000 |
| Hot Start Level (mm) | 0 | Inlet Coefficient | 0.800 |
| Manhole Headloss Coeff (Global) | 0.500 | Flow per Person per Day (l/per/day) | 0.000 |
| Foul Sewage per hectare (l/s) | 0.000 | | |

| | | | | | |
|-----------------------------|---|------------------------------|---|------------------------------|---|
| Number of Input Hydrographs | 0 | Number of Offline Controls | 0 | Number of Time/Area Diagrams | 1 |
| Number of Online Controls | 1 | Number of Storage Structures | 2 | Number of Real Time Controls | 0 |

Synthetic Rainfall Details

| | | | | | |
|----------------|----------------------|---------|-------------|-------------|-------|
| Rainfall Model | FSR M5-60 (mm) | 17.200 | Cv (Summer) | 0.750 | |
| Region | Scotland and Ireland | Ratio R | 0.300 | Cv (Winter) | 0.840 |

| | |
|------------------------------------|---------------------------------|
| Margin for Flood Risk Warning (mm) | 300.0 |
| Analysis Timestep | 2.5 Second Increment (Extended) |
| DTS Status | ON |
| DVD Status | OFF |
| Inertia Status | OFF |

| | |
|--------------------------|---|
| Profile(s) | Summer and Winter |
| Duration(s) (mins) | 15, 30, 60, 120, 180, 240, 360, 480, 600, 720, 960, 1440, 2160, 2880, 4320, 5760, 7200, 8640, 10080 |
| Return Period(s) (years) | 1, 30, 100 |
| Climate Change (%) | 20, 20, 20 |

| PN | US/MH Name | Storm | Return Period | Climate Change | First (X) Surge | First (Y) Flood | First (Z) Overflow | Overflow Act. | Water Level (m) | Surcharged Depth (m) | Flooded Volume (m ³) | Flow / Cap. |
|----------|------------|------------|---------------|----------------|-----------------|-----------------|--------------------|---------------|-----------------|----------------------|----------------------------------|-------------|
| S2 1.000 | S2 1-0 | 360 Winter | 1 | +20% | | | | | 6.012 | -0.213 | 0.000 | 0.01 |
| S2 2.000 | S2 2-0 | 15 Summer | 1 | +20% | | | | | 5.366 | -0.225 | 0.000 | 0.00 |
| S2 1.001 | S2 1-1 | 15 Winter | 1 | +20% | | | | | 4.376 | -0.238 | 0.000 | 0.10 |
| S2 1.002 | S2 1-2 | 15 Winter | 1 | +20% | | | | | 4.126 | -0.297 | 0.000 | 0.09 |
| S2 1.003 | S2 1-3 | 15 Winter | 1 | +20% | | | | | 4.071 | -0.295 | 0.000 | 0.09 |
| S2 1.004 | S2 1-4 | 15 Winter | 1 | +20% | | | | | 4.021 | -0.283 | 0.000 | 0.14 |
| S2 1.005 | S2 1-5 | 30 Winter | 1 | +20% | | | | | 3.971 | -0.529 | 0.000 | 0.00 |
| S2 1.006 | S2 1-6 | 30 Winter | 1 | +20% | | | | | 3.971 | -0.131 | 0.000 | 0.22 |
| S2 1.007 | S2 1-7 | 30 Winter | 1 | +20% | | | | | 3.968 | -0.126 | 0.000 | 0.19 |

| PN | US/MH Name | Overflow (l/s) | Half Drain Time (mins) | Pipe Flow (l/s) | Level Exceeded |
|----------|------------|----------------|------------------------|-----------------|----------------|
| S2 1.000 | S2 1-0 | | | 0.8 | OK |
| S2 2.000 | S2 2-0 | | | 0.0 | OK |
| S2 1.001 | S2 1-1 | | | 5.8 | OK |
| S2 1.002 | S2 1-2 | | 8 | 8.9 | OK |
| S2 1.003 | S2 1-3 | | | 8.9 | OK |
| S2 1.004 | S2 1-4 | | 10 | 11.3 | OK |
| S2 1.005 | S2 1-5 | | | 9.1 | OK |
| S2 1.006 | S2 1-6 | | | 6.1 | OK |
| S2 1.007 | S2 1-7 | | | 6.1 | OK |

| | | |
|---|--|---|
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| Innovyze | Network 2020.1.3 | |

30 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for Storm 2

Simulation Criteria

| | | | |
|---------------------------------|-------|--|-------|
| Areal Reduction Factor | 1.000 | Additional Flow - % of Total Flow | 0.000 |
| Hot Start (mins) | 0 | MADD Factor * 10m ³ /ha Storage | 2.000 |
| Hot Start Level (mm) | 0 | Inlet Coefficient | 0.800 |
| Manhole Headloss Coeff (Global) | 0.500 | Flow per Person per Day (l/per/day) | 0.000 |
| Foul Sewage per hectare (l/s) | 0.000 | | |

| | | | | | |
|-----------------------------|---|------------------------------|---|------------------------------|---|
| Number of Input Hydrographs | 0 | Number of Offline Controls | 0 | Number of Time/Area Diagrams | 1 |
| Number of Online Controls | 1 | Number of Storage Structures | 2 | Number of Real Time Controls | 0 |

Synthetic Rainfall Details

| | | | | | |
|----------------|----------------------|---------|-------------|-------------|-------|
| Rainfall Model | FSR M5-60 (mm) | 17.200 | Cv (Summer) | 0.750 | |
| Region | Scotland and Ireland | Ratio R | 0.300 | Cv (Winter) | 0.840 |

| | |
|------------------------------------|---------------------------------|
| Margin for Flood Risk Warning (mm) | 300.0 |
| Analysis Timestep | 2.5 Second Increment (Extended) |
| DTS Status | ON |
| DVD Status | OFF |
| Inertia Status | OFF |

| | |
|--------------------------|---|
| Profile(s) | Summer and Winter |
| Duration(s) (mins) | 15, 30, 60, 120, 180, 240, 360, 480, 600, 720, 960, 1440, 2160, 2880, 4320, 5760, 7200, 8640, 10080 |
| Return Period(s) (years) | 1, 30, 100 |
| Climate Change (%) | 20, 20, 20 |

| PN | US/MH Name | Storm | Return Period | Climate Change | First (X) Surge | First (Y) Flood | First (Z) Overflow | Overflow Act. | Water Level (m) | Surcharged Depth (m) | Flooded Volume (m ³) | Flow / Cap. |
|----------|------------|------------|---------------|----------------|-----------------|-----------------|--------------------|---------------|-----------------|----------------------|----------------------------------|-------------|
| S2 1.000 | S2 1-0 | 120 Winter | 30 | +20% | | | | | 6.032 | -0.193 | 0.000 | 0.05 |
| S2 2.000 | S2 2-0 | 15 Summer | 30 | +20% | | | | | 5.366 | -0.225 | 0.000 | 0.00 |
| S2 1.001 | S2 1-1 | 15 Winter | 30 | +20% | | | | | 4.419 | -0.195 | 0.000 | 0.25 |
| S2 1.002 | S2 1-2 | 15 Winter | 30 | +20% | | | | | 4.180 | -0.243 | 0.000 | 0.25 |
| S2 1.003 | S2 1-3 | 15 Winter | 30 | +20% | | | | | 4.129 | -0.237 | 0.000 | 0.24 |
| S2 1.004 | S2 1-4 | 15 Winter | 30 | +20% | | | | | 4.085 | -0.219 | 0.000 | 0.36 |
| S2 1.005 | S2 1-5 | 30 Winter | 30 | +20% | | | | | 4.033 | -0.467 | 0.000 | 0.01 |
| S2 1.006 | S2 1-6 | 30 Winter | 30 | +20% | | | | | 4.033 | -0.069 | 0.000 | 0.44 |
| S2 1.007 | S2 1-7 | 30 Winter | 30 | +20% | | | | | 4.031 | -0.063 | 0.000 | 0.39 |

| PN | US/MH Name | Overflow (l/s) | Half Drain Time (mins) | Pipe Flow (l/s) | Level Exceeded |
|----------|------------|----------------|------------------------|-----------------|----------------|
| S2 1.000 | S2 1-0 | | | 3.4 | OK |
| S2 2.000 | S2 2-0 | | | 0.0 | OK |
| S2 1.001 | S2 1-1 | | | 15.1 | OK |
| S2 1.002 | S2 1-2 | | 4 | 23.7 | OK |
| S2 1.003 | S2 1-3 | | | 23.2 | OK |
| S2 1.004 | S2 1-4 | | 6 | 29.3 | OK |
| S2 1.005 | S2 1-5 | | | 23.1 | OK |
| S2 1.006 | S2 1-6 | | | 12.2 | OK |
| S2 1.007 | S2 1-7 | | | 12.1 | OK |

| | | |
|---|--|---|
| AECOM | | Page 8 |
| Midpoint Alencon Link Basingstoke, RG21 7PP | 60710277 LDA Dyke Road Galway Surface Water Network 2 Hydraulic Design |  |
| Date 16/12/2024 15:57 File Storm Water Network.MDX | Designed by Thorne Prophet Checked by Emma McKendrick | |
| Innovyze | Network 2020.1.3 | |

100 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for Storm 2

Simulation Criteria

| | | | |
|---------------------------------|-------|--|-------|
| Areal Reduction Factor | 1.000 | Additional Flow - % of Total Flow | 0.000 |
| Hot Start (mins) | 0 | MADD Factor * 10m ³ /ha Storage | 2.000 |
| Hot Start Level (mm) | 0 | Inlet Coefficient | 0.800 |
| Manhole Headloss Coeff (Global) | 0.500 | Flow per Person per Day (l/per/day) | 0.000 |
| Foul Sewage per hectare (l/s) | 0.000 | | |

| | | | | | |
|-----------------------------|---|------------------------------|---|------------------------------|---|
| Number of Input Hydrographs | 0 | Number of Offline Controls | 0 | Number of Time/Area Diagrams | 1 |
| Number of Online Controls | 1 | Number of Storage Structures | 2 | Number of Real Time Controls | 0 |

Synthetic Rainfall Details

| | | | | | |
|----------------|----------------------|---------|-------------|-------------|-------|
| Rainfall Model | FSR M5-60 (mm) | 17.200 | Cv (Summer) | 0.750 | |
| Region | Scotland and Ireland | Ratio R | 0.300 | Cv (Winter) | 0.840 |

| | |
|------------------------------------|---------------------------------|
| Margin for Flood Risk Warning (mm) | 300.0 |
| Analysis Timestep | 2.5 Second Increment (Extended) |
| DTS Status | ON |
| DVD Status | OFF |
| Inertia Status | OFF |

| | |
|--------------------------|---|
| Profile(s) | Summer and Winter |
| Duration(s) (mins) | 15, 30, 60, 120, 180, 240, 360, 480, 600, 720, 960, 1440, 2160, 2880, 4320, 5760, 7200, 8640, 10080 |
| Return Period(s) (years) | 1, 30, 100 |
| Climate Change (%) | 20, 20, 20 |

| PN | US/MH Name | Storm | Return Period | Climate Change | First (X) Surge | First (Y) Flood | First (Z) Overflow | Overflow Act. | Water Level (m) | Surcharged Depth (m) | Flooded Volume (m ³) | Flow / Cap. |
|----------|------------|-----------|---------------|----------------|-----------------|-----------------|--------------------|---------------|-----------------|----------------------|----------------------------------|-------------|
| S2 1.000 | S2 1-0 | 60 Winter | 100 | +20% | | | | | 6.039 | -0.186 | 0.000 | 0.07 |
| S2 2.000 | S2 2-0 | 15 Summer | 100 | +20% | | | | | 5.366 | -0.225 | 0.000 | 0.00 |
| S2 1.001 | S2 1-1 | 15 Winter | 100 | +20% | | | | | 4.436 | -0.178 | 0.000 | 0.32 |
| S2 1.002 | S2 1-2 | 15 Winter | 100 | +20% | | | | | 4.202 | -0.221 | 0.000 | 0.33 |
| S2 1.003 | S2 1-3 | 15 Winter | 100 | +20% | | | | | 4.152 | -0.214 | 0.000 | 0.32 |
| S2 1.004 | S2 1-4 | 15 Winter | 100 | +20% | | | | | 4.110 | -0.193 | 0.000 | 0.46 |
| S2 1.005 | S2 1-5 | 30 Winter | 100 | +20% | | | | | 4.067 | -0.433 | 0.000 | 0.01 |
| S2 1.006 | S2 1-6 | 30 Winter | 100 | +20% | | | | | 4.067 | -0.035 | 0.000 | 0.52 |
| S2 1.007 | S2 1-7 | 30 Winter | 100 | +20% | | | | | 4.062 | -0.032 | 0.000 | 0.45 |

| PN | US/MH Name | Overflow (l/s) | Half Drain Time (mins) | Pipe Flow (l/s) | Status | Level Exceeded |
|----------|------------|----------------|------------------------|-----------------|--------|----------------|
| S2 1.000 | S2 1-0 | | | 4.9 | OK | |
| S2 2.000 | S2 2-0 | | | 0.0 | OK | |
| S2 1.001 | S2 1-1 | | | 19.6 | OK | |
| S2 1.002 | S2 1-2 | | 5 | 30.7 | OK | |
| S2 1.003 | S2 1-3 | | | 30.1 | OK | |
| S2 1.004 | S2 1-4 | | 9 | 38.0 | OK | |
| S2 1.005 | S2 1-5 | | | 29.7 | OK | |
| S2 1.006 | S2 1-6 | | | 14.5 | OK | |
| S2 1.007 | S2 1-7 | | | 14.2 | OK | |

| | | |
|---|--|---|
| AECOM | | Page 1 |
| Midpoint Alencon Link Basingstoke, RG21 7PP | 60710277 LDA Dyke Road Galway Surafce Water (Diversion) Hydraulic Design |  |
| Date 16/12/2024 16:12 File Storm Water Network.MDX | Designed by Thorne Prophet Checked by Emma McKendrick | |
| Innovyze | Network 2020.1.3 | |

STORM SEWER DESIGN by the Modified Rational Method

Design Criteria for Storm Diversion

Pipe Sizes STANDARD Manhole Sizes STANDARD

FSR Rainfall Model - Scotland and Ireland

| | | | |
|--------------------------------------|--------|---------------------------------------|-------|
| Return Period (years) | 5 | PIMP (%) | 100 |
| M5-60 (mm) | 17.200 | Add Flow / Climate Change (%) | 20 |
| Ratio R | 0.300 | Minimum Backdrop Height (m) | 0.200 |
| Maximum Rainfall (mm/hr) | 50 | Maximum Backdrop Height (m) | 1.500 |
| Maximum Time of Concentration (mins) | 30 | Min Design Depth for Optimisation (m) | 1.200 |
| Foul Sewage (l/s/ha) | 0.000 | Min Vel for Auto Design only (m/s) | 1.00 |
| Volumetric Runoff Coeff. | 0.750 | Min Slope for Optimisation (1:X) | 200 |

Designed with Level Soffits

Network Design Table for Storm Diversion

| PN | Length (m) | Fall (m) | Slope (1:X) | I.Area (ha) | T.E. (mins) | Base Flow (l/s) | k (mm) | HYD SECT | DIA (mm) | Section Type | Auto Design |
|--------|------------|----------|-------------|-------------|-------------|-----------------|--------|----------|----------|--------------|---|
| D1.000 | 17.805 | 0.079 | 225.4 | 0.000 | 4.00 | 11.0 | 0.600 | o | 225 | Pipe/Conduit |  |
| D1.001 | 80.189 | 0.356 | 225.0 | 0.000 | 0.00 | 0.0 | 0.600 | o | 225 | Pipe/Conduit |  |
| D1.002 | 19.124 | 0.085 | 225.0 | 0.000 | 0.00 | 0.0 | 0.600 | o | 225 | Pipe/Conduit |  |
| D2.000 | 13.864 | 0.164 | 84.5 | 0.360 | 4.00 | 0.0 | 0.600 | o | 225 | Pipe/Conduit |  |
| D1.003 | 6.532 | 0.048 | 137.5 | 0.000 | 0.00 | 0.0 | 0.600 | o | 300 | Pipe/Conduit |  |
| D3.000 | 14.067 | 0.056 | 250.0 | 0.000 | 4.00 | 11.0 | 0.600 | o | 450 | Pipe/Conduit |  |
| D3.001 | 59.020 | 0.295 | 200.0 | 0.000 | 0.00 | 0.0 | 0.600 | o | 450 | Pipe/Conduit |  |
| D3.002 | 9.749 | 0.049 | 200.0 | 0.000 | 0.00 | 0.0 | 0.600 | o | 450 | Pipe/Conduit |  |
| D3.003 | 37.209 | 0.248 | 150.0 | 0.000 | 0.00 | 0.0 | 0.600 | o | 450 | Pipe/Conduit |  |
| D3.004 | 35.667 | 0.224 | 159.0 | 0.000 | 0.00 | 0.0 | 0.600 | o | 600 | Pipe/Conduit |  |

Network Results Table

| PN | Rain (mm/hr) | T.C. (mins) | US/IL (m) | E I.Area (ha) | E Base Flow (l/s) | Foul (l/s) | Add Flow (l/s) | Vel (m/s) | Cap (l/s) | Flow (l/s) |
|--------|--------------|-------------|-----------|---------------|-------------------|------------|----------------|-----------|-----------|------------|
| D1.000 | 50.00 | 4.34 | 3.852 | 0.000 | 11.0 | 0.0 | 2.2 | 0.87 | 34.5 | 13.2 |
| D1.001 | 50.00 | 5.88 | 3.773 | 0.000 | 11.0 | 0.0 | 2.2 | 0.87 | 34.5 | 13.2 |
| D1.002 | 50.00 | 6.25 | 3.417 | 0.000 | 11.0 | 0.0 | 2.2 | 0.87 | 34.5 | 13.2 |
| D2.000 | 50.00 | 4.16 | 3.496 | 0.000 | 25.0 | 0.0 | 4.2 | 1.42 | 56.6 | 25.0 |
| D1.003 | 50.00 | 6.33 | 3.257 | 0.000 | 36.0 | 0.0 | 7.2 | 1.34 | 94.6 | 43.2 |
| D3.000 | 50.00 | 4.18 | 4.242 | 0.000 | 11.0 | 0.0 | 2.2 | 1.28 | 203.8 | 13.2 |
| D3.001 | 50.00 | 4.87 | 4.186 | 0.000 | 11.0 | 0.0 | 2.2 | 1.43 | 228.1 | 13.2 |
| D3.002 | 50.00 | 4.98 | 3.891 | 0.000 | 11.0 | 0.0 | 2.2 | 1.43 | 228.1 | 13.2 |
| D3.003 | 50.00 | 5.36 | 3.842 | 0.000 | 11.0 | 0.0 | 2.2 | 1.66 | 263.6 | 13.2 |
| D3.004 | 50.00 | 4.31 | 3.444 | 0.000 | 15.0 | 0.0 | 2.5 | 1.93 | 545.3 | 15.0 |

Midpoint
Alencon Link
Basingstoke, RG21 7PP

60710277 LDA Dyke Road Galway
Surface Water (Diversion)
Hydraulic Design



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Checked by Emma McKendrick

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Manhole Schedules for Storm Diversion

| MH Name | MH CL (m) | MH Depth (m) | MH Connection | MH Diam., L*W (mm) | PN | Pipe Out Invert Level (m) | Diameter (mm) | PN | Pipes In Invert Level (m) | Diameter (mm) | Backdrop (mm) |
|---------|-----------|--------------|---------------|--------------------|--------|---------------------------|---------------|--------|---------------------------|---------------|---------------|
| D1-0 | 5.330 | 1.478 | Open Manhole | 1200 | D1.000 | 3.852 | 225 | | | | |
| D1-1 | 5.114 | 1.341 | Open Manhole | 1200 | D1.001 | 3.773 | 225 | D1.000 | 3.773 | 225 | |
| D1-2 | 5.000 | 1.583 | Open Manhole | 1200 | D1.002 | 3.417 | 225 | D1.001 | 3.417 | 225 | |
| D2-0 | 4.977 | 1.481 | Open Manhole | 1200 | D2.000 | 3.496 | 225 | | | | |
| D1-3 | 5.000 | 1.743 | Open Manhole | 1200 | D1.003 | 3.257 | 300 | D1.002 | 3.332 | 225 | |
| | | | | | | | | D2.000 | 3.332 | 225 | |
| D | 5.000 | 1.791 | Open Manhole | 525 | | OUTFALL | | D1.003 | 3.209 | 300 | |
| D3-0 | 6.330 | 2.088 | Open Manhole | 1350 | D3.000 | 4.242 | 450 | | | | |
| D3-1 | 6.278 | 2.092 | Open Manhole | 1350 | D3.001 | 4.186 | 450 | D3.000 | 4.186 | 450 | |
| D3-2 | 6.001 | 2.110 | Open Manhole | 1350 | D3.002 | 3.891 | 450 | D3.001 | 3.891 | 450 | |
| D3-3 | 5.886 | 2.044 | Open Manhole | 1350 | D3.003 | 3.842 | 450 | D3.002 | 3.842 | 450 | |
| D3-4 | 5.131 | 1.687 | Open Manhole | 1500 | D3.004 | 3.444 | 600 | D3.003 | 3.594 | 450 | |
| D | 5.002 | 1.782 | Open Manhole | 600 | | OUTFALL | | D3.004 | 3.220 | 600 | |

| MH Name | Manhole Easting (m) | Manhole Northing (m) | Intersection Easting (m) | Intersection Northing (m) | Manhole Access | Layout (North) |
|---------|---------------------|----------------------|--------------------------|---------------------------|----------------|----------------|
| D1-0 | 529896.296 | 725973.256 | 529896.296 | 725973.256 | Required | |
| D1-1 | 529883.944 | 725986.080 | 529883.944 | 725986.080 | Required | |
| D1-2 | 529839.206 | 726052.629 | 529839.206 | 726052.629 | Required | |
| D2-0 | 529816.164 | 726057.896 | 529816.164 | 726057.896 | Required | |
| D1-3 | 529821.607 | 726045.145 | 529821.607 | 726045.145 | Required | |
| D | 529815.600 | 726042.580 | | | No Entry | |
| D3-0 | 529857.422 | 725890.199 | 529857.422 | 725890.199 | Required | |
| D3-1 | 529851.166 | 725902.798 | 529851.166 | 725902.798 | Required | |
| D3-2 | 529833.549 | 725959.127 | 529833.549 | 725959.127 | Required | |
| D3-3 | 529835.121 | 725968.749 | 529835.121 | 725968.749 | Required | |
| D3-4 | 529824.069 | 726004.278 | 529824.069 | 726004.278 | Required | |
| D | 529816.551 | 726039.144 | | | No Entry | |

Midpoint
Alencon Link
Basingstoke, RG21 7PP

60710277 LDA Dyke Road Galway
Surafce Water (Diversion)
Hydraulic Design



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PIPELINE SCHEDULES for Storm Diversion

Upstream Manhole

| PN | Hyd Sect | Diam (mm) | MH Name | C.Level (m) | I.Level (m) | D.Depth (m) | MH Connection | MH DIAM., L*W (mm) |
|--------|----------|-----------|---------|-------------|-------------|-------------|---------------|--------------------|
| D1.000 | o | 225 | D1-0 | 5.330 | 3.852 | 1.253 | Open Manhole | 1200 |
| D1.001 | o | 225 | D1-1 | 5.114 | 3.773 | 1.116 | Open Manhole | 1200 |
| D1.002 | o | 225 | D1-2 | 5.000 | 3.417 | 1.358 | Open Manhole | 1200 |
| D2.000 | o | 225 | D2-0 | 4.977 | 3.496 | 1.256 | Open Manhole | 1200 |
| D1.003 | o | 300 | D1-3 | 5.000 | 3.257 | 1.443 | Open Manhole | 1200 |
| D3.000 | o | 450 | D3-0 | 6.330 | 4.242 | 1.638 | Open Manhole | 1350 |
| D3.001 | o | 450 | D3-1 | 6.278 | 4.186 | 1.642 | Open Manhole | 1350 |
| D3.002 | o | 450 | D3-2 | 6.001 | 3.891 | 1.660 | Open Manhole | 1350 |
| D3.003 | o | 450 | D3-3 | 5.886 | 3.842 | 1.594 | Open Manhole | 1350 |
| D3.004 | o | 600 | D3-4 | 5.131 | 3.444 | 1.087 | Open Manhole | 1500 |

Downstream Manhole

| PN | Length (m) | Slope (1:X) | MH Name | C.Level (m) | I.Level (m) | D.Depth (m) | MH Connection | MH DIAM., L*W (mm) |
|--------|------------|-------------|---------|-------------|-------------|-------------|---------------|--------------------|
| D1.000 | 17.805 | 225.4 | D1-1 | 5.114 | 3.773 | 1.116 | Open Manhole | 1200 |
| D1.001 | 80.189 | 225.0 | D1-2 | 5.000 | 3.417 | 1.358 | Open Manhole | 1200 |
| D1.002 | 19.124 | 225.0 | D1-3 | 5.000 | 3.332 | 1.443 | Open Manhole | 1200 |
| D2.000 | 13.864 | 84.5 | D1-3 | 5.000 | 3.332 | 1.443 | Open Manhole | 1200 |
| D1.003 | 6.532 | 137.5 | D | 5.000 | 3.209 | 1.491 | Open Manhole | 525 |
| D3.000 | 14.067 | 250.0 | D3-1 | 6.278 | 4.186 | 1.642 | Open Manhole | 1350 |
| D3.001 | 59.020 | 200.0 | D3-2 | 6.001 | 3.891 | 1.660 | Open Manhole | 1350 |
| D3.002 | 9.749 | 200.0 | D3-3 | 5.886 | 3.842 | 1.594 | Open Manhole | 1350 |
| D3.003 | 37.209 | 150.0 | D3-4 | 5.131 | 3.594 | 1.087 | Open Manhole | 1500 |
| D3.004 | 35.667 | 159.0 | D | 5.002 | 3.220 | 1.182 | Open Manhole | 600 |

Free Flowing Outfall Details for Storm Diversion

| Outfall Pipe Number | Outfall Name | C. Level (m) | I. Level (m) | Min I. Level (m) | D,L (mm) | W (mm) |
|---------------------|--------------|--------------|--------------|------------------|----------|--------|
|---------------------|--------------|--------------|--------------|------------------|----------|--------|

| | | | | | | |
|--------|---|-------|-------|-------|-----|---|
| D1.003 | D | 5.000 | 3.209 | 3.209 | 525 | 0 |
|--------|---|-------|-------|-------|-----|---|

Free Flowing Outfall Details for Storm Diversion

| Outfall Pipe Number | Outfall Name | C. Level (m) | I. Level (m) | Min I. Level (m) | D,L (mm) | W (mm) |
|---------------------|--------------|--------------|--------------|------------------|----------|--------|
|---------------------|--------------|--------------|--------------|------------------|----------|--------|

| | | | | | | |
|--------|---|-------|-------|-------|-----|---|
| D3.004 | D | 5.002 | 3.220 | 3.220 | 600 | 0 |
|--------|---|-------|-------|-------|-----|---|

| | | |
|------------------------------|-------------------------------|---|
| AECOM | | Page 4 |
| Midpoint | 60710277 LDA Dyke Road Galway |  |
| Alencon Link | Surface Water (Diversion) | |
| Basingstoke, RG21 7PP | Hydraulic Design | |
| Date 16/12/2024 16:12 | Designed by Thorne Prophet | |
| File Storm Water Network.MDX | Checked by Emma McKendrick | |
| Innovyze | Network 2020.1.3 | |

Simulation Criteria for Storm Diversion

| | | | |
|---------------------------------|-------|--|-------|
| Volumetric Runoff Coeff | 0.750 | Additional Flow - % of Total Flow | 0.000 |
| Areal Reduction Factor | 1.000 | MADD Factor * 10m ³ /ha Storage | 2.000 |
| Hot Start (mins) | 0 | Inlet Coefficient | 0.800 |
| Hot Start Level (mm) | 0 | Flow per Person per Day (l/per/day) | 0.000 |
| Manhole Headloss Coeff (Global) | 0.500 | Run Time (mins) | 60 |
| Foul Sewage per hectare (l/s) | 0.000 | Output Interval (mins) | 1 |

Number of Input Hydrographs 0 Number of Offline Controls 0 Number of Time/Area Diagrams 0
Number of Online Controls 0 Number of Storage Structures 0 Number of Real Time Controls 0

Synthetic Rainfall Details

| | | | |
|-----------------------|----------------------|-----------------------|--------|
| Rainfall Model | FSR | Profile Type | Summer |
| Return Period (years) | 5 | Cv (Summer) | 0.750 |
| Region | Scotland and Ireland | Cv (Winter) | 0.840 |
| M5-60 (mm) | 17.100 | Storm Duration (mins) | 30 |
| Ratio R | 0.300 | | |

| | | |
|---|--|---|
| AECOM | | Page 5 |
| Midpoint Alencon Link Basingstoke, RG21 7PP | 60710277 LDA Dyke Road Galway Surafce Water (Diversion) Hydraulic Design |  |
| Date 16/12/2024 16:12 File Storm Water Network.MDX | Designed by Thorne Prophet Checked by Emma McKendrick | |
| Innovyze | Network 2020.1.3 | |

1 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for Storm Diversion

Simulation Criteria

| | | | |
|---------------------------------|-------|--|-------|
| Areal Reduction Factor | 1.000 | Additional Flow - % of Total Flow | 0.000 |
| Hot Start (mins) | 0 | MADD Factor * 10m ³ /ha Storage | 2.000 |
| Hot Start Level (mm) | 0 | Inlet Coeffiecient | 0.800 |
| Manhole Headloss Coeff (Global) | 0.500 | Flow per Person per Day (l/per/day) | 0.000 |
| Foul Sewage per hectare (l/s) | 0.000 | | |

| | | | | | |
|-----------------------------|---|------------------------------|---|------------------------------|---|
| Number of Input Hydrographs | 0 | Number of Offline Controls | 0 | Number of Time/Area Diagrams | 0 |
| Number of Online Controls | 0 | Number of Storage Structures | 0 | Number of Real Time Controls | 0 |

Synthetic Rainfall Details

| | | | | | |
|----------------|----------------------|---------|-------------|-------------|-------|
| Rainfall Model | FSR M5-60 (mm) | 17.200 | Cv (Summer) | 0.750 | |
| Region | Scotland and Ireland | Ratio R | 0.300 | Cv (Winter) | 0.840 |

| | |
|------------------------------------|---------------------------------|
| Margin for Flood Risk Warning (mm) | 300.0 |
| Analysis Timestep | 2.5 Second Increment (Extended) |
| DTS Status | ON |
| DVD Status | OFF |
| Inertia Status | OFF |

| | |
|--------------------------|---|
| Profile(s) | Summer and Winter |
| Duration(s) (mins) | 15, 30, 60, 120, 180, 240, 360, 480, 600, 720, 960, 1440, 2160, 2880, 4320, 5760, 7200, 8640, 10080 |
| Return Period(s) (years) | 1, 30, 100 |
| Climate Change (%) | 20, 20, 20 |

| PN | US/MH Name | Storm | Return Period | Climate Change | First (X) Surge | First (Y) Flood | First (Z) Overflow | Overflow Act. | Water Level (m) | Surcharged Depth (m) | Flooded Volume (m ³) |
|--------|------------|------------|---------------|----------------|-----------------|-----------------|--------------------|---------------|-----------------|----------------------|----------------------------------|
| D1.000 | D1-0 | 15 Summer | 1 | +20% | | | | | 3.945 | -0.132 | 0.000 |
| D1.001 | D1-1 | 960 Winter | 1 | +20% | | | | | 3.861 | -0.137 | 0.000 |
| D1.002 | D1-2 | 15 Winter | 1 | +20% | 30/15 Summer | | | | 3.541 | -0.100 | 0.000 |
| D2.000 | D2-0 | 15 Winter | 1 | +20% | 1/15 Summer | 100/15 Summer | | | 3.749 | 0.028 | 0.000 |
| D1.003 | D1-3 | 15 Winter | 1 | +20% | 30/15 Summer | | | | 3.518 | -0.039 | 0.000 |
| D3.000 | D3-0 | 15 Summer | 1 | +20% | | | | | 4.321 | -0.371 | 0.000 |
| D3.001 | D3-1 | 15 Summer | 1 | +20% | | | | | 4.252 | -0.384 | 0.000 |
| D3.002 | D3-2 | 15 Summer | 1 | +20% | | | | | 3.968 | -0.372 | 0.000 |
| D3.003 | D3-3 | 15 Summer | 1 | +20% | | | | | 3.905 | -0.387 | 0.000 |
| D3.004 | D3-4 | 15 Summer | 1 | +20% | | | | | 3.507 | -0.537 | 0.000 |

| PN | US/MH Name | Flow / Cap. | Overflow (l/s) | Half Drain Time (mins) | Pipe Flow (l/s) | Status | Level Exceeded |
|--------|------------|-------------|----------------|------------------------|-----------------|------------|----------------|
| D1.000 | D1-0 | 0.36 | | | 11.0 | OK | |
| D1.001 | D1-1 | 0.33 | | | 11.0 | OK | |
| D1.002 | D1-2 | 0.41 | | | 12.9 | OK | |
| D2.000 | D2-0 | 1.07 | | | 52.7 | SURCHARGED | 2 |
| D1.003 | D1-3 | 1.00 | | | 61.3 | OK | |
| D3.000 | D3-0 | 0.07 | | | 11.0 | OK | |
| D3.001 | D3-1 | 0.05 | | | 11.0 | OK | |
| D3.002 | D3-2 | 0.07 | | | 11.0 | OK | |
| D3.003 | D3-3 | 0.05 | | | 11.1 | OK | |
| D3.004 | D3-4 | 0.02 | | | 11.0 | OK | |

| | | |
|------------------------------|-------------------------------|---|
| AECOM | | Page 6 |
| Midpoint | 60710277 LDA Dyke Road Galway |  |
| Alencon Link | Surafce Water (Diversion) | |
| Basingstoke, RG21 7PP | Hydraulic Design | |
| Date 16/12/2024 16:12 | Designed by Thorne Prophet | |
| File Storm Water Network.MDX | Checked by Emma McKendrick | |
| Innovyze | Network 2020.1.3 | |

30 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for Storm Diversion

Simulation Criteria

| | | | |
|---------------------------------|-------|--|-------|
| Areal Reduction Factor | 1.000 | Additional Flow - % of Total Flow | 0.000 |
| Hot Start (mins) | 0 | MADD Factor * 10m ³ /ha Storage | 2.000 |
| Hot Start Level (mm) | 0 | Inlet Coeffiecient | 0.800 |
| Manhole Headloss Coeff (Global) | 0.500 | Flow per Person per Day (l/per/day) | 0.000 |
| Foul Sewage per hectare (l/s) | 0.000 | | |

| | | | | | |
|-----------------------------|---|------------------------------|---|------------------------------|---|
| Number of Input Hydrographs | 0 | Number of Offline Controls | 0 | Number of Time/Area Diagrams | 0 |
| Number of Online Controls | 0 | Number of Storage Structures | 0 | Number of Real Time Controls | 0 |

Synthetic Rainfall Details

| | | | | | |
|----------------|----------------------|---------|-------------|-------------|-------|
| Rainfall Model | FSR M5-60 (mm) | 17.200 | Cv (Summer) | 0.750 | |
| Region | Scotland and Ireland | Ratio R | 0.300 | Cv (Winter) | 0.840 |

| | |
|------------------------------------|---------------------------------|
| Margin for Flood Risk Warning (mm) | 300.0 |
| Analysis Timestep | 2.5 Second Increment (Extended) |
| DTS Status | ON |
| DVD Status | OFF |
| Inertia Status | OFF |

| | |
|--------------------------|---|
| Profile(s) | Summer and Winter |
| Duration(s) (mins) | 15, 30, 60, 120, 180, 240, 360, 480, 600, 720, 960, 1440, 2160, 2880, 4320, 5760, 7200, 8640, 10080 |
| Return Period(s) (years) | 1, 30, 100 |
| Climate Change (%) | 20, 20, 20 |

| PN | US/MH Name | Storm | Return Period | Climate Change | First (X) Surge | First (Y) Flood | First (Z) Overflow | Overflow Act. | Water Level (m) | Surcharged Depth (m) | Flooded Volume (m ²) |
|--------|------------|--------------|---------------|----------------|-----------------|-----------------|--------------------|---------------|-----------------|----------------------|----------------------------------|
| D1.000 | D1-0 | 15 Summer | 30 | +20% | | | | | 3.945 | -0.132 | 0.000 |
| D1.001 | D1-1 | 10080 Summer | 30 | +20% | | | | | 3.861 | -0.137 | 0.000 |
| D1.002 | D1-2 | 15 Winter | 30 | +20% | 30/15 Summer | | | | 3.729 | 0.087 | 0.000 |
| D2.000 | D2-0 | 15 Winter | 30 | +20% | 1/15 Summer | 100/15 Summer | | | 4.514 | 0.793 | 0.000 |
| D1.003 | D1-3 | 15 Winter | 30 | +20% | 30/15 Summer | | | | 3.710 | 0.153 | 0.000 |
| D3.000 | D3-0 | 15 Summer | 30 | +20% | | | | | 4.321 | -0.371 | 0.000 |
| D3.001 | D3-1 | 15 Summer | 30 | +20% | | | | | 4.252 | -0.384 | 0.000 |
| D3.002 | D3-2 | 15 Summer | 30 | +20% | | | | | 3.968 | -0.372 | 0.000 |
| D3.003 | D3-3 | 15 Summer | 30 | +20% | | | | | 3.905 | -0.387 | 0.000 |
| D3.004 | D3-4 | 15 Summer | 30 | +20% | | | | | 3.507 | -0.537 | 0.000 |

| PN | US/MH Name | Flow / Cap. | Overflow (l/s) | Half Drain Time (mins) | Pipe Flow (l/s) | Status | Level Exceeded |
|--------|------------|-------------|----------------|------------------------|-----------------|------------|----------------|
| D1.000 | D1-0 | 0.36 | | | 11.0 | OK | |
| D1.001 | D1-1 | 0.33 | | | 11.0 | OK | |
| D1.002 | D1-2 | 0.60 | | | 18.8 | SURCHARGED | |
| D2.000 | D2-0 | 2.21 | | | 108.8 | SURCHARGED | 2 |
| D1.003 | D1-3 | 1.87 | | | 114.8 | SURCHARGED | |
| D3.000 | D3-0 | 0.07 | | | 11.0 | OK | |
| D3.001 | D3-1 | 0.05 | | | 11.0 | OK | |
| D3.002 | D3-2 | 0.07 | | | 11.0 | OK | |
| D3.003 | D3-3 | 0.05 | | | 11.1 | OK | |
| D3.004 | D3-4 | 0.02 | | | 11.0 | OK | |

| | | |
|------------------------------|-------------------------------|---|
| AECOM | | Page 7 |
| Midpoint | 60710277 LDA Dyke Road Galway |  |
| Alencon Link | Surafce Water (Diversion) | |
| Basingstoke, RG21 7PP | Hydraulic Design | |
| Date 16/12/2024 16:12 | Designed by Thorne Prophet | |
| File Storm Water Network.MDX | Checked by Emma McKendrick | |
| Innovyze | Network 2020.1.3 | |

100 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for Storm Diversion

Simulation Criteria

| | | | |
|---------------------------------|-------|--|-------|
| Areal Reduction Factor | 1.000 | Additional Flow - % of Total Flow | 0.000 |
| Hot Start (mins) | 0 | MADD Factor * 10m ³ /ha Storage | 2.000 |
| Hot Start Level (mm) | 0 | Inlet Coefficient | 0.800 |
| Manhole Headloss Coeff (Global) | 0.500 | Flow per Person per Day (l/per/day) | 0.000 |
| Foul Sewage per hectare (l/s) | 0.000 | | |

| | | | | | |
|-----------------------------|---|------------------------------|---|------------------------------|---|
| Number of Input Hydrographs | 0 | Number of Offline Controls | 0 | Number of Time/Area Diagrams | 0 |
| Number of Online Controls | 0 | Number of Storage Structures | 0 | Number of Real Time Controls | 0 |

Synthetic Rainfall Details

| | | | | | |
|----------------|----------------------|---------|-------------|-------------|-------|
| Rainfall Model | FSR M5-60 (mm) | 17.200 | Cv (Summer) | 0.750 | |
| Region | Scotland and Ireland | Ratio R | 0.300 | Cv (Winter) | 0.840 |

| | |
|------------------------------------|---------------------------------|
| Margin for Flood Risk Warning (mm) | 300.0 |
| Analysis Timestep | 2.5 Second Increment (Extended) |
| DTS Status | ON |
| DVD Status | OFF |
| Inertia Status | OFF |

| | |
|--------------------------|---|
| Profile(s) | Summer and Winter |
| Duration(s) (mins) | 15, 30, 60, 120, 180, 240, 360, 480, 600, 720, 960, 1440, 2160, 2880, 4320, 5760, 7200, 8640, 10080 |
| Return Period(s) (years) | 1, 30, 100 |
| Climate Change (%) | 20, 20, 20 |

| PN | US/MH Name | Storm | Return Period | Climate Change | First (X) Surge | First (Y) Flood | First (Z) Overflow | Overflow Act. | Water | Surcharged | Flooded | Flow / Cap. |
|--------|------------|-----------|---------------|----------------|-----------------|-----------------|--------------------|---------------|-----------|------------|--------------------------|-------------|
| | | | | | | | | | Level (m) | Depth (m) | Volume (m ³) | |
| D1.000 | D1-0 | 15 Summer | 100 | +20% | | | | | 3.945 | -0.132 | 0.000 | 0.36 |
| D1.001 | D1-1 | 15 Winter | 100 | +20% | | | | | 3.875 | -0.123 | 0.000 | 0.33 |
| D1.002 | D1-2 | 15 Winter | 100 | +20% | 30/15 Summer | | | | 3.826 | 0.184 | 0.000 | 0.72 |
| D2.000 | D2-0 | 15 Winter | 100 | +20% | 1/15 Summer | 100/15 Summer | | | 4.978 | 1.257 | 0.896 | 2.69 |
| D1.003 | D1-3 | 15 Winter | 100 | +20% | 30/15 Summer | | | | 3.802 | 0.245 | 0.000 | 2.27 |
| D3.000 | D3-0 | 15 Summer | 100 | +20% | | | | | 4.321 | -0.371 | 0.000 | 0.07 |
| D3.001 | D3-1 | 15 Summer | 100 | +20% | | | | | 4.252 | -0.384 | 0.000 | 0.05 |
| D3.002 | D3-2 | 15 Summer | 100 | +20% | | | | | 3.968 | -0.372 | 0.000 | 0.07 |
| D3.003 | D3-3 | 15 Summer | 100 | +20% | | | | | 3.905 | -0.387 | 0.000 | 0.05 |
| D3.004 | D3-4 | 15 Summer | 100 | +20% | | | | | 3.507 | -0.537 | 0.000 | 0.02 |

| PN | US/MH Name | Overflow (l/s) | Half Drain | Pipe | Level Exceeded |
|--------|------------|----------------|-------------|------------|----------------|
| | | | Time (mins) | Flow (l/s) | |
| D1.000 | D1-0 | | 11.0 | | OK |
| D1.001 | D1-1 | | 11.2 | | OK |
| D1.002 | D1-2 | | 22.4 | | SURCHARGED |
| D2.000 | D2-0 | | 132.6 | | FLOOD 2 |
| D1.003 | D1-3 | | 139.4 | | SURCHARGED |
| D3.000 | D3-0 | | 11.0 | | OK |
| D3.001 | D3-1 | | 11.0 | | OK |
| D3.002 | D3-2 | | 11.0 | | OK |
| D3.003 | D3-3 | | 11.1 | | OK |
| D3.004 | D3-4 | | 11.0 | | OK |

Appendix C Wastewater Drainage Calculations

| | | |
|---|---|---|
| AECOM | | Page 1 |
| Midpoint Alencon Link Basingstoke, RG21 7PP | 60710277 LDA Dyke Road Galway Foul Network 1 Hydraulic Design |  |
| Date 16/12/2024 16:16 File Storm Water Network.MDX | Designed by Thorne Prophet Checked by Emma McKendrick | |
| Innovyze | Network 2020.1.3 | |

FOUL SEWERAGE DESIGN

Design Criteria for Foul

Pipe Sizes STANDARD Manhole Sizes STANDARD

| | | | |
|-----------------------------|--------|---------------------------------------|-------|
| Industrial Flow (l/s/ha) | 0.00 | Add Flow / Climate Change (%) | 20 |
| Industrial Peak Flow Factor | 0.00 | Minimum Backdrop Height (m) | 0.000 |
| Flow Per Person (l/per/day) | 222.00 | Maximum Backdrop Height (m) | 0.000 |
| Persons per House | 3.00 | Min Design Depth for Optimisation (m) | 0.000 |
| Domestic (l/s/ha) | 0.00 | Min Vel for Auto Design only (m/s) | 0.75 |
| Domestic Peak Flow Factor | 6.00 | Min Slope for Optimisation (1:X) | 500 |

Designed with Level Inverts

Network Design Table for Foul

| PN | Length (m) | Fall (m) | Slope (1:X) | Area (ha) | Houses | Base Flow (l/s) | k (mm) | HYD SECT | DIA (mm) | Section Type | Auto Design |
|--------|------------|----------|-------------|-----------|--------|-----------------|--------|----------|----------|--------------|-------------|
| F1.000 | 23.663 | 0.158 | 149.8 | 0.000 | 0 | 2.0 | 1.500 | o | 225 | Pipe/Conduit | 🚫 |
| F1.001 | 32.927 | 0.220 | 149.7 | 0.000 | 0 | 0.0 | 1.500 | o | 225 | Pipe/Conduit | 🟢 |
| F1.002 | 50.579 | 0.337 | 150.1 | 0.000 | 0 | 6.8 | 1.500 | o | 225 | Pipe/Conduit | 🟢 |
| F1.003 | 31.775 | 0.219 | 145.1 | 0.000 | 0 | 0.0 | 1.500 | o | 225 | Pipe/Conduit | 🟢 |
| F1.004 | 38.397 | 0.219 | 175.3 | 0.000 | 0 | 0.0 | 1.500 | o | 225 | Pipe/Conduit | 🟢 |
| F2.000 | 6.544 | 0.084 | 77.9 | 0.000 | 0 | 0.0 | 1.500 | o | 225 | Pipe/Conduit | 🚫 |
| F1.005 | 17.443 | 0.069 | 252.8 | 0.000 | 0 | 0.0 | 1.500 | o | 225 | Pipe/Conduit | 🟢 |
| F3.000 | 7.401 | 0.140 | 52.9 | 0.000 | 0 | 7.3 | 1.500 | o | 150 | Pipe/Conduit | 🚫 |
| F3.001 | 6.321 | 0.218 | 29.0 | 0.000 | 0 | 0.0 | 1.500 | o | 150 | Pipe/Conduit | 🟢 |
| F3.002 | 5.925 | 0.093 | 63.7 | 0.000 | 0 | 0.0 | 1.500 | o | 150 | Pipe/Conduit | 🟢 |
| F1.006 | 2.000 | 0.069 | 29.0 | 0.000 | 0 | 0.0 | 1.500 | o | 225 | Pipe/Conduit | 🟢 |

Network Results Table

| PN | US/IL (m) | E Area (ha) | E Base Flow (l/s) | E Hse | Add Flow (l/s) | P.Dep (mm) | P.Vel (m/s) | Vel (m/s) | Cap (l/s) | Flow (l/s) |
|--------|-----------|-------------|-------------------|-------|----------------|------------|-------------|-----------|-----------|------------|
| F1.000 | 4.150 | 0.000 | 2.0 | 0 | 0.4 | 39 | 0.52 | 0.94 | 37.3 | 2.4 |
| F1.001 | 3.992 | 0.000 | 2.0 | 0 | 0.4 | 39 | 0.52 | 0.94 | 37.3 | 2.4 |
| F1.002 | 3.772 | 0.000 | 8.8 | 0 | 1.8 | 82 | 0.81 | 0.94 | 37.2 | 10.6 |
| F1.003 | 3.435 | 0.000 | 8.8 | 0 | 1.8 | 81 | 0.82 | 0.95 | 37.9 | 10.6 |
| F1.004 | 3.216 | 0.000 | 8.8 | 0 | 1.8 | 85 | 0.76 | 0.87 | 34.4 | 10.6 |
| F2.000 | 3.081 | 0.000 | 0.0 | 0 | 0.0 | 0 | 0.00 | 1.30 | 51.7 | 0.0 |
| F1.005 | 2.997 | 0.000 | 8.8 | 0 | 1.8 | 95 | 0.67 | 0.72 | 28.6 | 10.6 |
| F3.000 | 5.500 | 0.000 | 7.3 | 0 | 1.5 | 67 | 1.15 | 1.21 | 21.3 | 8.8 |
| F3.001 | 5.360 | 0.000 | 7.3 | 0 | 1.5 | 57 | 1.43 | 1.63 | 28.8 | 8.8 |
| F3.002 | 5.142 | 0.000 | 7.3 | 0 | 1.5 | 71 | 1.07 | 1.10 | 19.4 | 8.8 |
| F1.006 | 2.928 | 0.000 | 16.1 | 0 | 3.2 | 73 | 1.73 | 2.14 | 85.0 | 19.3 |

Midpoint
Alencon Link
Basingstoke, RG21 7PP

60710277 LDA Dyke Road Galway
Foul Network 1
Hydraulic Design



Date 16/12/2024 16:16
File Storm Water Network.MDX

Designed by Thorne Prophet
Checked by Emma McKendrick

Innovyze

Network 2020.1.3

Manhole Schedules for Foul

| MH Name | MH CL (m) | MH Depth (m) | MH Connection | MH Diam., L*W (mm) | PN | Pipe Out Invert Level (m) | Diameter (mm) | PN | Pipes In Invert Level (m) | Diameter (mm) | Backdrop (mm) |
|---------|-----------|--------------|---------------|--------------------|--------|---------------------------|---------------|--------|---------------------------|---------------|---------------|
| F1-0 | 4.953 | 0.803 | Open Manhole | 1200 | F1.000 | 4.150 | 225 | | | | |
| F1-1 | 5.000 | 1.008 | Open Manhole | 1200 | F1.001 | 3.992 | 225 | F1.000 | 3.992 | 225 | |
| F1-2 | 5.805 | 2.033 | Open Manhole | 1200 | F1.002 | 3.772 | 225 | F1.001 | 3.772 | 225 | |
| F1-3 | 5.478 | 2.043 | Open Manhole | 1200 | F1.003 | 3.435 | 225 | F1.002 | 3.435 | 225 | |
| F1-4 | 5.589 | 2.373 | Open Manhole | 1200 | F1.004 | 3.216 | 225 | F1.003 | 3.216 | 225 | |
| F2-0 | 6.164 | 3.083 | Open Manhole | 1200 | F2.000 | 3.081 | 225 | | | | |
| F1-5 | 6.261 | 3.264 | Open Manhole | 1200 | F1.005 | 2.997 | 225 | F1.004 | 2.997 | 225 | |
| | | | | | | | | F2.000 | 2.997 | 225 | |
| F3-0 | 6.986 | 1.486 | Open Manhole | 1200 | F3.000 | 5.500 | 150 | | | | |
| F3-1 | 6.954 | 1.594 | Open Manhole | 1200 | F3.001 | 5.360 | 150 | F3.000 | 5.360 | 150 | |
| F3-2 | 6.817 | 1.675 | Open Manhole | 1200 | F3.002 | 5.142 | 150 | F3.001 | 5.142 | 150 | |
| F1-6 | 6.689 | 3.761 | Open Manhole | 1200 | F1.006 | 2.928 | 225 | F1.005 | 2.928 | 225 | |
| | | | | | | | | F3.002 | 5.049 | 150 | 2046 |
| FWWPS | 6.689 | 3.830 | Open Manhole | 0 | | OUTFALL | | F1.006 | 2.859 | 225 | |

| MH Name | Manhole Easting (m) | Manhole Northing (m) | Intersection Easting (m) | Intersection Northing (m) | Manhole Access | Layout (North) |
|---------|---------------------|----------------------|--------------------------|---------------------------|----------------|----------------|
| F1-0 | 529820.352 | 726061.058 | 529820.352 | 726061.058 | Required | |
| F1-1 | 529810.592 | 726039.502 | 529810.592 | 726039.502 | Required | |
| F1-2 | 529820.962 | 726008.251 | 529820.962 | 726008.251 | Required | |
| F1-3 | 529836.889 | 725960.245 | 529836.889 | 725960.245 | Required | |
| F1-4 | 529846.896 | 725930.087 | 529846.896 | 725930.087 | Required | |
| F2-0 | 529853.728 | 725889.749 | 529853.728 | 725889.749 | Required | |
| F1-5 | 529858.987 | 725893.643 | 529858.987 | 725893.643 | Required | |
| F3-0 | 529889.491 | 725896.046 | 529889.491 | 725896.046 | Required | |
| F3-1 | 529887.143 | 725903.065 | 529887.143 | 725903.065 | Required | |
| F3-2 | 529881.148 | 725901.059 | 529881.148 | 725901.059 | Required | |
| F1-6 | 529875.529 | 725899.178 | 529875.529 | 725899.178 | Required | |
| FWWPS | 529876.164 | 725897.282 | | | No Entry | |

Midpoint
Alencon Link
Basingstoke, RG21 7PP

60710277 LDA Dyke Road Galway
Foul Network 1
Hydraulic Design



Date 16/12/2024 16:16
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Designed by Thorne Prophet
Checked by Emma McKendrick

Innovyze

Network 2020.1.3

PIPELINE SCHEDULES for Foul

Upstream Manhole

| PN | Hyd Sect | Diam (mm) | MH Name | C.Level (m) | I.Level (m) | D.Depth (m) | MH Connection | MH DIAM., L*W (mm) |
|--------|----------|-----------|---------|-------------|-------------|-------------|---------------|--------------------|
| F1.000 | o | 225 | F1-0 | 4.953 | 4.150 | 0.578 | Open Manhole | 1200 |
| F1.001 | o | 225 | F1-1 | 5.000 | 3.992 | 0.783 | Open Manhole | 1200 |
| F1.002 | o | 225 | F1-2 | 5.805 | 3.772 | 1.808 | Open Manhole | 1200 |
| F1.003 | o | 225 | F1-3 | 5.478 | 3.435 | 1.818 | Open Manhole | 1200 |
| F1.004 | o | 225 | F1-4 | 5.589 | 3.216 | 2.148 | Open Manhole | 1200 |
| F2.000 | o | 225 | F2-0 | 6.164 | 3.081 | 2.858 | Open Manhole | 1200 |
| F1.005 | o | 225 | F1-5 | 6.261 | 2.997 | 3.039 | Open Manhole | 1200 |
| F3.000 | o | 150 | F3-0 | 6.986 | 5.500 | 1.336 | Open Manhole | 1200 |
| F3.001 | o | 150 | F3-1 | 6.954 | 5.360 | 1.444 | Open Manhole | 1200 |
| F3.002 | o | 150 | F3-2 | 6.817 | 5.142 | 1.525 | Open Manhole | 1200 |
| F1.006 | o | 225 | F1-6 | 6.689 | 2.928 | 3.536 | Open Manhole | 1200 |

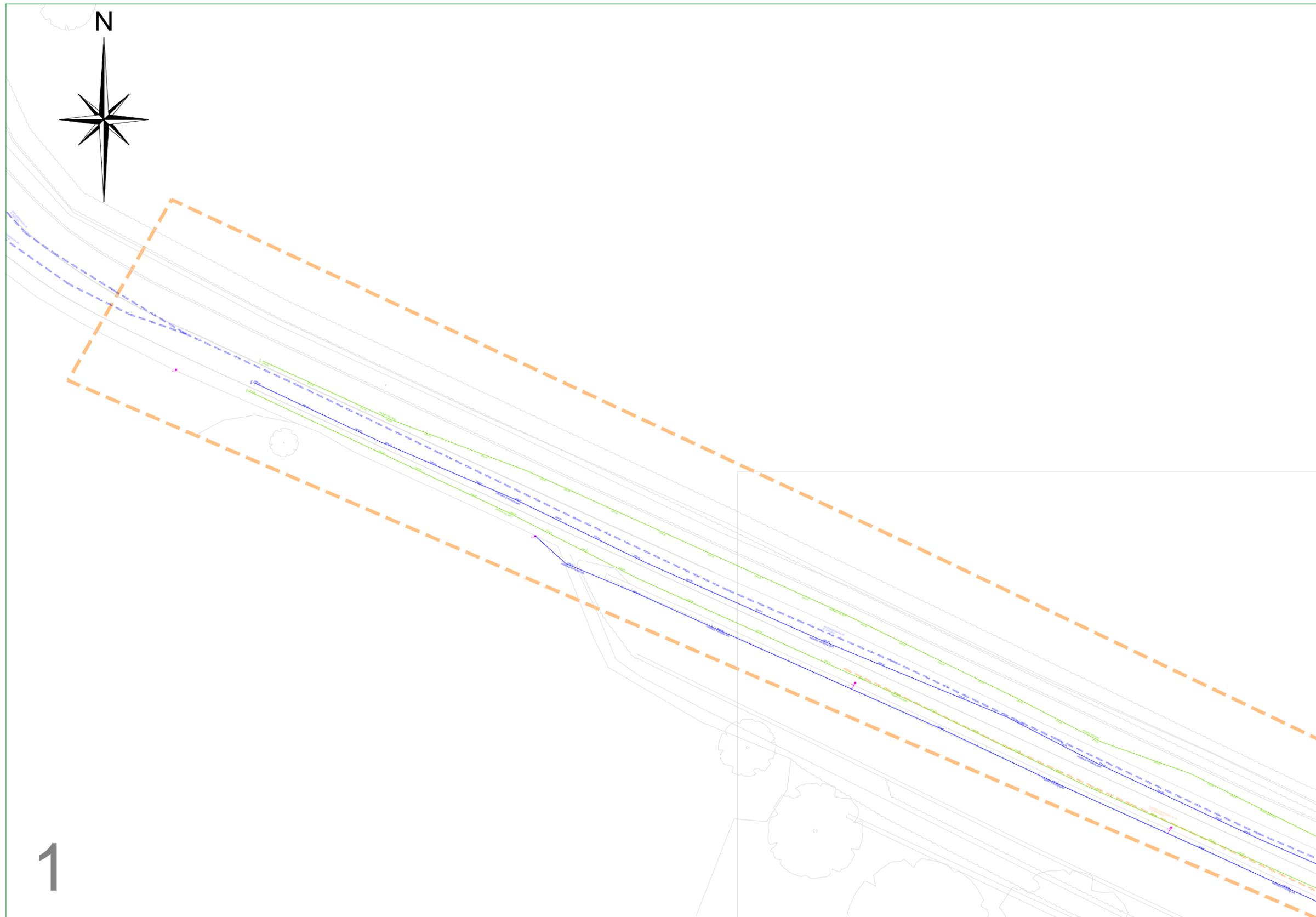
Downstream Manhole

| PN | Length (m) | Slope (1:X) | MH Name | C.Level (m) | I.Level (m) | D.Depth (m) | MH Connection | MH DIAM., L*W (mm) |
|--------|------------|-------------|---------|-------------|-------------|-------------|---------------|--------------------|
| F1.000 | 23.663 | 149.8 | F1-1 | 5.000 | 3.992 | 0.783 | Open Manhole | 1200 |
| F1.001 | 32.927 | 149.7 | F1-2 | 5.805 | 3.772 | 1.808 | Open Manhole | 1200 |
| F1.002 | 50.579 | 150.1 | F1-3 | 5.478 | 3.435 | 1.818 | Open Manhole | 1200 |
| F1.003 | 31.775 | 145.1 | F1-4 | 5.589 | 3.216 | 2.148 | Open Manhole | 1200 |
| F1.004 | 38.397 | 175.3 | F1-5 | 6.261 | 2.997 | 3.039 | Open Manhole | 1200 |
| F2.000 | 6.544 | 77.9 | F1-5 | 6.261 | 2.997 | 3.039 | Open Manhole | 1200 |
| F1.005 | 17.443 | 252.8 | F1-6 | 6.689 | 2.928 | 3.536 | Open Manhole | 1200 |
| F3.000 | 7.401 | 52.9 | F3-1 | 6.954 | 5.360 | 1.444 | Open Manhole | 1200 |
| F3.001 | 6.321 | 29.0 | F3-2 | 6.817 | 5.142 | 1.525 | Open Manhole | 1200 |
| F3.002 | 5.925 | 63.7 | F1-6 | 6.689 | 5.049 | 1.490 | Open Manhole | 1200 |
| F1.006 | 2.000 | 29.0 | FWWPS | 6.689 | 2.859 | 3.605 | Open Manhole | 0 |

Free Flowing Outfall Details for Foul

| Outfall Pipe Number | Outfall Name | C. Level (m) | I. Level (m) | Min I. Level (m) | D,L (mm) | W (mm) |
|---------------------|--------------|--------------|--------------|------------------|----------|--------|
| F1.006 | FWWPS | 6.689 | 2.859 | 0.000 | 0 | 0 |

Appendix D Utility Records and Utility Survey



PAS 128: 2014 (Quality of Survey Level Outputs):

| | |
|--------------------------------|---|
| DESKTOP UTILITY RECORDS SEARCH | |
| QL-D | Drafted from utility records |
| SITE RECONNAISSANCE | |
| QL-C | Location Demonstrated by visual reference to street furniture or evidence of previous streetworks, ie - reinstatement scars |
| DETECTION | |
| QL-B4 | A segment of utility suspected to exist but has not been detected by a geophysical technique |
| QL-B3 | Horizontal location only of the utility detected by one of the geophysical techniques used |
| QL-B2 | Horizontal and vertical location of the utility detected by one of the geophysical techniques used |
| QL-B1 | Horizontal and vertical location of the utility detected by multiple geophysical techniques |
| VERIFICATION | |
| QL-A | Horizontal and vertical location of the top and/or bottom of the utility |

Apex Surveys Ltd. Disclaimer - Utility Survey

The interpretative nature and the non-intrusive, indirect and non-destructive survey methods must be taken into account when considering the results of the surveys. Therefore Apex Surveys, while using appropriate practice to execute, interpret and present the data, gives no guarantees that all underground utilities and underground structures will be located and mapped. Furthermore, Apex Surveys cannot guarantee the accuracy of the utility depths annotated on the survey drawings. Apex Survey shall not be liable for any omissions or inaccuracies in the survey which arise due to the limitations of the service. No liability shall attach to Apex Surveys, in any circumstances, howsoever arising, in respect of any consequential loss or damages suffered by the Client.

The following is a non-exhaustive list of the limitations of utility surveys:

- Depth of Utility:** The depth and size of a utility affect the signal response and the degree with which a utility can be located. Due to attenuation of the radar signal with depth, resolution is restricted, hence making identification of utilities more difficult with increasing depth.
- Size of Utility:** The smaller the diameter of a utility the more difficult it is to locate. This difficulty increases with depth.
- Ground Conditions:** The depth penetration and quality of the data depends on the ground conditions of the site. GPR Surveying works best within high resistivity material. Clay overburden can impair GPR Surveying. Poor data may be a result of areas with high conductivity.
- Utility Congestion:** Where different utilities converge together into a service corridor or cross paths it becomes difficult to isolate a specific utility and to map its route. The reflected signal will display a single response to multiple utilities. Therefore multiple utilities may appear to be a single utility. Where similar services run on close proximity, separation may be impossible.
- Signal Jumping:** Signal from surrounding services may 'jump' to a highly conductive line masking its true identity.
- Shallowing:** (of deeper utilities by shallower objects) Shallow utilities will mask the existence of deeper utilities where they are in close proximity. Also, high reflective materials close to the surface i.e. rebar may hide deeper anomalies.
- Surface Obstructions:** The GPR system relies on a relatively flat and even surface on which to perform radar passes. If ground obstructions such as vehicles, organic material (long grass, scrub) or undulating ground surface are present then the acquired data will be of lower resolution and in some cases not viable.
- Loss of signal:** It is not always possible to trace the entire length of each underground service.
- Connections between manholes:** Connections between manhole chambers are assumed to be straight.
- Non-metallic objects:** Nonmetallic objects are amongst the most difficult to trace therefore successful tracing of non-metallic pipes/ utilities may be limited.
- Fiber Optic Cables:** Fiber optic cables may not be possible to locate except where laid with a built in tracer wire or similar conductor system.
- Defective / flooded manholes or pipework:** It may not be possible to establish connections between flooded or defective manholes or pipework.
- Acute bends in pipework:** It may not be possible to trace a pipe past an acute bend.

Accuracy estimates:

- Locational accuracy is determined by referring to the manufacturers guidelines for the detector used.
- In ideal conditions the spatial accuracies for the underground utilities may be +/- 5% for Radiodetection and +/- 10% of depth for the GPR to 2.5m deep. However variations within the subsurface, depth below the ground, close proximity of other services and local magnetic, atmospheric or ground conditions, bends, lateral service connections and any of the other limitations listed in this disclaimer may alter this estimated accuracy.
- Plan accuracies of + or - 150mm may be achieved but this figure will depend on the depth of service below ground level. However variations within the subsurface, depth below the ground, close proximity of other services and local magnetic, atmospheric or ground conditions, bends, lateral service connections and any of the other limitations listed in this disclaimer may alter this estimated accuracy.
- DP represents distance from the surface level to the top of the service/ target
- Where technically possible, depth indications will be given. These along with plan positions should be used for guidance only and wherever critical accuracy is required these should be confirmed by the client by undertaking trial excavations or similar.

Record Drawing Information

- Services which have been untraceable are shown from records where possible or available. These lines are annotated as "Taken From Records" or "From Records".
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APEX SURVEYS
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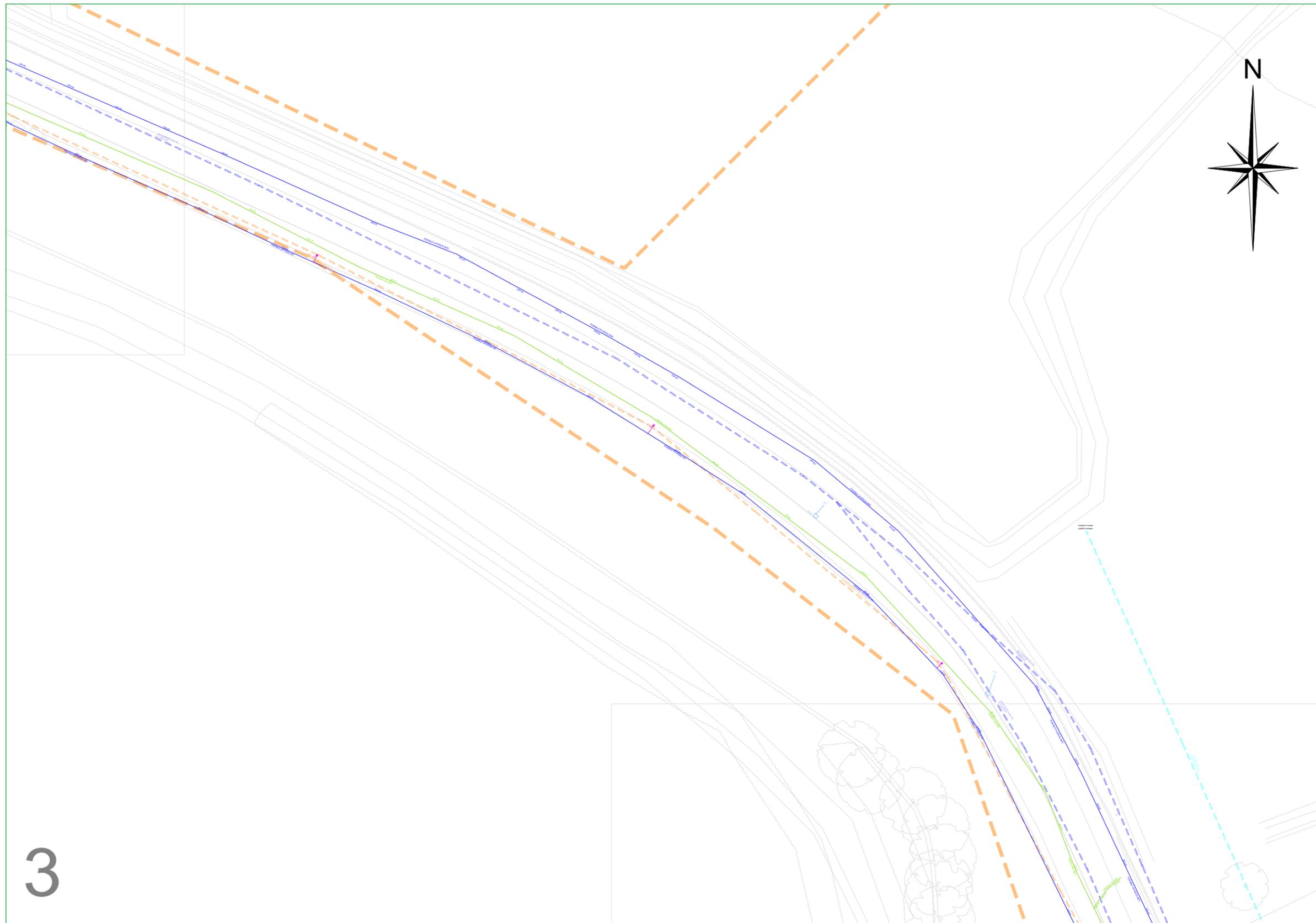
| | | | | | | | | |
|---------------------------|--|------------------------|---|--|--|-----------------------------|---|--|
| STREET FURNITURE : | BD + BS + CB EP + TP + ER + LP + MKR + TL + TB POST + RS - RS BH + TPIT + | SERVICES : | AV AJ CATV QL EIRCOM EIRCOM BOX ECP ESAT ESB ESB BOX FN GV IC MH SEPTIC SV | STOPCOCK SERVICE BOX (UNKNOWN) TRAFFIC COVER VENT WATER METER LEVELS : BED LEVEL FLOOR LEVEL INVERT LEVEL ROAD LEVEL SOFFIT LEVEL SPOT LEVEL TOP OF WALL LEVEL WATER LEVEL SURVEY CONTROL STATION | ST + BOX TLIC VENT + WM + +BED101.50 +FL101.50 +L101.50 +101.50 +SL101.50 +101.50 +TOP101.50 +WL101.50 | UNDERGROUND LEGEND : | WATER MAIN GAS MAIN STORM DRAIN FOUL SEWER COMBINED SEWER ELECTRIC CABLE ELECTRIC LIGHTING EIRCOM FIBRE OPTIC CABLE BROADBAND CABLE TV TRAFFIC AND SIGNAL CABLE CCTV IRRIGATION PIPE EMPTY DUCT GPR ANOMALY UNKNOWN CABLE O'HEAD ELECTRICITY O'HEAD TELECOM | WATER GAS STORM FOUL COMB POWER ELECTRIC EIRCOM F.OPTIC BROADBAND TV TRAFFIC CCTV IRRIGATION EMPTY ANOMALY CABLE OH |
| BOC C1 CONC DIA | DOWNPIPE EARTHENWARE NO FURTHER TRACE OFFSITE | DP EW NFT O/S | START OF RUN UNABLE TO OPEN UNABLE TO TRACE | SOR UTO UTT | | | | |

SHEET LAYOUT :

PLAN PRODUCED BY:

CONTACT INFORMATION:
Apex Surveys
Unit 78 Dunboyne Business Park
Dunboyne, Co. Meath, Ireland
www.apexsurveys.ie
info@apexsurveys.ie
00353 1 691 0156

| | | | |
|-------------------|-------------------------------|-------------------|------------|
| CLIENT: | | PROJECT: | |
| Aecom | | Dyke Road, Galway | |
| GRID SYSTEM: | Irish Transverse Mercator | SCALE : | 1/200 A1 |
| DATUM: | Malin Head (OSGM15) | DRG No: | 5999 |
| NOTES: | Drawing Contains Scale Factor | DATE : | 10/11/2023 |
| REVISIONS: | | | |
| No. | Date | Description | |
| 001 | 00/00 | Original Drawing | |
| 002 | 19/12/23 | Anomalies Updated | |
| SHEET: | | 1 of 11 | |
| DESCRIPTION: | | 2D Utilities | |
| SURVEYED BY : | | K.K. | |
| PROCESSED BY : | | J.P. | |
| CHECKED BY : | | Alan Brady | |



PAS 128: 2014 (Quality of Survey Level Outputs):

| | |
|--|---|
| DESKTOP UTILITY RECORDS SEARCH QL-D | Drafted from utility records |
| SITE RECONNAISSANCE QL-C | Location Demonstrated by visual reference to street furniture or evidence of previous streetworks, ie - reinstatement scars |
| DETECTION QL-B4 | A segment of utility suspected to exist but has not been detected by a geophysical technique |
| QL-B3 | Horizontal location only of the utility detected by one of the geophysical techniques used |
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| VERIFICATION QL-A | Horizontal and vertical location of the top and/or bottom of the utility |

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The following is a non-exhaustive list of the limitations of utility surveys:

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- Accuracy estimates:
 - Locational accuracy is determined by referring to the manufacturers guidelines for the detector used.
 - In ideal conditions the spatial accuracies for the underground utilities may be +/- 5% for Radiodetection and +/- 10% of depth for the GPR to 2.5m deep. However variations within the subsurface, depth below the ground, close proximity of other services and local magnetic, atmospheric or ground conditions, bends, lateral service connections and any of the other limitations listed in this disclaimer may alter this estimated accuracy.
 - Plan accuracies of + or - 150mm may be achieved but this figure will depend on the depth of service below ground level. However variations within the subsurface, depth below the ground, close proximity of other services and local magnetic, atmospheric or ground conditions, bends, lateral service connections and any of the other limitations listed in this disclaimer may alter this estimated accuracy.
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3

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00353 1 691 0156

STREET FURNITURE :

| | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
|----------|------|----------|------|---------------|----|------------------|------|----------------|------|--------------|------|-----------|------|-------------|-------|-----------|--------|---------------|------|---------------|----|----------|--------|-----------|-------|----------|------|----------|-----|
| BOLLARDS | BD + | BUS STOP | BS + | CRASH BARRIER | CB | ELECTRICITY POLE | EP + | TELEPHONE POLE | TP + | EARTHING ROD | ER + | LAMP POST | LP + | MARKER POST | MKR + | SIGN POST | SPSN + | TRAFFIC LIGHT | TL + | TELEPHONE BOX | TB | POST BOX | POST + | CAST-IRON | RS-RS | CONCRETE | CONC | DIAMETER | DIA |
|----------|------|----------|------|---------------|----|------------------|------|----------------|------|--------------|------|-----------|------|-------------|-------|-----------|--------|---------------|------|---------------|----|----------|--------|-----------|-------|----------|------|----------|-----|

SERVICES :

| | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
|-----------|----|--------------------|----|-------------|------|-------------|----|--------------|--------|---------------------|------------|----------------------|------|------------|-----|-----------|------------------|---------|--------------|----|-----------|----|-------|---|------------------|----|---------|----|-------------|--------|--------------|----|
| AIR VALVE | AV | ARMSTRONG JUNCTION | AJ | CABLE TV IC | CATV | COVER LEVEL | QL | EIRCOM COVER | EIRCOM | EIRCOM JUNCTION BOX | EIRCOM BOX | ELECTRICAL CABLE PIT | ESAT | ESAT COVER | ESB | ESB COVER | ESB JUNCTION BOX | ESB BOX | FIRE HYDRANT | FH | GAS VALVE | GV | GULLY | G | INSPECTION COVER | IC | MANHOLE | MH | SEPTIC TANK | SEPTIC | SLUICE VALVE | SV |
|-----------|----|--------------------|----|-------------|------|-------------|----|--------------|--------|---------------------|------------|----------------------|------|------------|-----|-----------|------------------|---------|--------------|----|-----------|----|-------|---|------------------|----|---------|----|-------------|--------|--------------|----|

STOPCOCK
SERVICE BOX (UNKNOWN)
TRAFFIC COVER
VENT
WATER METER

LEVELS :

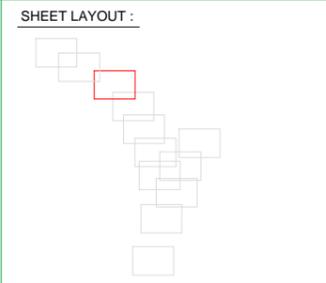
| | | | | | | | | | | | | | | | | | |
|-----------|------------|-------------|-----------|--------------|----------|------------|---------|--------------|-----------|------------|---------|-------------------|------------|-------------|-----------|------------------------|----|
| BED LEVEL | +BED101.50 | FLOOR LEVEL | +FL101.50 | INVERT LEVEL | +I101.50 | ROAD LEVEL | +101.50 | SOFFIT LEVEL | +SL101.50 | SPOT LEVEL | +101.50 | TOP OF WALL LEVEL | +TOW101.50 | WATER LEVEL | +WL101.50 | SURVEY CONTROL STATION | CS |
|-----------|------------|-------------|-----------|--------------|----------|------------|---------|--------------|-----------|------------|---------|-------------------|------------|-------------|-----------|------------------------|----|

START OF RUN
UNABLE TO OPEN
UNABLE TO TRACE

SOR
UTO
UTT

UNDERGROUND LEGEND :

| | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
|------------|-------|----------|-----|-------------|-------|------------|-------|----------------|------|----------------|----------|-------------------|----------|--------|-------------------|-------|-----------|-----------|----------|----|--------------------------|---------|------|-----------------|------------|------------|-------|-------------|---------|---------------|---------|----------------|--------|
| WATER MAIN | WATER | GAS MAIN | GAS | STORM DRAIN | STORM | FULL SEWER | SEWER | COMBINED SEWER | COMB | ELECTRIC CABLE | ELECTRIC | ELECTRIC LIGHTING | ELECTRIC | EIRCOM | FIBRE OPTIC CABLE | FIBRE | BROADBAND | BROADBAND | CABLE TV | TV | TRAFFIC AND SIGNAL CABLE | TRAFFIC | CCTV | IRRIGATION PIPE | IRRIGATION | EMPTY DUCT | EMPTY | GPR ANOMALY | ANOMALY | UNKNOWN CABLE | UNKNOWN | O'HEAD TELECOM | O'HEAD |
|------------|-------|----------|-----|-------------|-------|------------|-------|----------------|------|----------------|----------|-------------------|----------|--------|-------------------|-------|-----------|-----------|----------|----|--------------------------|---------|------|-----------------|------------|------------|-------|-------------|---------|---------------|---------|----------------|--------|



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CLIENT:

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GRID SYSTEM: Irish Transverse Mercator
DATUM: Malin Head (OSGM15)
NOTES: Drawing Contains Scale Factor

REVISIONS:

| No. | Date | Description |
|-----|----------|-------------------|
| 001 | 00/00 | Original Drawing |
| 002 | 19/12/23 | Anomalies Updated |

PROJECT:

Dyke Road, Galway

SCALE : 1/200 A1

DATE : 10/11/2023

DRG No: 5999

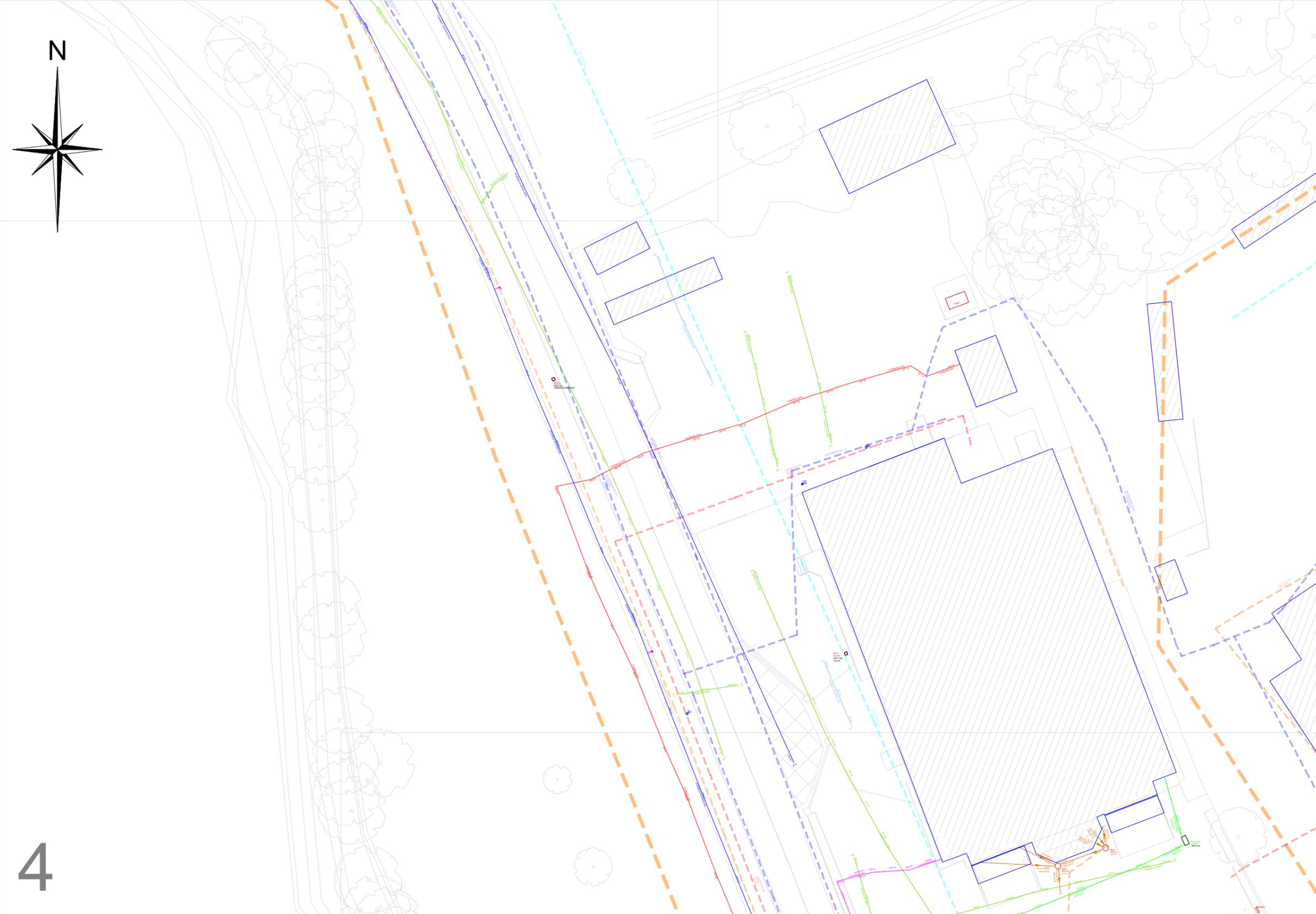
SHEET: 3 of 11

DESCRIPTION: 2D Utilities

SURVEYED BY: K.K.

PROCESSED BY: J.P.

CHECKED BY: Alan Brady



PAS 128: 2014 (Quality of Survey Level Outputs):

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| QL-A Horizontal and vertical location of the top and/or bottom of the utility |

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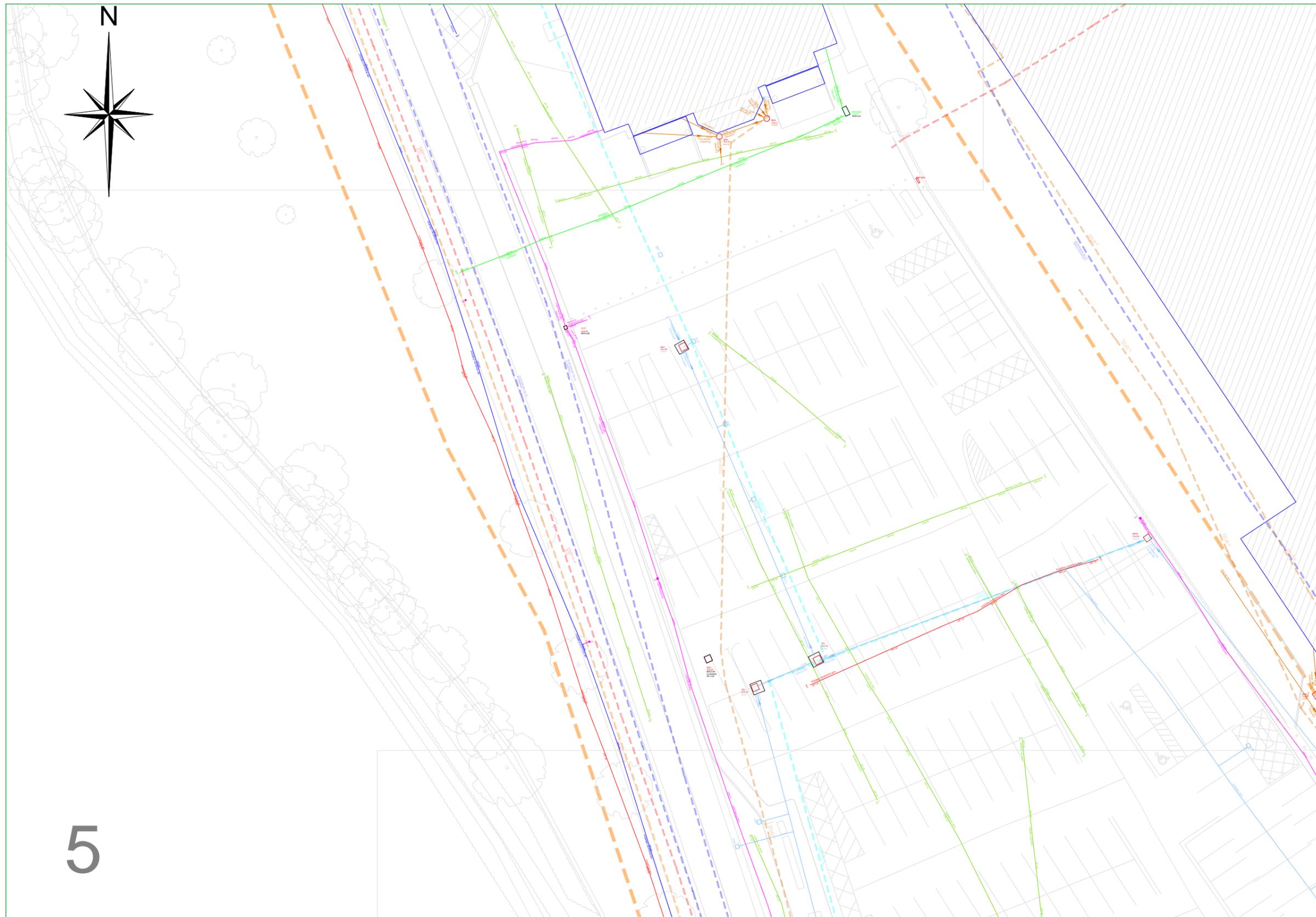
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00353 1 691 0156

| | | |
|--|---|---|
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| BOLLARDS BUS STOP CRASH BARRIER GATE ELECTRICITY POLE TELEPHONE POLE EARTHING ROD LAMP POST MARKER POST SIGN POST TRAFFIC LIGHT TELEPHONE BOX POST POST BOX ROADSIGN BORE HOLE TRIAL PIT | AIR VALVE ARMSTRONG JUNCTION CABLE TV IC COVER LEVEL EIRCOM COVER EIRCOM JUNCTION BOX ELECTRICAL CABLE PIT ESAT COVER ESB COVER ESB JUNCTION BOX FIRE HYDRANT GAS VALVE GULLY INSPECTION COVER MANHOLE SEPTIC TANK SLUICE VALVE | WATER MAIN GAS MAIN STORM DRAIN FOUL SEWER COMBINED SEWER ELECTRIC CABLE ELECTRIC LIGHTING EIRCOM FIBRE OPTIC CABLE BROADBAND CABLE TV TRAFFIC AND SIGNAL CABLE CCTV IRRIGATION PIPE EMPTY DUCT GPR ANOMALY UNKNOWN CABLE O'HEAD ELECTRICITY O'HEAD TELECOM |

| | | | |
|--|---|---|---|
| STREET FURNITURE : | SERVICES : | UNDERGROUND LEGEND : | SHEET LAYOUT : |
| BOLLARDS BUS STOP CRASH BARRIER GATE ELECTRICITY POLE TELEPHONE POLE EARTHING ROD LAMP POST MARKER POST SIGN POST TRAFFIC LIGHT TELEPHONE BOX POST POST BOX ROADSIGN BORE HOLE TRIAL PIT | AIR VALVE ARMSTRONG JUNCTION CABLE TV IC COVER LEVEL EIRCOM COVER EIRCOM JUNCTION BOX ELECTRICAL CABLE PIT ESAT COVER ESB COVER ESB JUNCTION BOX FIRE HYDRANT GAS VALVE GULLY INSPECTION COVER MANHOLE SEPTIC TANK SLUICE VALVE | WATER MAIN GAS MAIN STORM DRAIN FOUL SEWER COMBINED SEWER ELECTRIC CABLE ELECTRIC LIGHTING EIRCOM FIBRE OPTIC CABLE BROADBAND CABLE TV TRAFFIC AND SIGNAL CABLE CCTV IRRIGATION PIPE EMPTY DUCT GPR ANOMALY UNKNOWN CABLE O'HEAD ELECTRICITY O'HEAD TELECOM | PLAN PRODUCED BY: APEX SURVEYS CONTACT INFORMATION: Apex Surveys Unit 78 Dunboyne Business Park Dunboyne, Co. Meath, Ireland www.apexsurveys.ie info@apexsurveys.ie 00353 1 691 0156 |

| CLIENT: | PROJECT: | | | | | | | | | |
|--|---|-------------------|-------------|-----|-------|------------------|-----|----------|-------------------|--|
| Aecom | Dyke Road, Galway | | | | | | | | | |
| GRID SYSTEM: Irish Transverse Mercator DATUM: Malin Head (OSGM15) NOTES: Drawing Contains Scale Factor | SCALE : 1/200 A1 DATE : 10/11/2023 | | | | | | | | | |
| REVISIONS: | DRG No: 5999 DESCRIPTION: 2D Utilities | | | | | | | | | |
| <table border="1"> <tr> <th>No.</th> <th>Date</th> <th>Description</th> </tr> <tr> <td>001</td> <td>00/00</td> <td>Original Drawing</td> </tr> <tr> <td>002</td> <td>19/12/23</td> <td>Anomalies Updated</td> </tr> </table> | No. | Date | Description | 001 | 00/00 | Original Drawing | 002 | 19/12/23 | Anomalies Updated | SHEET: 4 of 11 SURVEYED BY : K.K. PROCESSED BY : J.P. CHECKED BY : Alan Brady |
| No. | Date | Description | | | | | | | | |
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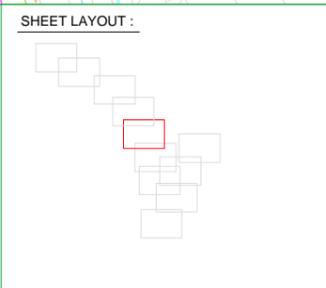
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APEX SURVEYS

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| | | | |
|---|--|---|---|
| <p>STREET FURNITURE :</p> <p>BOLLARDS BUS STOP CRASH BARRIER GATE ELECTRICITY POLE TELEPHONE POLE EARTHING ROD LAMP POST MARKER POST SIGN POST TRAFFIC LIGHT TELEPHONE BOX POST POST BOX ROADSIGN BORE HOLE TRIAL PIT</p> <p>BOC C1 CONC DIA</p> | <p>SERVICES :</p> <p>AIR VALVE ARMSTRONG JUNCTION CABLE TV IC COVER LEVEL EIRCOM COVER EIRCOM JUNCTION BOX ELECTRICAL CABLE PIT ESAT COVER ESB COVER ESB JUNCTION BOX FIRE HYDRANT GAS VALVE GULLY INSPECTION COVER MANHOLE SEPTIC TANK SLUICE VALVE</p> <p>AV AJ CATV QL EIRCOM EIRCOM BOX ECP ESAT ESB ESB BOX FH GV IC MH SEPTIC SV</p> <p>DOWNPIPE EARTHENWARE NO FURTHER TRACE OFFSITE</p> <p>DP E/W NFT O/S</p> | <p>STOPCOCK SERVICE BOX (UNKNOWN) TRAFFIC COVER VENT WATER METER</p> <p>LEVELS :</p> <p>BED LEVEL FLOOR LEVEL INVERT LEVEL ROAD LEVEL SOFFIT LEVEL SPOT LEVEL TOP OF WALL LEVEL WATER LEVEL SURVEY CONTROL STATION</p> <p>ST BOX TLIC VENT WM</p> <p>+BED101.50 +FL101.50 +LS101.50 +101.50 +SL101.50 +TOW101.50 +WL101.50</p> <p>SOR UTO UTT</p> | <p>UNDERGROUND LEGEND :</p> <p>WATER MAIN GAS MAIN STORM DRAIN FOUL SEWER COMBINED SEWER ELECTRIC CABLE ELECTRIC LIGHTING EIRCOM FIBRE OPTIC CABLE BROADBAND CABLE TV TRAFFIC AND SIGNAL CABLE CCTV IRRIGATION PIPE EMPTY DUCT GPR ANOMALY UNKNOWN CABLE O'HEAD ELECTRICITY O'HEAD TELECOM</p> <p>WATER GAS EIRCOM FOUL COMB POWER ELECTRIC EIRCOM E-OPTIC BROADBAND TV TRAFFIC CCTV IRRIGATION EMPTY ANOMALY CABLE OR</p> |
|---|--|---|---|



PLAN PRODUCED BY:

APEX SURVEYS

CONTACT INFORMATION:

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Dunboyne, Co. Meath, Ireland
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00353 1 691 0156

CLIENT:

Aecom

GRID SYSTEM: Irish Transverse Mercator
DATUM: Malin Head (OSGM15)
NOTES: Drawing Contains Scale Factor

REVISIONS:

| No. | Date | Description |
|-----|----------|-------------------|
| 001 | 00/00 | Original Drawing |
| 002 | 19/12/23 | Anomalies Updated |

PROJECT:

Dyke Road, Galway

SCALE : 1/200 A1

DATE : 10/11/2023

DRG No: 5999

SHEET: 5 of 11

DESCRIPTION: 2D Utilities

SURVEYED BY: K.K.

PROCESSED BY: J.P.

CHECKED BY: Alan Brady



6

PAS 128: 2014 (Quality of Survey Level Outputs):

| | |
|--------------------------------|---|
| DESKTOP UTILITY RECORDS SEARCH | |
| QL-D | Drafted from utility records |
| SITE RECONNAISSANCE | |
| QL-C | Location Demonstrated by visual reference to street furniture or evidence of previous streetworks, ie - reinstatement scars |
| DETECTION | |
| QL-B4 | A segment of utility suspected to exist but has not been detected by a geophysical technique |
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| VERIFICATION | |
| QL-A | Horizontal and vertical location of the top and/or bottom of the utility |

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- Size of Utility: The smaller the diameter of a utility the more difficult it is to locate. This difficulty increases with depth.
- Ground Conditions: The depth penetration and quality of the data depends on the ground conditions of the site. GPR Surveying works best within high resistivity material. Clay overburden can impair GPR Surveying. Poor data may be a result of areas with high conductivity.
- Utility Congestion: Where different utilities converge together into a service corridor or cross paths it becomes difficult to isolate a specific utility and to map its route. The reflected signal will display a single response to multiple utilities. Therefore multiple utilities may appear to be a single utility. Where similar services run on close proximity, separation may be impossible.
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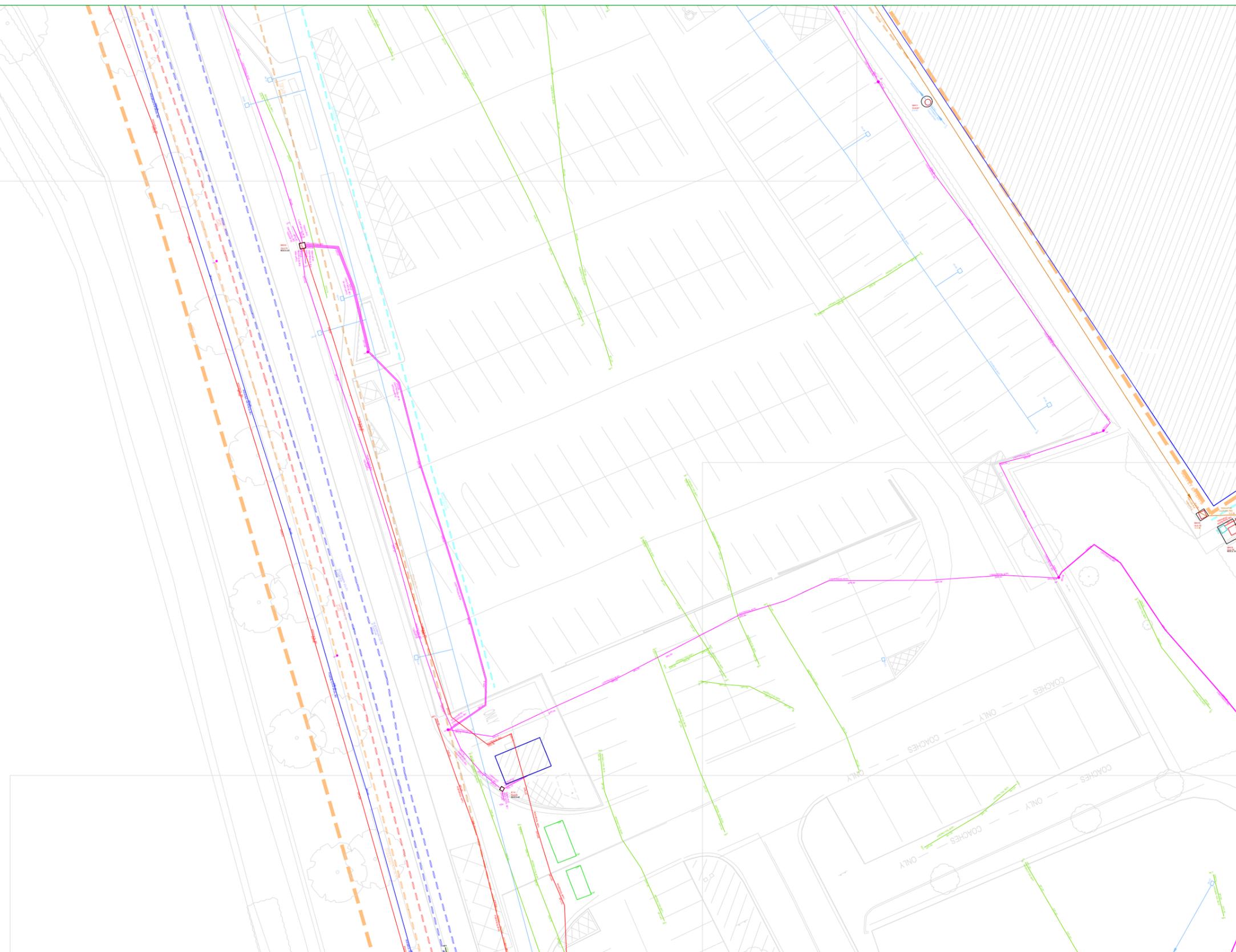
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STREET FURNITURE :

| | |
|------------------|---------|
| BOLLARDS | BD + |
| BUS STOP | BS + |
| CRASH BARRIER | CB |
| GATE | --- |
| ELECTRICITY POLE | EP + |
| TELEPHONE POLE | TP + |
| EARTHING ROD | ER + |
| LAMP POST | LP + |
| MARKER POST | MKR + |
| SIGN POST | SPSN + |
| TRAFFIC LIGHT | TL + |
| TELEPHONE BOX | TB |
| POST BOX | POST + |
| ROADSIGN | RS -/RS |
| BORE HOLE | BH + |
| TRIAL PIT | TPIT + |

SERVICES :

| | |
|----------------------|------------|
| AIR VALVE | AV |
| ARMSTRONG JUNCTION | AJ |
| CABLE TV IC | CATV |
| COVER LEVEL | CL |
| EIRCOM COVER | EIRCOM |
| EIRCOM JUNCTION BOX | EIRCOM BOX |
| ELECTRICAL CABLE PIT | ECP |
| ESAT COVER | ESAT |
| ESB COVER | ESB |
| ESB JUNCTION BOX | ESB BOX |
| FIRE HYDRANT | FH |
| GAS VALVE | GV |
| GULLY | G |
| INSPECTION COVER | IC |
| MANHOLE | MH |
| SEPTIC TANK | SEPTIC |
| SLUICE VALVE | SV |

STOPCOCK

| | |
|-----------------------|--------|
| SERVICE BOX (UNKNOWN) | ST + |
| TRAFFIC COVER | TLIC |
| VENT | VENT + |
| WATER METER | WM + |

LEVELS :

| | |
|------------------------|------------|
| BED LEVEL | +BED101.50 |
| FLOOR LEVEL | +FL101.50 |
| INVERT LEVEL | +I101.50 |
| ROAD LEVEL | +101.50 |
| SOFFIT LEVEL | +SL101.50 |
| SPOT LEVEL | +101.50 |
| TOP OF WALL LEVEL | +TOW101.50 |
| WATER LEVEL | +WL101.50 |
| SURVEY CONTROL STATION | CS |

UNDERGROUND LEGEND :

| | |
|--------------------------|------------|
| WATER MAIN | WATER |
| GAS MAIN | GAS |
| STORM DRAIN | STORM |
| POUL SEWER | SOIL |
| COMBINED SEWER | COMB |
| ELECTRIC CABLE | POWER |
| ELECTRIC LIGHTING | LIGHTING |
| EIRCOM | EIRCOM |
| FIBRE OPTIC CABLE | F.OPTIC |
| BROADBAND | BROADBAND |
| CABLE TV | TV |
| TRAFFIC AND SIGNAL CABLE | TRAFFIC |
| CCTV | CCTV |
| IRRIGATION PIPE | IRRIGATION |
| EMPTY DUCT | EMPTY |
| GPR ANOMALY | ANOMALY |
| UNKNOWN CABLE | CABLE |
| O'HEAD ELECTRICITY | OE |
| O'HEAD TELECOM | OT |



PLAN PRODUCED BY:

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GRID SYSTEM: Irish Transverse Mercator
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NOTES: Drawing Contains Scale Factor

REVISIONS:

| No. | Date | Description |
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| 001 | 00/00 | Original Drawing |
| 002 | 19/12/23 | Anomalies Updated |

PROJECT:

Dyke Road, Galway

SCALE : 1/200 A1

DATE : 10/11/2023

DRG No: 5999

SHEET: 6 of 11

| | |
|----------------|--------------|
| DESCRIPTION: | 2D Utilities |
| SURVEYED BY : | K.K. |
| PROCESSED BY : | J.P. |
| CHECKED BY : | Alan Brady |



PAS 128: 2014 (Quality of Survey Level Outputs):

| | |
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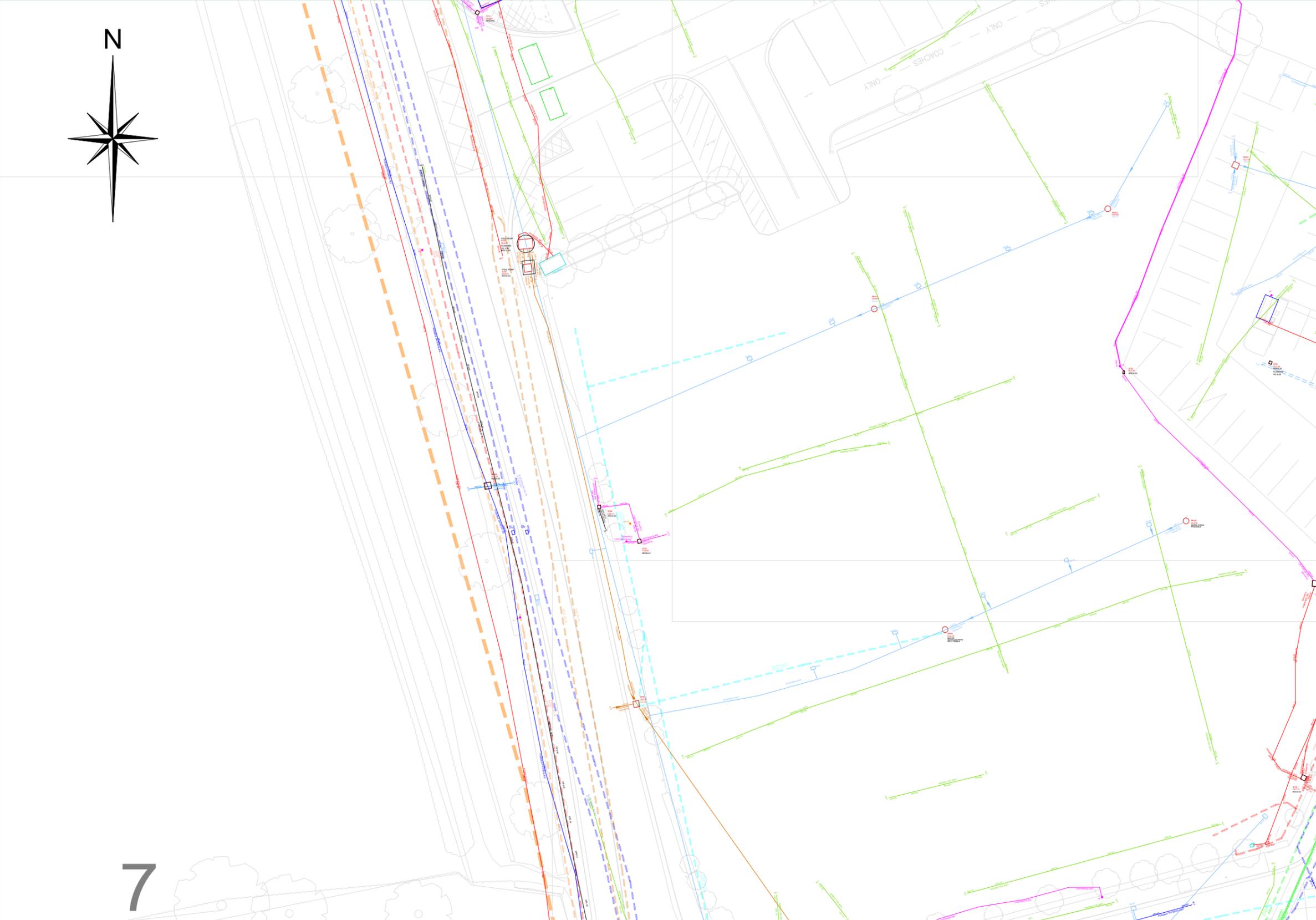
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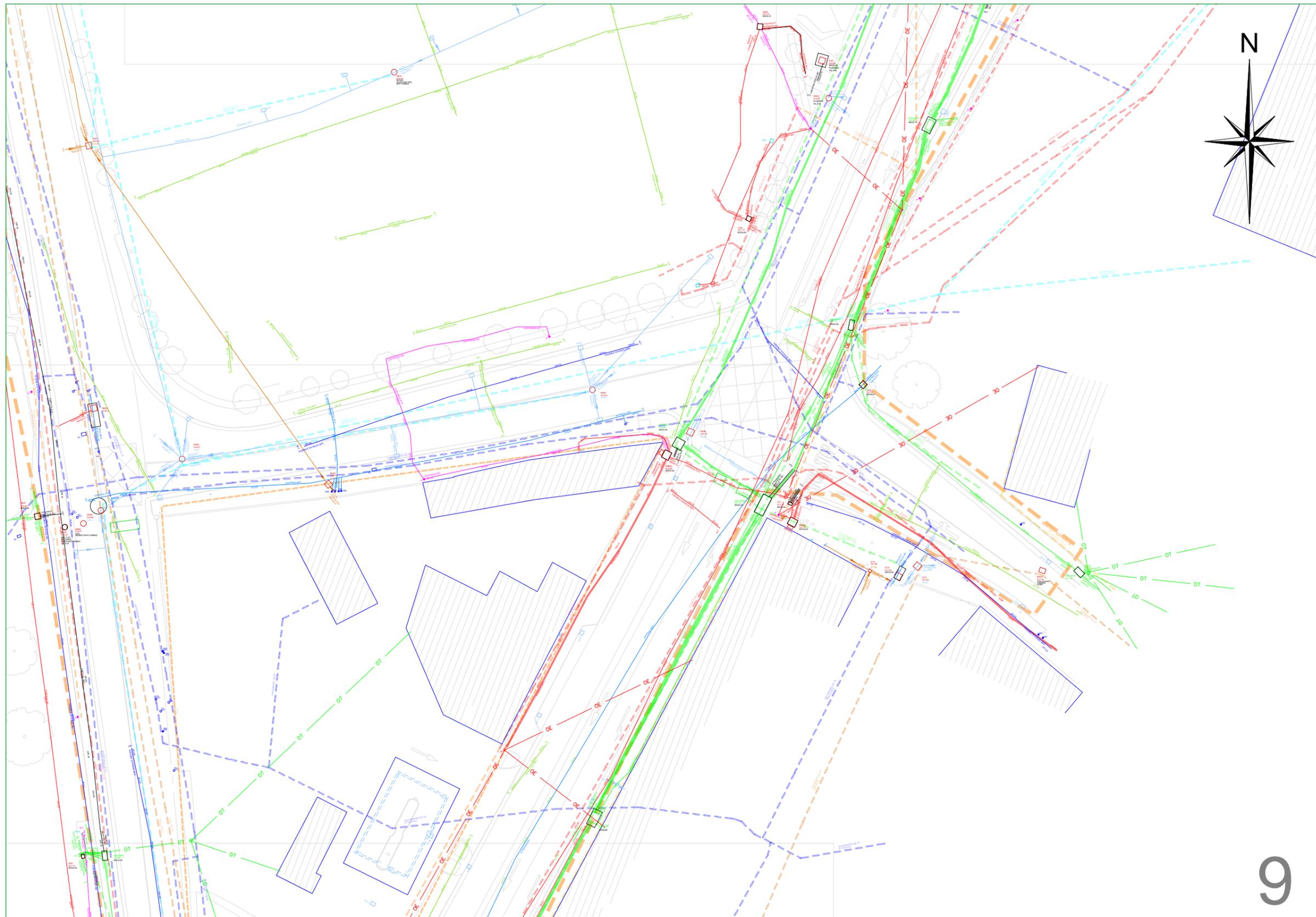
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| | | | | | | | | |
|--|--|---|---|---|------------------------------------|-----------------|--|-------------------|
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| | |
|---|--|
| UNDERGROUND LEGEND : | SHEET LAYOUT : |
| WATER MAIN GAS MAIN STORM DRAIN POUL SEWER COMBINED SEWER ELECTRIC CABLE ELECTRIC LIGHTING EIRCOM FIBRE OPTIC CABLE BROADBAND CABLE TV TRAFFIC AND SIGNAL CABLE CCTV IRRIGATION PIPE EMPTY DUCT GPR ANOMALY UNKNOWN CABLE O'HEAD ELECTRICITY O'HEAD TELECOM | WATER GAS STORM POUL COMB POWER LIGHTING EIRCOM F.OPTIC BROADBAND TV TRAFFIC CCTV IRRIGATION EMPTY ANOMALY CABLE OH |

| | |
|---|---|
| PLAN PRODUCED BY: | CLIENT: |
| APEX SURVEYS | Aecom |
| CONTACT INFORMATION: | PROJECT: |
| Apex Surveys Unit 78 Dunboyne Business Park Dunboyne, Co. Meath, Ireland www.apexsurveys.ie info@apexsurveys.ie 00353 1 691 0156 | Dyke Road, Galway |
| GRID SYSTEM: DATUM: NOTES: | Irish Transverse Mercator Malin Head (OSGM15) Drawing Contains Scale Factor |
| REVISIONS: | |
| No. Date Description | |
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| | | | |
|----------------|----------|-----------------------|--------------|
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 - Connections between manholes: Connections between manhole chambers are assumed to be straight.
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 - Fiber Optic Cables: Fiber optic cables may not be possible to locate except where laid with a built in tracer wire or similar conductor system.
 - Defective / flooded manholes or pipework: It may not be possible to establish connections between flooded or defective manholes or pipework.
 - Acute bends in pipework: It may not be possible to trace a pipe past an acute bend.
- Accuracy estimates:
- Locational accuracy is determined by referring to the manufacturers guidelines for the detector used.
 - In ideal conditions the spatial accuracies for the underground utilities may be +/- 5% for Radiodetection and +/- 10% of depth for the GPR to 2.5m deep. However variations within the subsurface, depth below the ground, close proximity of other services and local magnetic, atmospheric or ground conditions, bends, lateral service connections and any of the other limitations listed in this disclaimer may alter this estimated accuracy.
 - Plan accuracies of +/- 150mm may be achieved but this figure will depend on the depth of service below ground level. However variations within the subsurface, depth below the ground, close proximity of other services and local magnetic, atmospheric or ground conditions, bends, lateral service connections and any of the other limitations listed in this disclaimer may alter this estimated accuracy.
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 - Internal building services.
 - Small diameter cables less than 20mm diameter or pipes less than 40mm diameter.
 - Above ground services unless specifically requested.
 - Lifting manholes which require longer than 10 minutes effort using standard heavy duty apparatus.

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9

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STREET FURNITURE :

| | |
|------------------|---------|
| BOLLARDS | BD + |
| BUS STOP | BS + |
| CRASH BARRIER | CB |
| GATE | EP |
| ELECTRICITY POLE | EP + |
| TELEPHONE POLE | TP + |
| EARTHING ROD | ER + |
| LAMP POST | LP + |
| MARKER POST | MKR + |
| SIGN POST | SPSN + |
| TRAFFIC LIGHT | TL + |
| TELEPHONE BOX | TB |
| POST BOX | POST + |
| CAST-IRON | RS - RS |
| CONCRETE | BH + |
| DIAMETER | TPIT + |

SERVICES :

| | |
|----------------------|------------|
| AIR VALVE | AV |
| ARMSTRONG JUNCTION | AJ |
| CABLE TV IC | CATV |
| COVER LEVEL | QL |
| EIRCOM COVER | EIRCOM |
| EIRCOM JUNCTION BOX | EIRCOM BOX |
| ELECTRICAL CABLE PIT | ECP |
| ESAT COVER | ESAT |
| ESB COVER | ESB |
| ESB JUNCTION BOX | ESB BOX |
| FIRE HYDRANT | FH |
| GAS VALVE | GV |
| GULLY | G |
| INSPECTION COVER | IC |
| MANHOLE | MH |
| SEPTIC TANK | SEPTIC |
| SLUICE VALVE | SV |

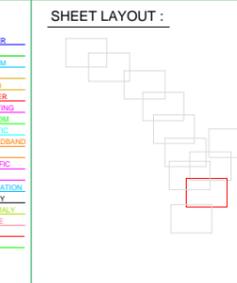
STOPCOCK
SERVICE BOX (UNKNOWN)
TRAFFIC COVER
VENT
WATER METER

LEVELS :

| | |
|------------------------|------------|
| BED LEVEL | +BED101.50 |
| FLOOR LEVEL | +FL101.50 |
| INVERT LEVEL | +I101.50 |
| ROAD LEVEL | +101.50 |
| SOFFIT LEVEL | +SL101.50 |
| SPOT LEVEL | +101.50 |
| TOP OF WALL LEVEL | +TOW101.50 |
| WATER LEVEL | +WL101.50 |
| SURVEY CONTROL STATION | |

UNDERGROUND LEGEND :

| | |
|--------------------------|--------------------------|
| WATER MAIN | WATER |
| GAS MAIN | GAS |
| STORM DRAIN | STORM |
| FULL SEWER | SEWER |
| COMBINED SEWER | COMB |
| ELECTRIC CABLE | ELECTRIC |
| ELECTRIC LIGHTING | ELECTRIC LIGHTING |
| EIRCOM | EIRCOM |
| FIBRE OPTIC CABLE | FIBRE OPTIC CABLE |
| BROADBAND | BROADBAND |
| CABLE TV | CABLE TV |
| TRAFFIC AND SIGNAL CABLE | TRAFFIC AND SIGNAL CABLE |
| CCTV | CCTV |
| IRRIGATION PIPE | IRRIGATION PIPE |
| EMPTY DUCT | EMPTY DUCT |
| GPR ANOMALY | GPR ANOMALY |
| UNKNOWN CABLE | UNKNOWN CABLE |
| O'HEAD ELECTRICITY | O'HEAD ELECTRICITY |
| O'HEAD TELECOM | O'HEAD TELECOM |



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CLIENT:

Aecom

GRID SYSTEM: Irish Transverse Mercator
DATUM: Malin Head (OSGM15)
NOTES: Drawing Contains Scale Factor

REVISIONS:

| No. | Date | Description |
|-----|----------|-------------------|
| 001 | 00/00 | Original Drawing |
| 002 | 19/12/23 | Anomalies Updated |

PROJECT:

Dyke Road, Galway

SCALE : 1/200 A1

DATE : 10/11/2023

DRG No: 5999

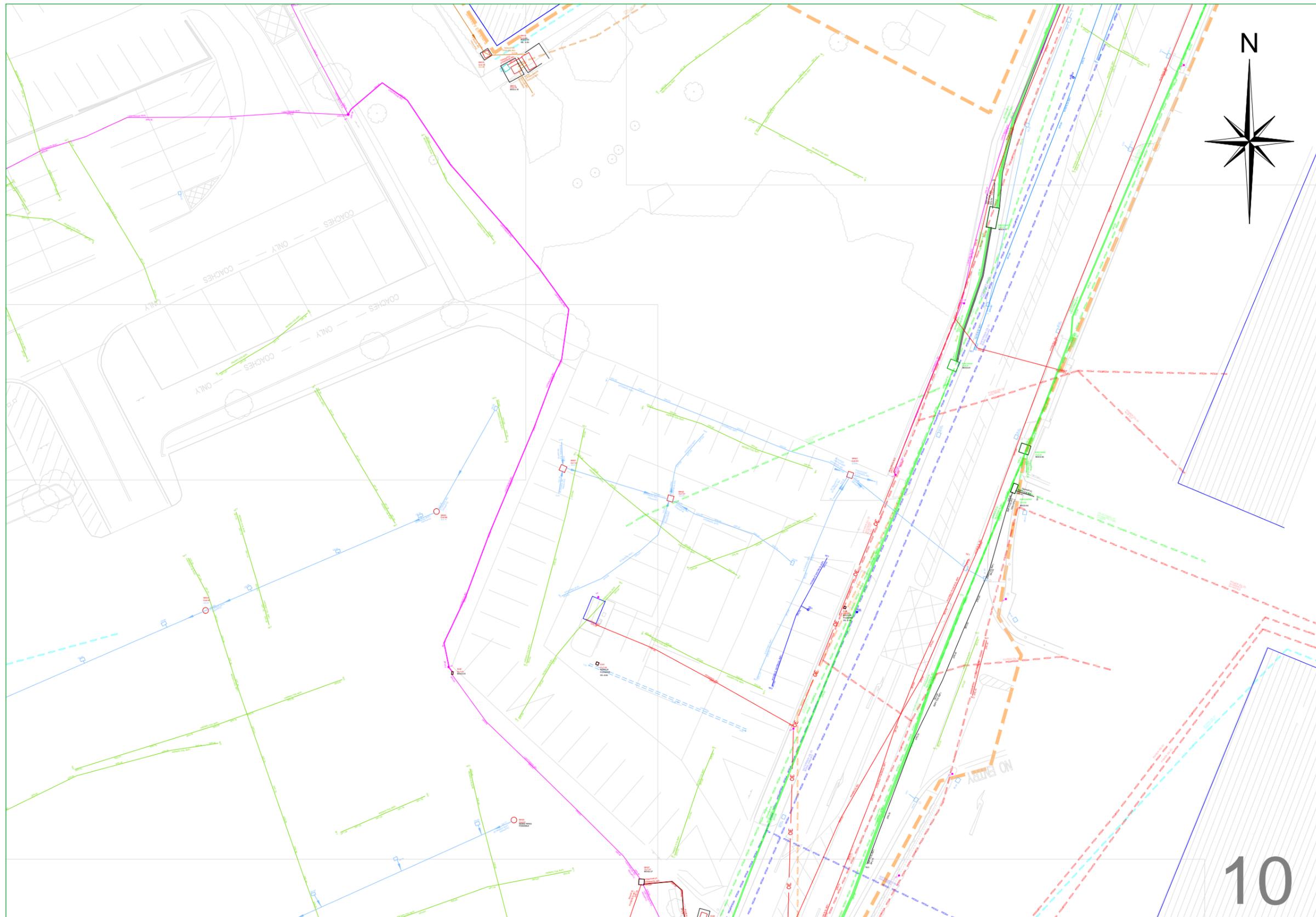
SHEET: 9 of 11

DESCRIPTION: 2D Utilities

SURVEYED BY: K.K.

PROCESSED BY: J.P.

CHECKED BY: Alan Brady



PAS 128: 2014 (Quality of Survey Level Outputs):

| | |
|--|---|
| DESKTOP UTILITY RECORDS SEARCH QL-D | Drafted from utility records |
| SITE RECONNAISSANCE QL-C | Location Demonstrated by visual reference to street furniture or evidence of previous streetworks, ie - reinstatement scars |
| DETECTION QL-B4 | A segment of utility suspected to exist but has not been detected by a geophysical technique |
| QL-B3 | Horizontal location only of the utility detected by one of the geophysical techniques used |
| QL-B2 | Horizontal and vertical location of the utility detected by one of the geophysical techniques used |
| QL-B1 | Horizontal and vertical location of the utility detected by multiple geophysical techniques |
| VERIFICATION QL-A | Horizontal and vertical location of the top and/or bottom of the utility |

Apex Surveys Ltd. Disclaimer - Utility Survey

The interpretative nature and the non-intrusive, indirect and non-destructive survey methods must be taken into account when considering the results of the survey. Therefore Apex Surveys, while using appropriate practice to execute, interpret and present the data, gives no guarantees that all underground utilities and underground structures will be located and mapped. Furthermore, Apex Surveys cannot guarantee the accuracy of the utility depths annotated on the survey drawings. Apex Survey shall not be liable for any omissions or inaccuracies in the survey which arise due to the limitations of the service. No liability shall attach to Apex Surveys, in any circumstances, howsoever arising, in respect of any consequential loss or damages suffered by the Client.

- The following is a non-exhaustive list of the limitations of utility surveys:
- The Survey aims to map existing utilities subsurface utilities and provide information with respect to pipe size, material type and drainage connectivity. However utility surveying is limited by the following guidelines and it may not be possible to accurately survey, define and locate all services and sub-surface features.
 - Depth of Utility: The depth and size of a utility affect the signal response and the degree with which a utility can be located. Due to attenuation of the radar signal with depth, resolution is restricted, hence making identification of utilities more difficult with increasing depth.
 - Size of Utility: The smaller the diameter of a utility the more difficult it is to locate. This difficulty increases with depth.
 - Ground Conditions: The depth penetration and quality of the data depends on the ground conditions of the site. GPR Surveying works best within high resistivity material. Clay overburden can impair GPR Surveying. Poor data may be a result of areas with high conductivity.
 - Utility Congestion: Where different utilities converge together into a service corridor or cross paths it becomes difficult to isolate a specific utility and to map its route. The reflected signal will display a single response to multiple utilities. Therefore multiple utilities may appear to be a single utility. Where similar services run on close proximity, separation may be impossible.
 - Signal Jumping: Signal from surrounding services may 'jump' to a highly conductive line masking its true identity.
 - Shadowing: (of deeper utilities by shallower objects) Shallow utilities will mask the existence of deeper utilities where they are in close proximity. Also, high reflective materials close to the surface i.e. rebar may hide deeper anomalies.
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| | | | | | | | | | |
|---------------------------|--|-------------------|---|---|-----------------------------------|-----------------|--|--|-------------------|
| STREET FURNITURE : | BD + BS + CB ELECTRICITY POLE TELEPHONE POLE EARTHING ROD LAMP POST MARKER SIGN POST TRAFFIC LIGHT TELEPHONE BOX POST POST BOX ROADSIGN BORE HOLE TRIAL PIT | SERVICES : | AV AJ CATV QL EIRCOM EIRCOM BOX ECP ESAT ESB ESB BOX FN GV IC MH SEPTIC SV | STOPCOCK SERVICE BOX (UNKNOWN) TRAFFIC COVER VENT WATER METER | ST BOX TLIC VENT WM + | LEVELS : | BED LEVEL FLOOR LEVEL INVERT LEVEL ROAD LEVEL SOFFIT LEVEL SPOT LEVEL TOP OF WALL LEVEL WATER LEVEL SURVEY CONTROL STATION | +BED101.50 +FL101.50 +LS101.50 +101.50 +SL101.50 +101.50 +TOW101.50 +WL101.50 | SOR UTO UTT |
| BOTTOM OF CHAMBER | BOC C1 CONC DIA | DOWNPIPE | DP EHW NFT O/S | START OF RUN | UNABLE TO OPEN UNABLE TO TRACE | | | | |

| | |
|---|--|
| UNDERGROUND LEGEND : | SHEET LAYOUT : |
| WATER MAIN GAS MAIN STORM DRAIN POUL SEWER COMBINED SEWER ELECTRIC CABLE ELECTRIC LIGHTING EIRCOM FIBRE OPTIC CABLE BROADBAND CABLE TV TRAFFIC AND SIGNAL CABLE CCTV IRRIGATION PIPE EMPTY DUCT GPR ANOMALY UNKNOWN CABLE O'HEAD ELECTRICITY O'HEAD TELECOM | WATER GAS STORM POUL COMB POWER ELECTRIC EIRCOM F.OPTIC BROADBAND TV TRAFFIC CCTV IRRIGATION EMPTY ANOMALY CABLE OH OT |

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00353 1 691 0156

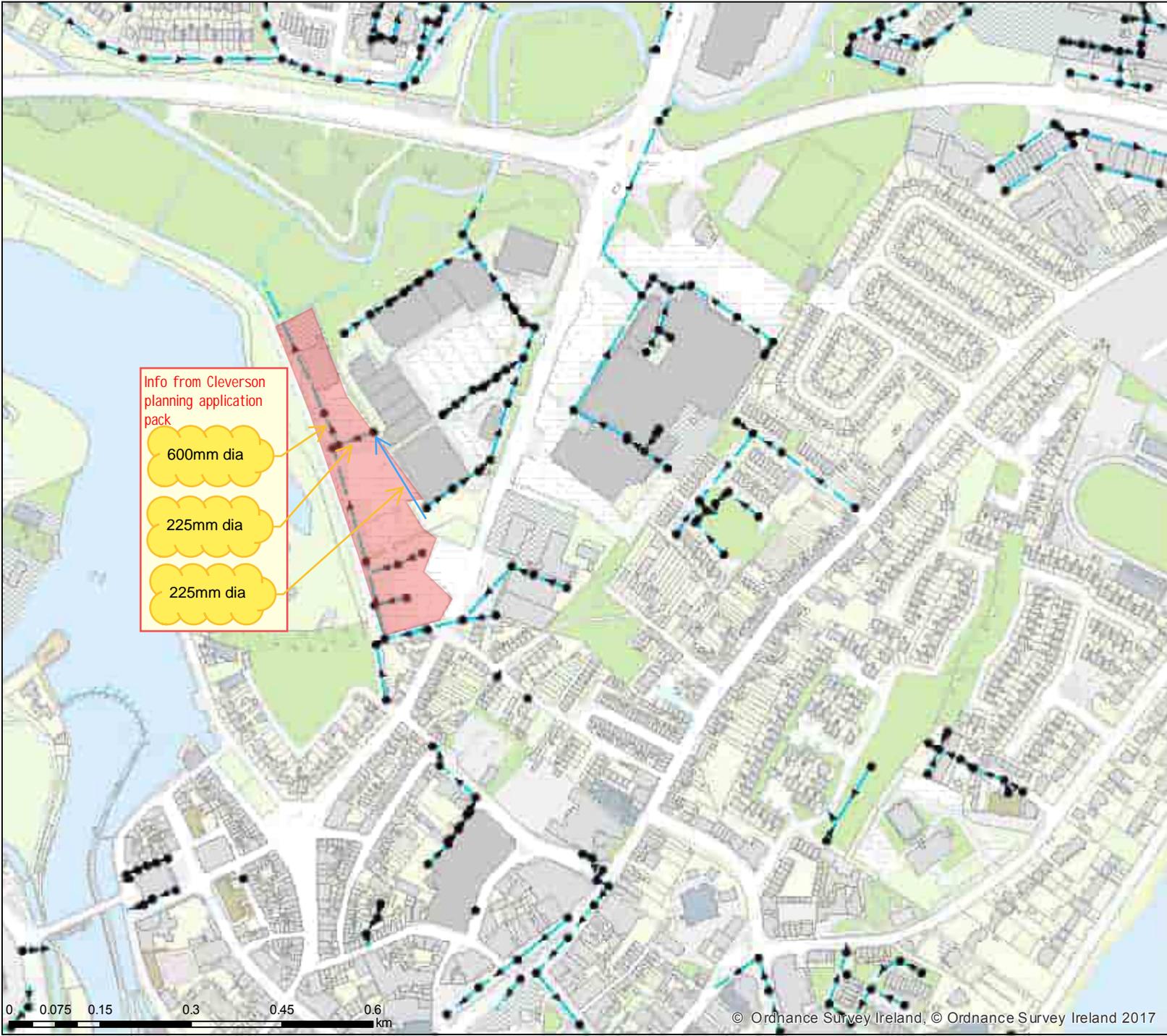
| | | |
|---------------------|-------------------------------|-------------------|
| CLIENT: | PROJECT: | |
| Aecom | Dyke Road, Galway | |
| GRID SYSTEM: | Irish Transverse Mercator | |
| DATUM: | Malin Head (OSGM15) | |
| NOTES: | Drawing Contains Scale Factor | |
| REVISIONS: | | |
| No. | Date | Description |
| 001 | 00/00 | Original Drawing |
| 002 | 19/12/23 | Anomalies Updated |

| | | | |
|----------------|----------|-----------------------|--------------|
| SCALE : | 1/200 A1 | DATE : | 10/11/2023 |
| DRG No: | 5999 | DESCRIPTION : | 2D Utilities |
| SHEET: | 10 of 11 | SURVEYED BY : | K.K. |
| | | PROCESSED BY : | J.P. |
| | | CHECKED BY : | Alan Brady |

Dyke Road - Storm



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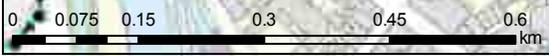
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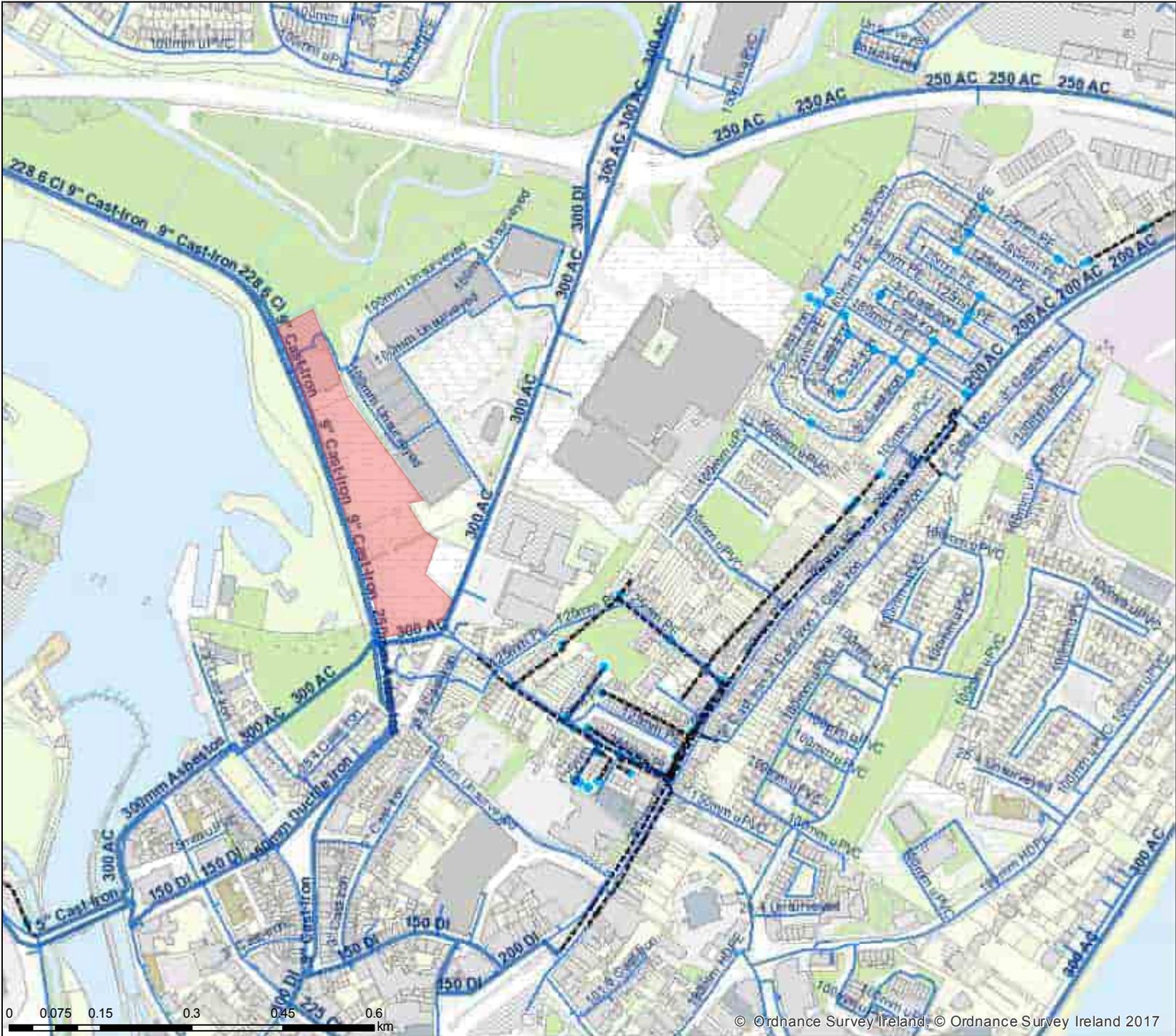
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| Legend | Legend | Legend |
|---|---|---|
| <ul style="list-style-type: none"> Water Distribution Water Main Water Service Water Valve Water Meter Water Stopcock Water Stopcock Box Water Stopcock Cover Water Stopcock Key Water Stopcock Lock Water Stopcock Padlock Water Stopcock Seal Water Stopcock Sealant Water Stopcock Sealant Box Water Stopcock Sealant Cover Water Stopcock Sealant Key Water Stopcock Sealant Lock Water Stopcock Sealant Padlock Water Stopcock Sealant Seal Water Stopcock Sealant Sealant Box Water Stopcock Sealant Sealant Cover Water Stopcock Sealant Sealant Key Water Stopcock Sealant Sealant Lock Water Stopcock Sealant Sealant Padlock Water Stopcock Sealant Sealant Seal | <ul style="list-style-type: none"> Gas Distribution Gas Main Gas Service Gas Valve Gas Meter Gas Stopcock Gas Stopcock Box Gas Stopcock Cover Gas Stopcock Key Gas Stopcock Lock Gas Stopcock Padlock Gas Stopcock Seal Gas Stopcock Sealant Gas Stopcock Sealant Box Gas Stopcock Sealant Cover Gas Stopcock Sealant Key Gas Stopcock Sealant Lock Gas Stopcock Sealant Padlock Gas Stopcock Sealant Seal | <ul style="list-style-type: none"> Electricity Distribution Electricity Main Electricity Service Electricity Valve Electricity Meter Electricity Stopcock Electricity Stopcock Box Electricity Stopcock Cover Electricity Stopcock Key Electricity Stopcock Lock Electricity Stopcock Padlock Electricity Stopcock Seal Electricity Stopcock Sealant Electricity Stopcock Sealant Box Electricity Stopcock Sealant Cover Electricity Stopcock Sealant Key Electricity Stopcock Sealant Lock Electricity Stopcock Sealant Padlock Electricity Stopcock Sealant Seal |



Dyke Road - Water



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| Legend | Legend | Legend |
|---------------------|---------------------|---------------------|
| Water Mains | Water Mains | Water Mains |
| Service Connections | Service Connections | Service Connections |
| Valves | Valves | Valves |
| Manholes | Manholes | Manholes |
| Access Points | Access Points | Access Points |
| Street Names | Street Names | Street Names |
| Topography | Topography | Topography |
| Scale | Scale | Scale |

Appendix E Ground Investigation Reports

- E.1 Ground Investigation Report (Ground Investigation Ireland Ltd)**
- E.2 Geophysical Survey (Minerex Geophysics Ltd)**



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Ground Investigations Ireland

Dyke Road Galway

Aecom

Ground Investigation Report

July 2024

Directors:

Fergal McNameara (MD), Conor Finlierty, Aisling McDonnell, Barry Sexton, Stephen Keady & Michael Sutton
Ground Investigations Ireland Limited | Registered in Ireland, Company Registration No.: 405220



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DOCUMENT CONTROL SHEET

| | |
|----------------|-----------------------------|
| Project Title | Dyke Road Galway |
| Client | Land Development Agency |
| Engineer | Aecom |
| Project No | 13614-02-24 |
| Document Title | Ground Investigation Report |

| Rev. | Status | Author(s) | Reviewed By | Approved By | Office of Origin | Issue Date |
|------|---------|-----------|-------------|-------------|------------------|--------------|
| A | Interim | A Mann | M Sutton | M Sutton | Dublin | 24 June 2024 |
| B | Final | A Mann | M Sutton | M Sutton | Dublin | 04 July 2024 |

Ground Investigations Ireland Ltd. present the results of the fieldworks and laboratory testing in accordance with the specification and related documents provided by or on behalf of the client. The possibility of variation in the ground and/or groundwater conditions between or below exploratory locations or due to the investigation techniques employed must be taken into account when this report and the appendices inform designs or decisions where such variation may be considered relevant. Ground and/or groundwater conditions may vary due to seasonal, man-made or other activities not apparent during the fieldworks and no responsibility can be taken for such variation. The data presented and the recommendations included in this report and associated appendices are intended for the use of the client and the client's geotechnical representative only and any duty of care to others is excluded unless approved in writing.



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GROUND INVESTIGATIONS IRELAND
Geotechnical & Environmental

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APPENDICES

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| Appendix 5 | Borehole Records |
| Appendix 6 | Insitu Plate Bearing Test Results |
| Appendix 7 | TRL Dynamic Cone Penetrometer Records |
| Appendix 8 | Laboratory Results |
| Appendix 9 | Groundwater Monitoring Records |



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1.0 Preamble

On the instructions of Aecom Engineers, a site investigation was carried out by Ground Investigations Ireland Ltd., between April and June 2024 at the site of the proposed development in Galway City.

2.0 Overview

2.1. Background

It is proposed to construct a new residential and commercial/retail development with associated services, access roads and car parking at the proposed site. The site is currently occupied by a car park and is situated near the centre of Galway City. The proposed construction is envisaged to consist of piled foundations and pavement make up with some local excavations for services and plant

2.2. Purpose and Scope

The purpose of the site investigation was to investigate subsurface conditions utilising a variety of investigative methods in accordance with the project specification. The scope of the work undertaken for this project included the following:

- Visit project site to observe existing conditions
- Carry out 6 No. Trial Pits to a maximum depth of 3.0m BGL
- Carry out 4 No. Slit Trenches to determine existing service details
- Carry out 2 No. Soakaways to determine a soil infiltration value to BRE digest 365
- Carry out 11 No. Cable Percussion boreholes to a maximum depth of 10.10m BGL
- Carry out 7 No. Rotary Core Boreholes to a maximum depth of 20.10m BGL
- Carry out 1 No. Insitu Plate Bearing Tests
- Carry out 4 No. TRL Dynamic Cone Penetrometer Tests
- Installation of 4 No. Groundwater monitoring wells
- Geotechnical & Environmental Laboratory testing
- Report with recommendations

3.0 Subsurface Exploration

3.1. General

During the ground investigation a programme of intrusive investigation specified by the Consulting Engineer was undertaken to determine the sub surface conditions at the proposed site. Regular sampling and in-situ testing was undertaken in the exploratory holes to facilitate the geotechnical descriptions and to enable laboratory testing to be carried out on the soil samples recovered during excavation and drilling.

The procedures used in this site investigation are in accordance with Eurocode 7 Part 2: Ground Investigation and testing (ISEN 1997 – 2:2007) and B.S. 5930:2015.

3.2. Trial Pits

The trial pits were excavated using a 3T tracked excavator at the locations shown in the exploratory hole location plan in Appendix 1. The locations were checked using a CAT scan to minimise the potential for encountering services during the excavation. The trial pits were sampled, logged and photographed by a Geotechnical Engineer/Engineering Geologist prior to backfilling with arisings. Notes were made of any services, inclusions, pit stability, groundwater encountered and the characteristics of the strata encountered and are presented on the trial pit logs which are provided in Appendix 2 of this Report.

3.3. Slit Trenching

The slit trenches were excavated using a 3T tracked excavator at the locations shown in the exploratory hole location plan in Appendix 1. The locations were checked using a CAT scan to minimise the potential for encountering services during the excavation. The soil was slowly stripped using a spotter on the trench to alert the driver if any services were seen, to avoid damage to any underlying services. The slit trenches were sampled, logged and photographed by a Geotechnical Engineer/Engineering Geologist prior to backfilling with arisings. Notes were made of any services, inclusions, pit stability, groundwater encountered and the characteristics of the strata encountered and are presented on the slit trench records which are provided in Appendix 3 of this Report.

3.4. Soakaway Testing

The soakaway testing was carried out in selected trial pits at the locations shown in the exploratory hole location plan in Appendix 1. These pits were carefully excavated and filled with water to assess the infiltration characteristics of the proposed site. The pits were allowed to drain and the drop in water level was recorded over time as required by BRE Digest 365. The pits were logged prior to completing the soakaway test and were backfilled with arising's upon completion. The soakaway test results are provided in Appendix 4 of this Report.

3.5. Cable Percussion Boreholes

The Cable Percussion Boreholes were drilled using a Dando 2000 drilling rig with regular in-situ testing and sampling undertaken to facilitate the production of geotechnical logs and laboratory testing.

The standard method of boring in soil for site investigation is known as the Cable Percussion method. It consists of using a Shell in non cohesive soils and a clay cutter in cohesive soils, both operated on a wire cable. Very hard soils, boulders and other hard obstructions are broken up by chiselling and the fragments removed with the Shell. Where ground conditions made it necessary, the borehole was lined with 200mm diameter steel casing. While the use of the Cable Percussion method of boring gives the maximum data on soil conditions, some mixing of laminated soil is inevitable. For this reason, thin lenses of granular material may not be noticed. Disturbed samples were taken from the boring tools at suitable depths, so that there is a representative sample at the top of each change in stratum and thereafter at regular intervals

down the borehole until the next stratum was encountered. The disturbed samples were then sealed and sent to the laboratory where they were visually examined to confirm the description of the relevant strata. Standard Penetration Tests were carried out in the boreholes. The results of these tests, together with the depths at which the tests were taken are shown on the accompanying borehole records. The test consists of a thick wall sampler tube, 50mm external diameter, being driven into the soil by a monkey weighing 63.5kg and with a free drop of 760mm. For gravels and glacial till the driving shoe was replaced by a solid 60° cone. The Standard Penetration Test number referred to as the 'N' value is the number of blows required to drive the tube 300mm, after an initial penetration of 150mm. The number gives a guide to the consistency of the soil and can also be used to estimate the relative strength/density at the depth of the test and also to estimate the bearing capacity and compressibility of the soil. The cable percussion borehole logs are provided in Appendix 5 of this Report.

3.6. Rotary Boreholes

The rotary coring was carried out by a track mounted T44 Beretta rig at the locations shown on the location plan in Appendix 1. The rotary boreholes were completed from the ground surface or alternatively, where noted on the individual borehole log, from the base of the cable percussion borehole where a temporary liner was installed to facilitate follow-on rotary coring.

The T44 Beretta is equipped with rubber tracks which allow for short travel on pavement surfaces avoiding any damage to the surface. The T44 Beretta utilises a triple tube core barrel system operated using a wireline drilling process. The outer barrel is rotated by the drill rods and at its lower end, carries the coring bit. The inner barrel is mounted on a swivel so that it does not rotate during the process. The third barrel or liner is placed within the second one to retain the core intact and to preserve as much as possible the fabric of the drilling stratum. The core is cut by the coring bit and passes to the inner liner. The core is brought up to the surface within the inner barrel on a small diameter wire rope or line attached to the "overshoot" recovery tool which is then placed into a core box in order of recovery. A drilling fluid, typically air mist or water flush is passed from the surface through hollow drill rods to the drill bit and is used to cool the drill bit. Temporary casing is used in some situations to support unstable ground or to seal off fissures or voids. It should be noted that the rotary coring can only achieve limited recovery in overburden, particularly granular or weakly cemented strata due to the flushing medium washing away the cohesive fraction during coring. The recovery achieved, where required is noted on the borehole logs and core photographs are provided to allow assessment of the core recovered. The rotary borehole logs are provided in Appendix 5 of this Report.

3.7. Surveying

The exploratory hole locations have been recorded using a KQ GEO Technologies KQ-M8 System which records the coordinates and elevation of the locations to ITM or Irish National Grid as required by the project specification. The coordinates and elevations are provided on the exploratory hole logs in the appendices of this Report.

3.8. Groundwater/Gas Monitoring Installations

Groundwater and or Gas Monitoring Installation were installed upon the completion of the boreholes to enable sampling and the determination of the equilibrium groundwater level. The typical groundwater monitoring installation consists of a 50mm uPVC/HDPE slotted pipe with a pea gravel response zone and bentonite seal installed to the Engineers specification. Where required the standpipe is sealed with a gas tap and finished with a durable steel cover fixed in place with a concrete surround. The installation details are provided on the exploratory hole logs in the appendices of this Report.

3.9. Insitu Plate Bearing Test

The plate bearing tests were carried out using a 450mm diameter plate at the locations shown on the site plan in Appendix 1. The plate was loaded in increments using a hydraulic jack and an excavator to provide a reaction and the displacement was monitored in accordance with BS1377 Part 9 using independently mounted digital strain gauges. The constrained modulus and equivalent CBR are calculated in accordance with HD29/75 and are provided on the test reports in Appendix 6 of this Report.

3.10. TRL Dynamic Cone Penetrometer

The TRL DCP tests were carried out at locations specified by the Consulting Engineer to determine a CBR design value for the design of external pavements. The testing was carried out below the Topsoil or existing pavement at the depths detailed on the test report. The test consists of dropping a 10kg weight on an anvil to drive a small diameter cone and recording the blows for a given penetration. The results of the DCP testing are included in Appendix 7 of this Report.

3.11. Laboratory Testing

Samples were selected from the exploratory holes for a range of geotechnical and environmental testing to assist in the classification of soils and to provide information for the proposed design.

Environmental & Chemical testing as required by the specification, including the Rilta Suite and Engineers Ireland Suites E and D was carried out by Element Materials Technology Laboratory in the UK. The Rilta suite testing includes both Solid Waste and Leachate Waste Acceptance Criteria. Chemical testing including organic matter content, sulphate content, chloride content and pH was carried out in Professional Soils Laboratory (PSL Ltd) in the UK.

Geotechnical testing consisting of moisture content, Atterberg limits, Particle Size Distribution (PSD), hydrometer and Moisture Condition Value (MCV) tests were also carried out in Professional Soils Laboratory (PSL Ltd). Specialist shear strength testing consisting of quick undrained, shear box and consolidation testing was carried out on undisturbed U100 or piston samples where recovered.

Rock strength testing including Point Load (Is_{50}) and Unconfined Compressive Strength (UCS) testing was carried out by Construction Materials Testing Laboratories (CTML) Ireland. The results of the laboratory testing are included in Appendix 8 of this Report.

4.0 Ground Conditions

4.1. General

The ground conditions encountered during the investigation are summarised below with reference to in situ and laboratory test results. The full details of the strata encountered during the ground investigation are provided in the exploratory hole logs included in the appendices of this report.

The sequence of strata encountered were generally consistent across the site and generally comprised;

- Surfacing
- Made Ground
- Organic Deposits
- Soft Cohesive Deposits
- Cohesive Deposits
- Granular Deposits
- Bedrock

SURFACING: Tarmac surfacing was present typically to a depth of 0.06m BGL.

MADE GROUND: Made Ground deposits were encountered beneath the Surfacing and were generally present to depths of between 0.5m and 1.0m BGL and a maximum of 3.4m BGL in BRC04. These deposits were described generally as *grey Sand and Gravel FILL and contained occasional fragments of tarmacadam* occasionally overlying *grey slightly sandy gravelly Clay* and *brownish black gravelly Peat with occasional red brick, ceramic and rubbish fragments.*

ORGANIC DEPOSITS: Organic deposits were generally encountered beneath the Made Ground and were described typically as *brownish black slightly clayey slightly gravelly PEAT.* The secondary constituents varied across the site, with silt and clay lenses occasionally present in the peat. The strength of the deposits was typically very soft based on SPT N values.

SOFT COHESIVE DEPOSITS: Soft Cohesive deposits were encountered beneath the organic deposits and were generally described as *beige or cream clayey SILT with frequent shell fragments* occasionally onto *light grey slightly sandy slightly gravelly clayey SILT with occasional cobbles.* The secondary sand and gravel constituents varied across the site and with depth, and peat lenses were occasionally present within the deposits. The strength of the soft cohesive deposits was typically very soft to soft.

COHESIVE DEPOSITS: Cohesive deposits were encountered beneath the soft cohesive deposits at some locations and were described typically as *light grey to grey slightly sandy slightly gravelly silty CLAY with occasional cobbles.* The secondary sand and gravel constituents varied across the site and with depth. The strength of the cohesive deposits typically increased with depth and was stiff or very stiff below 6.0m BGL

in the majority of the exploratory holes. These deposits had some occasional cobble content, where noted on the exploratory hole logs.

GRANULAR DEPOSITS: Granular deposits were occasionally encountered at the base of the cohesive deposits and were typically described as *grey very sandy subangular to subrounded fine to coarse GRAVEL with occasional cobbles*. The secondary sand constituents varied across the site while occasional cobble content was also present where noted on the exploratory hole logs.

Based on the SPT N values the deposits are typically medium dense to dense and become dense with depth. Groundwater strikes were occasionally noted in the boreholes on encountering the granular deposits.

BEDROCK: The rotary core boreholes recovered Strong thinly to medium bedded grey fine to medium grained fossiliferous LIMESTONE, with the exception of BRC04 which recovered strong to very strong thinly to thickly banded dark green medium to coarsely crystalline METAGABBRO. Occasional calcite veins were noted during logging.

The depth to rock increases to the southeast from 11.2m BGL in BH01 in the north western corner of the site to a maximum of 15.3m BGL in BRC03 in the centre of the site. The depth to rock decreases to 9.4m BGL in BRC06, and further decreases to between 6.6m and 6.1m BGL respectively in BRC04 and BRC05 in the southeastern portion of the site. The total core recovery is typically 100% within bedrock. The SCR and RQD are generally poorer in the upper weathered zone, however both indices show an increase with depth in each of the boreholes.

4.2. Groundwater

Groundwater strikes are noted on the exploratory hole logs where they occurred and where possible drilling was suspended for twenty minutes to allow the subsequent rise in groundwater to be recorded. We would point out that these exploratory holes did not remain open for sufficiently long periods of time to establish the hydrogeological regime and groundwater levels would be expected to vary with the tide, time of year, rainfall, nearby construction and other factors. For this reason, standpipes were installed in BRC1, BRC02, BRC04 and BRC05 to allow the equilibrium groundwater level to be determined. The groundwater monitoring is included in Appendix 9 of this Report.

4.3. Laboratory Testing

4.3.1. Geotechnical Laboratory Testing

The geotechnical testing carried out on soil samples recovered generally confirm the descriptions on the logs with the primary constituent of the cohesive deposits found to be a CLAY of high plasticity. The Particle Size Distribution tests confirm that generally the cohesive deposits are well-graded to gap graded with percentages of sands and gravels ranging between 1% and 48% generally with fines contents of 14 to 97%.

The Particle Size Distribution tests confirm that generally the granular deposits are gap graded with percentages of silt/clay typically between 1% and 2% with a sand content of typically 7% to 14%. gravel content of typically 53% to 68%.

Undrained shear strength testing on undisturbed samples gave results ranging between 3kPa and 13 kPa which correlated with results of laboratory vane tests which gave peak shear strengths ranging from 6 to 27 kPa.

MCV Values ranged between <1 to 4.5 in the cohesive deposits meaning the material would be considered unsuitable for reuse.

4.3.2. Chemical Laboratory Testing

The pH and sulphate testing carried out indicate that pH results are near neutral to alkaline, with values ranging from 7.1 to 9.6, and that the water soluble sulphate results range from low to elevated (DS-1 to DS-2) when compared to the guideline values from BRE Special Digest 1:2005. Samples of the Peat (TP3 0.6-1.1m BGL) have elevated Total Sulphur results (0.52%) which result in a high Total Potential Sulphate (TPS) and a subsequent higher classification of the Design Sulphate values in accordance with the BRE digest. Caution is recommended and the removal of the Peat and Made ground stratum to reduce the risk of sulphate attack adjacent to concrete elements as part of the permanent works. Piles in the organic and very soft deposits are recommended to incorporate appropriate measures to resist sulphate attack.

4.3.3. Environmental Laboratory Testing

A number of samples were analysed for a suite of parameters which allows for the assessment of the sampled material in terms of total pollutant content for classification of materials as *hazardous* or *non-hazardous*. The suite also allows for the assessment of the sampled material in terms of suitability for placement at licenced landfills (inert, stable non-reactive, hazardous etc.). The parameter list for the suite includes analysis of the solid samples for arsenic, barium, cadmium, chromium, copper, cyanide, lead, nickel, mercury, zinc, speciated aliphatic and aromatic petroleum hydrocarbons, pH, sulphate, sulphide, moisture content, soil organic matter and an asbestos screen.

The suite also includes those parameters specified in the EU Council Decision establishing criteria for the acceptance of waste at Landfills (Council Decision 2003/33/EC), which for the solid samples are total organic carbon (TOC), speciated aliphatic and aromatic petroleum hydrocarbons, BTEX, phenol, polychlorinated biphenyls (PCB) and PAH.

As part of the suite a leachate is generated from the solid sample which is analysed for antimony, arsenic, barium, cadmium, chromium, copper, lead, mercury, molybdenum, nickel, selenium, zinc, chloride, fluoride, soluble sulphate, sulphide, phenols, dissolved organic carbon (DOC) and total dissolved solids (TDS).

While the laboratory report provides a comparison with the waste acceptance criteria limits it does not provide a waste classification of the material sampled nor does it comment on any potentially hazardous properties of the materials tested. The possibility for contamination, not revealed by the testing undertaken should be borne in mind particularly where Made Ground deposits are present or the previous site use or

location indicate a risk of environmental variation. The waste classification report is included under the cover of a separate report by Ground Investigations Ireland.

4.3.4. Rock Laboratory Testing

The rock testing carried out on samples recovered from the boreholes reported Unconfined Compressive Strength (UCS) values ranging between 88.1 and 140.7 MPa while the point load testing gave I_{s50} values ranging between 2.93 to 8.79 MPa. The I_{s50} results correlate to the UCS values using a factor of approximately 20, giving values of 58.6 MPa and 175.8 MPa. These results correlate to the strength descriptions ranging between of Strong to very strong and confirming the descriptions on the logs.

The results from the completed laboratory testing are included in Appendix 8 of this report

4.4. Geotechnical Design Parameters

Preliminary geotechnical design parameters for the materials encountered during the ground investigation have been summarised in Table 1 Geotechnical Design Parameters. Both laboratory test and SPT N results, using standard empirical relationships, have been used to determine the geotechnical parameters of the overburden strata.

Shear strength parameters have been determined using laboratory testing and established empirical relationships for the relevant materials. Based on the relationship published by Stroud, the correlation of $C_u = f_1 \times N$ is used to estimate the undrained shear strength of the cohesive deposits, where f_1 is determined using a correlation with the plasticity index.

The shear strength parameters from the granular stratum are provided using the effective shear strength parameters determined from the uncorrected SPT N values after Peck et al. reported by Tomlinson Foundation Design and Construction 7th Ed. (2001).

A range is provided for the compressibility parameter m_v based on correlations with the SPT N value based on the relationship published by Stroud, the correlation of $M_v = 1/(f_2 \times N)$ where f_2 is determined using a correlation with the plasticity index

Table 1 Recommended Geotechnical Parameters based on GII GI Data

| Stratum | Bulk Density (kN/m ³) | SPT 'N' Correlated | Undrained Shear Strength C _u (kN/m ²) | Effective Strength Parameters | | Poisson's Ratio ν (ν _u) | Co-efficient of Compressibility m _v (m ² /MN) |
|--|--------------------------------------|-----------------------|--|-------------------------------------|------------------------|---|--|
| | | | | Cohesion c' (kN/m ²) | φ' degrees | | |
| Granular Made Ground Deposits | 16 – 20* ¹ | 1 - 20 | n/a | - | 28 – 30* ⁴ | 0.1 – 0.3 | n/a |
| Cohesive Made Ground Deposits | 16 – 20* ¹ | 1 - 20 | 5 – 100* ² | 0 | 25 - 30* ⁴ | 0.2 (0.5) | 0.1-1.5* ³ |
| Soft Peat | 10 | n/a | 0 – 5 kPa | 0 – 2 | n/a | n/a | >1.5 |
| Soft Cohesive Deposits | 16 – 20* ¹ | 1 - 8 | 5 - 40* ² | 0 - 1 | 25 - 28* ⁴ | 0.2 (0.5) | 0.1 – 1.5* ³ |
| Firm Cohesive Deposits | 18 – 20 ¹ | 8 – 15 | 40 - 75* ² | 0 - 3 | 28 – 30* ⁴ | 0.2 (0.5) | 0.1 – 0.3* ³ |
| Stiff Cohesive Deposits | 19 – 20* ¹ | 15 – 25 | 75 - 150* ² | 0 - 5 | 30 - 33* ⁴ | 0.2 (0.5) | 0.05 – 0.1* ³ |
| Very Stiff Cohesive Deposits | 20 – 22 | 25+ | 150+ | 0 – 5 | 30 – 33+* ⁴ | 0.2 (0.5) | 0.05 – 0.1* ³ |
| Loose Granular Deposits ¹ | 16 – 18* ¹ | 1 - 10 | n/a | n/a | 28 – 30 * ⁴ | 0.1 – 0.3 | n/a |
| Medium Dense Granular Deposits ¹ | 18 – 21* ¹ | 10 - 30 | n/a | n/a | 30 – 36 * ⁴ | 0.1 – 0.3 | n/a |
| Dense Granular Deposits ¹ | 21 - 23* ¹ | 30+ | n/a | n/a | 36+ * ⁴ | 0.1 – 0.3 | n/a |

*1 Values for bulk density assumed

*2 Based on correlated SPT N values

*3 Based on correlated SPT N values and published data. Caution should be exercised when selecting design values for the variable Made Ground Stratum.

*4 Testing on undisturbed samples is recommended to determine the design value of this parameter for detailed design.

NOTE: The values in Table 1 represent a range of recommended values based on the typical soil types, insitu testing and laboratory testing scheduled by the Consulting Engineer. The values presented are recommended for outline guidance only and specific designs should derive design values based on the exploratory hole logs and lab testing for each specific site. To determine specific design values relevant to the design being undertaken in a particular area, reference should be made to the relevant specific exploratory hole logs. Further testing is recommended to determine the specific geotechnical parameters required for foundation design and temporary works design

5.0 Recommendations & Conclusions

5.1. General

The recommendations given and opinions expressed in this report are based on the findings as detailed in the exploratory hole records. Where an opinion is expressed on the material between exploratory hole locations, this is for guidance only and no liability can be accepted for its accuracy. No responsibility can be accepted for conditions which have not been revealed by the exploratory holes. Limited information has been provided at the ground investigation stage and any designs based on the recommendations or conclusions should be completed in accordance with the current design codes, taking into account the variation and the specific details contained within the exploratory hole logs.

5.2. Foundations

Due to the presence of soft and compressible Cohesive deposits beneath the footprint of the proposed structure and high loading anticipated, piled foundations are recommended for the proposed building. The type, size and depth of the pile foundations should be confirmed by a specialist piling contractor based on the loading from the proposed building. The floor slab is recommended be suspended and also supported on the building piles.

Negative skin friction from the very soft cohesive deposits should be considered in the pile design due to the possibility of loading from working platforms or the adjacent pavement make up.

The pH and sulphate testing carried out indicate that pH results are near neutral to alkaline, with values ranging from 7.1 to 9.6, and that the water soluble sulphate results range from low to elevated (DS-1 to DS-2) when compared to the guideline values from BRE Special Digest 1:2005. Samples of the Peat (TP3 0.6-1.1m BGL) have elevated Total Sulphur results (0.52%) which result in a high Total Potential Sulphate (TPS) and a subsequent higher classification of the Design Sulphate values in accordance with the BRE digest. Caution is recommended and the removal of the Peat and Made ground stratum to reduce the risk of sulphate attack adjacent to concrete elements as part of the permanent works. Piles in the organic and very soft deposits are recommended to incorporate appropriate measures to resist sulphate attack.

5.3. External Pavements

The proposed pavements are recommended to be designed in accordance with the CBR test results included in the Appendices of this Report. The low CBR test results indicate that a capping layer or a sufficient depth of crushed stone fill may be required. Plate bearing tests are recommended at the time of construction to verify the design assumptions for the proposed pavement make up and to verify adequate compaction has been achieved.

The use of a geogrid and separation membrane may improve the performance of the proposed pavement and enable a more economical pavement design to be achieved, a specialist supplier is recommended to advise of the required strength, depth and type of geotextile for the proposed design.

5.4. Excavations

Short term temporary excavations in the cohesive deposits will remain stable for a limited time only and will require to be appropriately battered or the sides supported if the excavation is below 1.25m BGL or is required to permit man entry.

Excavations in the Made Ground, Peat or soft Cohesive Deposits will require to be appropriately battered or the sides supported due to the low strength of these deposits.

Any excavations which penetrate the granular deposits will require to be appropriately battered or the sides supported and are likely to require dewatering due to the groundwater seepages noted in the exploratory hole logs in the Appendices of this Report.

The groundwater and stability noted on the trial pit logs should be consulted when determining the most appropriate construction methods for excavations. Generally, where significant excavations are required in water bearing granular deposits a cut-off wall may be more cost effective than extensive dewatering. An assessment by a specialist dewatering contractor is recommended to determine the most cost effective approach to the proposed excavation.

Excavations in the upper cohesive are expected to be excavatable with conventional excavation equipment. Any waste material to be removed off site should be disposed of to a suitably licenced landfill.

The environmental testing completed during the ground investigation is reported under the cover of a separate GII Waste Classification/Subsoil Assessment Report.

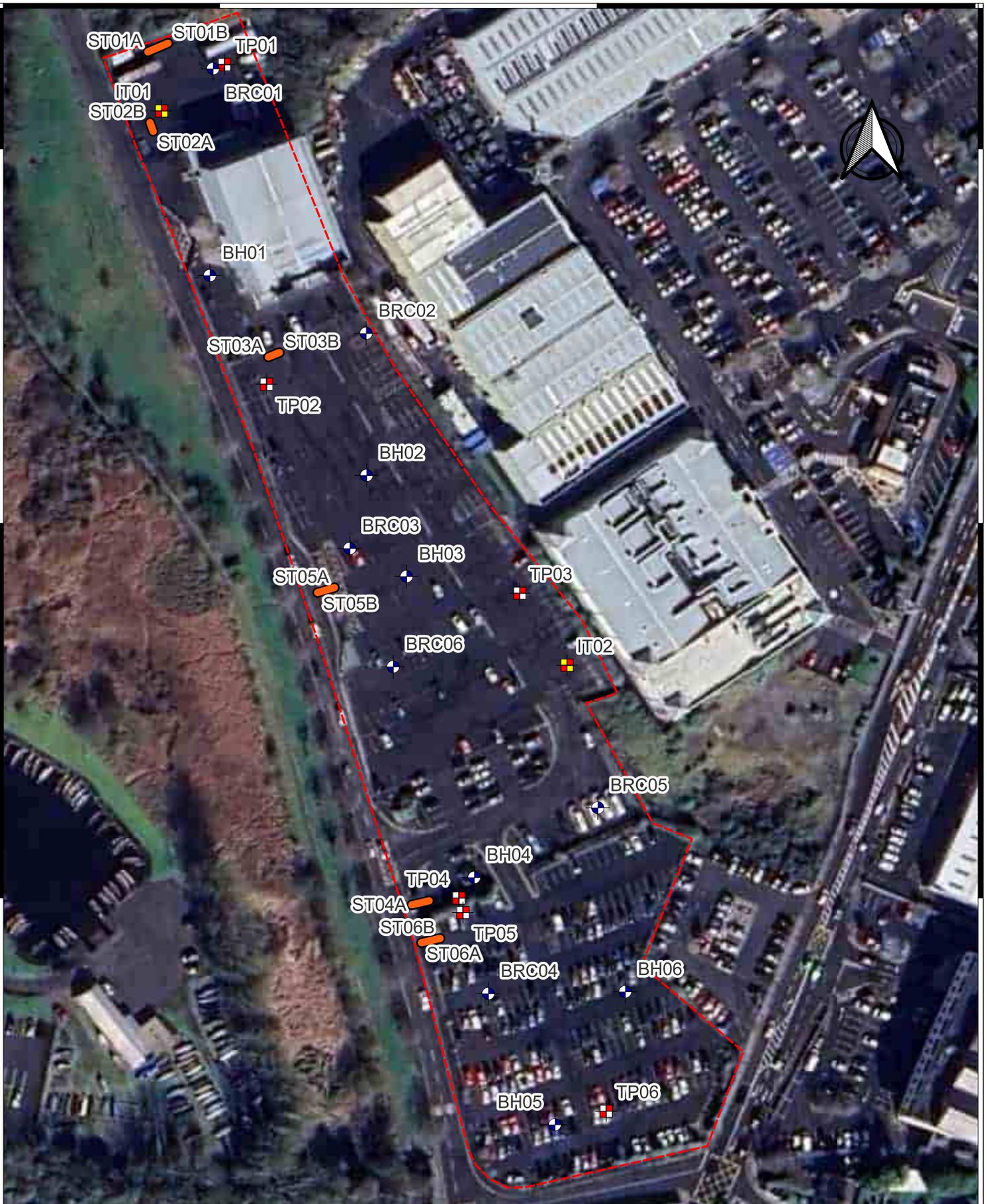
5.5. Soakaway Design

At the locations of IT01 and IT02 the water level dropped too slowly to allow calculation of 'f' the soil infiltration rate. These locations are therefore not recommended as suitable for soakaway design and construction.

The recommendations provided in this report should be verified in the design of the proposed buildings, using the full details of the loading conditions and taking into consideration the allowable tolerable settlements/movements that the building can accommodate. The founding strata should be inspected and verified by a suitably qualified engineer prior to construction of the building foundations.

APPENDIX 1 - Site Location Plan





GROUND INVESTIGATIONS IRELAND
Professional & Field Services

Ground Investigations Ireland Ltd.
 Catherinstown House,
 Hazelhatch Road,
 Newcastle, Co. Dublin
 www.gii.ie 01-6015175/5176

Client:



Project Title:
 Dyke Road Galway

Drawing Title:
 Investigation Locations

GII Project Reference:
 13614-02-24

Drawn By:
 MS

Date:
 24-06-24

-  Trial Pits
-  Slit Trench Locations
-  Borehole
-  Infiltration Test

APPENDIX 2 – Trial Pit Records





| | | | | |
|--|-----------------------------------|----------------------------|-----------------|---------------------------|
| Machine : 3T Tracked Excavator Method : Trial Pit | Dimensions 2.20 x 0.87 x 2.20m | Ground Level (mOD) 4.27 | Client Aecom | Job Number 13614-02-24 |
| | Location 529801.3 E 726122.3 N | Dates 17/04/2024 | Engineer | Sheet 1/1 |

| Depth (m) | Sample / Tests | Water Depth (m) | Field Records | Level (mOD) | Depth (m) (Thickness) | Description | Legend | Water |
|--------------|----------------|-----------------|--------------------|--------------|-----------------------|---|--------|-------|
| 0.50 | B | | | 4.20 | (0.07) 0.07 | TARMACADAM | | |
| | | | | | (0.67) | MADE GROUND: Grey sandy angular to subrounded fine to coarse Gravel | | |
| 1.00 1.00 | HV 34kPa B | | 44,38,20/Av. 34.00 | 3.53 3.52 | 0.74 0.75 | MADE GROUND: Black plastic net membrane | | |
| | | | | | (1.45) | Brown fibrous PEAT | | |
| | | | | 2.07 | 2.20 | Complete at 2.20m | | |

| | | | | | | |
|---|--|------------------|-----------|------------|------|----|
| Plan | Remarks No groundwater encountered during excavation Trial pit sidewalls stable DCP carried out at 0.8m BGL. Trial pit backfilled upon completion | | | | | |
| | <table border="1"> <tr> <td>Scale (approx)</td> <td>Logged By</td> <td>Figure No.</td> </tr> <tr> <td>1:25</td> <td>LB</td> <td>13614-02-24.TP01</td> </tr> </table> | Scale (approx) | Logged By | Figure No. | 1:25 | LB |
| Scale (approx) | Logged By | Figure No. | | | | |
| 1:25 | LB | 13614-02-24.TP01 | | | | |



Machine : 3T Tracked Excavator
Method : Trial Pit

Dimensions
2.10 x 0.88 x 2.20m

Ground Level (mOD)
5.01

Client
Aecom
Job Number
13614-02-24

Location
529812.4 E 726036.9 N

Dates
17/04/2024

Engineer
Sheet
1/1

| Depth (m) | Sample / Tests | Water Depth (m) | Field Records | Level (mOD) | Depth (m) (Thickness) | Description | Legend | Water |
|-----------|----------------|-----------------|--------------------|--------------|-----------------------|---|--------|-------|
| 0.50 | B | | | 4.94 | (0.07) | TARMACADAM | | |
| 0.70 | HV 14kPa | | 10,16,16/Av. 14.00 | 4.42 4.41 | (0.52) | MADE GROUND: Grey sandy subangular to subrounded fine to coarse Gravel with medium angular to subangular cobble content | | |
| 1.00 | B | | Slow(1) at 1.30m. | | 0.59 0.60 | MADE GROUND: Black plastic net membrane | | |
| 2.00 | HV 9kPa B | | 10,10,8/Av. 9.33 | | (1.20) | MADE GROUND: Brown fibrous Peat with frequent wood fragments and occasional plastic glass and wire fragments | | |
| 2.00 | | | | 3.21 | 1.80 | Very soft cream clayey SILT with frequent shell fragments | | |
| | | | | 2.81 | (0.40) | Complete at 2.20m | | |

Plan

| | | | | | | | | | |
|---|---|---|---|---|---|---|---|---|---|
| . | . | . | . | . | . | . | . | . | . |
| . | . | . | . | . | . | . | . | . | . |
| . | . | . | . | . | . | . | . | . | . |
| . | . | . | . | . | . | . | . | . | . |
| . | . | . | . | . | . | . | . | . | . |
| . | . | . | . | . | . | . | . | . | . |

Remarks

Groundwater encountered at 1.30m BGL. Slow
Trial pit sidewalls stable
DCP carried out at 0.7m BGL
Trail pit backfilled upon completion

| | | |
|-------------------------------|------------------------|---------------------------------------|
| Scale (approx) 1:25 | Logged By LB | Figure No. 13614-02-24.TP02 |
|-------------------------------|------------------------|---------------------------------------|



Machine : 3T Tracked Excavator
Method : Trial Pit

Dimensions
1.80 x 0.63 x 2.30m

Ground Level (mOD)
5.06

Client
Aecom

Job Number
13614-02-24

Location
529879.9 E 725980.6 N

Dates
15/04/2024

Engineer

Sheet
1/1

| Depth (m) | Sample / Tests | Water Depth (m) | Field Records | Level (mOD) | Depth (m) (Thickness) | Description | Legend | Water |
|-----------|----------------|-----------------|--------------------|--------------|-----------------------|---|--------|-------|
| 0.50 | B | | | 4.96 | (0.10) 0.10 | TARMACADAM | | |
| 0.65 | HV 53kPa | | 50,60,50/Av. 53.33 | 4.46 4.45 | 0.60 0.61 | MADE GROUND: Grey sandy angular to subrounded fine to coarse Gravel | | |
| 1.00 | B | | | | (0.49) | MADE GROUND: Membrane | | |
| 1.50 | HV 9kPa | | 10,10,6/Av. 8.67 | | | Dark brown fibrous PEAT with occasional rootlets | | |
| 2.00 | B | | | 3.96 | 1.10 | Very soft cream clayey SILT with frequent shell fragments | | |
| | | | | | (1.20) | | | |
| | | | | 2.76 | 2.30 | Complete at 2.30m | | |

Plan

| | | | | | | | | | |
|---|---|---|---|---|---|---|---|---|---|
| . | . | . | . | . | . | . | . | . | . |
| . | . | . | . | . | . | . | . | . | . |
| . | . | . | . | . | . | . | . | . | . |
| . | . | . | . | . | . | . | . | . | . |
| . | . | . | . | . | . | . | . | . | . |
| . | . | . | . | . | . | . | . | . | . |

Remarks

No groundwater encountered during excavation
 Trial pit sidewalls stable
 Plate bearing test carried out at 0.20m BGL
 DCP carried out at 0.7m BGL
 Trial pit backfilled upon completion

| | | |
|-------------------------------|------------------------|---------------------------------------|
| Scale (approx) 1:25 | Logged By LB | Figure No. 13614-02-24.TP03 |
|-------------------------------|------------------------|---------------------------------------|



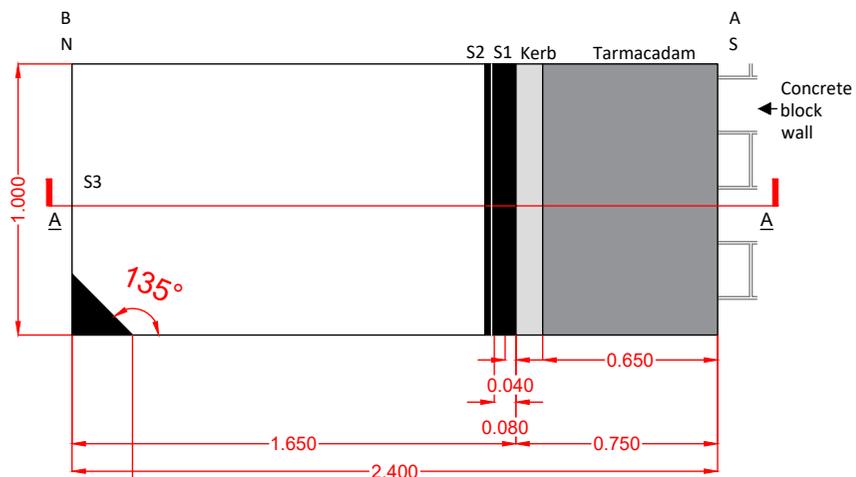
| | | | | |
|--|-----------------------------------|----------------------------|-----------------|---------------------------|
| Machine : 3T Tracked Excavator Method : Trial Pit | Dimensions 2.40 x 1.00 x 1.20m | Ground Level (mOD) 6.24 | Client Aecom | Job Number 13614-02-24 |
| | Location 529863.4 E 725899.1 N | Dates 15/04/2024 | Engineer | Sheet 1/1 |

| Depth (m) | Sample / Tests | Water Depth (m) | Field Records | Level (mOD) | Depth (m) (Thickness) | Description | Legend | Water |
|-----------|----------------|-----------------|---------------|-------------|-----------------------|--|--------|-------|
| 0.50 | B | | | 6.17 | (0.07) | TARMACADAM | | |
| | | | | | (0.93) | MADE GROUND: Grey slightly clayey sandy angular to subrounded fine to coarse Gravel with medium cobble and boulder content | | |
| 1.00 | B | | | 5.24 | 1.00 (0.20) | MADE GROUND: Brown slightly sandy slightly gravelly Clay with occasional red brick fragments | | |
| | | | | 5.04 | 1.20 | Complete at 1.20m | | |

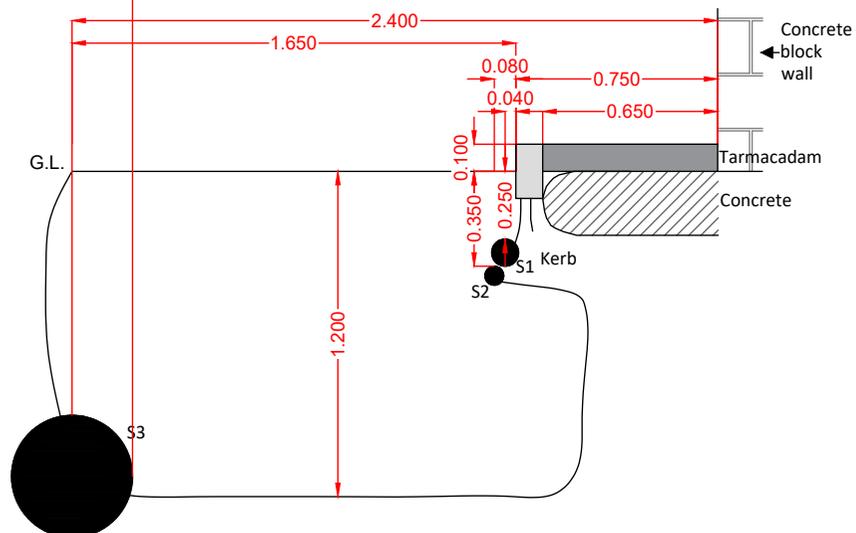
| | | | | | | |
|---|--|------------------|-----------|------------|------|----|
| Plan | Remarks No groundwater encountered during excavation Trial pit sidewalls collapsing DCP carried out at 0.9m BGL. Trial pit backfilled upon completion | | | | | |
| | <table border="1"> <tr> <td>Scale (approx)</td> <td>Logged By</td> <td>Figure No.</td> </tr> <tr> <td>1:25</td> <td>LB</td> <td>13614-02-24.TP04</td> </tr> </table> | Scale (approx) | Logged By | Figure No. | 1:25 | LB |
| Scale (approx) | Logged By | Figure No. | | | | |
| 1:25 | LB | 13614-02-24.TP04 | | | | |

TP04

PLAN VIEW



SECTION A-A



FOUNDATION PIT LOG

- 0.00 - 0.07 TARMACADAM.
- 0.07 - 1.00 MADE GROUND: Grey slightly clayey sandy angular to subrounded fine to coarse Gravel with some cobbles and boulders.
- 1.00 - 1.20 MADE GROUND: Brown slightly sandy slightly gravelly Clay with some red brick fragments.

SERVICES:

- S1 - Ø0.100m black duct 90°
- S2 - Ø0.070m black duct 90°
- S3 - Approximately Ø0.450m concrete 45°



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| | |
|--------------|--------------------------------|
| PROJECT: | 13614-02-24 - Dyke Road Galway |
| DRAWING No.: | TP-04 |
| DATE: | 15/04/2024 |
| CLIENT: | Aecom |
| SCALE: | NTS |

| | | | |
|----------|------------|-----------|-------------|
| Version: | Date: | Drawn By: | Checked By: |
| 1 | 14/05/2024 | J.S. | M.S. |



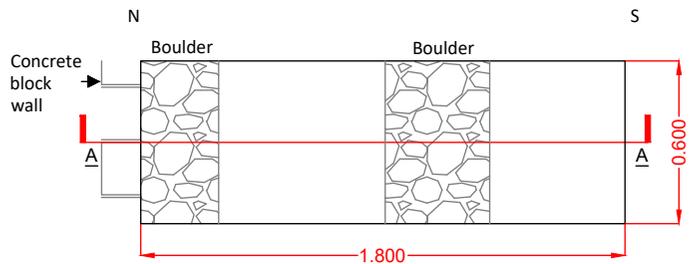
| | | | | |
|--|-----------------------------------|----------------------------|-----------------|---------------------------|
| Machine : 3T Tracked Excavator Method : Trial Pit | Dimensions 1.80 x 0.60 x 1.35m | Ground Level (mOD) 6.24 | Client Aecom | Job Number 13614-02-24 |
| | Location 529863.4 E 725899.1 N | Dates 15/04/2024 | Engineer | Sheet 1/1 |

| Depth (m) | Sample / Tests | Water Depth (m) | Field Records | Level (mOD) | Depth (m) (Thickness) | Description | Legend | Water |
|-----------|----------------|-----------------|---------------|-------------|-----------------------|---|--------|-------|
| 0.50 | B | | | 6.17 | (0.07) | TARMACADAM | | |
| | | | | | (0.48) | MADE GROUND: Grey slightly sandy angular to subrounded fine to coarse Gravel | | |
| 1.00 | B | | | 5.69 | 0.55 | MADE GROUND: Light brown slightly clayey slightly gravelly fine to coarse Sand with medium angular to subrounded cobble and boulder content. Gravel is subangular to subrounded fine to coarse. | | |
| | | | | | (0.80) | | | |
| | | | | 4.89 | 1.35 | Complete at 1.35m | | |

| | | | | | | |
|---|--|------------------|-----------|------------|------|----|
| Plan | Remarks No groundwater encountered during excavation Trial pit sidewalls stable Trial pit backfilled upon completion | | | | | |
| | <table border="1"> <tr> <td>Scale (approx)</td> <td>Logged By</td> <td>Figure No.</td> </tr> <tr> <td>1:25</td> <td>LB</td> <td>13614-02-24.TP05</td> </tr> </table> | Scale (approx) | Logged By | Figure No. | 1:25 | LB |
| Scale (approx) | Logged By | Figure No. | | | | |
| 1:25 | LB | 13614-02-24.TP05 | | | | |

TP05

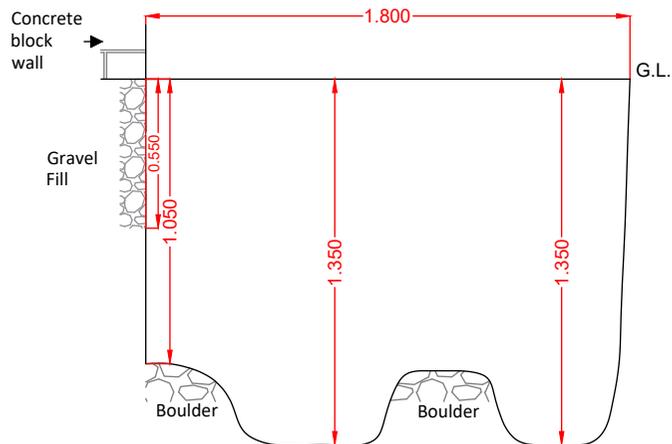
PLAN VIEW



FOUNDATION PIT LOG

- 0.00 - 0.07 TARMACADAM.
- 0.07 - 0.55 MADE GROUND: Grey slightly sandy angular to subrounded fine to coarse Gravel.
- 0.55 - 1.35 MADE GROUND: Light brown slightly clayey slightly gravelly fine to coarse Sand with some angular to subrounded cobbles and boulders.

SECTION A-A



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| | |
|--------------|--------------------------------|
| PROJECT: | 13614-02-24 - Dyke Road Galway |
| DRAWING No.: | TP-05 |
| DATE: | 15/04/2024 |
| CLIENT: | Aecom |
| SCALE: | NTS |

| | | | |
|----------|------------|-----------|-------------|
| Version: | Date: | Drawn By: | Checked By: |
| 1 | 14/05/2024 | J.S. | M.S. |



| | | | | |
|--|-----------------------------------|----------------------------|-----------------|---------------------------|
| Machine : 3T Tracked Excavator Method : Trial Pit | Dimensions 2.30 x 0.80 x 0.80m | Ground Level (mOD) 7.16 | Client Aecom | Job Number 13614-02-24 |
| | Location 529902.4 E 725842.8 N | Dates 17/04/2024 | Engineer | Sheet 1/1 |

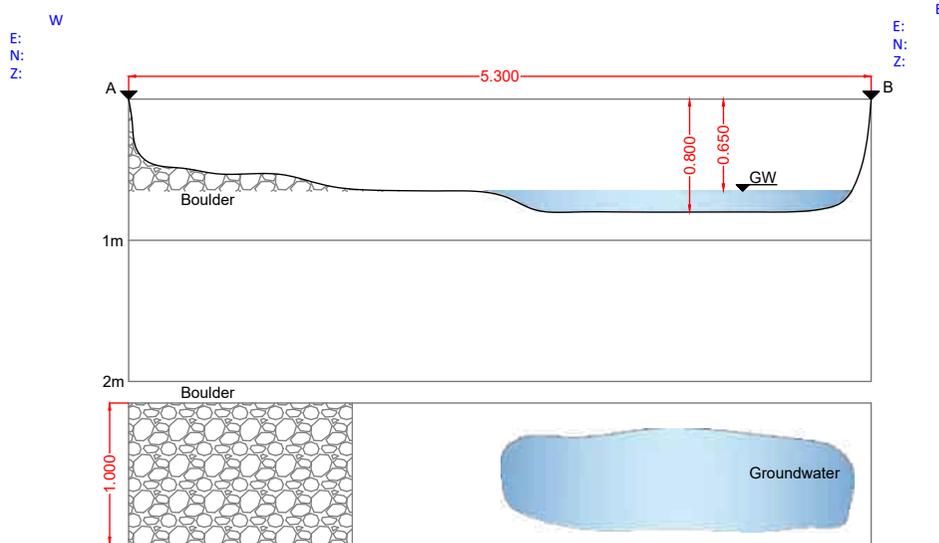
| Depth (m) | Sample / Tests | Water Depth (m) | Field Records | Level (mOD) | Depth (m) (Thickness) | Description | Legend | Water |
|-----------|----------------|-----------------|---------------|-------------|-----------------------|---|--------|-------|
| 0.50 | B | | | 7.06 | (0.10) 0.10 | TARMACADAM | | |
| 0.70 | B | | | 6.56 | (0.50) | MADE GROUND: Grey slightly sandy angular to subrounded fine to coarse Gravel with high angular to subangular cobble content and an old wire | | |
| | | | | 6.36 | 0.60 (0.20) | POSSIBLE MADE GROUND: Brown clayey slightly gravelly fine to coarse Sand | | |
| | | | | | 0.80 | Complete at 0.80m | | |

| | | | | | | |
|---|--|------------------|-----------|------------|------|----|
| Plan | Remarks No groundwater encountered during excavation Trial pit sidewalls stable Trial pit backfilled upon completion | | | | | |
| | <table border="1"> <tr> <td>Scale (approx)</td> <td>Logged By</td> <td>Figure No.</td> </tr> <tr> <td>1:25</td> <td>LB</td> <td>13614-02-24.TP06</td> </tr> </table> | Scale (approx) | Logged By | Figure No. | 1:25 | LB |
| Scale (approx) | Logged By | Figure No. | | | | |
| 1:25 | LB | 13614-02-24.TP06 | | | | |

APPENDIX 3 – Slit Trench Records



ST-01



| Service No | ø (m) | Colour - Material | Utility | Angle to trench | Coordinates | | Level |
|------------|-------|-------------------|---------|-----------------|-------------|-------|-------|
| | | | | | East | North | |
| | | | | | | | |

| Surface from/to (m) | Surface type |
|---------------------|--------------|
| 0.00 - 5.30 | TARMACADAM |

| Sample depth (m) | Sample type |
|------------------|-------------|
| 0.5 | B |

| From (m) | To (m) | Description |
|----------|--------|--|
| 0.00 | 0.05 | TARMACADAM. |
| 0.05 | 0.20 | MADE GROUND: Dark grey slightly sandy angular to subangular fine to coarse Gravel. |
| 0.20 | 0.80 | MADE GROUND: Greyish brown sandy gravelly Clay with some fragments of metal, cans and some angular to subangular cobble content. |

| Groundwater | Y/N | Depth | Notes |
|-------------|-----|-------|-------|
| Slow | Y | 0.65 | |



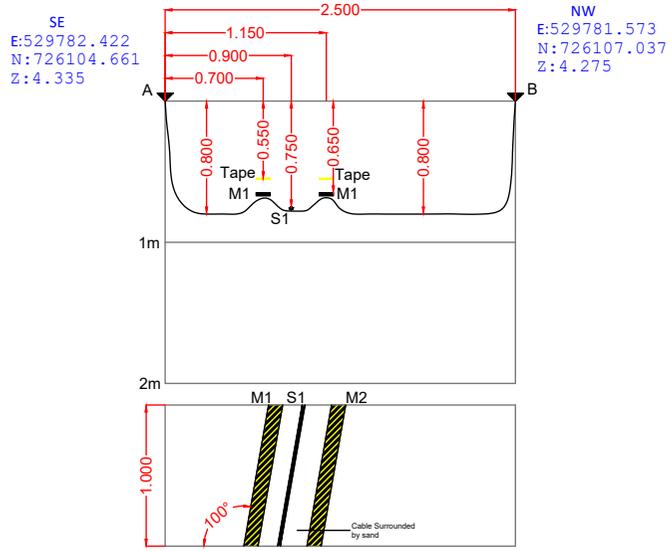
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Tel: +353-(0)1 6015175/6 Fax: +353-(0)1 6015173
Email: info@gii.ie Web: www.gii.ie

| | |
|--------------|--------------------------------|
| PROJECT: | 13614-02-24 - Dyke Road Galway |
| DRAWING No.: | ST-01 |
| DATE: | 11/04/2024 |
| CLIENT: | Aecom |
| SCALE: | NTS |

| Version: | Date: | Drawn By: | Checked By: |
|----------|------------|-----------|-------------|
| 1 | 23/04/2024 | J.S. | L.B. |

ST-02



| Service No | ø (m) | Colour - Material | Utility | Angle to trench | Coordinates | | Level |
|------------|-------|-------------------|---------|-----------------|-------------|------------|-------|
| | | | | | East | North | |
| S1 | 0.020 | Black | ESB | 100° | 529782.061 | 726105.5 | 3.559 |
| M1 | 0.100 | Black rubber | - | 100° | 529782.112 | 726105.334 | 3.698 |
| M2 | 0.100 | Black rubber | - | 100° | 529782.016 | 726105.736 | 3.676 |

| Surface from/to (m) | Surface type | |
|---------------------|--------------|------------|
| 0.00 | 2.50 | TARMACADAM |

| Sample depth (m) | Sample type |
|------------------|-------------|
| 0.5 | B |

| From (m) | To (m) | Description |
|----------|--------|--|
| 0.00 | 0.05 | TARMACADAM. |
| 0.05 | 0.15 | MADE GROUND: Dark grey slightly sandy angular to subrounded fine to coarse Gravel. |
| 0.15 | 0.80 | MADE GROUND: Grey sandy gravelly Clay. |

| Groundwater | Y/N | Depth | Notes |
|-------------|-----|-------|-------|
| | N | | |



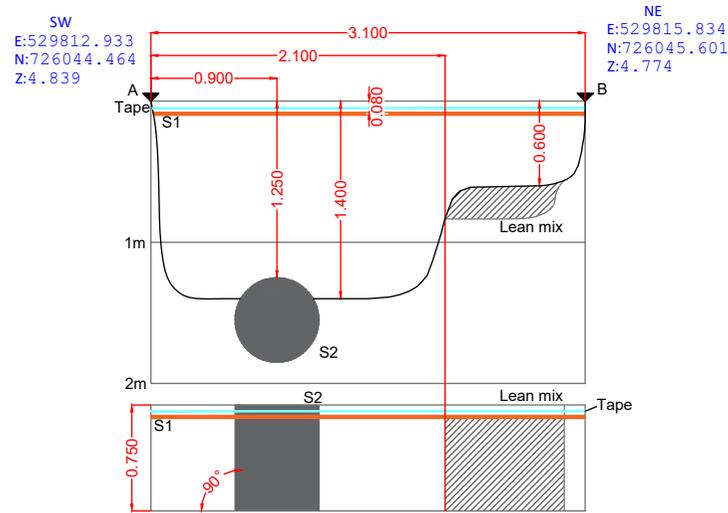
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| | |
|--------------|--------------------------------|
| PROJECT: | 13614-02-24 - Dyke Road Galway |
| DRAWING No.: | ST-02 |
| DATE: | 11/04/2024 |
| CLIENT: | Aecom |
| SCALE: | NTS |

| Version: | Date: | Drawn By: | Checked By: |
|----------|------------|-----------|-------------|
| 1 | 23/04/2024 | J.S. | L.B. |

ST-03



| Service No | ø (m) | Colour - Material | Utility | Angle to trench | Coordinates | | Level |
|------------|--------|-------------------|-------------|-----------------|-------------|------------|-------|
| | | | | | East | North | |
| S1 | 0.020 | Orange - Plastic | Fibre optic | 0° | 529813.155 | 726044.790 | 4.710 |
| S1 | 0.600? | Concrete | Storm | 90° | 529813.831 | 726044.651 | 3.539 |

| Surface from/to (m) | Surface type |
|---------------------|--------------|
| 0.00 | 3.10 |
| | TARMACADAM |

| Sample depth (m) | Sample type |
|------------------|-------------|
| 0.5 | B |

| From (m) | To (m) | Description |
|----------|--------|--|
| 0.00 | 0.07 | TARMACADAM. |
| 0.07 | 1.40 | MADE GROUND: Grey clayey angular to subrounded fine to coarse Sand and Gravel. |

| Groundwater | Y/N | Depth | Notes |
|-------------|-----|-------|-------|
| | N | | |



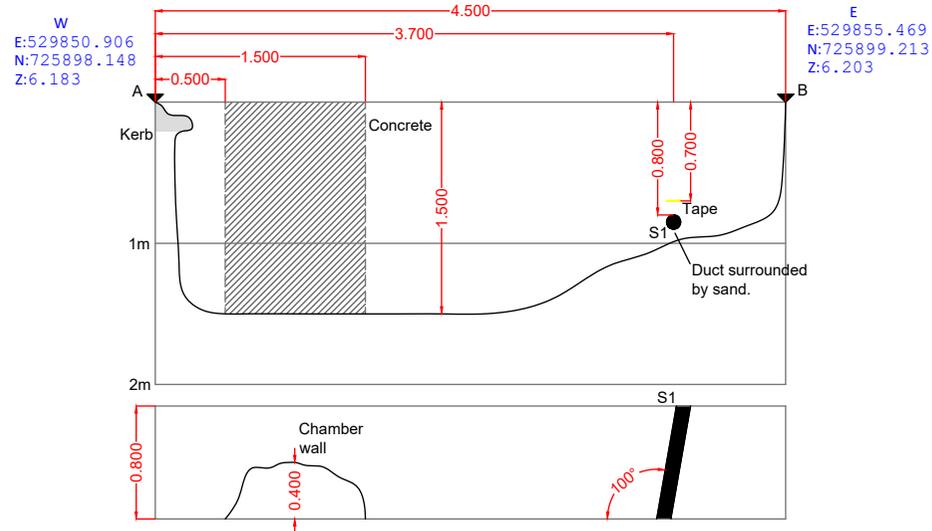
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| | |
|--------------|--------------------------------|
| PROJECT: | 13614-02-24 - Dyke Road Galway |
| DRAWING No.: | ST-03 |
| DATE: | 12/04/2024 |
| CLIENT: | Aecom |
| SCALE: | NTS |

| Version: | Date: | Drawn By: | Checked By: |
|----------|------------|-----------|-------------|
| 1 | 23/04/2024 | J.S. | L.B. |

ST-04



| Service No | ø (m) | Colour - Material | Utility | Angle to trench | Coordinates | | Level |
|------------|-------|-------------------|---------|-----------------|-------------|------------|-------|
| | | | | | East | North | |
| S1 | 0.100 | Black - Duct | ESB | 100° | 529854.585 | 725898.986 | 5.482 |

| Surface from/to (m) | Surface type |
|---------------------|--------------|
| 0.00 | 4.50 |
| | TARMACADAM |

| Sample depth (m) | Sample type |
|------------------|-------------|
| 1.0 | B |

| From (m) | To (m) | Description |
|----------|--------|---|
| 0.00 | 0.08 | TARMACADAM. |
| 0.08 | 0.30 | MADE GROUND: Grey sandy angular to subrounded fine to coarse Gravel. |
| 0.30 | 0.50 | MADE GROUND: Brown clayey gravelly fine to coarse Sand. |
| 0.50 | 1.50 | MADE GROUND: Grey sandy angular to subrounded fine to coarse Gravel with some plastic fragments and plastic bags. |

| Groundwater | Y/N | Depth | Notes |
|-------------|-----|-------|-------|
| | N | | |



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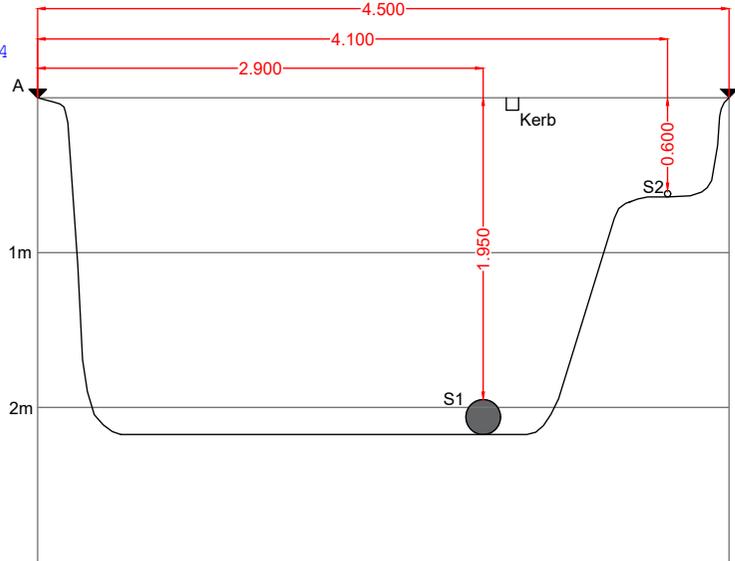
| | |
|--------------|--------------------------------|
| PROJECT: | 13614-02-24 - Dyke Road Galway |
| DRAWING No.: | ST-04 |
| DATE: | 17/04/2024 |
| CLIENT: | Aecom |
| SCALE: | NTS |

| Version: | Date: | Drawn By: | Checked By: |
|----------|------------|-----------|-------------|
| 1 | 23/06/2024 | S.F. | L.B. |

ST-05

E
E:529830.397
N:725982.844
Z:5.685

W
E:529825.979
N:725981.532
Z:5.701



| Service No | ø (m) | Colour - Material | Utility | Angle to trench | Coordinates | | Level |
|------------|-------|-------------------|---------|-----------------|-------------|------------|-------|
| | | | | | East | North | |
| S1 | 0.225 | Grey | Foul | 100° | 529827.542 | 725981.833 | 3.884 |
| S2 | 0.040 | Steel crushed | - | 85° | 529826.391 | 725981.661 | 4.869 |

| From (m) | To (m) | Description |
|----------|--------|-----------------------|
| 0.00 | 0.10 | MADE GROUND: Concrete |
| 0.10 | 0.20 | Tarmacadam |
| 0.20 | 0.90 | MADE GROUND: 804 |
| 0.90 | 0.91 | MADE GROUND: Membrane |
| 0.20 | 0.90 | Black fibrous PEAT. |

| Surface from/to (m) | | Surface type |
|---------------------|------|--------------|
| 0.00 | 3.05 | Concrete |
| 3.05 | 3.13 | Kerb |
| 3.15 | 4.50 | Tarmacadam |

| Sample depth (m) | Sample type |
|------------------|-------------|
| | |
| | |

| Groundwater | Y/N | Depth | Notes |
|-------------|-----|-------|-------|
| ? | Y | 1.50 | |



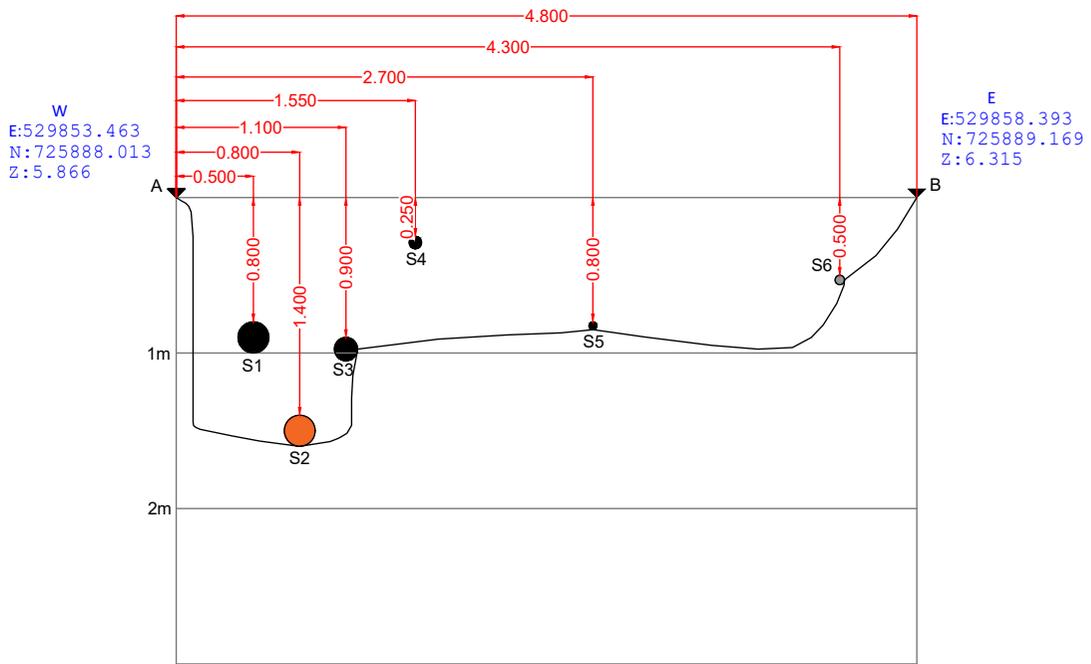
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| | |
|--------------|--------------------------------|
| PROJECT: | 13614-02-24 - Dyke Road Galway |
| DRAWING No.: | ST-05 |
| DATE: | 13/06/2024 |
| CLIENT: | Aecom |
| SCALE: | NTS |

| Version: | Date: | Drawn By: | Checked By: |
|----------|------------|-----------|-------------|
| 1 | 23/04/2024 | S.F | ? |

ST-06



| Service No | ø (m) | Colour - Material | Utility | Angle to trench | Coordinates | | Level |
|------------|-------|-------------------|---------|-----------------|-------------|------------|-------|
| | | | | | East | North | |
| S1 | 0.200 | Black PVC | | 90° | 529853.931 | 725888.107 | 5.167 |
| S2 | 0.200 | Orange PVC | Foul? | 90° | 529854.062 | 725888.283 | 4.477 |
| S3 | 0.150 | Black PVC | | 90° | 529854.473 | 725888.281 | 4.929 |
| S4 | 0.080 | Black Duct | ESB? | 90° | 529854.87 | 725888.265 | 5.760 |
| S5 | 0.050 | Black PVC | | 90° | 529856.04 | 725888.724 | 5.411 |
| S6 | 0.060 | Steel bent | ? | 90° | 529857.489 | 725889.112 | 5.844 |

| From (m) | To (m) | Description |
|----------|--------|--|
| 0.00 | 0.30 | MADE GROUND: Tarmacadam |
| 0.30 | 0.60 | MADE GROUND: 804 |
| 0.60 | 0.61 | MADE GROUND: Membrane |
| 0.61 | 1.5 | MADE GROUND: Grey slightly clayey slightly sandy subangular to subrounded fine to coarse Gravel. |

| Surface from/to (m) | | Surface type |
|---------------------|------|----------------------|
| 0.00 | 2.70 | TARMACADAM Foot path |
| 2.70 | 4.80 | TARMACADAM Car park |

| Groundwater | Y/N | Depth | Notes |
|-------------|-----|-------|-------|
| Slow | Y | 0.65 | |



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| | |
|--------------|--------------------------------|
| PROJECT: | 13614-02-24 - Dyke Road Galway |
| DRAWING No.: | ST-01 |
| DATE: | 13/06/2024 |
| CLIENT: | Aecom |
| SCALE: | NTS |

| Version: | Date: | Drawn By: | Checked By: |
|----------|------------|-----------|-------------|
| 1 | 16/06/2024 | S.F. | ? |

Dyke Road Galway – Slit Trench Photographs

ST06



ST06



Dyke Road Galway – Slit Trench Photographs

ST06



APPENDIX 4 – Soakaway Records





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 D22 YD52.

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IT01

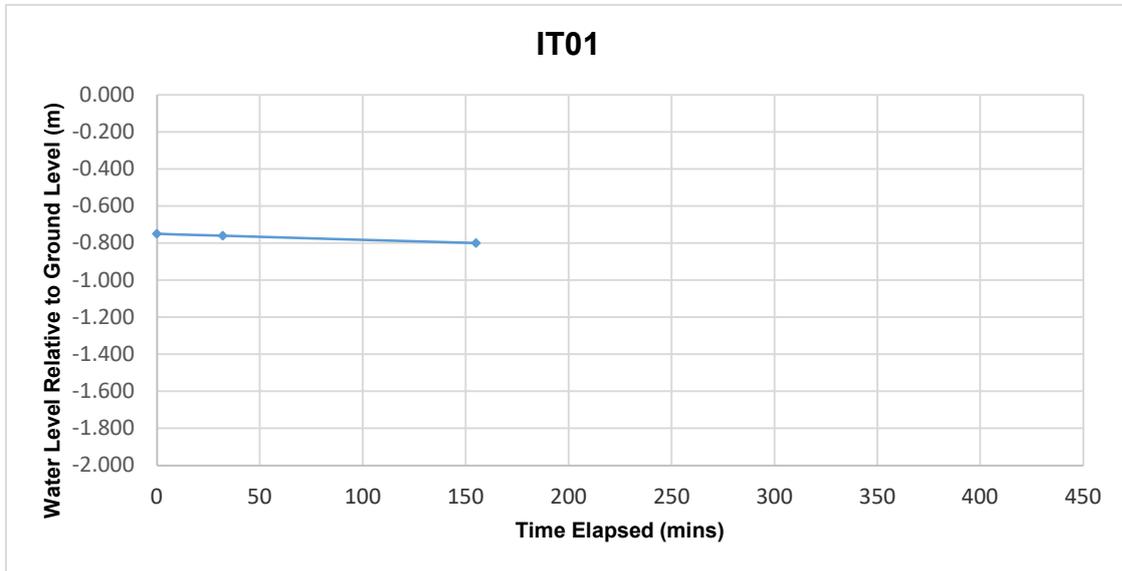
Soakaway Test to BRE Digest 365

Trial Pit Dimensions: 1.60m x 0.60m x 0.85m (L x W x D)

| Date | Time | Water level (m bgl) |
|------------|--------------------------|---------------------|
| 15/04/2024 | Groundwater at 0.75m BGL | |
| 15/04/2024 | 0 | -0.750 |
| 15/04/2024 | 32 | -0.760 |
| 15/04/2024 | 155 | -0.800 |
| | | |
| | | |
| | | |

***Soakaway failed - Pit backfilled**

| Start depth | Depth of Pit | Diff | 75% full | 25%full |
|-------------|--------------|-------|----------|---------|
| 0.75 | 0.850 | 0.100 | 0.775 | 0.825 |





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IT02

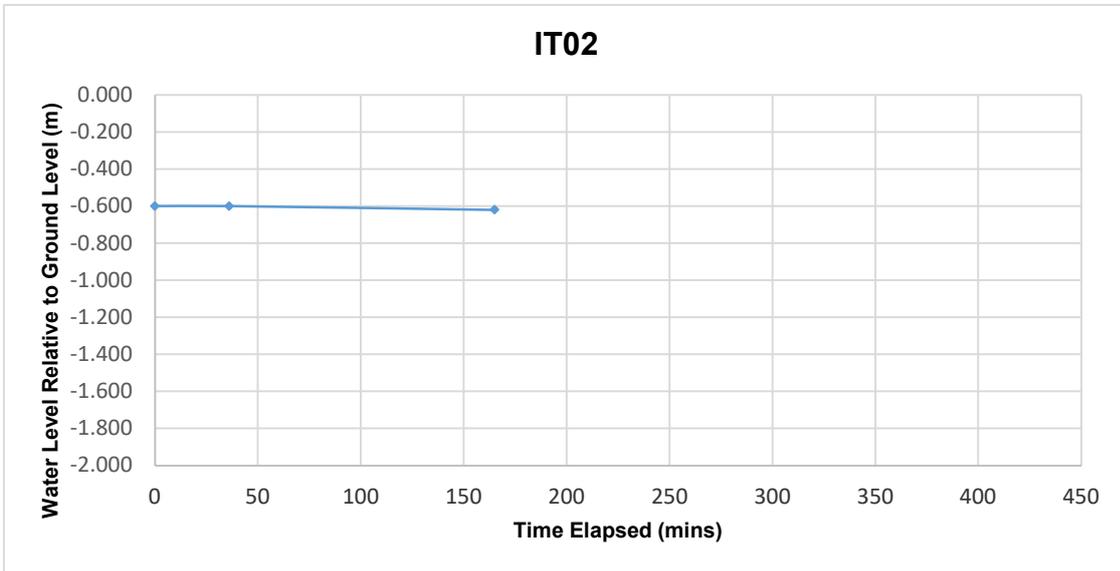
Soakaway Test to BRE Digest 365

Trial Pit Dimensions: 1.70m x 0.75m x 1.60m (L x W x D)

| Date | Time | Water level (m bgl) |
|------------|------|---------------------|
| 15/04/2024 | 0 | -0.600 |
| 15/04/2024 | 36 | -0.600 |
| 15/04/2024 | 165 | -0.620 |
| | | |
| | | |
| | | |
| | | |

***Soakaway failed - Pit backfilled**

| Start depth | Depth of Pit | Diff | 75% full | 25%full |
|-------------|--------------|-------|----------|---------|
| 0.60 | 1.600 | 1.000 | 0.85 | 1.35 |



APPENDIX 5 – Borehole Records





| | | | | |
|--|---|---|------------------------|----------------------------------|
| Machine : Dando 2000 & Beretta T-44 | Casing Diameter 200mm cased to 10.50m 96mm cased to 16.30m | Ground Level (mOD) 4.73 | Client Aecom | Job Number 13614-02-24 |
| Method : Cable Percussion with Rotary follow on | Location 529797.4 E 726065.9 N | Dates 26/04/2024- 22/05/2024 | Engineer | Sheet 1/2 |

| Depth (m) | Sample / Tests | Casing Depth (m) | Water Depth (m) | Field Records | Level (mOD) | Depth (m) (Thickness) | Description | Legend | Water |
|-------------|-----------------|------------------|-----------------|---|-------------|-----------------------|---|--------|-------|
| 0.25 | ES | | | | 4.67 | 0.06 | TARMACADAM | | |
| 0.50 | ES | | | | 4.23 | 0.50 | MADE GROUND: Grey slightly clayey sandy angular to subrounded fine to coarse Gravel | | |
| 1.00-1.45 | SPT(C) N=2 B | | | 2,2/1,0,0,1 | | (1.00) | MADE GROUND: Brownish black slightly clayey slightly sandy gravelly Peat with occasional red brick and ceramic fragments. Gravel is subangular to subrounded fine to coarse | | |
| 1.50 | ES | | | | 3.23 | 1.50 | MADE GROUND: Brownish black clayey slightly sandy slightly gravelly Peat with occasional ceramic and red brick fragments and rubbish. Gravel is subangular to subrounded fine to coarse | | |
| 2.00-2.45 | SPT(C) N=0 B | | | 1,0/0,0,0,0 | | (1.00) | | | |
| 2.50 | B | | | | 2.23 | 2.50 | Very soft beige clayey SILT and brown CLAY with frequent shell fragments | | |
| 3.00-3.45 | UT 100 | | | | | (1.00) | | | |
| 3.50 | B | | | | 1.23 | 3.50 | Very soft beige clayey SILT with frequent shell fragments | | |
| 4.00-4.45 | SPT(C) N=0 | | | 0,0/0,0,0,0 | | | | | |
| 4.50 | B | | | | | (2.20) | | | |
| 5.00-5.45 | UT 0 | | | | | | | | |
| 5.70 | B | | | | -0.97 | 5.70 | Very soft dark grey silty CLAY with occasional shell fragments | | |
| 6.00-6.45 | SPT(C) N=2 | | | 1,1/1,0,1,0 | | | | | |
| 6.70 | B | | | | | | | | |
| 7.50-7.95 | SPT(C) N=1 B | | | 0,1/0,0,0,1 | | (3.80) | | | |
| 8.70 | B | | | | | | | | |
| 9.00-9.45 | SPT(C) N=2 | | | 1,0/0,1,1,0 | | | | | |
| 9.50 | B | | | Water strike(1) at 9.50m, rose to 4.00m in 20 mins. 0,0/0,1,0,0 | -4.77 | 9.50 | Grey slightly sandy slightly gravelly silty CLAY. Gravel is subangular to subrounded fine to coarse | | |
| 10.00-10.45 | SPT(C) N=1 | | | | | | | | |

| | | |
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| Remarks Cable percussion drilling completed at 10.50m BGL, Rotary drilling completed at 16.30m BGL. Groundwater encountered at 9.50m BGL Borehole backfilled upon completion | Scale (approx) | Logged By |
| | 1:50 | AM |
| | Figure No. 13614-02-24.BH01 | |



| | | | | |
|--|---|---|------------------------|----------------------------------|
| Machine : Dando 2000 & Beretta T-44 Method : Cable Percussion with Rotary follow on | Casing Diameter 200mm cased to 10.50m 96mm cased to 16.30m | Ground Level (mOD) 4.73 | Client Aecom | Job Number 13614-02-24 |
| | Location 529797.4 E 726065.9 N | Dates 26/04/2024- 22/05/2024 | Engineer | Sheet 2/2 |

| Depth (m) | Sample / Tests | | Casing Depth (m) | Water Depth (m) | Field Records | Level (mOD) | Depth (m) (Thickness) | Description | Legend | Water | |
|----------------------|----------------|-----|------------------|-----------------|------------------------|-------------|-----------------------|--|--|-------|-----|
| | TCR | SCR | | | | | | | | | RQD |
| 10.50 | 74 | | | | 1,10/50 SPT(C) 50/5 | -5.77 | 10.50 | Very dense grey subangular to subrounded fine to coarse GRAVEL with low cobble content | | | |
| 11.00-11.16 11.00 | | | | | | | (0.70) | | | | |
| 11.20 | 100 | 87 | 61 | | | | -6.47 | 11.20 | Strong thinly to medium bedded grey finely to medium grained fossiliferous LIMESTONE. Fresh (11.20 - 16.30m BGL) 1 fracture set. 10-30 degrees, closely to medium spaced, undulating, rough with occasional Clay staining | | |
| 12.50 | 100 | 88 | 72 | | | 6 | | (5.10) | | | |
| 14.00 | 100 | 87 | 69 | | | | | | | | |
| 15.50 | 100 | 100 | 84 | | | | | | | | |
| 16.30 | | | | | | -11.57 | 16.30 | Complete at 16.30m | | | |

| | | |
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| Remarks | Scale (approx) | Logged By |
| | 1:50 | AM |
| | Figure No. 13614-02-24.BH01 | |



| | | | | |
|----------------------------------|--|---|------------------------|----------------------------------|
| Machine : Dando 2000 | Casing Diameter 200mm cased to 9.70m | Ground Level (mOD) 5.08 | Client Aecom | Job Number 13614-02-24 |
| Method : Cable Percussion | Location 529838.9 E 726012.6 N | Dates 10/04/2024- 11/04/2024 | Engineer | Sheet 1/1 |

| Depth (m) | Sample / Tests | Casing Depth (m) | Water Depth (m) | Field Records | Level (mOD) | Depth (m) (Thickness) | Description | Legend | Water |
|-----------|----------------|------------------|-----------------|---|-------------|-----------------------|---|--------|-------|
| 0.50 | ES | | | | 5.03 | 0.05 | TARMACADAM | | |
| 1.00 | B | | | | 4.58 | 0.50 | Light grey slightly sandy subangular to subrounded fine to coarse gravel FILL | | |
| 1.00 | ES | | | | | (1.00) | Brownish black mottled light brown slightly clayey slightly sandy slightly gravelly PEAT. Gravel is subangular to subrounded fine to coarse | | |
| 1.00-1.45 | UT 60% | | | 2 blows | | | | | |
| 1.50 | B | | | | 3.58 | 1.50 | Very soft greyish light brown slightly sandy clayey SILT | | |
| 2.00-2.45 | SPT(C) N=1 | | | 0,0/0,0,1,0 | | (1.50) | | | |
| 2.50 | B | | | | | | | | |
| 3.00 | B | | | | 2.08 | 3.00 | Grey slightly sandy slightly gravelly clayey SILT. Gravel is subangular to subrounded fine to coarse | | |
| 3.00-3.45 | UT 100% | | | 3 blows | | (0.60) | | | |
| 3.60 | B | | | | 1.48 | 3.60 | Soft to firm grey slightly sandy slightly gravelly clayey SILT with occasional cobbles. Gravel is subangular to subrounded fine to coarse | | |
| 4.00-4.45 | SPT(C) N=7 | | | 0,1/1,0,2,4 | | | | | |
| 4.60 | B | | | | | (2.40) | | | |
| 5.00-5.45 | SPT(C) N=12 | | | 2,2/1,3,2,6 | | | | | |
| 5.60 | B | | | | | | | | |
| 6.00-6.45 | SPT(C) N=27 | | | 9,6/5,6,8,8 | -0.92 | 6.00 | Stiff to very stiff grey slightly sandy slightly gravelly CLAY with occasional cobbles. Gravel is subangular to subrounded fine to coarse | | |
| 6.00 | B | | | | | | | | |
| 7.00 | B | | | | | | | | |
| 7.50-7.78 | SPT(C) 50/125 | | | 9,15/37,13 | | (3.40) | | | |
| 8.00 | B | | | | | | | | |
| 9.00-9.45 | SPT(C) N=32 | | | 14,15/13,3,2,14 | | | | | |
| 9.00 | B | | | | | | | | |
| 9.40 | B | | | Water strike(1) at 9.50m, rose to 2.80m in 20 mins. | -4.32 | 9.40 | Grey slightly clayey very sandy subangular to subrounded fine to coarse GRAVEL with occasional cobbles | | |
| | | | | | -4.62 | 9.70 | Complete at 9.70m | | |

| | | |
|--|---------------------------------------|------------------|
| Remarks Cable percussion drilling refused at 9.70m BGL Groundwater encountered at 9.50m BGL Borehole backfilled upon completion Chiselling from 7.50m to 8.00m for 0.5 hours. | Scale (approx) | Logged By |
| | 1:50 | AM |
| | Figure No. 13614-02-24.BH02 | |



| | | | | |
|----------------------------------|--|---|------------------------|----------------------------------|
| Machine : Dando 2000 | Casing Diameter 200mm cased to 9.10m | Ground Level (mOD) 5.21 | Client Aecom | Job Number 13614-02-24 |
| Method : Cable Percussion | Location 529849.5 E 725985.6 N | Dates 12/04/2024- 15/04/2024 | Engineer | Sheet 1/1 |

| Depth (m) | Sample / Tests | Casing Depth (m) | Water Depth (m) | Field Records | Level (mOD) | Depth (m) (Thickness) | Description | Legend | Water |
|-----------|-----------------------|------------------|-----------------|---------------|-------------|-----------------------|---|--------|-------|
| 1.00 | B | | | | 5.15 | 0.06 | TARMACADAM | | |
| 1.00-1.45 | UT 100 | | | 2 blows | 4.21 | 1.00 (0.94) | Grey sandy angular to subrounded fine to coarse gravel FILL | | |
| 2.00-2.45 | SPT(C) N=1 | | | 0,0/0,0,1 | 3.21 | 2.00 (1.00) | Greyish brown and beige slightly sandy slightly gravelly clayey SILT with pockets of brownish black Peat. Gravel is subangular to subrounded fine to coarse | | |
| 2.00 | B | | | | 2.21 | 3.00 (0.40) | Very soft greyish beige clayey SILT with occasional shell fragments | | |
| 3.00-3.45 | UT 100 | | | 3 blows | 1.81 | 3.40 (1.60) | Brownish grey peaty silty CLAY with frequent organics | | |
| 3.40 | B | | | | 0.21 | 5.00 (1.00) | Very soft to soft light grey slightly sandy slightly gravelly clayey SILT with occasional cobbles. Gravel is subangular to subrounded fine to coarse | | |
| 4.00-4.45 | SPT(C) N=4 | | | 0,1/0,0,1,3 | -0.79 | 6.00 (2.00) | Firm light grey slightly sandy slightly gravelly clayey SILT with occasional cobbles. Gravel is subangular to subrounded fine to coarse | | |
| 4.40 | B | | | | -2.79 | 8.00 (1.10) | Stiff to very stiff light grey slightly sandy slightly gravelly CLAY with occasional cobbles. Gravel is subangular to subrounded fine to coarse | | |
| 5.00-5.45 | SPT(C) N=12 | | | 2,5/4,4,3,1 | -3.89 | 9.10 | Very stiff light grey sandy gravelly CLAY with occasional cobbles. Gravel is subangular to subrounded fine to coarse | | |
| 5.40 | B | | | | | | Complete at 9.10m | | |
| 6.00-6.45 | SPT(C) N=32 | | | 3,5/9,7,8,8 | | | | | |
| 6.00 | B | | | | | | | | |
| 7.00 | B | | | | | | | | |
| 7.50-7.95 | SPT(C) N=41 | | | 2,8/10,8,9,14 | | | | | |
| 8.00 | B | | | | | | | | |
| 9.00-9.08 | SPT(C) 25*/75 50/0 | | | 25/50 | | | | | |

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|---|---------------------------------------|------------------|
| Remarks Cable percussion drilling refused at 9.10m BGL No groundwater encountered during drilling Borehole backfilled upon completion Chiselling from 9.10m to 9.10m for 1 hour. | Scale (approx) | Logged By |
| | 1:50 | AM |
| | Figure No. 13614-02-24.BH03 | |



| | | | | |
|----------------------------------|--|-----------------------------------|------------------------|----------------------------------|
| Machine : Dando 2000 | Casing Diameter 200mm cased to 2.80m | Ground Level (mOD) 6.09 | Client Aecom | Job Number 13614-02-24 |
| Method : Cable Percussion | Location 529867.4 E 725905.2 N | Dates 19/04/2024 | Engineer | Sheet 1/1 |

| Depth (m) | Sample / Tests | Casing Depth (m) | Water Depth (m) | Field Records | Level (mOD) | Depth (m) (Thickness) | Description | Legend | Water |
|-----------|----------------|------------------|-----------------|----------------|-------------|-----------------------|---|--------|-------|
| 0.30 | B | | | | 6.03 | 0.06 (0.24) | TARMACADAM | | |
| 0.50 | ES | | | | 5.79 | 0.30 (0.50) | Grey Sand and Gravel FILL. Gravel is subangular to subrounded fine to coarse | | |
| 0.80 | B | | | | 5.29 | 0.80 | Brownish black slightly gravelly PEAT. Gravel is subangular to subrounded fine to coarse | | |
| 1.00-1.45 | SPT(C) N=38 | | | 14,11/18,8,5,7 | | (1.00) | Very stiff greenish grey mottled brown sandy gravelly CLAY with occasional cobbles. Possible residual soil. Gravel is subangular to subrounded fine to coarse | | |
| 1.50 | ES | | | | | | | | |
| 1.80 | B | | | | 4.29 | 1.80 | Dense greenish grey sandy angular to subrounded fine to coarse GRAVEL. Possible weathered rock | | |
| 2.30-2.68 | SPT(C) 50/230 | | | 5,7/9,10,18,13 | | (1.00) | | | |
| | | | | | 3.29 | 2.80 | Complete at 2.80m | | |

| | | |
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| Remarks Cable percussion drilling refused at 2.80m BGL No groundwater encountered during drilling Borehole backfilled upon completion Chiselling from 0.80m to 1.00m for 0.25 hours. Chiselling from 1.00m to 2.30m for 1 hour. Chiselling from 2.80m to 2.80m for 1 hour. | Scale (approx) | Logged By |
| | 1:50 | AM |
| Figure No. 13614-02-24.BH04 | | |



| | | | | |
|---------------------------|---|----------------------------|-----------------|---------------------------|
| Machine : Dando 2000 | Casing Diameter 200mm cased to 4.50m | Ground Level (mOD) 7.05 | Client Aecom | Job Number 13614-02-24 |
| Method : Cable Percussion | Location 529888.8 E 725839.2 N | Dates 25/04/2024 | Engineer | Sheet 1/1 |

| Depth (m) | Sample / Tests | Casing Depth (m) | Water Depth (m) | Field Records | Level (mOD) | Depth (m) (Thickness) | Description | Legend | Water |
|-------------------|--------------------|------------------|-----------------|---------------------------|-------------|-----------------------|---|--------|-------|
| 0.25 | ES | | | | 6.99 | 0.06 (0.34) | TARMACADAM | | |
| 0.50 | ES | | | | 6.65 | 0.40 | Grey Sand and Gravel FILL. Gravel is subangular to subrounded fine to coarse | | |
| 1.00-1.30 1.00 | SPT(C) 50/145 B | | | 14,11/24,26 | | (1.60) | MADE GROUND: Greyish light brown slightly sandy slightly gravelly CLAY with occasional cobbles. Gravel is subangular to subrounded fine to coarse | | |
| 1.50 | ES | | | | | | | | |
| 2.00-2.45 2.00 | SPT(C) N=7 B | | | 10,8/3,2,1,1 | 5.05 | 2.00 (0.40) | Soft greyish light brown slightly sandy slightly gravelly CLAY with occasional cobbles. Gravel is subangular to subrounded fine to coarse | | |
| 2.40 | B | | | | 4.65 | 2.40 (0.60) | Soft brownish black slightly clayey slightly gravelly PEAT. Gravel is subangular to subrounded fine to coarse | | |
| 3.00-3.45 | SPT(C) N=43 | | | 7,10/6,5,15,17 | 4.05 | 3.00 (0.50) | Brownish black slightly clayey slightly gravelly PEAT. Gravel is subangular to subrounded fine to coarse | | |
| 3.50 | B | | | Water strike(1) at 3.50m. | 3.55 | 3.50 | Dense bluish grey very sandy angular to subrounded fine to coarse GRAVEL | | ✓ |
| 4.00-4.24 4.00 | SPT(C) 50/85 B | | | 17,8/31,19 | | (1.00) | | | |
| | | | | | 2.55 | 4.50 | Terminated on possible bedrock or large boulder | | |
| | | | | | | | Complete at 4.50m | | |

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|--|---------------------------------------|-----------|
| Remarks Cable percussion drilling refused at 4.50m BGL Groundwater encountered at 3.50m BGL Borehole backfilled upon completion Chiselling from 1.00m to 2.00m for 1 hour. Chiselling from 4.50m to 4.50m for 1 hour. | Scale (approx) | Logged By |
| | 1:50 | AM |
| | Figure No. 13614-02-24.BH05 | |



| | | | | |
|---------------------------|---|----------------------------|-----------------|---------------------------|
| Machine : Dando 2000 | Casing Diameter 200mm cased to 5.70m | Ground Level (mOD) 6.89 | Client Aecom | Job Number 13614-02-24 |
| Method : Cable Percussion | Location 529907.4 E 725874.7 N | Dates 24/04/2024 | Engineer | Sheet 1/1 |

| Depth (m) | Sample / Tests | Casing Depth (m) | Water Depth (m) | Field Records | Level (mOD) | Depth (m) (Thickness) | Description | Legend | Water |
|-----------|------------------------|------------------|-----------------|--|-------------|-----------------------|---|--------|-------|
| | | | | | 6.79 | 0.10 | TARMACADAM | | |
| | | | | | | (0.50) | Grey sandy angular to subrounded fine to coarse Gravel FILL | | |
| | | | | | 6.29 | 0.60 | MADE GROUND: Grey slightly sandy gravelly Clay with occasional cobbles. Gravel is subangular to subrounded fine to coarse | | |
| 1.00-1.09 | SPT(C) 25*/75 50/15 | | | 25/50 | | (1.20) | | | |
| 1.00 | B | | | | | | | | |
| 1.80 | B | | | | 5.09 | 1.80 | MADE GROUND: Greyish brown slightly sandy slightly gravelly Clay with occasional cobbles and occasional red brick and charcoal fragments. Gravel is subangular to subrounded fine to coarse | | |
| 2.00-2.45 | SPT(C) N=9 | | | 17,8/2,3,2,2 | | (0.70) | | | |
| 2.50 | B | | | | 4.39 | 2.50 | MADE GROUND: Brownish black slightly gravelly Peat with rare red brick fragments. Gravel is subangular to subrounded fine to coarse | | |
| 2.70 | B | | | | 4.19 | (0.20) 2.70 | | | |
| 3.00-3.45 | UT 100 | | | 1 blows | | (0.70) | Brownish black PEAT | | |
| 3.40 | B | | | | 3.49 | 3.40 | Stiff to very stiff light grey slightly sandy slightly gravelly silty CLAY with occasional cobbles. Gravel is subangular to subrounded fine to coarse | | |
| 4.00-4.45 | SPT(C) N=32 | | | Water strike(1) at 3.50m, rose to 3.00m in 20 mins. 4,6/8,7,8,9 | | (1.10) | | | |
| 4.50 | B | | | | 2.39 | 4.50 | Very stiff light grey sandy slightly gravelly silty CLAY with occasional cobbles. Gravel is subangular to subrounded fine to coarse | | |
| 5.00-5.25 | SPT(C) 50/95 | | | 16,9/33,17 | | (1.20) | | | |
| 5.70-5.70 | SPT(C) 25*/0 50/0 | | | 25/50 | 1.19 | 5.70 | Complete at 5.70m | | |

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| Remarks Cable percussion drilling refused at 5.70m BGL Groundwater encountered at 3.50m BGL Borehole backfilled upon completion Chiselling from 1.00m to 1.80m for 1 hour. Chiselling from 5.00m to 5.70m for 0.5 hours. Chiselling from 5.70m to 5.70m for 1 hour. | Scale (approx) | Logged By |
| | 1:50 | AM |
| | Figure No. 13614-02-24.BH06 | |



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|--|---|---|------------------------|----------------------------------|
| Machine : Dando 2000 & Beretta T-44 Method : Cable Percussion with Rotary follow on | Casing Diameter 200mm cased to 10.50m 96mm cased to 16.50m | Ground Level (mOD) 4.15 | Client Aecom | Job Number 13614-02-24 |
| | Location 529798.3 E 726121.2 N | Dates 25/04/2024- 21/05/2024 | Engineer | Sheet 1/2 |

| Depth (m) | Sample / Tests | Casing Depth (m) | Water Depth (m) | Field Records | Level (mOD) | Depth (m) (Thickness) | Description | Legend | Water | Instr |
|-------------|-----------------|------------------|-----------------|---|-------------|-----------------------|---|--------|-------|-------|
| 0.25 | ES | | | | 4.09 | 0.06 | TARMACADAM | | | |
| 0.50 | ES | | | | | (0.64) | Grey slightly clayey Sand and Gravel FILL with occasional tarmacadam fragments. Gravel is subangular to subrounded fine to coarse | | | |
| 1.00-1.45 | B UT 60 | | | 6 blows | 3.45 | 0.70 | Very soft brownish black slightly clayey slightly gravelly PEAT. Gravel is subangular to subrounded fine to coarse | | | |
| 1.50 | ES | | | | | (1.80) | | | | |
| 2.00-2.45 | SPT(C) N=0 B | | | 1,0/0,0,0,0 | | | | | | |
| 2.50 | B | | | | 1.65 | 2.50 | Beige clayey SILT and brownish black slightly clayey PEAT with frequent shell fragments | | | |
| 3.00-3.45 | UT 0 | | | 2 blows | | (1.00) | | | | |
| 3.50 | B | | | | 0.65 | 3.50 | Very soft beige clayey SILT with occasional shell fragments | | | |
| 4.00-4.45 | SPT(C) N=1 | | | 0,0/0,1,0,0 | | | | | | |
| 4.50 | B | | | | | (2.00) | | | | |
| 5.30-5.75 | UT 100 | | | 2 blows | | | | | | |
| 5.50 | B | | | | -1.35 | 5.50 | Soft brownish dark grey silty CLAY and beige clayey SILT with occasional shell fragments and occasional pockets of Peat | | | |
| 6.00-6.45 | SPT(C) N=7 | | | 2,1/2,1,2,2 | | | | | | |
| 6.50 | B | | | | | (2.00) | | | | |
| 7.50-7.95 | B UT 100 | | | 2 blows | -3.35 | 7.50 | Very soft greyish brown silty CLAY | | | |
| 8.50 | B | | | | | (1.50) | | | | |
| 9.00 | B | | | Water strike(1) at 9.00m, rose to 8.00m in 20 mins. | -4.85 | 9.00 | Very soft to soft light grey slightly sandy slightly gravelly silty CLAY with occasional cobbles. Gravel is subangular to subrounded fine to coarse | | | |
| 9.00-9.45 | SPT(C) N=0 | | | 1,0/0,0,0,0 | | | | | | |
| 10.00-10.45 | SPT(C) N=4 | | | 1,0/0,1,2,1 | | (1.50) | | | | |

| | | |
|--|--|------------------------|
| Remarks Cable percussion drilling completed at 10.50m BGL, Rotary drilling completed at 16.50m BGL Groundwater encountered at 9.00m BGL 50mm standpipe installed to 10.00m BGL. Slotted standpipe installed from 10.00 - 1.00m BGL. Solid standpipe installed from 1.00m BGL - GL with gas tap and flush cover | Scale (approx) 1:50 | Logged By AM |
| | Figure No. 13614-02-24.BRC01 | |



| | | | | |
|--|---|---|------------------------|----------------------------------|
| Machine : Dando 2000 & Beretta T-44 Method : Cable Percussion with Rotary follow on | Casing Diameter 200mm cased to 10.50m 96mm cased to 16.50m | Ground Level (mOD) 4.15 | Client Aecom | Job Number 13614-02-24 |
| | Location 529798.3 E 726121.2 N | Dates 25/04/2024- 21/05/2024 | Engineer | Sheet 2/2 |

| Depth (m) | Sample / Tests | | Casing Depth (m) | Water Depth (m) | Field Records | Level (mOD) | Depth (m) (Thickness) | Description | Legend | Water | Instr |
|----------------------|----------------|-----|------------------|-----------------|---------------------------|-------------|-----------------------|--|--------|-------|-------|
| | TCR | SCR | | | | | | | | | |
| 10.50 | 64 | | | | 0,0/0,1,4,3 SPT(C) N=8 | -6.35 | 10.50 (0.50) | Poor recovery. Recovery consists of grey peaty clayey SILT onto grey clayey silty subangular to subrounded fine to coarse GRAVEL | | | |
| 11.00-11.45 11.00 | | | | | | -6.85 | 11.00 (0.50) | Poor recovery. Recovery consists of slightly clayey slightly sandy subangular to subrounded fine to coarse GRAVEL. | | | |
| 11.50 | 80 | 50 | 45 | | C | -7.35 | 11.50 (1.50) | Strong thinly to medium bedded grey finely to medium grained fossiliferous LIMESTONE. Slightly weathered (11.50 - 13.00m BGL) 2 fracture sets. FS1: 10-30 degrees, closely to medium spaced, undulating, rough with occasional Clay staining. FS2: 70-80 degrees, widely spaced, undulating, rough with occasional Clay staining | | | |
| 12.50-12.60 12.50 | | | | 9 | | -8.85 | 13.00 (3.50) | Strong thinly to medium bedded grey finely to medium grained fossiliferous LIMESTONE. Fresh (13.00 - 16.50m BGL) 3 fracture sets. FS1: 0-20 degrees, closely to medium spaced, undulating, rough with occasional Clay staining. FS2: 30-40 degrees, medium to widely spaced, undulating, rough. FS3: 70-80 degrees, undulating, rough with occasional Clay staining | | | |
| 13.00 | 100 | 97 | 77 | | | | | | | | |
| 14.00 14.65-14.88 | 100 | 97 | 77 | 7 | C | | | | | | |
| 15.50 16.05-16.15 | 100 | 90 | 70 | | C | | | | | | |
| 16.50 | | | | | | -12.35 | 16.50 | Complete at 16.50m | | | |

| | | |
|---------|---------------------------------|-----------|
| Remarks | Scale (approx) | Logged By |
| | 1:50 | AM |
| | Figure No. 13614-02-24.BRC01 | |



| | | | | |
|--|--|---|------------------------|----------------------------------|
| Machine : Dando 2000 & Beretta T-44 Method : Cable Percussion with Rotary follow on | Casing Diameter 200mm cased to 10.10m 146mm cased to 12.40m 96mm cased to 18.60m | Ground Level (mOD) 5.08 | Client Aecom | Job Number 13614-02-24 |
| Location 529838.8 E 726050.5 N | | Dates 15/04/2024- 15/05/2024 | Engineer | Sheet 1/2 |

| Depth (m) | Sample / Tests | Casing Depth (m) | Water Depth (m) | Field Records | Level (mOD) | Depth (m) (Thickness) | Description | Legend | Water | Instr |
|-----------|--------------------|------------------|-----------------|---|-------------|-----------------------|---|--------|-------|-------|
| 0.25 | ES | | | | 5.02 | 0.06 | TARMACADAM | | | |
| 0.50 | ES | | | | | (0.94) | Grey Sand and Gravel FILL with occasional tarmacadam fragments. Gravel is subangular to subrounded fine to coarse | | | |
| 1.00 | B | | | | 4.08 | 1.00 | Very soft brownish black slightly gravelly PEAT with occasional pockets of beige clayey SILT. Gravel is subangular to subrounded fine to coarse | | | |
| 1.50-2.05 | ES UT 60 | | | 4 blows | | (1.50) | | | | |
| 2.00-2.45 | SPT N=0 B | | | 0,0/0,0,0,0 | | | | | | |
| 2.50 | B | | | | 2.58 | 2.50 | Very soft beige clayey SILT with frequent shell fragments | | | |
| 3.00-3.45 | UT 100 | | | 3 blows | | | | | | |
| 3.50 | B | | | | | (2.20) | | | | |
| 4.00-4.45 | SPT N=0 | | | 1,0/0,0,0,0 | | | | | | |
| 4.70 | B | | | | 0.38 | 4.70 | Brownish dark grey peaty silty CLAY | | | |
| 5.00-5.45 | UT 100 | | | 3 blows | | (1.30) | | | | |
| 5.70 | B | | | | | | | | | |
| 6.00 | B | | | Water strike(1) at 6.00m, rose to 3.00m in 20 mins. | -0.92 | 6.00 | Very stiff light grey slightly sandy very gravelly silty CLAY with low cobble content. Gravel is subangular to subrounded fine to coarse. | | | |
| 6.00-6.45 | SPT N=31 | | | 2,1/1,10,5,15 | | | | | | |
| 7.00 | B | | | | | | | | | |
| 7.50-7.79 | SPT(C) 50/135 | | | 21,4/32,18 | | | | | | |
| 8.00 | B | | | | | (4.10) | | | | |
| 9.00-9.31 | SPT(C) 50/160 B | | | 10,13/20,24,6 | | | | | | |
| 9.70-9.95 | SPT(C) 50/95 | | | 25/31,19 | | | | | | |

| | | |
|---|--|------------------------|
| Remarks Cable percussion drilling completed at 10.10m BGL, Rotary drilling completed at 18.60m BGL Groundwater encountered at 6.00m BGL 50mm standpipe installed to 6.00m BGL. Slotted standpipe installed from 6.00 - 0.50m BGL. Solid standpipe installed from 0.50m BGL - GL with flush cover Chiselling from 7.50m to 8.00m for 0.5 hours. Chiselling from 8.30m to 8.70m for 0.5 hours. Chiselling from 9.50m to 9.70m for 0.5 hours. | Scale (approx) 1:50 | Logged By AM |
| | Figure No. 13614-02-24.BRC02 | |



| | | | | |
|--|--|---|------------------------|----------------------------------|
| Machine : Dando 2000 & Beretta T-44 Method : Cable Percussion with Rotary follow on | Casing Diameter 200mm cased to 10.10m 146mm cased to 12.40m 96mm cased to 18.60m | Ground Level (mOD) 5.08 | Client Aecom | Job Number 13614-02-24 |
| | Location 529838.8 E 726050.5 N | Dates 15/04/2024- 15/05/2024 | Engineer | Sheet 2/2 |

| Depth (m) | Sample / Tests | | Casing Depth (m) | Water Depth (m) | Field Records | Level (mOD) | Depth (m) (Thickness) | Description | Legend | Water | Instr |
|----------------------------|----------------|-----|------------------|-----------------|---------------|-------------|-----------------------|---|--------|-------|-------|
| | TCR | SCR | | | | | | | | | |
| 10.10 | 11 | | | | | -5.02 | 10.10 (0.90) | Poor recovery. Recovery consists of grey slightly sandy slightly gravelly silty CLAY. Gravel is subangular to subrounded fine to coarse. | | | |
| 11.00 | 29 | | | | | -5.92 | 11.00 (1.40) | Poor recovery. Recovery consists of grey slightly sandy slightly gravelly silty CLAY with medium cobble content. Gravel is subangular to subrounded fine to coarse | | | |
| 12.40 | 25 | | | | | -7.32 | 12.40 (1.10) | Grey subangular to subrounded fine to coarse GRAVEL | | | |
| 13.50 | 100 | 100 | 22 | | | -8.42 | 13.50 | Strong thinly to medium bedded grey fine to medium grained fossiliferous LIMESTONE with occasional calcite veins. Fresh | | | |
| 14.00 | 100 | 91 | 50 | 6 | | | (2.70) | (13.50 - 16.20m BGL) 3 fracture sets. FS1: 0-20 degrees, closely to medium spaced, undulating, rough with occasional Clay staining. FS2: 40-60 degrees, widely spaced, undulating, rough with occasional Clay staining. FS3: 80-90 degrees, undulating, rough with occasional Clay staining | | | |
| 15.37-15.50 | | | | | C | | | | | | |
| 15.50 | | | | | | | | | | | |
| 16.20 | 100 | 97 | 86 | | | -11.12 | 16.20 | Strong thinly to medium bedded grey fine to medium grained fossiliferous LIMESTONE with occasional calcite veins. Fresh | | | |
| 17.00 | | | | 3 | | | (2.40) | (16.20 - 18.60m BGL) 1 fracture set. 0-20 degrees, medium to widely spaced, undulating, rough with occasional Clay staining | | | |
| 18.20-18.35 18.37-18.60 | 100 | 100 | 97 | | C C | | | | | | |
| 18.60 | | | | | | -13.52 | 18.60 | Complete at 18.60m | | | |

| | | |
|---------|---------------------------------|-----------|
| Remarks | Scale (approx) | Logged By |
| | 1:50 | AM |
| | Figure No. 13614-02-24.BRC02 | |



| | | | | |
|--|---|---------------------------------------|------------------------|----------------------------------|
| Machine : Dando 2000 & Beretta T-44 Method : Cable Percussion with Rotary follow on | Casing Diameter 200mm cased to 10.30m 96mm cased to 20.10m | Ground Level (mOD) 5.37 | Client Aecom | Job Number 13614-02-24 |
| | Location 529834.4 E 725993.1 N | Dates 17/04/2024-16/05/2024 | Engineer | Sheet 1/3 |

| Depth (m) | Sample / Tests | Casing Depth (m) | Water Depth (m) | Field Records | Level (mOD) | Depth (m) (Thickness) | Description | Legend | Water |
|-------------------|------------------|------------------|-----------------|---------------|-------------|-----------------------|---|--------|-------|
| 0.25 | ES | | | | 5.31 | 0.06 | TARMACADAM | | |
| 0.50 | ES | | | | | (1.94) | Grey very sandy subangular to subrounded fine to coarse Gravel FILL | | |
| 1.50 | ES | | | | | | | | |
| 2.00-2.45 2.00 | SPT(C) N=0 B | | | 2,0/0,0,0,0 | 3.37 | 2.00 | Very soft brownish grey slightly sandy slightly gravelly silty CLAY with occasional cobbles. Gravel is subangular to subrounded fine to coarse | | |
| 3.00 3.00-3.45 | B U4 100 | | | 2 blows | 2.37 | 3.00 | Very soft light grey slightly sandy slightly gravelly clayey SILT with occasional cobbles. Gravel is subangular to subrounded fine to coarse | | |
| 4.00-4.45 4.00 | SPT(C) N=3 B | | | 1,1/2,1,0,0 | | (2.00) | | | |
| 5.00-5.45 5.00 | SPT(C) N=6 B | | | 1,0/0,2,2,2 | 0.37 | 5.00 | Soft to firm light grey slightly sandy slightly gravelly silty CLAY with occasional cobbles. Gravel is subangular to subrounded fine to coarse | | |
| 6.00-6.45 6.00 | SPT(C) N=16 B | | | 1,1/2,4,5,5 | -0.63 | 6.00 | Firm to stiff light grey slightly sandy slightly gravelly silty CLAY with occasional cobbles. Gravel is subangular to subrounded fine to coarse | | |
| 7.50-7.95 7.50 | SPT(C) N=49 B | | | 6,8/9,8,9,23 | -2.13 | 7.50 | Very stiff grey slightly sandy slightly gravelly CLAY with occasional cobbles. Gravel is subangular to subrounded fine to coarse | | |
| 8.50 | B | | | | | (2.80) | | | |
| 9.00-9.23 | SPT(C) 50/80 | | | 15,10/46,4 | | | | | |
| 9.50 | B | | | | | | | | |
| 10.00-10.22 | SPT(C) 50/70 | | | 23,2/50 | | | | | |

| | | |
|--|--|------------------------|
| Remarks Cable percussion drilling refused at 10.30m BGL, Rotary drilling completed at 20.10m BGL No groundwater encountered during drilling Borehole backfilled upon completion Chiselling from 8.00m to 8.30m for 0.25 hours. Chiselling from 9.00m to 9.50m for 0.5 hours. Chiselling from 9.50m to 10.00m for 0.5 hours. | Scale (approx) 1:50 | Logged By AM |
| | Figure No. 13614-02-24.BRC03 | |



| | | | | |
|--|---|---------------------------------------|------------------------|----------------------------------|
| Machine : Dando 2000 & Beretta T-44 Method : Cable Percussion with Rotary follow on | Casing Diameter 200mm cased to 10.30m 96mm cased to 20.10m | Ground Level (mOD) 5.37 | Client Aecom | Job Number 13614-02-24 |
| | Location 529834.4 E 725993.1 N | Dates 17/04/2024-16/05/2024 | Engineer | Sheet 2/3 |

| Depth (m) | Sample / Tests | | Casing Depth (m) | Water Depth (m) | Field Records | Level (mOD) | Depth (m) (Thickness) | Description | Legend | Water |
|-------------|----------------|-----|------------------|-----------------|--------------------------|-------------|-----------------------|---|--------|-------|
| | TCR | SCR | | | | | | | | |
| 10.30 | | | | | | -4.93 | 10.30 | Very stiff to hard brownish grey slightly sandy slightly gravelly CLAY with low cobble content. Gravel is subangular to subrounded fine to coarse | | |
| 11.00-11.08 | 100 | | | | 25/50 SPT(C) 25*/75 50/0 | | | | | |
| 11.00 | | | | | | | | | | |
| 12.50-12.58 | 100 | | | | 25/50 SPT(C) 25*/75 50/0 | (5.00) | | | | |
| 12.50 | | | | | | | | | | |
| 14.00-14.08 | 100 | 12 | 7 | | 25/50 SPT(C) 25*/75 50/0 | | | | | |
| 14.00 | | | | | | | | | | |
| 15.30 | | | | | | -9.93 | 15.30 | | | |
| 15.50 | | | | | | | | | | |
| 16.40-16.50 | 100 | 90 | 70 | 8 | C | | (2.20) | | | |
| 17.17-17.45 | | | | | C | | | | | |
| 17.00 | | | | | | | | | | |
| 17.50 | 100 | 97 | 71 | | | -12.13 | 17.50 | | | |
| 18.50 | | | | 7 | | | | | | |
| 18.50 | 100 | 100 | 84 | | | | (2.60) | | | |
| 19.95-20.05 | | | | | C | | | | | |

| | | |
|---------|---------------------------------|-----------|
| Remarks | Scale (approx) | Logged By |
| | 1:50 | AM |
| | Figure No. 13614-02-24.BRC03 | |



| | | | | |
|---|---|---|------------------------|----------------------------------|
| Machine : Dando 2000 & Beretta T-44 Flush : Water Core Dia : 63.5 mm Method : Cable Percussion with Rotary follow on | Casing Diameter 200mm cased to 10.30m 96mm cased to 20.10m | Ground Level (mOD) 5.37 | Client Aecom | Job Number 13614-02-24 |
| | Location 529834.4 E 725993.1 N | Dates 17/04/2024- 16/05/2024 | Engineer | Sheet 3/3 |

| Depth (m) | TCR (%) | SCR (%) | RQD (%) | FI | Field Records | Level (mOD) | Depth (m) (Thickness) | Description | Legend | Water |
|-----------|---------|---------|---------|----|---------------|-------------|-----------------------|--------------------|--------|-------|
| 20.10 | | | | | | -14.73 | 20.10 | Complete at 20.10m | | |

| | | |
|----------------|--|------------------------|
| Remarks | Scale (approx) 1:50 | Logged By AM |
| | Figure No. 13614-02-24.BRC03 | |



| | | | | |
|---|---|------------------------------------|-----------------|---------------------------|
| Machine : Dando 2000 and Beretta T-44 | Casing Diameter 200mm cased to 6.50m 96mm cased to 12.00m | Ground Level (mOD) 6.94 | Client Aecom | Job Number 13614-02-24 |
| Method : Cable Percussion with Rotary follow on | Location 529871.1 E 725874.2 N | Dates 22/04/2024- 16/05/2024 | Engineer | Sheet 1/2 |

| Depth (m) | Sample / Tests | Casing Depth (m) | Water Depth (m) | Field Records | Level (mOD) | Depth (m) (Thickness) | Description | Legend | Water | Instr |
|-----------|----------------|------------------|-----------------|--|-------------|-----------------------|--|--------|-------|-------|
| 0.25 | ES | | | | 6.88 | 0.06 | TARMACADAM | | | |
| 0.50 | ES | | | | | (0.94) | MADE GROUND: Grey Sand and Gravel. Gravel is angular to subangular fine to coarse | | | |
| 1.00-1.45 | SPT(C) N=15 | | | 5,4/6,4,2,3 | 5.94 | 1.00 | MADE GROUND: Brownish grey sandy slightly gravelly Clay with occasional glass wire and rubbish fragments and occasional cobbles. Gravel is subangular to subrounded fine to coarse | | | |
| 1.50 | ES | | | | | (1.60) | | | | |
| 2.00-2.45 | SPT(C) N=11 | | | 3,2/2,3,3,3 | | | | | | |
| 2.00 | B | | | | 4.34 | 2.60 | | | | |
| 2.60 | B | | | | 4.14 | (0.20) 2.80 | MADE GROUND: Brownish black slightly gravelly Peat. Gravel is subangular to subrounded fine to coarse | | | |
| 2.80 | | | | | | | | | | |
| 3.00-3.45 | UT 100 | | | 14 blows | | (0.60) | MADE GROUND: Beige slightly gravelly clayey SILT with pockets of brownish black Peat and wire fragments. Gravel is subangular to subrounded fine to coarse | | | |
| 3.40 | B | | | | 3.54 | 3.40 | | | | |
| 4.00 | B | | | Water strike(1) at 4.00m, rose to 3.50m in 20 mins. 19,6/5,4,4,4 | 2.94 | 4.00 | Light grey slightly sandy gravelly silty CLAY with occasional cobbles. Gravel is subangular to subrounded fine to coarse. | | | |
| 4.00-4.45 | SPT(C) N=17 | | | | | | Medium dense to dense greenish grey very sandy subangular to subrounded fine to coarse GRAVEL with occasional cobbles | | | |
| 5.00-5.38 | SPT(C) 50/226 | | | 8,12/14,14,21,1 | | (2.50) | | | | |
| 5.00 | B | | | | | | | | | |
| 6.00-6.30 | SPT(C) 50/151 | | | 10,15/19,29,2 | | | | | | |
| 6.40 | TCR | SCR | RQD | FI | | | | | | |
| 6.50 | 100 | | | | 0.44 | 6.50 | Dark green angular to subangular fine to coarse GRAVEL. Possible weathered bedrock | | | |
| 6.60 | | | | | 0.34 | 6.60 | Strong to very strong thinly to thickly banded dark green medium to coarsely crystalline METAGABBRO with occasional calcite veins. Fresh | | | |
| 8.00 | | | | | | | (6.60 - 12.00m BGL) 2 fracture sets. FS1: 10-30 degrees, closely to medium spaced, planar to undulating, rough to striated. FS2: 30-40 degrees, widely spaced, planar to undulating, rough | | | |
| 8.40-8.57 | 100 | 93 | 87 | C | | | | | | |
| 9.50-9.60 | | | | | | (5.40) | | | | |
| 9.50 | | | | 5 | | | | | | |

| | | |
|---|--|-----------|
| Remarks Cable percussion drilling refused at 6.50m BGL on bedrock, Rotary drilling completed at 12.00m BGL Groundwater encountered at 4.00m BGL 50mm standpipe installed to 6.00m BGL. Slotted standpipe installed from 6.00 - 0.50m BGL. Solid standpipe installed from 0.5m BGL - GL with flush cover Chiselling from 4.00m to 4.50m for 0.5 hours. Chiselling from 6.50m to 6.50m for 1 hour. | Scale (approx) | Logged By |
| | 1:50 | AM |
| | Figure No. 13614-02-24.BRC04 | |



| | | | | |
|---|--|---------------------------------------|------------------------|----------------------------------|
| Machine : Dando 2000 and Beretta T-44 Flush : Water Core Dia : 63.5 mm Method : Cable Percussion with Rotary follow on | Casing Diameter 200mm cased to 6.50m 96mm cased to 12.00m | Ground Level (mOD) 6.94 | Client Aecom | Job Number 13614-02-24 |
| | Location 529871.1 E 725874.2 N | Dates 22/04/2024-16/05/2024 | Engineer | Sheet 2/2 |

| Depth (m) | TCR (%) | SCR (%) | RQD (%) | FI | Field Records | Level (mOD) | Depth (m) (Thickness) | Description | Legend | Water | Instr |
|-------------|---------|---------|---------|----|---------------|-------------|-----------------------|--------------------|--------|-------|-------|
| 10.11-10.30 | 100 | 93 | 75 | | C | | | | | | |
| 11.00 | 100 | 100 | 83 | | | | | | | | |
| 12.00 | | | | | | -5.06 | 12.00 | Complete at 12.00m | | | |

| | | |
|--|-----------------------|------------------|
| Remarks | Scale (approx) | Logged By |
| | 1:50 | AM |
| Figure No. 13614-02-24.BRC04 | | |



| | | | | |
|---|---|------------------------------------|-----------------|---------------------------|
| Machine : Dando 2000 & Beretta T-44 | Casing Diameter 200mm cased to 6.10m 96mm cased to 11.00m | Ground Level (mOD) 5.53 | Client Aecom | Job Number 13614-02-24 |
| Method : Cable Percussion with Rotary follow on | Location 529900.1 E 725923.8 N | Dates 18/04/2024- 20/05/2024 | Engineer | Sheet 1/2 |

| Depth (m) | Sample / Tests | Casing Depth (m) | Water Depth (m) | Field Records | Level (mOD) | Depth (m) (Thickness) | Description | Legend | Water | Instr |
|-----------|----------------|------------------|-----------------|--|-------------|-----------------------|--|--|-------|-------|
| 0.50 | ES | | | | 5.47 | 0.06 | TARMACADAM | | | |
| 0.80 | B | | | | 4.73 | 0.80 | MADE GROUND: Grey slightly clayey Sand and Gravel. Gravel is subangular to subrounded fine to coarse | | | |
| 1.00-1.45 | UT 80 | | | 3 blows | | (0.74) | Brownish black slightly clayey slightly gravelly PEAT. Gravel is subangular to subrounded fine to coarse | | | |
| 1.50 | B | | | | 4.03 | 1.50 | Very soft beige clayey SILT with occasional shell fragments | | | |
| 1.50 | ES | | | | | (0.90) | | | | |
| 2.00-2.45 | SPT N=0 | | | 0,0/0,0,0,0 | | | | | | |
| 2.40 | B | | | | 3.13 | 2.40 | Dark brown peaty CLAY | | | |
| 2.60 | B | | | | 2.93 | (0.20) 2.60 | Light grey slightly gravelly clayey SILT with occasional cobbles. Gravel is subangular to subrounded fine to coarse | | | |
| 3.00-3.45 | UT 0 | | | Water strike(1) at 3.00m, rose to 2.90m in 20 mins. 8 blows | | (0.90) | | | | |
| 3.50 | B | | | | 2.03 | 3.50 | Very stiff light grey slightly sandy slightly gravelly silty CLAY with occasional cobbles. Gravel is subangular to subrounded fine to coarse | | | |
| 4.00-4.45 | SPT(C) N=39 | | | 7,8/9,9,11,10 | | | | | | |
| 4.50 | B | | | | | (2.30) | | | | |
| 5.00-5.45 | SPT(C) N=40 | | | Water strike(2) at 5.00m, rose to 3.00m in 20 mins. 6,11/9,14,7,10 | | | | | | |
| 5.80 | B | | | | -0.27 | 5.80 | Dense bluish grey sandy angular to subangular fine to coarse GRAVEL with occasional cobbles. Gravel is subangular to subrounded fine to coarse | | | |
| 5.80-6.09 | SPT(C) 50/135 | | | 8,12/10,40 | | (0.30) | | | | |
| 6.10 | TCR | SCR | RQD | FI | | -0.57 | 6.10 | Strong thinly to medium bedded grey finely to medium grained fossiliferous LIMESTONE with occasional calcite veins. Slightly weathered | | |
| 6.50 | 100 | 85 | 27 | 15 | | (1.10) | (6.10 - 7.20m BGL) 1 fracture set. 10-30 degrees, very closely to closely spaced, undulating, rough with occasional Clay staining | | | |
| 7.20 | 100 | 82 | 56 | | | 7.20 | Strong thinly to medium bedded grey finely to medium grained fossiliferous LIMESTONE with occasional calcite veins. Fresh | | | |
| 7.71-7.80 | | | | C | | | (7.20 - 11.00m BGL) 1 fracture set. 10-30 degrees, closely to medium spaced, undulating, rough with occasional Clay staining | | | |
| 8.00 | | | | | | | | | | |
| 9.25-9.36 | 100 | 97 | 38 | 8 | | (3.80) | | | | |
| 9.65-9.88 | | | | C | | | | | | |
| 9.50 | | | | | | | | | | |

| | | |
|---|---------------------------------|-----------|
| Remarks Cable percussion drilling refused at 6.10m BGL. Rotary drilling completed at 11.00m BGL Groundwater encountered at 3.00m and 5.00m BGL 50mm standpipe installed to 11.00m BGL. Slotted standpipe installed from 11.00 - 1.00m BGL. Solid standpipe installed from 1.00m BGL - GL with gas tap and flush cover Chiselling from 5.50m to 5.80m for 0.5 hours. Chiselling from 6.10m to 6.10m for 1 hour. | Scale (approx) | Logged By |
| | 1:50 | AM |
| | Figure No. 13614-02-24.BRC05 | |



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Site
Dyke Road Galway

Borehole
Number
BRC05

| | | | | |
|--|--|---------------------------------------|------------------------|----------------------------------|
| Machine : Dando 2000 & Beretta T-44 Flush : Water Core Dia : 63.50 mm Method : Cable Percussion with Rotary follow on | Casing Diameter 200mm cased to 6.10m 96mm cased to 11.00m | Ground Level (mOD) 5.53 | Client Aecom | Job Number 13614-02-24 |
| | Location 529900.1 E 725923.8 N | Dates 18/04/2024-20/05/2024 | Engineer | Sheet 2/2 |

| Depth (m) | TCR (%) | SCR (%) | RQD (%) | FI | Field Records | Level (mOD) | Depth (m) (Thickness) | Description | Legend | Water | Instr |
|-----------|---------|---------|---------|----|---------------|-------------|-----------------------|--------------------|--------|-------|-------|
| 11.00 | 100 | 97 | 73 | | | -5.47 | 11.00 | Complete at 11.00m | | | |

| | | |
|--|-----------------------|------------------|
| Remarks | Scale (approx) | Logged By |
| | 1:50 | AM |
| Figure No. 13614-02-24.BRC05 | | |



| | | | | | | | | | |
|------------------------|--|---|--|----------------------------|--|-----------------|--|---------------------------|--|
| Machine : Beretta T-44 | | Casing Diameter 96mm cased to 14.40m | | Ground Level (mOD) 5.43 | | Client Aecom | | Job Number 13614-02-24 | |
| Flush : Water | | Location 529845.9 E 725961.5 N | | Dates 22/05/2024 | | Engineer | | Sheet 1/2 | |
| Core Dia: 63.5 mm | | | | | | | | | |
| Method : Rotary Cored | | | | | | | | | |

| Depth (m) | TCR (%) | SCR (%) | RQD (%) | FI | Field Records | Level (mOD) | Depth (m) (Thickness) | Description | Legend | Water |
|-----------|---------|---------|---------|----|-----------------------------|-------------|--|---|--------|-------|
| 2.00 | 33 | | | | 1,0/0,0,0,0 SPT(C) N=0 | 5.40 | 0.03 | TARMACADAM | | |
| 2.00-2.45 | | | | | | (1.97) | Possible MADEGROUND: Poor recovery. Recovery consists of grey slightly clayey subangular to subrounded fine to coarse GRAVEL | | | |
| 3.50 | 10 | | | | 0,0/0,0,0,0 SPT(C) N=0 | 3.43 | 2.00 | Poor recovery. Recovery consists of grey sandy slightly gravelly silty CLAY with low cobble content. Gravel is subangular to subrounded fine to coarse. Driller notes: Grey silts (Very soft) | | |
| 3.50-3.95 | | | | | | (4.50) | | | | |
| 5.00 | 27 | | | | 0,0/1,0,0,0 SPT(C) N=1 | | | | | |
| 5.00-5.45 | | | | | | | | | | |
| 6.50 | 100 | | | | 3,4/4,5,4,7 SPT(C) N=20 | -1.07 | 6.50 | Grey clayey sandy subangular to subrounded fine to coarse GRAVEL with low cobble content. Driller notes: sandy gravelly Clay (Stiff) | | |
| 6.50-6.95 | | | | | | (0.90) | | | | |
| 8.00 | 37 | 7 | 0 | | 0,5/7,8,8,12 SPT(C) N=35 | -1.97 | 7.40 | Grey gravelly fine to coarse SAND. Gravel is subangular to subrounded fine to medium | | |
| 8.00-8.45 | | | | | | (0.60) | | | | |
| 9.40 | | | | | | -2.57 | 8.00 | Poor recovery. Recovery consists of grey slightly clayey subangular to rounded fine to coarse GRAVEL with low cobble content (Dense) | | |
| 9.50 | | | | | | -3.97 | 9.40 | Medium strong to strong thinly to medium bedded grey finely to medium grained fossiliferous LIMESTONE with occasional calcite veins. Slightly weathered | | |

| | | |
|--|--|-----------|
| Remarks Rotary drilling completed at 14.40m BGL Borehole backfilled upon completion | Scale (approx) | Logged By |
| | 1:50 | AM |
| | Figure No. 13614-02-24.BRC06 | |



| | | | | | | | | | |
|------------------------|--|---|--|----------------------------|--|-----------------|--|---------------------------|--|
| Machine : Beretta T-44 | | Casing Diameter 96mm cased to 14.40m | | Ground Level (mOD) 5.43 | | Client Aecom | | Job Number 13614-02-24 | |
| Flush : Water | | Location 529845.9 E 725961.5 N | | Dates 22/05/2024 | | Engineer | | Sheet 2/2 | |
| Core Dia: 63.5 mm | | | | | | | | | |
| Method : Rotary Cored | | | | | | | | | |

| Depth (m) | TCR (%) | SCR (%) | RQD (%) | FI | Field Records | Level (mOD) | Depth (m) (Thickness) | Description | Legend | Water |
|-------------|---------|---------|---------|----|---------------|-------------|-----------------------|--|--------|-------|
| 10.70 | 100 | 83 | 67 | 6 | | | (1.30) | (9.40 - 10.70m BGL) 1 fracture set. 10-30 degrees, closely to medium spaced, undulating, rough with occasional Clay staining | | |
| 11.00 | | | | | | -5.27 | 10.70 | Medium strong to strong thinly to medium bedded grey and dark grey finely grained argillaceous LIMESTONE with frequent mudstone laminations and occasional calcite veins. Fresh | | |
| 11.00-11.17 | | | | | C | | | | | |
| 12.50 | 100 | 80 | 43 | | | | (3.70) | (10.70 - 14.40m BGL) 3 fracture sets. FS1: 0-20 degrees, closely to medium spaced, planar to undulating, rough with occasional Clay staining. FS2: 40-50 degrees, medium to widely spaced, planar to undulating, rough with occasional Clay staining. FS3: 60-70 degrees, widely spaced, undulating, rough | | |
| 13.34-13.44 | | | | 11 | | | | | | |
| 14.00 | 100 | 93 | 20 | | C | | | | | |
| 14.40 | 100 | 100 | 75 | | | | 14.40 | Complete at 14.40m | | |
| | | | | | | -8.97 | | | | |

| | | |
|---------|---------------------------------|-----------|
| Remarks | Scale (approx) | Logged By |
| | 1:50 | AM |
| | Figure No. 13614-02-24.BRC06 | |

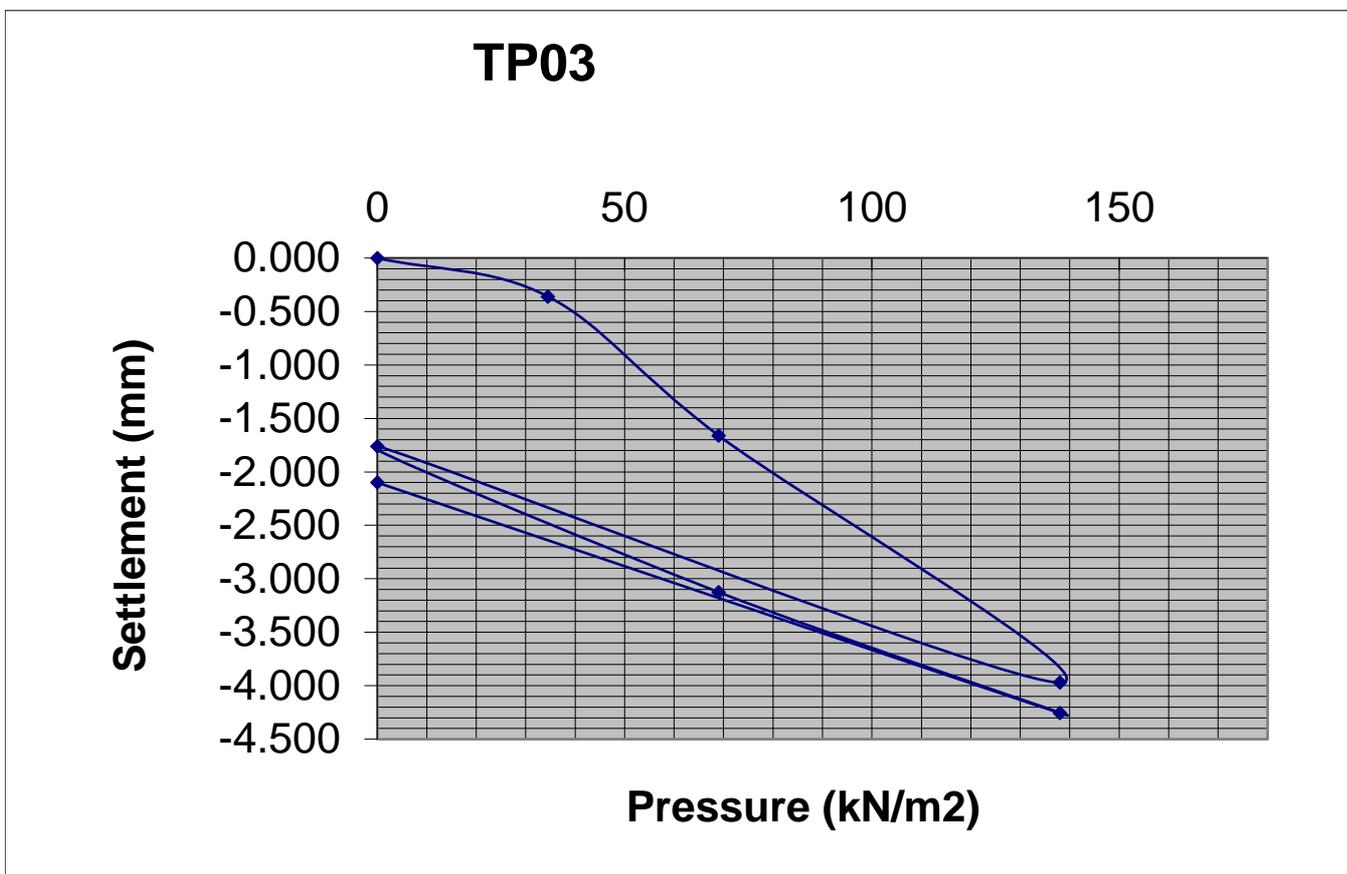
APPENDIX 6 – Insitu Plate Bearing Test Records



| Applied Load | Gauge settlement |
|--------------|------------------|
| 0 | 0.000 |
| 34.5 | -0.36 |
| 69 | -1.66 |
| 138 | -3.97 |
| 0 | -1.76 |
| 69 | -3.125 |
| 138 | -4.255 |
| 0 | -2.1 |
| | |



| | | | |
|-----------------------|-------------------|-----------------|---|
| LOCATION | Dyke Road, Galway | MATERIAL | MADE GROUND: Grey sandy angular to subrounded fine to coarse Gravel |
| CONTRACT NO. | 13614-02-24 | | |
| DATE | 15/04/2024 | | |
| CLIENT | Aecom | DEPTH | 0.20m |
| PLATE DIAMETER | 457mm | NOTES | |
| TEST NO. | TP03 | SAMPLES | |



Modulus of subgrade reaction, K (Initial) = **28.09 MN/m²/m**

Modulus of subgrade reaction, K (Reload) = **34.16 MN/m²/m**

Equivalent CBR(initial)in accordance with HD25/94 volume7 section2 = **3.12 %**

Equivalent CBR(reload)in accordance with HD25/94 volume7 section2 = **4.38 %**

APPENDIX 7 - TRL Dynamic Cone Penetrometer Records





GROUND INVESTIGATIONS IRELAND
Geotechnical & Environmental

Cathelinstown House
Hazelhatch Road,
Newcastle,
Co. Dublin
D22 YC52

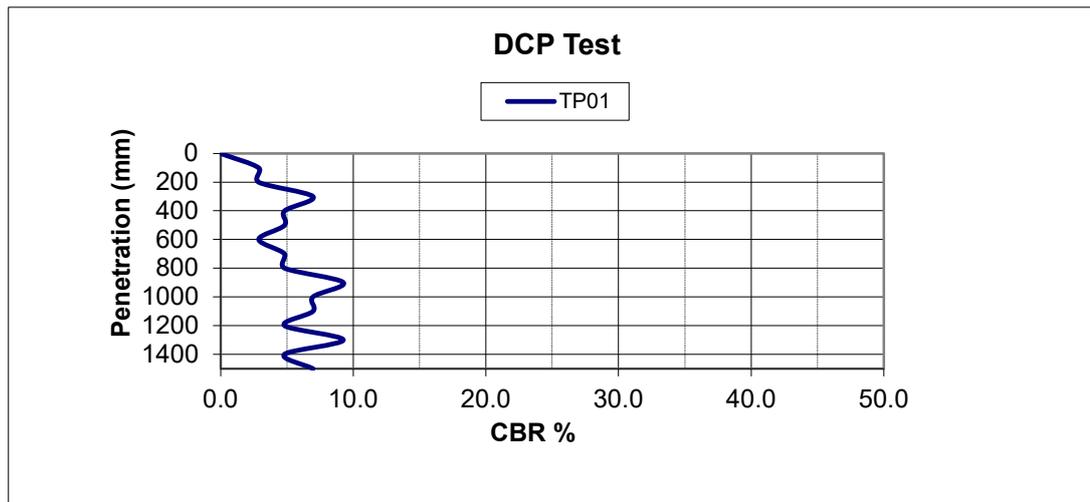
Tel: 01 801 5176 / 5178
Email: info@gii.ie
Web: www.gii.ie

Job Name Dyke Road Galway
Job No. 13614-02-24
Client Aecom
Initial Depth 0.8

Test Type Dynamic Cone Penetration Test
Test Reference TP01
By LB
Date 17/04/2024

| Depth below start depth (mm) | No. of Blows per 100mm | Penetration per Blow (mm) | CBR (%) |
|------------------------------|------------------------|---------------------------|---------|
| 0 | - | - | 0.0 |
| 100 | 2 | 50.0 | 2.9 |
| 200 | 2 | 50.0 | 2.9 |
| 300 | 4 | 25.0 | 7.0 |
| 400 | 3 | 33.3 | 4.8 |
| 500 | 3 | 33.3 | 4.8 |
| 600 | 2 | 50.0 | 2.9 |
| 700 | 3 | 33.3 | 4.8 |
| 800 | 3 | 33.3 | 4.8 |
| 900 | 5 | 20.0 | 9.3 |
| 1000 | 4 | 25.0 | 7.0 |
| 1100 | 4 | 25.0 | 7.0 |
| 1200 | 3 | 33.3 | 4.8 |
| 1300 | 5 | 20.0 | 9.3 |
| 1400 | 3 | 33.3 | 4.8 |
| 1500 | 4 | 25.0 | 7.0 |

Reference Kleyn and Van Heerden (60° Cone)
Formula $\text{Log}_{10}(\text{CBR}) = 2.632 - 1.28 \text{Log}_{10}(\text{mm/blow})$

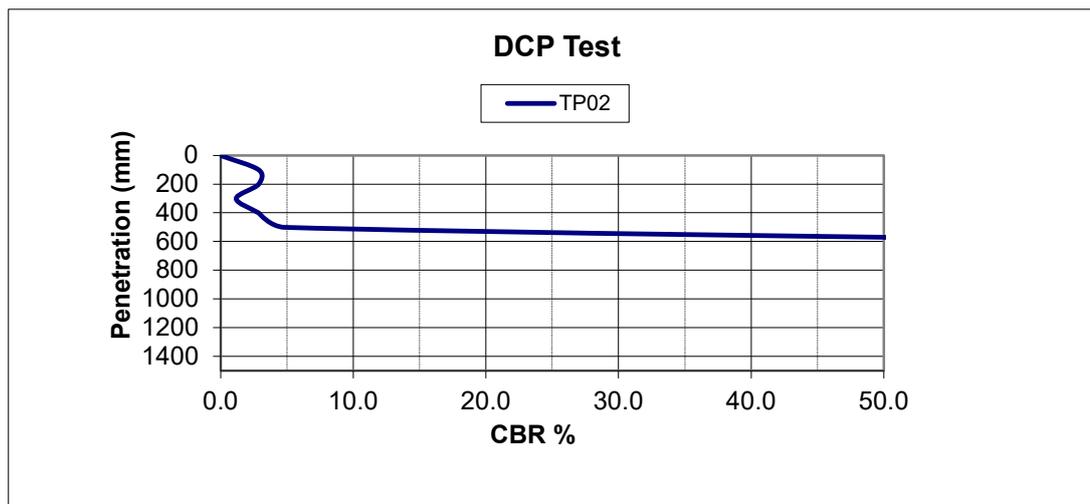


Job Name Dyke Road Galway
Job No. 13614-02-24
Client Aecom
Initial Depth 0.7

Test Type Dynamic Cone Penetration Test
Test Reference TP02
By LB
Date 17/04/2024

| Depth below start depth (mm) | No. of Blows per 100mm | Penetration per Blow (mm) | CBR (%) |
|------------------------------|------------------------|---------------------------|---------|
| 0 | - | - | 0.0 |
| 100 | 2 | 50.0 | 2.9 |
| 200 | 2 | 50.0 | 2.9 |
| 300 | 1 | 100.0 | 1.2 |
| 400 | 2 | 50.0 | 2.9 |
| 500 | 3 | 33.3 | 4.8 |
| 600 | 25 | 4.0 | 72.7 |
| 700 | | | |
| 800 | | | |
| 900 | | | |
| 1000 | | | |
| 1100 | | | |
| 1200 | | | |
| 1300 | | | |
| 1400 | | | |
| 1500 | | | |

Reference Kleyn and Van Heerden (60° Cone)
Formula $\text{Log}_{10}(\text{CBR}) = 2.632 - 1.28 \text{Log}_{10}(\text{mm/blow})$

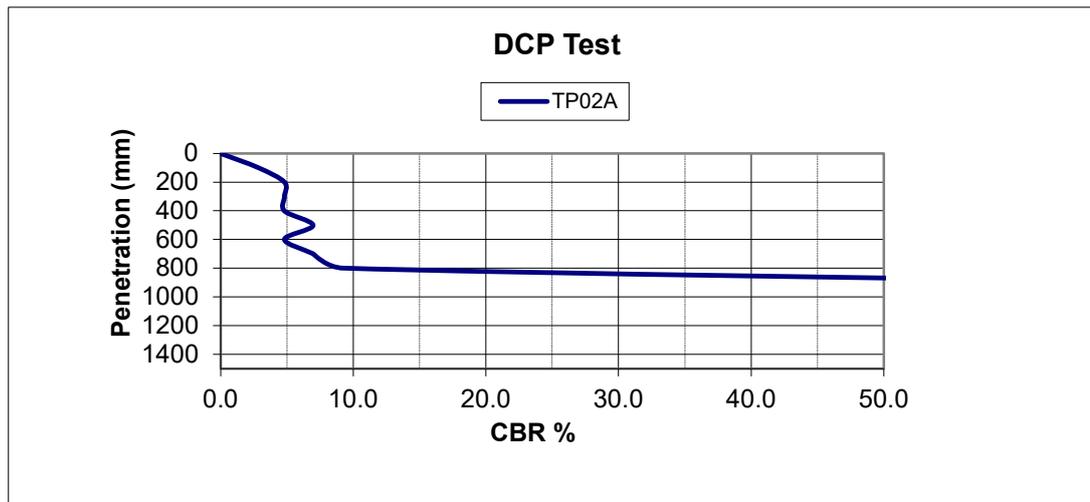


Job Name Dyke Road Galway
Job No. 13614-02-24
Client Aecom
Initial Depth 0.7

Test Type Dynamic Cone Penetration Test
Test Reference TP02A
By LB
Date 17/04/2024

| Depth below start depth (mm) | No. of Blows per 100mm | Penetration per Blow (mm) | CBR (%) |
|------------------------------|------------------------|---------------------------|---------|
| 0 | - | - | 0.0 |
| 100 | 2 | 50.0 | 2.9 |
| 200 | 3 | 33.3 | 4.8 |
| 300 | 3 | 33.3 | 4.8 |
| 400 | 3 | 33.3 | 4.8 |
| 500 | 4 | 25.0 | 7.0 |
| 600 | 3 | 33.3 | 4.8 |
| 700 | 4 | 25.0 | 7.0 |
| 800 | 5 | 20.0 | 9.3 |
| 900 | 25 | 4.0 | 72.7 |
| 1000 | | | |
| 1100 | | | |
| 1200 | | | |
| 1300 | | | |
| 1400 | | | |
| 1500 | | | |

Reference Kleyn and Van Heerden (60° Cone)
Formula $\text{Log}_{10}(\text{CBR}) = 2.632 - 1.28 \text{Log}_{10}(\text{mm/blow})$

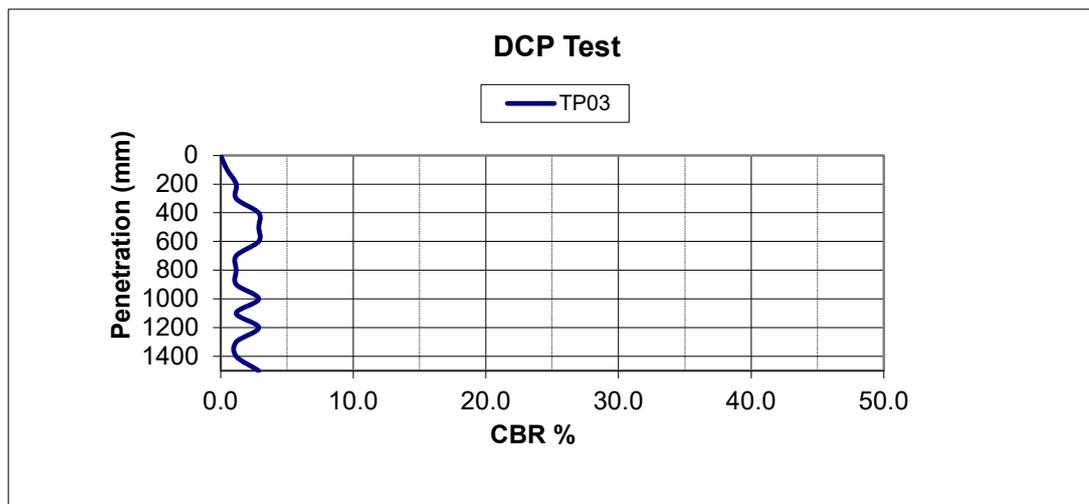


Job Name Dyke Road Galway
Job No. 13614-02-24
Client Aecom
Initial Depth 0.7

Test Type Dynamic Cone Penetration Test
Test Reference TP03
By LB
Date 15/04/2024

| Depth below start depth (mm) | No. of Blows per 100mm | Penetration per Blow (mm) | CBR (%) |
|------------------------------|------------------------|---------------------------|---------|
| 0 | - | - | 0.0 |
| 100 | 0 | 200.0 | 0.5 |
| 200 | 1 | 100.0 | 1.2 |
| 300 | 1 | 100.0 | 1.2 |
| 400 | 2 | 50.0 | 2.9 |
| 500 | 2 | 50.0 | 2.9 |
| 600 | 2 | 50.0 | 2.9 |
| 700 | 1 | 100.0 | 1.2 |
| 800 | 1 | 100.0 | 1.2 |
| 900 | 1 | 100.0 | 1.2 |
| 1000 | 2 | 50.0 | 2.9 |
| 1100 | 1 | 100.0 | 1.2 |
| 1200 | 2 | 50.0 | 2.9 |
| 1300 | 1 | 100.0 | 1.2 |
| 1400 | 1 | 100.0 | 1.2 |
| 1500 | 2 | 50.0 | 2.9 |

Reference Kleyn and Van Heerden (60° Cone)
Formula $\text{Log}_{10}(\text{CBR}) = 2.632 - 1.28 \text{Log}_{10}(\text{mm/blow})$

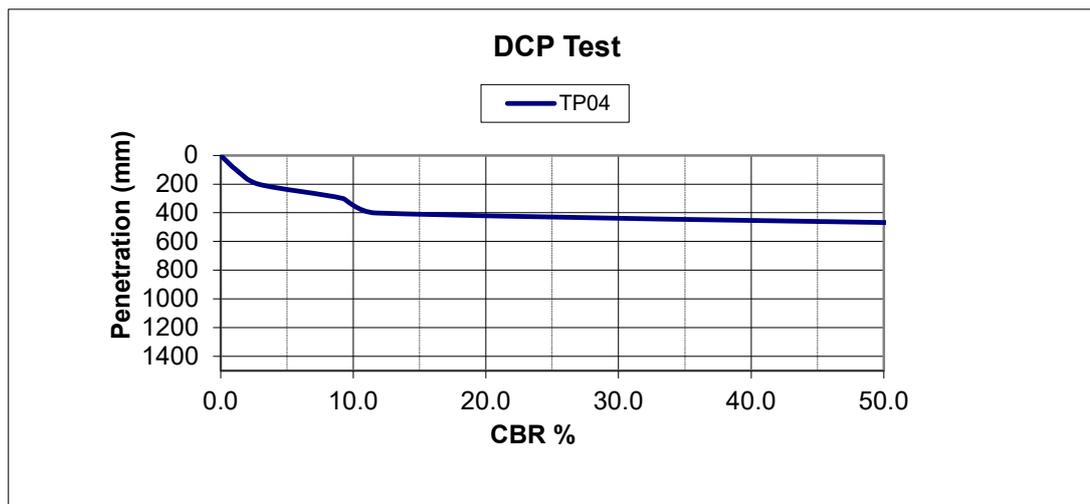


Job Name Dyke Road Galway
Job No. 13614-02-24
Client Aecom
Initial Depth 0.9

Test Type Dynamic Cone Penetration Test
Test Reference TP04
By LB
Date 15/04/2024

| Depth below start depth (mm) | No. of Blows per 100mm | Penetration per Blow (mm) | CBR (%) |
|------------------------------|------------------------|---------------------------|---------|
| 0 | - | - | 0.0 |
| 100 | 4 | 100.0 | 1.2 |
| 200 | 2 | 50.0 | 2.9 |
| 300 | 5 | 20.0 | 9.3 |
| 400 | 6 | 16.7 | 11.7 |
| 500 | 25 | 4.0 | 72.7 |
| 600 | | | |
| 700 | | | |
| 800 | | | |
| 900 | | | |
| 1000 | | | |
| 1100 | | | |
| 1200 | | | |
| 1300 | | | |
| 1400 | | | |
| 1500 | | | |

Reference Kleyn and Van Heerden (60° Cone)
Formula $\text{Log}_{10}(\text{CBR}) = 2.632 - 1.28 \text{Log}_{10}(\text{mm/blow})$



APPENDIX 8 – Laboratory Results



Ground Investigations Ireland
Catherinstown House
Hazelhatch Road
Newcastle
Co. Dublin
Ireland
D22 K5P8



Attention : Mike Sutton
Date : 30th April, 2024
Your reference : 13614-02-24
Our reference : Test Report 24/6265 Batch 1
Location : Dyke Road Galway
Date samples received : 12th April, 2024
Status : Final Report
Issue : 202404301446

Two samples were received for analysis on 12th April, 2024 of which two were scheduled for analysis. Please find attached our Test Report which should be read with notes at the end of the report and should include all sections if reproduced. Interpretations and opinions are outside the scope of any accreditation, and all results relate only to samples supplied.

All analysis is carried out on as received samples and reported on a dry weight basis unless stated otherwise. Results are not surrogate corrected.

The greenhouse gas emissions generated (in Carbon – Co2e) to obtain the results in this report are estimated as:

Scope 1&2 emissions - 9.412 kg of CO2

Scope 1&2&3 emissions - 22.243 kg of CO2

Authorised By:



Phil Sommerton BSc

Senior Project Manager

Please include all sections of this report if it is reproduced

NOTES TO ACCOMPANY ALL SCHEDULES AND REPORTS

EMT Job No.: 24/6265

SOILS and ASH

Please note we are only MCERTS accredited (UK soils only) for sand, loam and clay and any other matrix is outside our scope of accreditation.

Where an MCERTS report has been requested, you will be notified within 48 hours of any samples that have been identified as being outside our MCERTS scope. As validation has been performed on clay, sand and loam, only samples that are predominantly these matrices, or combinations of them will be within our MCERTS scope. If samples are not one of a combination of the above matrices they will not be marked as MCERTS accredited.

It is assumed that you have taken representative samples on site and require analysis on a representative subsample. Stones will generally be included unless we are requested to remove them.

All samples will be discarded one month after the date of reporting, unless we are instructed to the contrary. Asbestos samples are retained for 6 months.

If you have not already done so, please send us a purchase order if this is required by your company.

Where appropriate please make sure that our detection limits are suitable for your needs, if they are not, please notify us immediately.

All analysis is reported on a dry weight basis unless stated otherwise. Limits of detection for analyses carried out on as received samples are not moisture content corrected. Results are not surrogate corrected. Samples are dried at 35°C ±5°C unless otherwise stated. Moisture content for CEN Leachate tests are dried at 105°C ±5°C. Ash samples are dried at 35°C ±5°C.

Where Mineral Oil is quoted, this refers to Total Aliphatics C10-C40.

Where a CEN 10:1 ZERO Headspace VOC test has been carried out, a 10:1 ratio of water to wet (as received) soil has been used.

% Asbestos in Asbestos Containing Materials (ACMs) is determined by reference to HSG 264 The Survey Guide - Appendix 2 : ACMs in buildings listed in order of ease of fibre release.

Sufficient amount of sample must be received to carry out the testing specified. Where an insufficient amount of sample has been received the testing may not meet the requirements of our accredited methods, as such accreditation may be removed.

Negative Neutralization Potential (NP) values are obtained when the volume of NaOH (0.1N) titrated (pH 8.3) is greater than the volume of HCl (1N) to reduce the pH of the sample to 2.0 - 2.5. Any negative NP values are corrected to 0.

The calculation of Pyrite content assumes that all oxidisable sulphides present in the sample are pyrite. This may not be the case. The calculation may be an overestimate when other sulphides such as Barite (Barium Sulphate) are present.

WATERS

Please note we are not a UK Drinking Water Inspectorate (DWI) Approved Laboratory .

ISO17025 accreditation applies to surface water and groundwater and usually one other matrix which is analysis specific, any other liquids are outside our scope of accreditation.

As surface waters require different sample preparation to groundwaters the laboratory must be informed of the water type when submitting samples.

Where Mineral Oil is quoted, this refers to Total Aliphatics C10-C40.

STACK EMISSIONS

Where an MCERTS report has been requested, you will be notified within 48 hours of any samples that have been identified as being outside our MCERTS scope. As validation for Dioxins and Furans and Dioxin like PCBs has been performed on XAD-2 Resin, only samples which use this resin will be within our MCERTS scope.

Where appropriate please make sure that our detection limits are suitable for your needs, if they are not, please notify us immediately.

DEVIATING SAMPLES

All samples should be submitted to the laboratory in suitable containers with sufficient ice packs to sustain an appropriate temperature for the requested analysis. The temperature of sample receipt is recorded on the confirmation schedules in order that the client can make an informed decision as to whether testing should still be undertaken.

SURROGATES

Surrogate compounds are added during the preparation process to monitor recovery of analytes. However low recovery in soils is often due to peat, clay or other organic rich matrices. For waters this can be due to oxidants, surfactants, organic rich sediments or remediation fluids. Acceptable limits for most organic methods are 70 - 130% and for VOCs are 50 - 150%. When surrogate recoveries are outside the performance criteria but the associated AQC passes this is assumed to be due to matrix effect. Results are not surrogate corrected.

DILUTIONS

A dilution suffix indicates a dilution has been performed and the reported result takes this into account. No further calculation is required.

BLANKS

Where analytes have been found in the blank, the sample will be treated in accordance with our laboratory procedure for dealing with contaminated blanks.

NOTE

Data is only reported if the laboratory is confident that the data is a true reflection of the samples analysed. Data is only reported as accredited when all the requirements of our Quality System have been met. In certain circumstances where all the requirements of the Quality System have not been met, for instance if the associated AQC has failed, the reason is fully investigated and documented. The sample data is then evaluated alongside the other quality control checks performed during analysis to determine its suitability. Following this evaluation, provided the sample results have not been effected, the data is reported but accreditation is removed. It is a requirement of our Accreditation Body for data not reported as accredited to be considered indicative only, but this does not mean the data is not valid.

Where possible, and if requested, samples will be re-extracted and a revised report issued with accredited results. Please do not hesitate to contact the laboratory if further details are required of the circumstances which have led to the removal of accreditation.

Laboratory records are kept for a period of no less than 6 years.

REPORTS FROM THE SOUTH AFRICA LABORATORY

Any method number not prefixed with SA has been undertaken in our UK laboratory unless reported as subcontracted.

Measurement Uncertainty

Measurement uncertainty defines the range of values that could reasonably be attributed to the measured quantity. This range of values has not been included within the reported results. Uncertainty expressed as a percentage can be provided upon request.

Customer Provided Information

Sample ID and depth is information provided by the customer.

Age of Diesel

The age of release estimation is based on the nC17/pristane ratio only as prescribed by Christensen and Larsen (1993) and Kaplan, Galperin, Alimi et al., (1996).

Age estimation should be treated with caution as it can be influenced by site specific factors of which the laboratory are not aware.

Tentatively Identified Compounds (TICs)

Where Tentatively Identified Compounds (TICs) are reported, up to 10 Tentatively Identified Compounds will be listed where there is found to be a greater than 80% match with the NIST library. The reported concentration is determined semi-quantitatively, with a matrix specific limit of detection.

Note, other compounds may be present but are not reported.

ABBREVIATIONS and ACRONYMS USED

| | |
|---------|--|
| # | ISO17025 (UKAS Ref No. 4225) accredited - UK. |
| SA | ISO17025 (SANAS Ref No.T0729) accredited - South Africa |
| B | Indicates analyte found in associated method blank. |
| DR | Dilution required. |
| M | MCERTS accredited. |
| NA | Not applicable |
| NAD | No Asbestos Detected. |
| ND | None Detected (usually refers to VOC and/SVOC TICs). |
| NDP | No Determination Possible |
| SS | Calibrated against a single substance |
| SV | Surrogate recovery outside performance criteria. This may be due to a matrix effect. |
| W | Results expressed on as received basis. |
| + | AQC failure, accreditation has been removed from this result, if appropriate, see 'Note' on previous page. |
| >> | Results above quantitative calibration range. The result should be considered the minimum value and is indicative only. The actual result could be significantly higher. |
| * | Analysis subcontracted to an Element Materials Technology approved laboratory. |
| CO | Suspected carry over |
| LOD/LOR | Limit of Detection (Limit of Reporting) in line with ISO 17025 and MCERTS |
| ME | Matrix Effect |
| NFD | No Fibres Detected |
| BS | AQC Sample |
| LB | Blank Sample |
| N | Client Sample |
| TB | Trip Blank Sample |
| OC | Outside Calibration Range |
| | |

HWOL ACRONYMS AND OPERATORS USED

| | |
|-------|--|
| HS | Headspace Analysis. |
| EH | Extractable Hydrocarbons - i.e. everything extracted by the solvent. |
| CU | Clean-up - e.g. by florisil, silica gel. |
| 1D | GC - Single coil gas chromatography. |
| Total | Aliphatics & Aromatics. |
| AL | Aliphatics only. |
| AR | Aromatics only. |
| 2D | GC-GC - Double coil gas chromatography. |
| #1 | EH_Total but with humics mathematically subtracted |
| #2 | EU_Total but with fatty acids mathematically subtracted |
| _ | Operator - underscore to separate acronyms (exception for +). |
| + | Operator to indicate cumulative e.g. EH+HS_Total or EH_CU+HS_Total |
| MS | Mass Spectrometry. |

EMT Job No: 24/6265

| Test Method No. | Description | Prep Method No. (if appropriate) | Description | ISO 17025 (UKAS/S ANAS) | MCERTS (UK soils only) | Analysis done on As Received (AR) or Dried (AD) | Reported on dry weight basis |
|-----------------|--|----------------------------------|---|-------------------------|------------------------|---|------------------------------|
| PM4 | Gravimetric measurement of Natural Moisture Content and % Moisture Content at either 35°C or 105°C. Calculation based on ISO 11465:1993(E) and BS1377-2:1990. | PM0 | No preparation is required. | | | AR | |
| TM4 | Modified USEPA 8270D v5:2014 method for the solvent extraction and determination of PAHs by GC-MS. | PM8 | End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required. | | | AR | Yes |
| TM4 | Modified USEPA 8270D v5:2014 method for the solvent extraction and determination of PAHs by GC-MS. | PM8 | End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required. | Yes | | AR | Yes |
| TM5 | Modified 8015B v2:1996 method for the determination of solvent Extractable Petroleum Hydrocarbons (EPH) within the range C8-C40 by GCFID. For waters the solvent extracts dissolved phase plus a sheen if present. | PM16 | Fractionation into aliphatic and aromatic fractions using a Rapid Trace SPE. | | | AR | |
| TM5 | Modified 8015B v2:1996 method for the determination of solvent Extractable Petroleum Hydrocarbons (EPH) within the range C8-C40 by GCFID. For waters the solvent extracts dissolved phase plus a sheen if present. | PM8/PM16 | End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required/Fractionation into aliphatic and aromatic fractions using a Rapid Trace SPE. | | | AR | Yes |
| TM5 | Modified 8015B v2:1996 method for the determination of solvent Extractable Petroleum Hydrocarbons (EPH) within the range C8-C40 by GCFID. For waters the solvent extracts dissolved phase plus a sheen if present. | PM8/PM16 | End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required/Fractionation into aliphatic and aromatic fractions using a Rapid Trace SPE. | Yes | | AR | Yes |
| TM5/TM36 | please refer to TM5 and TM36 for method details | PM8/PM12/PM16 | please refer to PM8/PM16 and PM12 for method details | | | AR | Yes |
| TM17 | Modified US EPA method 8270D v5:2014. Determination of specific Polychlorinated Biphenyl congeners by GC-MS. | PM8 | End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required. | Yes | | AR | Yes |
| TM20 | Modified BS 1377-3:1990/USEPA 160.1/3 (TDS/TS: 1971) Gravimetric determination of Total Dissolved Solids/Total Solids | PM0 | No preparation is required. | Yes | | AR | Yes |
| TM21 | Modified BS 7755-3:1995, ISO10694:1995 Determination of Total Organic Carbon or Total Carbon by combustion in an Eltra TOC furnace/analyser in the presence of oxygen. The CO2 generated is quantified using infra-red detection. Organic Matter (SOM) calculated as per EA MCERTS Chemical Testing of Soil. | PM24 | Preparation of Soil and Marine Sediment Samples for Total Organic Carbon. | Yes | | AD | Yes |

EMT Job No: 24/6265

| Test Method No. | Description | Prep Method No. (if appropriate) | Description | ISO 17025 (UKAS/S ANAS) | MCERTS (UK soils only) | Analysis done on As Received (AR) or Dried (AD) | Reported on dry weight basis |
|-----------------|--|----------------------------------|---|-------------------------|------------------------|---|------------------------------|
| TM26 | Determination of phenols by Reversed Phased High Performance Liquid Chromatography and Electro-Chemical Detection. | PM0 | No preparation is required. | | | AR | Yes |
| TM30 | Determination of Trace Metals by ICP-OES (Inductively Coupled Plasma – Optical Emission Spectrometry): WATERS by Modified USEPA Method 200.7, Rev. 4.4, 1994; Modified EPA Method 6010B, Rev.2, Dec 1996; Modified BS EN ISO 11885:2009: SOILS by Modified USEP 6010B, Rev.2, Dec.1996; Modified EPA Method 3050B, Rev.2, Dec.1996 | PM15 | Acid digestion of dried and ground solid samples using Aqua Regia refluxed at 112.5 °C. Samples containing asbestos are not dried and ground. | | | AD | Yes |
| TM30 | Determination of Trace Metals by ICP-OES (Inductively Coupled Plasma – Optical Emission Spectrometry): WATERS by Modified USEPA Method 200.7, Rev. 4.4, 1994; Modified EPA Method 6010B, Rev.2, Dec 1996; Modified BS EN ISO 11885:2009: SOILS by Modified USEP 6010B, Rev.2, Dec.1996; Modified EPA Method 3050B, Rev.2, Dec.1996 | PM15 | Acid digestion of dried and ground solid samples using Aqua Regia refluxed at 112.5 °C. Samples containing asbestos are not dried and ground. | Yes | | AD | Yes |
| TM30 | Determination of Trace Metals by ICP-OES (Inductively Coupled Plasma – Optical Emission Spectrometry): WATERS by Modified USEPA Method 200.7, Rev. 4.4, 1994; Modified EPA Method 6010B, Rev.2, Dec 1996; Modified BS EN ISO 11885:2009: SOILS by Modified USEP 6010B, Rev.2, Dec.1996; Modified EPA Method 3050B, Rev.2, Dec.1996 | PM17 | Modified method BS EN12457-2:2002 As received solid samples are leached with water in a 10:1 water to soil ratio for 24 hours, the moisture content of the sample is included in the ratio. | Yes | | AR | Yes |
| TM36 | Modified US EPA method 8015B v2:1996. Determination of Gasoline Range Organics (GRO) in the carbon chain range of C4-12 by headspace GC-FID. MTBE by GC/FID co-elutes with 3-methylpentane if present and therefore can give a false positive. Positive MTBE results will be re-run using GC-MS to double check, when requested. | PM12 | Modified US EPA method 5021A v2:2014. Preparation of solid and liquid samples for GC headspace analysis. | | | AR | Yes |
| TM36 | Modified US EPA method 8015B v2:1996. Determination of Gasoline Range Organics (GRO) in the carbon chain range of C4-12 by headspace GC-FID. MTBE by GC/FID co-elutes with 3-methylpentane if present and therefore can give a false positive. Positive MTBE results will be re-run using GC-MS to double check, when requested. | PM12 | Modified US EPA method 5021A v2:2014. Preparation of solid and liquid samples for GC headspace analysis. | Yes | | AR | Yes |
| TM38 | Soluble Ion analysis using Discrete Analyser. Modified US EPA methods: Chloride 325.2 (1978), Sulphate 375.4 (Rev.2 1993), o-Phosphate 365.2 (Rev.2 1993), TON 353.1 (Rev.2 1993), Nitrite 354.1 (1971), Hex Cr 7196A (1992), NH4+ 350.1 (Rev.2 1993) – All anions comparable to BS ISO 15923-1: 2013 | PM0 | No preparation is required. | Yes | | AR | Yes |
| TM38 | Soluble Ion analysis using Discrete Analyser. Modified US EPA methods: Chloride 325.2 (1978), Sulphate 375.4 (Rev.2 1993), o-Phosphate 365.2 (Rev.2 1993), TON 353.1 (Rev.2 1993), Nitrite 354.1 (1971), Hex Cr 7196A (1992), NH4+ 350.1 (Rev.2 1993) – All anions comparable to BS ISO 15923-1: 2013 | PM20 | Extraction of dried and ground or as received samples with deionised water in a 2:1 water to solid ratio using a reciprocal shaker for all analytes except hexavalent chromium. Extraction of as received sample using 10:1 ratio of 0.2M sodium hydroxide to soil for hexavalent chromium using a reciprocal shaker. | Yes | | AR | Yes |
| TM60 | TC/TOC analysis of Waters by High Temperature Combustion followed by NDIR detection. Based on the following modified standard methods: USEPA 9060A (2002), APHA SMEWW 5310B:1999 22nd Edition, ASTM D 7573, and USEPA 415.1. | PM0 | No preparation is required. | | | AR | Yes |
| TM61 | Determination of Mercury by Cold Vapour Atomic Fluorescence - WATERS: Modified USEPA Method 245.7, Rev 2, Feb 2005. SOILS: Modified USEPA Method 7471B, Rev.2, Feb 2007 | PM0 | No preparation is required. | Yes | | AR | Yes |

EMT Job No: 24/6265

| Test Method No. | Description | Prep Method No. (if appropriate) | Description | ISO 17025 (UKAS/S ANAS) | MCERTS (UK soils only) | Analysis done on As Received (AR) or Dried (AD) | Reported on dry weight basis |
|-----------------|---|----------------------------------|---|-------------------------|------------------------|---|------------------------------|
| TM65 | Asbestos Bulk Identification method based on HSG 248 Second edition (2021) | PM42 | Modified SCA Blue Book V.12 draft 2017 and WM3 1st Edition v1.1:2018. Solid samples undergo a thorough visual inspection for asbestos fibres prior to asbestos identification using TM065. | Yes | | AR | |
| TM73 | Modified US EPA methods 150.1 (1982) and 9045D Rev. 4 - 2004) and BS1377-3:1990. Determination of pH by Metrohm automated probe analyser. | PM0 | No preparation is required. | | | AR | Yes |
| TM73 | Modified US EPA methods 150.1 (1982) and 9045D Rev. 4 - 2004) and BS1377-3:1990. Determination of pH by Metrohm automated probe analyser. | PM11 | Extraction of as received solid samples using one part solid to 2.5 parts deionised water. | Yes | | AR | No |
| TM173 | Analysis of fluoride by ISE (Ion Selective Electrode) using modified ISE method 9214 - 340.2 (EPA 1998) | PM0 | No preparation is required. | | | AR | Yes |
| NONE | No Method Code | NONE | No Method Code | | | AD | Yes |
| NONE | No Method Code | PM17 | Modified method BS EN12457-2:2002 As received solid samples are leached with water in a 10:1 water to soil ratio for 24 hours, the moisture content of the sample is included in the ratio. | | | | |
| NONE | No Method Code | PM4 | Gravimetric measurement of Natural Moisture Content and % Moisture Content at either 35°C or 105°C. Calculation based on ISO 11465:1993(E) and BS1377-2:1990. | | | AR | |
| | | | | | | | |
| | | | | | | | |
| | | | | | | | |

Ground Investigations Ireland
Catherinstown House
Hazelhatch Road
Newcastle
Co. Dublin
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D22 K5P8



Attention : Mike Sutton
Date : 30th April, 2024
Your reference : 13614-02-24
Our reference : Test Report 24/6663 Batch 1
Location : Dyke Road Galway
Date samples received : 18th April, 2024
Status : Final Report
Issue : 202404301121

Two samples were received for analysis on 18th April, 2024 of which two were scheduled for analysis. Please find attached our Test Report which should be read with notes at the end of the report and should include all sections if reproduced. Interpretations and opinions are outside the scope of any accreditation, and all results relate only to samples supplied.

All analysis is carried out on as received samples and reported on a dry weight basis unless stated otherwise. Results are not surrogate corrected.

The greenhouse gas emissions generated (in Carbon – Co2e) to obtain the results in this report are estimated as:

Scope 1&2 emissions - 6.371 kg of CO2

Scope 1&2&3 emissions - 15.057 kg of CO2

Authorised By:



Bruce Leslie
Project Manager

Please include all sections of this report if it is reproduced

Client Name: Ground Investigations Ireland
Reference: 13614-02-24
Location: Dyke Road Galway
Contact: Mike Sutton

Note:

Asbestos Screen analysis is carried out in accordance with our documented in-house methods PM042 and TM065 and HSG 248 by Stereo and Polarised Light Microscopy using Dispersion Staining Techniques and is covered by our UKAS accreditation. Detailed Gravimetric Quantification and PCOM Fibre Analysis is carried out in accordance with our documented in-house methods PM042 and TM131 and HSG 248 using Stereo and Polarised Light Microscopy and Phase Contrast Optical Microscopy (PCOM). Asbestos sub-samples are retained for not less than 6 months from the date of analysis unless specifically requested.

The LOQ of the Asbestos Quantification is 0.001% dry fibre of dry mass of sample.

Where the sample is not taken by a Element Materials Technology consultant, Element Materials Technology cannot be responsible for inaccurate or unrepresentative sampling.

Where trace asbestos is reported the amount of asbestos will be <0.1%.

| EMT Job No. | Batch | Sample ID | Depth | EMT Sample No. | Analyst Name | Date Of Analysis | Analysis | Result |
|-------------|-------|-----------|-------|----------------|---------------|------------------|--|------------------------|
| 24/6663 | 1 | BRC-02 | 0.25 | 4 | Bart Kuznicki | 26/04/2024 | General Description (Bulk Analysis) | Brown soil with stones |
| | | | | | Bart Kuznicki | 26/04/2024 | Asbestos Fibres | NAD |
| | | | | | Bart Kuznicki | 26/04/2024 | Asbestos ACM | NAD |
| | | | | | Bart Kuznicki | 26/04/2024 | Asbestos Type | NAD |
| 24/6663 | 1 | BRC-02 | 1.50 | 8 | Kieran Hunt | 26/04/2024 | General Description (Bulk Analysis) | Brown soil / debris |
| | | | | | Kieran Hunt | 26/04/2024 | Asbestos Fibres | NAD |
| | | | | | Kieran Hunt | 26/04/2024 | Asbestos ACM | NAD |
| | | | | | Kieran Hunt | 26/04/2024 | Asbestos Type | NAD |

NOTES TO ACCOMPANY ALL SCHEDULES AND REPORTS

EMT Job No.: 24/6663

SOILS and ASH

Please note we are only MCERTS accredited (UK soils only) for sand, loam and clay and any other matrix is outside our scope of accreditation.

Where an MCERTS report has been requested, you will be notified within 48 hours of any samples that have been identified as being outside our MCERTS scope. As validation has been performed on clay, sand and loam, only samples that are predominantly these matrices, or combinations of them will be within our MCERTS scope. If samples are not one of a combination of the above matrices they will not be marked as MCERTS accredited.

It is assumed that you have taken representative samples on site and require analysis on a representative subsample. Stones will generally be included unless we are requested to remove them.

All samples will be discarded one month after the date of reporting, unless we are instructed to the contrary. Asbestos samples are retained for 6 months.

If you have not already done so, please send us a purchase order if this is required by your company.

Where appropriate please make sure that our detection limits are suitable for your needs, if they are not, please notify us immediately.

All analysis is reported on a dry weight basis unless stated otherwise. Limits of detection for analyses carried out on as received samples are not moisture content corrected. Results are not surrogate corrected. Samples are dried at 35°C ±5°C unless otherwise stated. Moisture content for CEN Leachate tests are dried at 105°C ±5°C. Ash samples are dried at 35°C ±5°C.

Where Mineral Oil is quoted, this refers to Total Aliphatics C10-C40.

Where a CEN 10:1 ZERO Headspace VOC test has been carried out, a 10:1 ratio of water to wet (as received) soil has been used.

% Asbestos in Asbestos Containing Materials (ACMs) is determined by reference to HSG 264 The Survey Guide - Appendix 2 : ACMs in buildings listed in order of ease of fibre release.

Sufficient amount of sample must be received to carry out the testing specified. Where an insufficient amount of sample has been received the testing may not meet the requirements of our accredited methods, as such accreditation may be removed.

Negative Neutralization Potential (NP) values are obtained when the volume of NaOH (0.1N) titrated (pH 8.3) is greater than the volume of HCl (1N) to reduce the pH of the sample to 2.0 - 2.5. Any negative NP values are corrected to 0.

The calculation of Pyrite content assumes that all oxidisable sulphides present in the sample are pyrite. This may not be the case. The calculation may be an overestimate when other sulphides such as Barite (Barium Sulphate) are present.

WATERS

Please note we are not a UK Drinking Water Inspectorate (DWI) Approved Laboratory .

ISO17025 accreditation applies to surface water and groundwater and usually one other matrix which is analysis specific, any other liquids are outside our scope of accreditation.

As surface waters require different sample preparation to groundwaters the laboratory must be informed of the water type when submitting samples.

Where Mineral Oil is quoted, this refers to Total Aliphatics C10-C40.

STACK EMISSIONS

Where an MCERTS report has been requested, you will be notified within 48 hours of any samples that have been identified as being outside our MCERTS scope. As validation for Dioxins and Furans and Dioxin like PCBs has been performed on XAD-2 Resin, only samples which use this resin will be within our MCERTS scope.

Where appropriate please make sure that our detection limits are suitable for your needs, if they are not, please notify us immediately.

DEVIATING SAMPLES

All samples should be submitted to the laboratory in suitable containers with sufficient ice packs to sustain an appropriate temperature for the requested analysis. The temperature of sample receipt is recorded on the confirmation schedules in order that the client can make an informed decision as to whether testing should still be undertaken.

SURROGATES

Surrogate compounds are added during the preparation process to monitor recovery of analytes. However low recovery in soils is often due to peat, clay or other organic rich matrices. For waters this can be due to oxidants, surfactants, organic rich sediments or remediation fluids. Acceptable limits for most organic methods are 70 - 130% and for VOCs are 50 - 150%. When surrogate recoveries are outside the performance criteria but the associated AQC passes this is assumed to be due to matrix effect. Results are not surrogate corrected.

DILUTIONS

A dilution suffix indicates a dilution has been performed and the reported result takes this into account. No further calculation is required.

BLANKS

Where analytes have been found in the blank, the sample will be treated in accordance with our laboratory procedure for dealing with contaminated blanks.

Please include all sections of this report if it is reproduced

All solid results are expressed on a dry weight basis unless stated otherwise.

NOTE

Data is only reported if the laboratory is confident that the data is a true reflection of the samples analysed. Data is only reported as accredited when all the requirements of our Quality System have been met. In certain circumstances where all the requirements of the Quality System have not been met, for instance if the associated AQC has failed, the reason is fully investigated and documented. The sample data is then evaluated alongside the other quality control checks performed during analysis to determine its suitability. Following this evaluation, provided the sample results have not been effected, the data is reported but accreditation is removed. It is a requirement of our Accreditation Body for data not reported as accredited to be considered indicative only, but this does not mean the data is not valid.

Where possible, and if requested, samples will be re-extracted and a revised report issued with accredited results. Please do not hesitate to contact the laboratory if further details are required of the circumstances which have led to the removal of accreditation.

Laboratory records are kept for a period of no less than 6 years.

REPORTS FROM THE SOUTH AFRICA LABORATORY

Any method number not prefixed with SA has been undertaken in our UK laboratory unless reported as subcontracted.

Measurement Uncertainty

Measurement uncertainty defines the range of values that could reasonably be attributed to the measured quantity. This range of values has not been included within the reported results. Uncertainty expressed as a percentage can be provided upon request.

Customer Provided Information

Sample ID and depth is information provided by the customer.

Age of Diesel

The age of release estimation is based on the nC17/pristane ratio only as prescribed by Christensen and Larsen (1993) and Kaplan, Galperin, Alimi et al., (1996).

Age estimation should be treated with caution as it can be influenced by site specific factors of which the laboratory are not aware.

Tentatively Identified Compounds (TICs)

Where Tentatively Identified Compounds (TICs) are reported, up to 10 Tentatively Identified Compounds will be listed where there is found to be a greater than 80% match with the NIST library. The reported concentration is determined semi-quantitatively, with a matrix specific limit of detection.

Note, other compounds may be present but are not reported.

ABBREVIATIONS and ACRONYMS USED

| | |
|---------|--|
| # | ISO17025 (UKAS Ref No. 4225) accredited - UK. |
| SA | ISO17025 (SANAS Ref No.T0729) accredited - South Africa |
| B | Indicates analyte found in associated method blank. |
| DR | Dilution required. |
| M | MCERTS accredited. |
| NA | Not applicable |
| NAD | No Asbestos Detected. |
| ND | None Detected (usually refers to VOC and/SVOC TICs). |
| NDP | No Determination Possible |
| SS | Calibrated against a single substance |
| SV | Surrogate recovery outside performance criteria. This may be due to a matrix effect. |
| W | Results expressed on as received basis. |
| + | AQC failure, accreditation has been removed from this result, if appropriate, see 'Note' on previous page. |
| >> | Results above quantitative calibration range. The result should be considered the minimum value and is indicative only. The actual result could be significantly higher. |
| * | Analysis subcontracted to an Element Materials Technology approved laboratory. |
| CO | Suspected carry over |
| LOD/LOR | Limit of Detection (Limit of Reporting) in line with ISO 17025 and MCERTS |
| ME | Matrix Effect |
| NFD | No Fibres Detected |
| BS | AQC Sample |
| LB | Blank Sample |
| N | Client Sample |
| TB | Trip Blank Sample |
| OC | Outside Calibration Range |
| | |

HWOL ACRONYMS AND OPERATORS USED

| | |
|-------|--|
| HS | Headspace Analysis. |
| EH | Extractable Hydrocarbons - i.e. everything extracted by the solvent. |
| CU | Clean-up - e.g. by florisil, silica gel. |
| 1D | GC - Single coil gas chromatography. |
| Total | Aliphatics & Aromatics. |
| AL | Aliphatics only. |
| AR | Aromatics only. |
| 2D | GC-GC - Double coil gas chromatography. |
| #1 | EH_Total but with humics mathematically subtracted |
| #2 | EU_Total but with fatty acids mathematically subtracted |
| _ | Operator - underscore to separate acronyms (exception for +). |
| + | Operator to indicate cumulative e.g. EH+HS_Total or EH_CU+HS_Total |
| MS | Mass Spectrometry. |

EMT Job No: 24/6663

| Test Method No. | Description | Prep Method No. (if appropriate) | Description | ISO 17025 (UKAS/S ANAS) | MCERTS (UK soils only) | Analysis done on As Received (AR) or Dried (AD) | Reported on dry weight basis |
|-----------------|--|----------------------------------|---|-------------------------|------------------------|---|------------------------------|
| PM4 | Gravimetric measurement of Natural Moisture Content and % Moisture Content at either 35°C or 105°C. Calculation based on ISO 11465:1993(E) and BS1377-2:1990. | PM0 | No preparation is required. | | | AR | |
| TM4 | Modified USEPA 8270D v5:2014 method for the solvent extraction and determination of PAHs by GC-MS. | PM8 | End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required. | | | AR | Yes |
| TM4 | Modified USEPA 8270D v5:2014 method for the solvent extraction and determination of PAHs by GC-MS. | PM8 | End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required. | Yes | | AR | Yes |
| TM5 | Modified 8015B v2:1996 method for the determination of solvent Extractable Petroleum Hydrocarbons (EPH) within the range C8-C40 by GCFID. For waters the solvent extracts dissolved phase plus a sheen if present. | PM8 | End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required. | Yes | | AR | Yes |
| TM21 | Modified BS 7755-3:1995, ISO10694:1995 Determination of Total Organic Carbon or Total Carbon by combustion in an Eltra TOC furnace/analyser in the presence of oxygen. The CO2 generated is quantified using infra-red detection. Organic Matter (SOM) calculated as per EA MCERTS Chemical Testing of Soil. | PM24 | Preparation of Soil and Marine Sediment Samples for Total Organic Carbon. | | | AD | Yes |
| TM26 | Determination of phenols by Reversed Phased High Performance Liquid Chromatography and Electro-Chemical Detection. | PM21B | As Received samples are extracted in Methanol: Water (60:40) by reciprocal shaker. | Yes | | AR | Yes |
| TM30 | Determination of Trace Metals by ICP-OES (Inductively Coupled Plasma – Optical Emission Spectrometry): WATERS by Modified USEPA Method 200.7, Rev. 4.4, 1994; Modified EPA Method 6010B, Rev.2, Dec 1996; Modified BS EN ISO 11885:2009: SOILS by Modified USEP 6010B, Rev.2, Dec.1996; Modified EPA Method 3050B, Rev.2, Dec.1996 | PM15 | Acid digestion of dried and ground solid samples using Aqua Regia refluxed at 112.5 °C. Samples containing asbestos are not dried and ground. | Yes | | AD | Yes |
| TM38 | Soluble Ion analysis using Discrete Analyser. Modified US EPA methods: Chloride 325.2 (1978), Sulphate 375.4 (Rev.2 1993), o-Phosphate 365.2 (Rev.2 1993), TON 353.1 (Rev.2 1993), Nitrite 354.1 (1971), Hex Cr 7196A (1992), NH4+ 350.1 (Rev.2 1993) – All anions comparable to BS ISO 15923-1: 2013l | PM20 | Extraction of dried and ground or as received samples with deionised water in a 2:1 water to solid ratio using a reciprocal shaker for all analytes except hexavalent chromium. Extraction of as received sample using 10:1 ratio of 0.2M sodium hydroxide to soil for hexavalent chromium using a reciprocal shaker. | Yes | | AD | Yes |
| TM65 | Asbestos Bulk Identification method based on HSG 248 Second edition (2021) | PM42 | Modified SCA Blue Book V.12 draft 2017 and WM3 1st Edition v1.1:2018. Solid samples undergo a thorough visual inspection for asbestos fibres prior to asbestos identification using TM065. | Yes | | AR | |
| TM73 | Modified US EPA methods 150.1 (1982) and 9045D Rev. 4 - 2004) and BS1377-3:1990. Determination of pH by Metrohm automated probe analyser. | PM11 | Extraction of as received solid samples using one part solid to 2.5 parts deionised water. | Yes | | AR | No |

Ground Investigations Ireland
Catherinstown House
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D22 K5P8



Attention : Mike Sutton
Date : 16th May, 2024
Your reference : 13614-02-24
Our reference : Test Report 24/6996 Batch 1
Location : Dyke Road, Galway
Date samples received : 24th April, 2024
Status : Final Report
Issue : 202405161600

Twenty samples were received for analysis on 24th April, 2024 of which nine were scheduled for analysis. Please find attached our Test Report which should be read with notes at the end of the report and should include all sections if reproduced. Interpretations and opinions are outside the scope of any accreditation, and all results relate only to samples supplied.

All analysis is carried out on as received samples and reported on a dry weight basis unless stated otherwise. Results are not surrogate corrected.

The greenhouse gas emissions generated (in Carbon – Co2e) to obtain the results in this report are estimated as:

Scope 1&2 emissions - 43.223 kg of CO2

Scope 1&2&3 emissions - 102.147 kg of CO2

Authorised By:



Bruce Leslie
Project Manager

Please include all sections of this report if it is reproduced

Element Materials Technology

Client Name: Ground Investigations Ireland
Reference: 13614-02-24
Location: Dyke Road, Galway
Contact: Mike Sutton
EMT Job No: 24/6996

Report : Solid

Solids: V=60g VOC jar, J=250g glass jar, T=plastic tub

| EMT Sample No. | 1-4 | 5-8 | 9-12 | 33-36 | 37-40 | 41-44 | 45-48 | 49-52 | 57-60 | | | | |
|-------------------------------------|------------|------------|------------|------------|------------|------------|------------|------------|------------|---------|-------|--------------|--|
| Sample ID | ST01 | ST02 | ST03 | TP03 | TP03 | TP03 | TP04 | TP04 | TP05 | | | | |
| Depth | 0.20-0.80 | 0.15-0.80 | 0.07-1.40 | 0.10-0.60 | 0.61-1.10 | 1.10-2.30 | 0.07-1.00 | 1.00-1.20 | 0.55-1.35 | | | | |
| COC No / misc | | | | | | | | | | | | | |
| Containers | V J T | V J T | V J T | V J T | V J T | V J T | V J T | V J T | V J T | | | | |
| Sample Date | 11/04/2024 | 11/04/2024 | 12/04/2024 | 15/04/2024 | 15/04/2024 | 15/04/2024 | 15/04/2024 | 15/04/2024 | 15/04/2024 | | | | |
| Sample Type | Soil | | | | |
| Batch Number | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | | | | |
| Date of Receipt | 24/04/2024 | 24/04/2024 | 24/04/2024 | 24/04/2024 | 24/04/2024 | 24/04/2024 | 24/04/2024 | 24/04/2024 | 24/04/2024 | | | | |
| | | | | | | | | | | LOD/LOR | Units | Method No. | |
| Antimony | <1 | <1 | <1 | <1 | <1 | <1 | <1 | <1 | <1 | <1 | mg/kg | TM30/PM15 | |
| Arsenic # | <0.5 | 2.4 | 1.6 | 1.3 | 7.7 | 1.3 | 1.7 | 2.6 | 1.7 | <0.5 | mg/kg | TM30/PM15 | |
| Barium # | 8 | 12 | 7 | 7 | 34 | 99 | 13 | 12 | 10 | <1 | mg/kg | TM30/PM15 | |
| Cadmium # | 0.2 | 0.4 | 0.4 | 0.3 | 0.6 | 0.2 | 0.4 | 0.4 | 0.9 | <0.1 | mg/kg | TM30/PM15 | |
| Chromium # | 13.3 | 16.2 | 11.7 | 4.1 | 11.6 | 17.9 | 23.0 | 10.2 | 24.5 | <0.5 | mg/kg | TM30/PM15 | |
| Copper # | 4 | 9 | 5 | 3 | 26 | 6 | 5 | 6 | 5 | <1 | mg/kg | TM30/PM15 | |
| Lead # | <5 | 9 | <5 | <5 | 30 | <5 | <5 | 7 | <5 | <5 | mg/kg | TM30/PM15 | |
| Mercury # | <0.1 | <0.1 | <0.1 | <0.1 | 0.2 | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 | mg/kg | TM30/PM15 | |
| Molybdenum # | 0.7 | 1.5 | 0.9 | 0.4 | 1.0 | 1.3 | 1.4 | 0.9 | 1.5 | <0.1 | mg/kg | TM30/PM15 | |
| Nickel # | 3.4 | 9.4 | 7.2 | 2.8 | 8.7 | 3.0 | 6.3 | 7.6 | 7.0 | <0.7 | mg/kg | TM30/PM15 | |
| Selenium # | <1 | <1 | <1 | <1 | 3 | <1 | <1 | <1 | <1 | <1 | mg/kg | TM30/PM15 | |
| Sulphur as S | - | - | - | - | 0.52 | 0.13 | - | 0.07 | - | <0.01 | % | TM30/PM15 | |
| Total Sulphate as SO4 BRE | - | - | - | - | 0.58 | 0.10 | - | 0.03 | - | <0.01 | % | TM50/PM29 | |
| Zinc # | 6 | 16 | 11 | <5 | 29 | 6 | 11 | 10 | 16 | <5 | mg/kg | TM30/PM15 | |
| Magnesium | - | - | - | - | 0.0106 | 0.0035 | - | 0.0019 | - | <0.0001 | g/l | TM30/PM20 | |
| PAH MS | | | | | | | | | | | | | |
| Naphthalene # | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | mg/kg | TM4/PM8 | |
| Acenaphthylene | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | mg/kg | TM4/PM8 | |
| Acenaphthene # | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | mg/kg | TM4/PM8 | |
| Fluorene # | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | mg/kg | TM4/PM8 | |
| Phenanthrene # | <0.03 | <0.03 | <0.03 | <0.03 | 0.12 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | mg/kg | TM4/PM8 | |
| Anthracene # | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | mg/kg | TM4/PM8 | |
| Fluoranthene # | <0.03 | <0.03 | <0.03 | <0.03 | 0.21 | <0.03 | <0.03 | 0.05 | 0.07 | <0.03 | mg/kg | TM4/PM8 | |
| Pyrene # | <0.03 | <0.03 | <0.03 | <0.03 | 0.18 | <0.03 | <0.03 | 0.04 | 0.08 | <0.03 | mg/kg | TM4/PM8 | |
| Benzo(a)anthracene # | <0.06 | <0.06 | <0.06 | <0.06 | 0.21 | <0.06 | <0.06 | <0.06 | 0.08 | <0.06 | mg/kg | TM4/PM8 | |
| Chrysene # | <0.02 | <0.02 | <0.02 | <0.02 | 0.15 | <0.02 | <0.02 | <0.02 | 0.07 | <0.02 | mg/kg | TM4/PM8 | |
| Benzo(k)fluoranthene # | <0.07 | <0.07 | <0.07 | <0.07 | <0.07 | <0.07 | <0.07 | <0.07 | 0.15 | <0.07 | mg/kg | TM4/PM8 | |
| Benzo(a)pyrene # | <0.04 | <0.04 | <0.04 | <0.04 | 0.12 | <0.04 | <0.04 | <0.04 | 0.10 | <0.04 | mg/kg | TM4/PM8 | |
| Indeno(123cd)pyrene # | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | 0.06 | <0.04 | mg/kg | TM4/PM8 | |
| Dibenzo(ah)anthracene # | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | mg/kg | TM4/PM8 | |
| Benzo(ghi)perylene # | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | 0.06 | <0.04 | mg/kg | TM4/PM8 | |
| Coronene | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | mg/kg | TM4/PM8 | |
| PAH 6 Total # | <0.22 | <0.22 | <0.22 | <0.22 | 0.33 | <0.22 | <0.22 | <0.22 | 0.44 | <0.22 | mg/kg | TM4/PM8 | |
| PAH 17 Total | <0.64 | <0.64 | <0.64 | <0.64 | 0.99 | <0.64 | <0.64 | <0.64 | 0.67 | <0.64 | mg/kg | TM4/PM8 | |
| Benzo(b)fluoranthene | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | 0.11 | <0.05 | mg/kg | TM4/PM8 | |
| Benzo(k)fluoranthene | <0.02 | <0.02 | <0.02 | <0.02 | <0.02 | <0.02 | <0.02 | <0.02 | 0.04 | <0.02 | mg/kg | TM4/PM8 | |
| Benzo(j)fluoranthene | <1 | <1 | <1 | <1 | <1 | <1 | <1 | <1 | <1 | <1 | mg/kg | TM4/PM8 | |
| PAH Surrogate % Recovery | 95 | 94 | 97 | 97 | 94 | 96 | 95 | 94 | 96 | <0 | % | TM4/PM8 | |
| Mineral Oil (C10-C40) (EH_CU_1D_AL) | <30 | <30 | 69 | <30 | 124 | 173 | <30 | <30 | <30 | <30 | mg/kg | TM5/PM8/PM16 | |

Please see attached notes for all abbreviations and acronyms

Element Materials Technology

Client Name: Ground Investigations Ireland
Reference: 13614-02-24
Location: Dyke Road, Galway
Contact: Mike Sutton
EMT Job No: 24/6996

Report : Solid

Solids: V=60g VOC jar, J=250g glass jar, T=plastic tub

| EMT Sample No. | 1-4 | 5-8 | 9-12 | 33-36 | 37-40 | 41-44 | 45-48 | 49-52 | 57-60 | | | |
|---|------------|------------|------------|------------|--------------------|------------|------------|------------|------------|---------|-------|------------------------|
| Sample ID | ST01 | ST02 | ST03 | TP03 | TP03 | TP03 | TP04 | TP04 | TP05 | | | |
| Depth | 0.20-0.80 | 0.15-0.80 | 0.07-1.40 | 0.10-0.60 | 0.61-1.10 | 1.10-2.30 | 0.07-1.00 | 1.00-1.20 | 0.55-1.35 | | | |
| COC No / misc | | | | | | | | | | | | |
| Containers | V J T | V J T | V J T | V J T | V J T | V J T | V J T | V J T | V J T | | | |
| Sample Date | 11/04/2024 | 11/04/2024 | 12/04/2024 | 15/04/2024 | 15/04/2024 | 15/04/2024 | 15/04/2024 | 15/04/2024 | 15/04/2024 | | | |
| Sample Type | Soil | Soil | Soil | Soil | Soil | Soil | Soil | Soil | Soil | | | |
| Batch Number | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | | | |
| Date of Receipt | 24/04/2024 | 24/04/2024 | 24/04/2024 | 24/04/2024 | 24/04/2024 | 24/04/2024 | 24/04/2024 | 24/04/2024 | 24/04/2024 | | | |
| | | | | | | | | | | LOD/LOR | Units | Method No. |
| TPH CWG | | | | | | | | | | | | |
| Aliphatics | | | | | | | | | | | | |
| >C5-C6 (HS_1D_AL) # | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 ^{SV} | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 | mg/kg | TM36/PM12 |
| >C6-C8 (HS_1D_AL) # | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 ^{SV} | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 | mg/kg | TM36/PM12 |
| >C8-C10 (HS_1D_AL) | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 ^{SV} | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 | mg/kg | TM36/PM12 |
| >C10-C12 (EH_CU_1D_AL) # | <0.2 | <0.2 | <0.2 | <0.2 | <0.2 | <0.2 | <0.2 | <0.2 | <0.2 | <0.2 | mg/kg | TMS/IPM8/PM16 |
| >C12-C16 (EH_CU_1D_AL) # | <4 | <4 | <4 | <4 | <4 | <4 | <4 | <4 | <4 | <4 | mg/kg | TMS/IPM8/PM16 |
| >C16-C21 (EH_CU_1D_AL) # | <7 | <7 | <7 | <7 | 21 | 71 | <7 | <7 | <7 | <7 | mg/kg | TMS/IPM8/PM16 |
| >C21-C35 (EH_CU_1D_AL) # | <7 | <7 | 69 | <7 | 103 | 102 | 19 | <7 | <7 | <7 | mg/kg | TMS/IPM8/PM16 |
| >C35-C40 (EH_CU_1D_AL) | <7 | <7 | <7 | <7 | <7 | <7 | <7 | <7 | <7 | <7 | mg/kg | TMS/IPM8/PM16 |
| Total aliphatics C5-40 (EH_CU+HS_1D_AL) | <26 | <26 | 69 | <26 | 124 | 173 | <26 | <26 | <26 | <26 | mg/kg | TMS/TMS/IPM8/PM12/PM16 |
| >C6-C10 (HS_1D_AL) | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 ^{SV} | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 | mg/kg | TM36/PM12 |
| >C10-C25 (EH_CU_1D_AL) | <10 | <10 | <10 | <10 | 35 | 112 | <10 | <10 | <10 | <10 | mg/kg | TMS/IPM8/PM16 |
| >C25-C35 (EH_CU_1D_AL) | <10 | <10 | 56 | <10 | 73 | 61 | 19 | <10 | <10 | <10 | mg/kg | TMS/IPM8/PM16 |
| Aromatics | | | | | | | | | | | | |
| >C5-EC7 (HS_1D_AR) # | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 ^{SV} | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 | mg/kg | TM36/PM12 |
| >EC7-EC8 (HS_1D_AR) # | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 ^{SV} | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 | mg/kg | TM36/PM12 |
| >EC8-EC10 (HS_1D_AR) # | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 ^{SV} | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 | mg/kg | TM36/PM12 |
| >EC10-EC12 (EH_CU_1D_AR) # | <0.2 | <0.2 | <0.2 | <0.2 | <0.2 | <0.2 | <0.2 | <0.2 | <0.2 | <0.2 | mg/kg | TMS/IPM8/PM16 |
| >EC12-EC16 (EH_CU_1D_AR) # | <4 | <4 | <4 | <4 | <4 | <4 | <4 | <4 | <4 | <4 | mg/kg | TMS/IPM8/PM16 |
| >EC16-EC21 (EH_CU_1D_AR) # | <7 | <7 | <7 | <7 | <7 | <7 | <7 | <7 | <7 | <7 | mg/kg | TMS/IPM8/PM16 |
| >EC21-EC35 (EH_CU_1D_AR) # | <7 | <7 | 68 | <7 | 237 | 156 | 56 | <7 | <7 | <7 | mg/kg | TMS/IPM8/PM16 |
| >EC35-EC40 (EH_CU_1D_AR) | <7 | <7 | 9 | <7 | 21 | <7 | 13 | <7 | <7 | <7 | mg/kg | TMS/IPM8/PM16 |
| Total aromatics C5-40 (EH_CU+HS_1D_AR) | <26 | <26 | 77 | <26 | 258 | 156 | 69 | <26 | <26 | <26 | mg/kg | TMS/TMS/IPM8/PM12/PM16 |
| Total aliphatics and aromatics(C5-40) (EH_CU+HS_1D_Total) | <52 | <52 | 146 | <52 | 382 | 329 | 69 | <52 | <52 | <52 | mg/kg | TMS/TMS/IPM8/PM12/PM16 |
| >EC6-EC10 (HS_1D_AR) # | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 ^{SV} | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 | mg/kg | TM36/PM12 |
| >EC10-EC25 (EH_CU_1D_AR) | <10 | <10 | <10 | <10 | 53 | 54 | <10 | <10 | <10 | <10 | mg/kg | TMS/IPM8/PM16 |
| >EC25-EC35 (EH_CU_1D_AR) | <10 | <10 | 68 | <10 | 176 | 115 | 56 | <10 | <10 | <10 | mg/kg | TMS/IPM8/PM16 |
| MTBE # | <5 | <5 | <5 | <5 | <5 ^{SV} | <5 | <5 | <5 | <5 | <5 | ug/kg | TM36/PM12 |
| Benzene # | <5 | <5 | <5 | <5 | <5 ^{SV} | <5 | <5 | <5 | <5 | <5 | ug/kg | TM36/PM12 |
| Toluene # | <5 | 7 | <5 | <5 | <5 ^{SV} | <5 | <5 | <5 | <5 | <5 | ug/kg | TM36/PM12 |
| Ethylbenzene # | <5 | <5 | <5 | <5 | <5 ^{SV} | <5 | <5 | <5 | <5 | <5 | ug/kg | TM36/PM12 |
| m/p-Xylene # | <5 | 7 | <5 | <5 | <5 ^{SV} | <5 | <5 | <5 | <5 | <5 | ug/kg | TM36/PM12 |
| o-Xylene # | <5 | <5 | <5 | <5 | <5 ^{SV} | <5 | <5 | <5 | <5 | <5 | ug/kg | TM36/PM12 |
| PCB 28 # | <5 | <5 | <5 | <5 | <5 | <5 | <5 | <5 | <5 | <5 | ug/kg | TM17/PM8 |
| PCB 52 # | <5 | <5 | <5 | <5 | <5 | <5 | <5 | <5 | <5 | <5 | ug/kg | TM17/PM8 |
| PCB 101 # | <5 | <5 | <5 | <5 | <5 | <5 | <5 | <5 | <5 | <5 | ug/kg | TM17/PM8 |
| PCB 118 # | <5 | <5 | <5 | <5 | <5 | <5 | <5 | <5 | <5 | <5 | ug/kg | TM17/PM8 |
| PCB 138 # | <5 | <5 | <5 | <5 | <5 | <5 | <5 | <5 | <5 | <5 | ug/kg | TM17/PM8 |
| PCB 153 # | <5 | <5 | <5 | <5 | <5 | <5 | <5 | <5 | <5 | <5 | ug/kg | TM17/PM8 |
| PCB 180 # | <5 | <5 | <5 | <5 | <5 | <5 | <5 | <5 | <5 | <5 | ug/kg | TM17/PM8 |
| Total 7 PCBs # | <35 | <35 | <35 | <35 | <35 | <35 | <35 | <35 | <35 | <35 | ug/kg | TM17/PM8 |

Please see attached notes for all abbreviations and acronyms

Element Materials Technology

Client Name: Ground Investigations Ireland
Reference: 13614-02-24
Location: Dyke Road, Galway
Contact: Mike Sutton
EMT Job No: 24/6996

Report : Solid

Solids: V=60g VOC jar, J=250g glass jar, T=plastic tub

| EMT Sample No. | 1-4 | 5-8 | 9-12 | 33-36 | 37-40 | 41-44 | 45-48 | 49-52 | 57-60 | | | | |
|---------------------------------|------------|------------|------------|------------|------------|------------|------------|------------|------------|--|---------|----------|---------------|
| Sample ID | ST01 | ST02 | ST03 | TP03 | TP03 | TP03 | TP04 | TP04 | TP05 | | | | |
| Depth | 0.20-0.80 | 0.15-0.80 | 0.07-1.40 | 0.10-0.60 | 0.61-1.10 | 1.10-2.30 | 0.07-1.00 | 1.00-1.20 | 0.55-1.35 | | | | |
| COC No / misc | | | | | | | | | | | | | |
| Containers | V J T | V J T | V J T | V J T | V J T | V J T | V J T | V J T | V J T | | | | |
| Sample Date | 11/04/2024 | 11/04/2024 | 12/04/2024 | 15/04/2024 | 15/04/2024 | 15/04/2024 | 15/04/2024 | 15/04/2024 | 15/04/2024 | | | | |
| Sample Type | Soil | | | | |
| Batch Number | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | | | | |
| Date of Receipt | 24/04/2024 | 24/04/2024 | 24/04/2024 | 24/04/2024 | 24/04/2024 | 24/04/2024 | 24/04/2024 | 24/04/2024 | 24/04/2024 | | | | |
| | | | | | | | | | | | LOD/LOR | Units | Method No. |
| Natural Moisture Content | 2.0 | 5.7 | 3.6 | 1.8 | 193.1 | 239.1 | 4.8 | 4.0 | 5.7 | | <0.1 | % | PM4/PM0 |
| Moisture Content (% Wet Weight) | 1.9 | 5.4 | 3.5 | 1.8 | 65.9 | 70.5 | 4.6 | 3.8 | 5.4 | | <0.1 | % | PM4/PM0 |
| Chloride (2:1 Ext BRE) # | - | - | - | - | 0.060 | 0.027 | - | 0.007 | - | | <0.002 | g/l | TM38/PM20 |
| Hexavalent Chromium # | <0.3 | <0.3 | <0.3 | <0.3 | <0.3 | <0.3 | <0.3 | <0.3 | <0.3 | | <0.3 | mg/kg | TM38/PM20 |
| Nitrate as NO3 (2:1 Ext BRE) | - | - | - | - | 0.0064 | <0.0025 | - | <0.0025 | - | | <0.0025 | g/l | TM38/PM20 |
| Sulphate as SO4 (2:1 Ext) # | - | - | - | - | 0.4223 | 0.0352 | - | 0.0235 | - | | <0.0015 | g/l | TM38/PM20 |
| Chromium III | 13.3 | 16.2 | 11.7 | 4.1 | 11.6 | 17.9 | 23.0 | 10.2 | 24.5 | | <0.5 | mg/kg | NONE/NONE |
| Total Organic Carbon # | 0.05 | 0.57 | 0.11 | 0.18 | 27.30 | 3.80 | 0.17 | 0.20 | 0.13 | | <0.02 | % | TM21/PM24 |
| pH # | 9.43 | 8.81 | 8.92 | 8.19 | 7.14 | 8.41 | 9.59 | 8.82 | 8.82 | | <0.01 | pH units | TM73/PM11 |
| Asbestos Type* | NAD | | | None | Subcontracted |

Please see attached notes for all abbreviations and acronyms

Element Materials Technology

Client Name: Ground Investigations Ireland
Reference: 13614-02-24
Location: Dyke Road, Galway
Contact: Mike Sutton
EMT Job No: 24/6996

Report : CEN 10:1 1 Batch

Solids: V=60g VOC jar, J=250g glass jar, T=plastic tub

| EMT Sample No. | 1-4 | 5-8 | 9-12 | 33-36 | 37-40 | 41-44 | 45-48 | 49-52 | 57-60 | | | | |
|---|------------|------------|------------|------------|------------|------------|------------|------------|------------|--|----------|----------|------------|
| Sample ID | ST01 | ST02 | ST03 | TP03 | TP03 | TP03 | TP04 | TP04 | TP05 | | | | |
| Depth | 0.20-0.80 | 0.15-0.80 | 0.07-1.40 | 0.10-0.60 | 0.61-1.10 | 1.10-2.30 | 0.07-1.00 | 1.00-1.20 | 0.55-1.35 | | | | |
| COC No / misc | | | | | | | | | | | | | |
| Containers | V J T | V J T | V J T | V J T | V J T | V J T | V J T | V J T | V J T | | | | |
| Sample Date | 11/04/2024 | 11/04/2024 | 12/04/2024 | 15/04/2024 | 15/04/2024 | 15/04/2024 | 15/04/2024 | 15/04/2024 | 15/04/2024 | | | | |
| Sample Type | Soil | | | | |
| Batch Number | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | | | | |
| Date of Receipt | 24/04/2024 | 24/04/2024 | 24/04/2024 | 24/04/2024 | 24/04/2024 | 24/04/2024 | 24/04/2024 | 24/04/2024 | 24/04/2024 | | | | |
| | | | | | | | | | | | LOD/LOR | Units | Method No. |
| Dissolved Antimony [#] | <0.002 | <0.002 | <0.002 | <0.002 | 0.007 | 0.002 | <0.002 | <0.002 | <0.002 | | <0.002 | mg/l | TM30/PM17 |
| Dissolved Antimony (A10) [#] | <0.02 | <0.02 | <0.02 | <0.02 | 0.07 | <0.02 | <0.02 | <0.02 | <0.02 | | <0.02 | mg/kg | TM30/PM17 |
| Dissolved Arsenic [#] | <0.0025 | <0.0025 | <0.0025 | <0.0025 | 0.0068 | 0.0039 | <0.0025 | <0.0025 | <0.0025 | | <0.0025 | mg/l | TM30/PM17 |
| Dissolved Arsenic (A10) [#] | <0.025 | <0.025 | <0.025 | <0.025 | 0.068 | 0.039 | <0.025 | <0.025 | <0.025 | | <0.025 | mg/kg | TM30/PM17 |
| Dissolved Barium [#] | <0.003 | <0.003 | <0.003 | <0.003 | 0.021 | 0.008 | <0.003 | <0.003 | <0.003 | | <0.003 | mg/l | TM30/PM17 |
| Dissolved Barium (A10) [#] | <0.03 | <0.03 | <0.03 | <0.03 | 0.21 | 0.08 | <0.03 | <0.03 | <0.03 | | <0.03 | mg/kg | TM30/PM17 |
| Dissolved Cadmium [#] | <0.0005 | <0.0005 | <0.0005 | <0.0005 | <0.0005 | <0.0005 | <0.0005 | <0.0005 | <0.0005 | | <0.0005 | mg/l | TM30/PM17 |
| Dissolved Cadmium (A10) [#] | <0.005 | <0.005 | <0.005 | <0.005 | <0.005 | <0.005 | <0.005 | <0.005 | <0.005 | | <0.005 | mg/kg | TM30/PM17 |
| Dissolved Chromium [#] | <0.0015 | <0.0015 | <0.0015 | <0.0015 | <0.0015 | <0.0015 | 0.0036 | <0.0015 | <0.0015 | | <0.0015 | mg/l | TM30/PM17 |
| Dissolved Chromium (A10) [#] | <0.015 | <0.015 | <0.015 | <0.015 | <0.015 | <0.015 | 0.036 | <0.015 | <0.015 | | <0.015 | mg/kg | TM30/PM17 |
| Dissolved Copper [#] | <0.007 | <0.007 | <0.007 | <0.007 | <0.007 | <0.007 | <0.007 | <0.007 | <0.007 | | <0.007 | mg/l | TM30/PM17 |
| Dissolved Copper (A10) [#] | <0.07 | <0.07 | <0.07 | <0.07 | <0.07 | <0.07 | <0.07 | <0.07 | <0.07 | | <0.07 | mg/kg | TM30/PM17 |
| Dissolved Lead [#] | <0.005 | <0.005 | <0.005 | <0.005 | <0.005 | <0.005 | <0.005 | <0.005 | <0.005 | | <0.005 | mg/l | TM30/PM17 |
| Dissolved Lead (A10) [#] | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | | <0.05 | mg/kg | TM30/PM17 |
| Dissolved Molybdenum [#] | <0.002 | 0.004 | <0.002 | <0.002 | 0.005 | 0.005 | <0.002 | <0.002 | <0.002 | | <0.002 | mg/l | TM30/PM17 |
| Dissolved Molybdenum (A10) [#] | <0.02 | 0.04 | <0.02 | <0.02 | 0.05 | 0.05 | <0.02 | <0.02 | <0.02 | | <0.02 | mg/kg | TM30/PM17 |
| Dissolved Nickel [#] | <0.002 | <0.002 | <0.002 | <0.002 | <0.002 | <0.002 | <0.002 | <0.002 | <0.002 | | <0.002 | mg/l | TM30/PM17 |
| Dissolved Nickel (A10) [#] | <0.02 | <0.02 | <0.02 | <0.02 | <0.02 | <0.02 | <0.02 | <0.02 | <0.02 | | <0.02 | mg/kg | TM30/PM17 |
| Dissolved Selenium [#] | <0.003 | <0.003 | <0.003 | <0.003 | <0.003 | <0.003 | <0.003 | <0.003 | <0.003 | | <0.003 | mg/l | TM30/PM17 |
| Dissolved Selenium (A10) [#] | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | | <0.03 | mg/kg | TM30/PM17 |
| Dissolved Zinc [#] | 0.003 | <0.003 | <0.003 | <0.003 | 0.006 | 0.004 | <0.003 | 0.003 | 0.003 | | <0.003 | mg/l | TM30/PM17 |
| Dissolved Zinc (A10) [#] | 0.03 | <0.03 | <0.03 | <0.03 | 0.06 | 0.04 | <0.03 | <0.03 | 0.03 | | <0.03 | mg/kg | TM30/PM17 |
| Mercury Dissolved by CVAF [#] | <0.00001 | <0.00001 | <0.00001 | <0.00001 | <0.00001 | <0.00001 | 0.00001 | <0.00001 | <0.00001 | | <0.00001 | mg/l | TM61/PM0 |
| Mercury Dissolved by CVAF [#] | <0.0001 | <0.0001 | <0.0001 | <0.0001 | <0.0001 | <0.0001 | 0.0001 | <0.0001 | <0.0001 | | <0.0001 | mg/kg | TM61/PM0 |
| Phenol | <0.01 | <0.01 | <0.01 | <0.01 | <0.01 | <0.01 | <0.01 | <0.01 | <0.01 | | <0.01 | mg/l | TM26/PM0 |
| Phenol | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 | | <0.1 | mg/kg | TM26/PM0 |
| Fluoride | <0.3 | 0.3 | <0.3 | 0.4 | 0.4 | <0.3 | <0.3 | <0.3 | <0.3 | | <0.3 | mg/l | TM173/PM0 |
| Fluoride | <3 | 3 | <3 | 4 | 4 | <3 | <3 | <3 | <3 | | <3 | mg/kg | TM173/PM0 |
| Sulphate as SO4 [#] | 0.7 | 3.4 | 1.6 | 203.0 | 63.6 | <0.5 | 11.6 | 0.9 | <0.5 | | <0.5 | mg/l | TM38/PM0 |
| Sulphate as SO4 [#] | 7 | 34 | 16 | 2031 | 636 | <5 | 116 | 9 | <5 | | <5 | mg/kg | TM38/PM0 |
| Mass of raw test portion | 0.0914 | 0.0954 | 0.0942 | 0.0923 | 0.369 | 0.1394 | 0.0961 | 0.0922 | 0.096 | | | kg | NONE/PM17 |
| Chloride [#] | <0.3 | <0.3 | 0.4 | 0.7 | 17.8 | 2.4 | 0.8 | <0.3 | 0.3 | | <0.3 | mg/l | TM38/PM0 |
| Chloride [#] | <3 | <3 | 4 | 7 | 178 | 24 | 8 | <3 | 3 | | <3 | mg/kg | TM38/PM0 |
| Mass of dried test portion | 0.09 | 0.09 | 0.09 | 0.09 | 0.09 | 0.09 | 0.09 | 0.09 | 0.09 | | | kg | NONE/PM17 |
| Dissolved Organic Carbon | <2 | <2 | <2 | <2 | 35 | 9 | <2 | <2 | <2 | | <2 | mg/l | TM60/PM0 |
| Dissolved Organic Carbon | <20 | <20 | <20 | <20 | 350 | 90 | <20 | <20 | <20 | | <20 | mg/kg | TM60/PM0 |
| pH | 7.88 | 7.93 | 7.50 | 8.23 | 8.21 | 8.17 | 10.43 | 8.13 | 8.09 | | <0.01 | pH units | TM73/PM0 |

Please see attached notes for all abbreviations and acronyms

Element Materials Technology

Client Name: Ground Investigations Ireland
Reference: 13614-02-24
Location: Dyke Road, Galway
Contact: Mike Sutton
EMT Job No: 24/6996

Report : EN12457_2
Solids: V=60g VOC jar, J=250g glass jar, T=plastic tub

| EMT Sample No. | 1-4 | 5-8 | 9-12 | 33-36 | 37-40 | 41-44 | 45-48 | 49-52 | 57-60 | Please see attached notes for all abbreviations and acronyms | | | | | | |
|--------------------------------------|------------|------------|------------|------------|----------------------|------------|------------|------------|------------|--|-------|---------------------|-----------|---------|----------|--------------|
| Sample ID | ST01 | ST02 | ST03 | TP03 | TP03 | TP03 | TP04 | TP04 | TP05 | | | | | | | |
| Depth | 0.20-0.80 | 0.15-0.80 | 0.07-1.40 | 0.10-0.60 | 0.61-1.10 | 1.10-2.30 | 0.07-1.00 | 1.00-1.20 | 0.55-1.35 | | | | | | | |
| COC No / misc | | | | | | | | | | | | | | | | |
| Containers | V J T | V J T | V J T | V J T | V J T | V J T | V J T | V J T | V J T | | | | | | | |
| Sample Date | 11/04/2024 | 11/04/2024 | 12/04/2024 | 15/04/2024 | 15/04/2024 | 15/04/2024 | 15/04/2024 | 15/04/2024 | 15/04/2024 | | | | | | | |
| Sample Type | Soil | Soil | Soil | Soil | Soil | Soil | Soil | Soil | Soil | | | | | | | |
| Batch Number | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | | | | | | | |
| Date of Receipt | 24/04/2024 | 24/04/2024 | 24/04/2024 | 24/04/2024 | 24/04/2024 | 24/04/2024 | 24/04/2024 | 24/04/2024 | 24/04/2024 | | Inert | Stable Non-reactive | Hazardous | LOD LOR | Units | Method No. |
| Solid Waste Analysis | | | | | | | | | | | | | | | | |
| Total Organic Carbon # | 0.05 | 0.57 | 0.11 | 0.18 | 27.30 | 3.80 | 0.17 | 0.20 | 0.13 | | 3 | 5 | 6 | <0.02 | % | TM21/PM24 |
| Sum of BTEX | <0.025 | <0.025 | <0.025 | <0.025 | <0.025 ^{SV} | <0.025 | <0.025 | <0.025 | <0.025 | | 6 | - | - | <0.025 | mg/kg | TM36/PM12 |
| Sum of 7 PCBs # | <0.035 | <0.035 | <0.035 | <0.035 | <0.035 | <0.035 | <0.035 | <0.035 | <0.035 | | 1 | - | - | <0.035 | mg/kg | TM17/PM8 |
| Mineral Oil | <30 | <30 | 69 | <30 | 124 | 173 | <30 | <30 | <30 | | 500 | - | - | <30 | mg/kg | TM5/PM8/PM16 |
| PAH Sum of 6 # | <0.22 | <0.22 | <0.22 | <0.22 | 0.33 | <0.22 | <0.22 | <0.22 | 0.44 | | - | - | - | <0.22 | mg/kg | TM4/PM8 |
| PAH Sum of 17 | <0.64 | <0.64 | <0.64 | <0.64 | 0.99 | <0.64 | <0.64 | <0.64 | 0.67 | | 100 | - | - | <0.64 | mg/kg | TM4/PM8 |
| CEN 10:1 Leachate | | | | | | | | | | | | | | | | |
| Arsenic # | <0.025 | <0.025 | <0.025 | <0.025 | 0.068 | 0.039 | <0.025 | <0.025 | <0.025 | | 0.5 | 2 | 25 | <0.025 | mg/kg | TM30/PM17 |
| Barium # | <0.03 | <0.03 | <0.03 | <0.03 | 0.21 | 0.08 | <0.03 | <0.03 | <0.03 | | 20 | 100 | 300 | <0.03 | mg/kg | TM30/PM17 |
| Cadmium # | <0.005 | <0.005 | <0.005 | <0.005 | <0.005 | <0.005 | <0.005 | <0.005 | <0.005 | | 0.04 | 1 | 5 | <0.005 | mg/kg | TM30/PM17 |
| Chromium # | <0.015 | <0.015 | <0.015 | <0.015 | <0.015 | <0.015 | 0.036 | <0.015 | <0.015 | | 0.5 | 10 | 70 | <0.015 | mg/kg | TM30/PM17 |
| Copper # | <0.07 | <0.07 | <0.07 | <0.07 | <0.07 | <0.07 | <0.07 | <0.07 | <0.07 | | 2 | 50 | 100 | <0.07 | mg/kg | TM30/PM17 |
| Mercury # | <0.0001 | <0.0001 | <0.0001 | <0.0001 | <0.0001 | <0.0001 | 0.0001 | <0.0001 | <0.0001 | | 0.01 | 0.2 | 2 | <0.0001 | mg/kg | TM61/PM0 |
| Molybdenum # | <0.02 | 0.04 | <0.02 | <0.02 | 0.05 | 0.05 | <0.02 | <0.02 | <0.02 | | 0.5 | 10 | 30 | <0.02 | mg/kg | TM30/PM17 |
| Nickel # | <0.02 | <0.02 | <0.02 | <0.02 | <0.02 | <0.02 | <0.02 | <0.02 | <0.02 | | 0.4 | 10 | 40 | <0.02 | mg/kg | TM30/PM17 |
| Lead # | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | | 0.5 | 10 | 50 | <0.05 | mg/kg | TM30/PM17 |
| Antimony # | <0.02 | <0.02 | <0.02 | <0.02 | 0.07 | <0.02 | <0.02 | <0.02 | <0.02 | | 0.06 | 0.7 | 5 | <0.02 | mg/kg | TM30/PM17 |
| Selenium # | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | | 0.1 | 0.5 | 7 | <0.03 | mg/kg | TM30/PM17 |
| Zinc # | 0.03 | <0.03 | <0.03 | <0.03 | 0.06 | 0.04 | <0.03 | <0.03 | 0.03 | | 4 | 50 | 200 | <0.03 | mg/kg | TM30/PM17 |
| Total Dissolved Solids # | <350 | <350 | <350 | 3562 | 2761 | 1030 | 790 | 370 | 440 | | 4000 | 60000 | 100000 | <350 | mg/kg | TM20/PM0 |
| Dissolved Organic Carbon | <20 | <20 | <20 | <20 | 350 | 90 | <20 | <20 | <20 | | 500 | 800 | 1000 | <20 | mg/kg | TM60/PM0 |
| Mass of raw test portion | 0.0914 | 0.0954 | 0.0942 | 0.0923 | 0.369 | 0.1394 | 0.0961 | 0.0922 | 0.096 | | - | - | - | | kg | NONE/PM17 |
| Dry Matter Content Ratio | 98.6 | 94.4 | 95.6 | 97.4 | 24.4 | 64.4 | 93.7 | 97.3 | 93.4 | | - | - | - | <0.1 | % | NONE/PM4 |
| Leachant Volume | 0.899 | 0.895 | 0.896 | 0.898 | 0.622 | 0.85 | 0.894 | 0.897 | 0.894 | | - | - | - | | l | NONE/PM17 |
| Moisture Content 105C (% Dry Weight) | 1.4 | 5.9 | 4.6 | 2.7 | 309.2 | 55.4 | 6.8 | 2.8 | 7.0 | | - | - | - | <0.1 | % | PM4/PM0 |
| pH # | 9.43 | 8.81 | 8.92 | 8.19 | 7.14 | 8.41 | 9.59 | 8.82 | 8.82 | | - | - | - | <0.01 | pH units | TM73/PM11 |
| Phenol | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 | | 1 | - | - | <0.1 | mg/kg | TM26/PM0 |
| Fluoride | <3 | 3 | <3 | 4 | 4 | <3 | <3 | <3 | <3 | | 10 | 150 | 500 | <3 | mg/kg | TM173/PM0 |
| Sulphate as SO4 # | 7 | 34 | 16 | 2031 | 636 | <5 | 116 | 9 | <5 | | 1000 | 20000 | 50000 | <5 | mg/kg | TM38/PM0 |
| Chloride # | <3 | <3 | 4 | 7 | 178 | 24 | 8 | <3 | 3 | | 800 | 15000 | 25000 | <3 | mg/kg | TM38/PM0 |

NOTES TO ACCOMPANY ALL SCHEDULES AND REPORTS

EMT Job No.: 24/6996

SOILS and ASH

Please note we are only MCERTS accredited (UK soils only) for sand, loam and clay and any other matrix is outside our scope of accreditation.

Where an MCERTS report has been requested, you will be notified within 48 hours of any samples that have been identified as being outside our MCERTS scope. As validation has been performed on clay, sand and loam, only samples that are predominantly these matrices, or combinations of them will be within our MCERTS scope. If samples are not one of a combination of the above matrices they will not be marked as MCERTS accredited.

It is assumed that you have taken representative samples on site and require analysis on a representative subsample. Stones will generally be included unless we are requested to remove them.

All samples will be discarded one month after the date of reporting, unless we are instructed to the contrary. Asbestos samples are retained for 6 months.

If you have not already done so, please send us a purchase order if this is required by your company.

Where appropriate please make sure that our detection limits are suitable for your needs, if they are not, please notify us immediately.

All analysis is reported on a dry weight basis unless stated otherwise. Limits of detection for analyses carried out on as received samples are not moisture content corrected. Results are not surrogate corrected. Samples are dried at 35°C ±5°C unless otherwise stated. Moisture content for CEN Leachate tests are dried at 105°C ±5°C. Ash samples are dried at 35°C ±5°C.

Where Mineral Oil is quoted, this refers to Total Aliphatics C10-C40.

Where a CEN 10:1 ZERO Headspace VOC test has been carried out, a 10:1 ratio of water to wet (as received) soil has been used.

% Asbestos in Asbestos Containing Materials (ACMs) is determined by reference to HSG 264 The Survey Guide - Appendix 2 : ACMs in buildings listed in order of ease of fibre release.

Sufficient amount of sample must be received to carry out the testing specified. Where an insufficient amount of sample has been received the testing may not meet the requirements of our accredited methods, as such accreditation may be removed.

Negative Neutralization Potential (NP) values are obtained when the volume of NaOH (0.1N) titrated (pH 8.3) is greater than the volume of HCl (1N) to reduce the pH of the sample to 2.0 - 2.5. Any negative NP values are corrected to 0.

The calculation of Pyrite content assumes that all oxidisable sulphides present in the sample are pyrite. This may not be the case. The calculation may be an overestimate when other sulphides such as Barite (Barium Sulphate) are present.

WATERS

Please note we are not a UK Drinking Water Inspectorate (DWI) Approved Laboratory .

ISO17025 accreditation applies to surface water and groundwater and usually one other matrix which is analysis specific, any other liquids are outside our scope of accreditation.

As surface waters require different sample preparation to groundwaters the laboratory must be informed of the water type when submitting samples.

Where Mineral Oil is quoted, this refers to Total Aliphatics C10-C40.

STACK EMISSIONS

Where an MCERTS report has been requested, you will be notified within 48 hours of any samples that have been identified as being outside our MCERTS scope. As validation for Dioxins and Furans and Dioxin like PCBs has been performed on XAD-2 Resin, only samples which use this resin will be within our MCERTS scope.

Where appropriate please make sure that our detection limits are suitable for your needs, if they are not, please notify us immediately.

DEVIATING SAMPLES

All samples should be submitted to the laboratory in suitable containers with sufficient ice packs to sustain an appropriate temperature for the requested analysis. The temperature of sample receipt is recorded on the confirmation schedules in order that the client can make an informed decision as to whether testing should still be undertaken.

SURROGATES

Surrogate compounds are added during the preparation process to monitor recovery of analytes. However low recovery in soils is often due to peat, clay or other organic rich matrices. For waters this can be due to oxidants, surfactants, organic rich sediments or remediation fluids. Acceptable limits for most organic methods are 70 - 130% and for VOCs are 50 - 150%. When surrogate recoveries are outside the performance criteria but the associated AQC passes this is assumed to be due to matrix effect. Results are not surrogate corrected.

DILUTIONS

A dilution suffix indicates a dilution has been performed and the reported result takes this into account. No further calculation is required.

BLANKS

Where analytes have been found in the blank, the sample will be treated in accordance with our laboratory procedure for dealing with contaminated blanks.

NOTE

Data is only reported if the laboratory is confident that the data is a true reflection of the samples analysed. Data is only reported as accredited when all the requirements of our Quality System have been met. In certain circumstances where all the requirements of the Quality System have not been met, for instance if the associated AQC has failed, the reason is fully investigated and documented. The sample data is then evaluated alongside the other quality control checks performed during analysis to determine its suitability. Following this evaluation, provided the sample results have not been effected, the data is reported but accreditation is removed. It is a requirement of our Accreditation Body for data not reported as accredited to be considered indicative only, but this does not mean the data is not valid.

Where possible, and if requested, samples will be re-extracted and a revised report issued with accredited results. Please do not hesitate to contact the laboratory if further details are required of the circumstances which have led to the removal of accreditation.

Laboratory records are kept for a period of no less than 6 years.

REPORTS FROM THE SOUTH AFRICA LABORATORY

Any method number not prefixed with SA has been undertaken in our UK laboratory unless reported as subcontracted.

Measurement Uncertainty

Measurement uncertainty defines the range of values that could reasonably be attributed to the measured quantity. This range of values has not been included within the reported results. Uncertainty expressed as a percentage can be provided upon request.

Customer Provided Information

Sample ID and depth is information provided by the customer.

Age of Diesel

The age of release estimation is based on the nC17/pristane ratio only as prescribed by Christensen and Larsen (1993) and Kaplan, Galperin, Alimi et al., (1996).

Age estimation should be treated with caution as it can be influenced by site specific factors of which the laboratory are not aware.

Tentatively Identified Compounds (TICs)

Where Tentatively Identified Compounds (TICs) are reported, up to 10 Tentatively Identified Compounds will be listed where there is found to be a greater than 80% match with the NIST library. The reported concentration is determined semi-quantitatively, with a matrix specific limit of detection.

Note, other compounds may be present but are not reported.

ABBREVIATIONS and ACRONYMS USED

| | |
|---------|--|
| # | ISO17025 (UKAS Ref No. 4225) accredited - UK. |
| SA | ISO17025 (SANAS Ref No.T0729) accredited - South Africa |
| B | Indicates analyte found in associated method blank. |
| DR | Dilution required. |
| M | MCERTS accredited. |
| NA | Not applicable |
| NAD | No Asbestos Detected. |
| ND | None Detected (usually refers to VOC and/SVOC TICs). |
| NDP | No Determination Possible |
| SS | Calibrated against a single substance |
| SV | Surrogate recovery outside performance criteria. This may be due to a matrix effect. |
| W | Results expressed on as received basis. |
| + | AQC failure, accreditation has been removed from this result, if appropriate, see 'Note' on previous page. |
| >> | Results above quantitative calibration range. The result should be considered the minimum value and is indicative only. The actual result could be significantly higher. |
| * | Analysis subcontracted to an Element Materials Technology approved laboratory. |
| CO | Suspected carry over |
| LOD/LOR | Limit of Detection (Limit of Reporting) in line with ISO 17025 and MCERTS |
| ME | Matrix Effect |
| NFD | No Fibres Detected |
| BS | AQC Sample |
| LB | Blank Sample |
| N | Client Sample |
| TB | Trip Blank Sample |
| OC | Outside Calibration Range |
| | |

HWOL ACRONYMS AND OPERATORS USED

| | |
|-------|--|
| HS | Headspace Analysis. |
| EH | Extractable Hydrocarbons - i.e. everything extracted by the solvent. |
| CU | Clean-up - e.g. by florisil, silica gel. |
| 1D | GC - Single coil gas chromatography. |
| Total | Aliphatics & Aromatics. |
| AL | Aliphatics only. |
| AR | Aromatics only. |
| 2D | GC-GC - Double coil gas chromatography. |
| #1 | EH_Total but with humics mathematically subtracted |
| #2 | EU_Total but with fatty acids mathematically subtracted |
| _ | Operator - underscore to separate acronyms (exception for +). |
| + | Operator to indicate cumulative e.g. EH+HS_Total or EH_CU+HS_Total |
| MS | Mass Spectrometry. |

EMT Job No: 24/6996

| Test Method No. | Description | Prep Method No. (if appropriate) | Description | ISO 17025 (UKAS/S ANAS) | MCERTS (UK soils only) | Analysis done on As Received (AR) or Dried (AD) | Reported on dry weight basis |
|-----------------|--|----------------------------------|---|-------------------------|------------------------|---|------------------------------|
| PM4 | Gravimetric measurement of Natural Moisture Content and % Moisture Content at either 35°C or 105°C. Calculation based on ISO 11465:1993(E) and BS1377-2:1990. | PM0 | No preparation is required. | | | AR | |
| TM4 | Modified USEPA 8270D v5:2014 method for the solvent extraction and determination of PAHs by GC-MS. | PM8 | End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required. | | | AR | Yes |
| TM4 | Modified USEPA 8270D v5:2014 method for the solvent extraction and determination of PAHs by GC-MS. | PM8 | End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required. | Yes | | AR | Yes |
| TM5 | Modified 8015B v2:1996 method for the determination of solvent Extractable Petroleum Hydrocarbons (EPH) within the range C8-C40 by GCFID. For waters the solvent extracts dissolved phase plus a sheen if present. | PM16 | Fractionation into aliphatic and aromatic fractions using a Rapid Trace SPE. | | | AR | |
| TM5 | Modified 8015B v2:1996 method for the determination of solvent Extractable Petroleum Hydrocarbons (EPH) within the range C8-C40 by GCFID. For waters the solvent extracts dissolved phase plus a sheen if present. | PM8/PM16 | End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required/Fractionation into aliphatic and aromatic fractions using a Rapid Trace SPE. | | | AR | Yes |
| TM5 | Modified 8015B v2:1996 method for the determination of solvent Extractable Petroleum Hydrocarbons (EPH) within the range C8-C40 by GCFID. For waters the solvent extracts dissolved phase plus a sheen if present. | PM8/PM16 | End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required/Fractionation into aliphatic and aromatic fractions using a Rapid Trace SPE. | Yes | | AR | Yes |
| TM5/TM36 | please refer to TM5 and TM36 for method details | PM8/PM12/PM16 | please refer to PM8/PM16 and PM12 for method details | | | AR | Yes |
| TM17 | Modified US EPA method 8270D v5:2014. Determination of specific Polychlorinated Biphenyl congeners by GC-MS. | PM8 | End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required. | Yes | | AR | Yes |
| TM20 | Modified BS 1377-3:1990/USEPA 160.1/3 (TDS/TS: 1971) Gravimetric determination of Total Dissolved Solids/Total Solids | PM0 | No preparation is required. | Yes | | AR | Yes |
| TM21 | Modified BS 7755-3:1995, ISO10694:1995 Determination of Total Organic Carbon or Total Carbon by combustion in an Eltra TOC furnace/analyser in the presence of oxygen. The CO2 generated is quantified using infra-red detection. Organic Matter (SOM) calculated as per EA MCERTS Chemical Testing of Soil. | PM24 | Preparation of Soil and Marine Sediment Samples for Total Organic Carbon. | Yes | | AD | Yes |

EMT Job No: 24/6996

| Test Method No. | Description | Prep Method No. (if appropriate) | Description | ISO 17025 (UKAS/S ANAS) | MCERTS (UK soils only) | Analysis done on As Received (AR) or Dried (AD) | Reported on dry weight basis |
|-----------------|--|----------------------------------|---|-------------------------|------------------------|---|------------------------------|
| TM26 | Determination of phenols by Reversed Phased High Performance Liquid Chromatography and Electro-Chemical Detection. | PM0 | No preparation is required. | | | AR | Yes |
| TM30 | Determination of Trace Metals by ICP-OES (Inductively Coupled Plasma – Optical Emission Spectrometry): WATERS by Modified USEPA Method 200.7, Rev. 4.4, 1994; Modified EPA Method 6010B, Rev.2, Dec 1996; Modified BS EN ISO 11885:2009: SOILS by Modified USEP 6010B, Rev.2, Dec.1996; Modified EPA Method 3050B, Rev.2, Dec.1996 | PM15 | Acid digestion of dried and ground solid samples using Aqua Regia refluxed at 112.5 °C. Samples containing asbestos are not dried and ground. | | | AD | Yes |
| TM30 | Determination of Trace Metals by ICP-OES (Inductively Coupled Plasma – Optical Emission Spectrometry): WATERS by Modified USEPA Method 200.7, Rev. 4.4, 1994; Modified EPA Method 6010B, Rev.2, Dec 1996; Modified BS EN ISO 11885:2009: SOILS by Modified USEP 6010B, Rev.2, Dec.1996; Modified EPA Method 3050B, Rev.2, Dec.1996 | PM15 | Acid digestion of dried and ground solid samples using Aqua Regia refluxed at 112.5 °C. Samples containing asbestos are not dried and ground. | Yes | | AD | Yes |
| TM30 | Determination of Trace Metals by ICP-OES (Inductively Coupled Plasma – Optical Emission Spectrometry): WATERS by Modified USEPA Method 200.7, Rev. 4.4, 1994; Modified EPA Method 6010B, Rev.2, Dec 1996; Modified BS EN ISO 11885:2009: SOILS by Modified USEP 6010B, Rev.2, Dec.1996; Modified EPA Method 3050B, Rev.2, Dec.1996 | PM17 | Modified method BS EN12457-2:2002 As received solid samples are leached with water in a 10:1 water to soil ratio for 24 hours, the moisture content of the sample is included in the ratio. | Yes | | AR | Yes |
| TM30 | Determination of Trace Metals by ICP-OES (Inductively Coupled Plasma – Optical Emission Spectrometry): WATERS by Modified USEPA Method 200.7, Rev. 4.4, 1994; Modified EPA Method 6010B, Rev.2, Dec 1996; Modified BS EN ISO 11885:2009: SOILS by Modified USEP 6010B, Rev.2, Dec.1996; Modified EPA Method 3050B, Rev.2, Dec.1996 | PM20 | Extraction of dried and ground or as received samples with deionised water in a 2:1 water to solid ratio using a reciprocal shaker for all analytes except hexavalent chromium. Extraction of as received sample using 10:1 ratio of 0.2M sodium hydroxide to soil for hexavalent chromium using a reciprocal shaker. | | | AD | Yes |
| TM36 | Modified US EPA method 8015B v2:1996. Determination of Gasoline Range Organics (GRO) in the carbon chain range of C4-12 by headspace GC-FID. MTBE by GC/FID co-elutes with 3-methylpentane if present and therefore can give a false positive. Positive MTBE results will be re-run using GC-MS to double check, when requested. | PM12 | Modified US EPA method 5021A v2:2014. Preparation of solid and liquid samples for GC headspace analysis. | | | AR | Yes |
| TM36 | Modified US EPA method 8015B v2:1996. Determination of Gasoline Range Organics (GRO) in the carbon chain range of C4-12 by headspace GC-FID. MTBE by GC/FID co-elutes with 3-methylpentane if present and therefore can give a false positive. Positive MTBE results will be re-run using GC-MS to double check, when requested. | PM12 | Modified US EPA method 5021A v2:2014. Preparation of solid and liquid samples for GC headspace analysis. | Yes | | AR | Yes |
| TM38 | Soluble Ion analysis using Discrete Analyser. Modified US EPA methods: Chloride 325.2 (1978), Sulphate 375.4 (Rev.2 1993), o-Phosphate 365.2 (Rev.2 1993), TON 353.1 (Rev.2 1993), Nitrite 354.1 (1971), Hex Cr 7196A (1992), NH4+ 350.1 (Rev.2 1993) – All anions comparable to BS ISO 15923-1: 2013 | PM0 | No preparation is required. | Yes | | AR | Yes |
| TM38 | Soluble Ion analysis using Discrete Analyser. Modified US EPA methods: Chloride 325.2 (1978), Sulphate 375.4 (Rev.2 1993), o-Phosphate 365.2 (Rev.2 1993), TON 353.1 (Rev.2 1993), Nitrite 354.1 (1971), Hex Cr 7196A (1992), NH4+ 350.1 (Rev.2 1993) – All anions comparable to BS ISO 15923-1: 2013 | PM20 | Extraction of dried and ground or as received samples with deionised water in a 2:1 water to solid ratio using a reciprocal shaker for all analytes except hexavalent chromium. Extraction of as received sample using 10:1 ratio of 0.2M sodium hydroxide to soil for hexavalent chromium using a reciprocal shaker. | | | AD | Yes |
| TM38 | Soluble Ion analysis using Discrete Analyser. Modified US EPA methods: Chloride 325.2 (1978), Sulphate 375.4 (Rev.2 1993), o-Phosphate 365.2 (Rev.2 1993), TON 353.1 (Rev.2 1993), Nitrite 354.1 (1971), Hex Cr 7196A (1992), NH4+ 350.1 (Rev.2 1993) – All anions comparable to BS ISO 15923-1: 2013 | PM20 | Extraction of dried and ground or as received samples with deionised water in a 2:1 water to solid ratio using a reciprocal shaker for all analytes except hexavalent chromium. Extraction of as received sample using 10:1 ratio of 0.2M sodium hydroxide to soil for hexavalent chromium using a reciprocal shaker. | Yes | | AD | Yes |

EMT Job No: 24/6996

| Test Method No. | Description | Prep Method No. (if appropriate) | Description | ISO 17025 (UKAS/S ANAS) | MCERTS (UK soils only) | Analysis done on As Received (AR) or Dried (AD) | Reported on dry weight basis |
|-----------------|---|----------------------------------|---|-------------------------|------------------------|---|------------------------------|
| TM38 | Soluble Ion analysis using Discrete Analyser. Modified US EPA methods: Chloride 325.2 (1978), Sulphate 375.4 (Rev.2 1993), o-Phosphate 365.2 (Rev.2 1993), TON 353.1 (Rev.2 1993), Nitrite 354.1 (1971), Hex Cr 7196A (1992), NH4+ 350.1 (Rev.2 1993) – All anions comparable to BS ISO 15923-1: 2013 | PM20 | Extraction of dried and ground or as received samples with deionised water in a 2:1 water to solid ratio using a reciprocal shaker for all analytes except hexavalent chromium. Extraction of as received sample using 10:1 ratio of 0.2M sodium hydroxide to soil for hexavalent chromium using a reciprocal shaker. | Yes | | AR | Yes |
| TM50 | Acid soluble sulphate (Total Sulphate) analysed by ICP-OES | PM29 | A hot hydrochloric acid digest is performed on a dried and ground sample, and the resulting liquor is analysed. | | | AD | Yes |
| TM60 | TC/TOC analysis of Waters by High Temperature Combustion followed by NDIR detection. Based on the following modified standard methods: USEPA 9060A (2002), APHA SMEWW 5310B:1999 22nd Edition, ASTM D 7573, and USEPA 415.1. | PM0 | No preparation is required. | | | AR | Yes |
| TM61 | Determination of Mercury by Cold Vapour Atomic Fluorescence - WATERS: Modified USEPA Method 245.7, Rev 2, Feb 2005. SOILS: Modified USEPA Method 7471B, Rev.2, Feb 2007 | PM0 | No preparation is required. | Yes | | AR | Yes |
| TM73 | Modified US EPA methods 150.1 (1982) and 9045D Rev. 4 - 2004) and BS1377-3:1990. Determination of pH by Metrohm automated probe analyser. | PM0 | No preparation is required. | | | AR | Yes |
| TM73 | Modified US EPA methods 150.1 (1982) and 9045D Rev. 4 - 2004) and BS1377-3:1990. Determination of pH by Metrohm automated probe analyser. | PM11 | Extraction of as received solid samples using one part solid to 2.5 parts deionised water. | Yes | | AR | No |
| TM173 | Analysis of fluoride by ISE (Ion Selective Electrode) using modified ISE method 9214 - 340.2 (EPA 1998) | PM0 | No preparation is required. | | | AR | Yes |
| NONE | No Method Code | NONE | No Method Code | | | AD | Yes |
| NONE | No Method Code | PM17 | Modified method BS EN12457-2:2002 As received solid samples are leached with water in a 10:1 water to soil ratio for 24 hours, the moisture content of the sample is included in the ratio. | | | | |
| NONE | No Method Code | PM4 | Gravimetric measurement of Natural Moisture Content and % Moisture Content at either 35°C or 105°C. Calculation based on ISO 11465:1993(E) and BS1377-2:1990. | | | AR | |

Ground Investigations Ireland
Catherinstown House
Hazelhatch Road
Newcastle
Co. Dublin
Ireland
D22 K5P8



Attention : Mike Sutton
Date : 21st May, 2024
Your reference : 13614-02-24
Our reference : Test Report 24/7795 Batch 1
Location : Dykes Road Galway
Date samples received : 8th May, 2024
Status : Final Report
Issue : 202405211146

Fifteen samples were received for analysis on 8th May, 2024 of which fifteen were scheduled for analysis. Please find attached our Test Report which should be read with notes at the end of the report and should include all sections if reproduced. Interpretations and opinions are outside the scope of any accreditation, and all results relate only to samples supplied.

All analysis is carried out on as received samples and reported on a dry weight basis unless stated otherwise. Results are not surrogate corrected.

The greenhouse gas emissions generated (in Carbon – Co2e) to obtain the results in this report are estimated as:

Scope 1&2 emissions - 69.721 kg of CO2

Scope 1&2&3 emissions - 164.769 kg of CO2

Authorised By:



Bruce Leslie
Project Manager

Please include all sections of this report if it is reproduced

Element Materials Technology

Client Name: Ground Investigations Ireland
Reference: 13614-02-24
Location: Dykes Road Galway
Contact: Mike Sutton
EMT Job No: 24/7795

Report : Solid

Solids: V=60g VOC jar, J=250g glass jar, T=plastic tub

| EMT Sample No. | 1-4 | 5-8 | 9-12 | 13-16 | 17-20 | 21-24 | 25-28 | 29-32 | 33-36 | 37-40 | LOD/LOR | Units | Method No. | | | |
|-------------------------------------|------------|------------|------------|------------|------------|------------|------------|------------|------------|------------|--|-------|--------------|-------|-------|-----------|
| | Sample ID | BH01 | BH01 | BH04 | BH05 | BH05 | BH05 | BRC01 | BRC01 | BRC01 | | | | BRC02 | | |
| Depth | 0.50 | 1.50 | 1.50 | 0.25 | 0.50 | 1.50 | 0.50 | 1.50 | 3.50 | 0.50 | Please see attached notes for all abbreviations and acronyms | | | | | |
| COC No / misc | | | | | | | | | | | | | | | | |
| Containers | V J T | V J T | V J T | V J T | V J T | V J T | V J T | V J T | V J T | V J T | | | | | | |
| Sample Date | 06/05/2024 | 06/05/2024 | 06/05/2024 | 06/05/2024 | 06/05/2024 | 06/05/2024 | 06/05/2024 | 06/05/2024 | 06/05/2024 | 06/05/2024 | | | | | | |
| Sample Type | Soil | | | | | | |
| Batch Number | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | | | | | | |
| Date of Receipt | 08/05/2024 | 08/05/2024 | 08/05/2024 | 08/05/2024 | 08/05/2024 | 08/05/2024 | 08/05/2024 | 08/05/2024 | 08/05/2024 | 08/05/2024 | | | | | | |
| Antimony | 2 | 4 | 3 | <1 | <1 | <1 | <1 | <1 | <1 | <1 | | | | <1 | mg/kg | TM30/PM15 |
| Arsenic # | 8.5 | 15.0 | 1.4 | 1.6 | 2.2 | 2.3 | 1.2 | 2.9 | 1.2 | 1.8 | | | | <0.5 | mg/kg | TM30/PM15 |
| Barium # | 96 | 209 | 69 | 8 | 28 | 25 | 8 | 21 | 103 | 8 | | | | <1 | mg/kg | TM30/PM15 |
| Cadmium # | 0.9 | 0.5 | <0.1 | 0.2 | 0.6 | 0.5 | 0.2 | 0.5 | 0.2 | 0.4 | <0.1 | mg/kg | TM30/PM15 | | | |
| Chromium # | 28.1 | 42.9 | 172.7 | 11.6 | 19.2 | 22.6 | 15.5 | 23.2 | 19.0 | 9.5 | <0.5 | mg/kg | TM30/PM15 | | | |
| Copper # | 43 | 77 | 4 | 4 | 8 | 8 | 4 | 13 | 8 | 5 | <1 | mg/kg | TM30/PM15 | | | |
| Lead # | 97 | 184 | <5 | <5 | 9 | 8 | <5 | 14 | <5 | <5 | <5 | mg/kg | TM30/PM15 | | | |
| Mercury # | 0.2 | 0.5 | <0.1 | <0.1 | 0.1 | 0.1 | <0.1 | 0.3 | <0.1 | <0.1 | <0.1 | mg/kg | TM30/PM15 | | | |
| Molybdenum # | 3.0 | 4.3 | 2.1 | 1.0 | 1.0 | 1.2 | 2.1 | 1.7 | 1.2 | 0.8 | <0.1 | mg/kg | TM30/PM15 | | | |
| Nickel # | 15.1 | 31.3 | 75.3 | 5.9 | 8.3 | 8.3 | 6.9 | 9.9 | 5.3 | 8.7 | <0.7 | mg/kg | TM30/PM15 | | | |
| Selenium # | 3 | <1 | 2 | <1 | <1 | <1 | <1 | 1 | <1 | <1 | <1 | mg/kg | TM30/PM15 | | | |
| Zinc # | 152 | 232 | 90 | <5 | 21 | 19 | 6 | 20 | 12 | 9 | <5 | mg/kg | TM30/PM15 | | | |
| PAH MS | | | | | | | | | | | | | | | | |
| Naphthalene # | <0.04 | 0.27 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | mg/kg | TM4/PM8 | | | |
| Acenaphthylene | <0.03 | 0.55 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | mg/kg | TM4/PM8 | | | |
| Acenaphthene # | <0.05 | 0.07 | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | mg/kg | TM4/PM8 | | | |
| Fluorene # | <0.04 | 0.21 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | mg/kg | TM4/PM8 | | | |
| Phenanthrene # | 0.15 | 1.43 | <0.03 | 0.08 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | mg/kg | TM4/PM8 | | | |
| Anthracene # | <0.04 | 0.78 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | mg/kg | TM4/PM8 | | | |
| Fluoranthene # | 0.57 | 5.19 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | 0.18 | <0.03 | <0.03 | <0.03 | mg/kg | TM4/PM8 | | | |
| Pyrene # | 0.64 | 4.73 | <0.03 | 0.05 | <0.03 | <0.03 | <0.03 | 0.14 | <0.03 | 0.05 | <0.03 | mg/kg | TM4/PM8 | | | |
| Benzo(a)anthracene # | 0.35 | 2.82 | <0.06 | <0.06 | <0.06 | <0.06 | <0.06 | 0.14 | <0.06 | <0.06 | <0.06 | mg/kg | TM4/PM8 | | | |
| Chrysene # | 0.38 | 3.01 | <0.02 | 0.15 | <0.02 | <0.02 | 0.05 | 0.11 | <0.02 | 0.08 | <0.02 | mg/kg | TM4/PM8 | | | |
| Benzo(bk)fluoranthene # | 0.92 | 5.80 | <0.07 | <0.07 | <0.07 | <0.07 | <0.07 | 0.20 | <0.07 | <0.07 | <0.07 | mg/kg | TM4/PM8 | | | |
| Benzo(a)pyrene # | 0.60 | 3.70 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | 0.11 | <0.04 | <0.04 | <0.04 | mg/kg | TM4/PM8 | | | |
| Indeno(123cd)pyrene # | 0.46 | 2.50 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | mg/kg | TM4/PM8 | | | |
| Dibenzo(ah)anthracene # | 0.08 | 0.60 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | mg/kg | TM4/PM8 | | | |
| Benzo(ghi)perylene # | 0.49 | 2.49 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | mg/kg | TM4/PM8 | | | |
| Coronene | 0.10 | 0.39 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | <0.04 | mg/kg | TM4/PM8 | | | |
| PAH 6 Total # | 3.04 | 19.68 | <0.22 | <0.22 | <0.22 | <0.22 | <0.22 | 0.49 | <0.22 | <0.22 | <0.22 | mg/kg | TM4/PM8 | | | |
| PAH 17 Total | 4.74 | 34.54 | <0.64 | <0.64 | <0.64 | <0.64 | <0.64 | 0.88 | <0.64 | <0.64 | <0.64 | mg/kg | TM4/PM8 | | | |
| Benzo(b)fluoranthene | 0.66 | 4.18 | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | 0.14 | <0.05 | <0.05 | <0.05 | mg/kg | TM4/PM8 | | | |
| Benzo(k)fluoranthene | 0.26 | 1.62 | <0.02 | <0.02 | <0.02 | <0.02 | <0.02 | 0.06 | <0.02 | <0.02 | <0.02 | mg/kg | TM4/PM8 | | | |
| Benzo(j)fluoranthene | <1 | <1 | <1 | <1 | <1 | <1 | <1 | <1 | <1 | <1 | <1 | mg/kg | TM4/PM8 | | | |
| PAH Surrogate % Recovery | 100 | 97 | 99 | 99 | 95 | 99 | 99 | 99 | 98 | 100 | <0 | % | TM4/PM8 | | | |
| Mineral Oil (C10-C40) (EH_CU_1D_AL) | 88 | 344 | <30 | 1047 | <30 | <30 | 129 | 52 | <30 | 296 | <30 | mg/kg | TM5/PM8/PM16 | | | |

Element Materials Technology

Client Name: Ground Investigations Ireland
Reference: 13614-02-24
Location: Dykes Road Galway
Contact: Mike Sutton
EMT Job No: 24/7795

Report : Solid

Solids: V=60g VOC jar, J=250g glass jar, T=plastic tub

| EMT Sample No. | 1-4 | 5-8 | 9-12 | 13-16 | 17-20 | 21-24 | 25-28 | 29-32 | 33-36 | 37-40 | Please see attached notes for all abbreviations and acronyms | | |
|---|--------------------|--------------------|------------|--------------------|------------|------------|--------------------|--------------------|------------|------------|--|-------|------------------------|
| Sample ID | BH01 | BH01 | BH04 | BH05 | BH05 | BH05 | BRC01 | BRC01 | BRC01 | BRC02 | | | |
| Depth | 0.50 | 1.50 | 1.50 | 0.25 | 0.50 | 1.50 | 0.50 | 1.50 | 3.50 | 0.50 | | | |
| COC No / misc | | | | | | | | | | | | | |
| Containers | V J T | V J T | V J T | V J T | V J T | V J T | V J T | V J T | V J T | V J T | | | |
| Sample Date | 06/05/2024 | 06/05/2024 | 06/05/2024 | 06/05/2024 | 06/05/2024 | 06/05/2024 | 06/05/2024 | 06/05/2024 | 06/05/2024 | 06/05/2024 | | | |
| Sample Type | Soil | Soil | Soil | Soil | Soil | Soil | Soil | Soil | Soil | Soil | | | |
| Batch Number | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | | | |
| Date of Receipt | 08/05/2024 | 08/05/2024 | 08/05/2024 | 08/05/2024 | 08/05/2024 | 08/05/2024 | 08/05/2024 | 08/05/2024 | 08/05/2024 | 08/05/2024 | LOD/LOR | Units | Method No. |
| TPH CWG | | | | | | | | | | | | | |
| Aliphatics | | | | | | | | | | | | | |
| >C5-C6 (HS_1D_AL) # | <0.1 ^{SV} | <0.1 ^{SV} | <0.1 | <0.1 ^{SV} | <0.1 | <0.1 | <0.1 ^{SV} | <0.1 ^{SV} | <0.1 | <0.1 | <0.1 | mg/kg | TM36/PM12 |
| >C6-C8 (HS_1D_AL) # | <0.1 ^{SV} | <0.1 ^{SV} | <0.1 | <0.1 ^{SV} | <0.1 | <0.1 | <0.1 ^{SV} | <0.1 ^{SV} | <0.1 | <0.1 | <0.1 | mg/kg | TM36/PM12 |
| >C8-C10 (HS_1D_AL) | <0.1 ^{SV} | <0.1 ^{SV} | <0.1 | 0.2 ^{SV} | <0.1 | <0.1 | <0.1 ^{SV} | <0.1 ^{SV} | <0.1 | <0.1 | <0.1 | mg/kg | TM36/PM12 |
| >C10-C12 (EH_CU_1D_AL) # | <0.2 | <0.2 | <0.2 | <0.2 | <0.2 | <0.2 | <0.2 | <0.2 | <0.2 | <0.2 | <0.2 | mg/kg | TMS/PM8/PM16 |
| >C12-C16 (EH_CU_1D_AL) # | <4 | <4 | <4 | <4 | <4 | <4 | <4 | <4 | <4 | <4 | <4 | mg/kg | TMS/PM8/PM16 |
| >C16-C21 (EH_CU_1D_AL) # | <7 | 22 | <7 | 61 | <7 | <7 | <7 | <7 | 16 | 21 | <7 | mg/kg | TMS/PM8/PM16 |
| >C21-C35 (EH_CU_1D_AL) # | 88 | 300 | <7 | 887 | 19 | 29 | 118 | 52 | <7 | 246 | <7 | mg/kg | TMS/PM8/PM16 |
| >C35-C40 (EH_CU_1D_AL) | <7 | 22 | <7 | 99 | <7 | <7 | 11 | <7 | <7 | 29 | <7 | mg/kg | TMS/PM8/PM16 |
| Total aliphatics C5-40 (EH_CU+HS_1D_AL) | 88 | 344 | <26 | 1047 | <26 | 29 | 129 | 52 | <26 | 296 | <26 | mg/kg | TMS/PM8/PM16/PM12/PM15 |
| >C6-C10 (HS_1D_AL) | <0.1 ^{SV} | <0.1 ^{SV} | <0.1 | 0.2 ^{SV} | <0.1 | <0.1 | <0.1 ^{SV} | <0.1 ^{SV} | <0.1 | <0.1 | <0.1 | mg/kg | TM36/PM12 |
| >C10-C25 (EH_CU_1D_AL) | <10 | 76 | <10 | 198 | <10 | <10 | 10 | <10 | 23 | 58 | <10 | mg/kg | TMS/PM8/PM16 |
| >C25-C35 (EH_CU_1D_AL) | 76 | 246 | <10 | 753 | 19 | 24 | 108 | 45 | <10 | 209 | <10 | mg/kg | TMS/PM8/PM16 |
| Aromatics | | | | | | | | | | | | | |
| >C5-EC7 (HS_1D_AR) # | <0.1 ^{SV} | <0.1 ^{SV} | <0.1 | <0.1 ^{SV} | <0.1 | <0.1 | <0.1 ^{SV} | <0.1 ^{SV} | <0.1 | <0.1 | <0.1 | mg/kg | TM36/PM12 |
| >EC7-EC8 (HS_1D_AR) # | <0.1 ^{SV} | <0.1 ^{SV} | <0.1 | <0.1 ^{SV} | <0.1 | <0.1 | <0.1 ^{SV} | <0.1 ^{SV} | <0.1 | <0.1 | <0.1 | mg/kg | TM36/PM12 |
| >EC8-EC10 (HS_1D_AR) # | <0.1 ^{SV} | <0.1 ^{SV} | <0.1 | <0.1 ^{SV} | <0.1 | <0.1 | <0.1 ^{SV} | <0.1 ^{SV} | <0.1 | <0.1 | <0.1 | mg/kg | TM36/PM12 |
| >EC10-EC12 (EH_CU_1D_AR) # | <0.2 | <0.2 | <0.2 | <0.2 | <0.2 | <0.2 | <0.2 | <0.2 | <0.2 | <0.2 | <0.2 | mg/kg | TMS/PM8/PM16 |
| >EC12-EC16 (EH_CU_1D_AR) # | <4 | <4 | <4 | <4 | <4 | <4 | <4 | <4 | <4 | <4 | <4 | mg/kg | TMS/PM8/PM16 |
| >EC16-EC21 (EH_CU_1D_AR) # | <7 | 64 | <7 | 91 | <7 | <7 | <7 | <7 | <7 | 28 | <7 | mg/kg | TMS/PM8/PM16 |
| >EC21-EC35 (EH_CU_1D_AR) # | 256 | 917 | <7 | 1788 | <7 | 86 | 238 | 141 | <7 | 562 | <7 | mg/kg | TMS/PM8/PM16 |
| >EC35-EC40 (EH_CU_1D_AR) | 43 | 122 | <7 | 266 | <7 | <7 | 39 | 20 | <7 | 100 | <7 | mg/kg | TMS/PM8/PM16 |
| Total aromatics C5-40 (EH_CU+HS_1D_AR) | 299 | 1103 | <26 | 2145 | <26 | 86 | 277 | 161 | <26 | 690 | <26 | mg/kg | TMS/PM8/PM16/PM12/PM15 |
| Total aliphatics and aromatics(C5-40) (EH_CU+HS_1D_Total) | 387 | 1447 | <52 | 3192 | <52 | 115 | 406 | 213 | <52 | 986 | <52 | mg/kg | TMS/PM8/PM16/PM12/PM15 |
| >EC6-EC10 (HS_1D_AR) # | <0.1 ^{SV} | <0.1 ^{SV} | <0.1 | <0.1 ^{SV} | <0.1 | <0.1 | <0.1 ^{SV} | <0.1 ^{SV} | <0.1 | <0.1 | <0.1 | mg/kg | TM36/PM12 |
| >EC10-EC25 (EH_CU_1D_AR) | 40 | 224 | <10 | 369 | <10 | 24 | 36 | <10 | <10 | 115 | <10 | mg/kg | TMS/PM8/PM16 |
| >EC25-EC35 (EH_CU_1D_AR) | 224 | 757 | <10 | 1512 | <10 | 68 | 208 | 131 | <10 | 477 | <10 | mg/kg | TMS/PM8/PM16 |
| MTBE # | <5 ^{SV} | <5 ^{SV} | <5 | <5 ^{SV} | <5 | <5 | <5 ^{SV} | <5 ^{SV} | <5 | <5 | <5 | ug/kg | TM36/PM12 |
| Benzene # | <5 ^{SV} | <5 ^{SV} | <5 | <5 ^{SV} | <5 | <5 | <5 ^{SV} | <5 ^{SV} | <5 | <5 | <5 | ug/kg | TM36/PM12 |
| Toluene # | <5 ^{SV} | <5 ^{SV} | <5 | <5 ^{SV} | <5 | <5 | <5 ^{SV} | <5 ^{SV} | <5 | <5 | <5 | ug/kg | TM36/PM12 |
| Ethylbenzene # | <5 ^{SV} | <5 ^{SV} | <5 | <5 ^{SV} | <5 | <5 | <5 ^{SV} | <5 ^{SV} | <5 | <5 | <5 | ug/kg | TM36/PM12 |
| m/p-Xylene # | <5 ^{SV} | <5 ^{SV} | <5 | 7 ^{SV} | <5 | <5 | <5 ^{SV} | <5 ^{SV} | <5 | <5 | <5 | ug/kg | TM36/PM12 |
| o-Xylene # | <5 ^{SV} | <5 ^{SV} | <5 | <5 ^{SV} | <5 | <5 | <5 ^{SV} | <5 ^{SV} | <5 | <5 | <5 | ug/kg | TM36/PM12 |
| PCB 28 # | <5 | <50 ^{AA} | <5 | <50 ^{AA} | <5 | <5 | <5 | <5 | <5 | <5 | <5 | ug/kg | TM17/PM8 |
| PCB 52 # | <5 | <50 ^{AA} | <5 | <50 ^{AA} | <5 | <5 | <5 | <5 | <5 | <5 | <5 | ug/kg | TM17/PM8 |
| PCB 101 # | <5 | <50 ^{AA} | <5 | <50 ^{AA} | <5 | <5 | <5 | <5 | <5 | <5 | <5 | ug/kg | TM17/PM8 |
| PCB 118 # | <5 | <50 ^{AA} | <5 | <50 ^{AA} | <5 | <5 | <5 | <5 | <5 | <5 | <5 | ug/kg | TM17/PM8 |
| PCB 138 # | <5 | <50 ^{AA} | <5 | <50 ^{AA} | <5 | <5 | <5 | <5 | <5 | <5 | <5 | ug/kg | TM17/PM8 |
| PCB 153 # | <5 | <50 ^{AA} | <5 | <50 ^{AA} | <5 | <5 | <5 | <5 | <5 | <5 | <5 | ug/kg | TM17/PM8 |
| PCB 180 # | <5 | <50 ^{AA} | <5 | <50 ^{AA} | <5 | <5 | <5 | <5 | <5 | <5 | <5 | ug/kg | TM17/PM8 |
| Total 7 PCBs # | <35 | <350 ^{AA} | <35 | <350 ^{AA} | <35 | <35 | <35 | <35 | <35 | <35 | <35 | ug/kg | TM17/PM8 |

Element Materials Technology

Client Name: Ground Investigations Ireland
Reference: 13614-02-24
Location: Dykes Road Galway
Contact: Mike Sutton
EMT Job No: 24/7795

Report : CEN 10:1 1 Batch

Solids: V=60g VOC jar, J=250g glass jar, T=plastic tub

| EMT Sample No. | 1-4 | 5-8 | 9-12 | 13-16 | 17-20 | 21-24 | 25-28 | 29-32 | 33-36 | 37-40 | Please see attached notes for all abbreviations and acronyms | | |
|---|------------|------------|------------|------------|------------|------------|------------|------------|------------|------------|--|----------|------------|
| Sample ID | BH01 | BH01 | BH04 | BH05 | BH05 | BH05 | BRC01 | BRC01 | BRC01 | BRC02 | | | |
| Depth | 0.50 | 1.50 | 1.50 | 0.25 | 0.50 | 1.50 | 0.50 | 1.50 | 3.50 | 0.50 | | | |
| COC No / misc | | | | | | | | | | | | | |
| Containers | V J T | V J T | V J T | V J T | V J T | V J T | V J T | V J T | V J T | V J T | | | |
| Sample Date | 06/05/2024 | 06/05/2024 | 06/05/2024 | 06/05/2024 | 06/05/2024 | 06/05/2024 | 06/05/2024 | 06/05/2024 | 06/05/2024 | 06/05/2024 | | | |
| Sample Type | Soil | | | |
| Batch Number | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | | | |
| Date of Receipt | 08/05/2024 | 08/05/2024 | 08/05/2024 | 08/05/2024 | 08/05/2024 | 08/05/2024 | 08/05/2024 | 08/05/2024 | 08/05/2024 | 08/05/2024 | LOD/LOR | Units | Method No. |
| Dissolved Antimony [#] | 0.009 | 0.013 | <0.002 | <0.002 | <0.002 | <0.002 | <0.002 | <0.002 | <0.002 | <0.002 | <0.002 | mg/l | TM30/PM17 |
| Dissolved Antimony (A10) [#] | 0.09 | 0.13 | <0.02 | <0.02 | <0.02 | <0.02 | <0.02 | <0.02 | <0.02 | <0.02 | <0.02 | mg/kg | TM30/PM17 |
| Dissolved Arsenic [#] | 0.0026 | 0.0051 | <0.0025 | <0.0025 | <0.0025 | <0.0025 | <0.0025 | 0.0071 | 0.0035 | <0.0025 | <0.0025 | mg/l | TM30/PM17 |
| Dissolved Arsenic (A10) [#] | 0.026 | 0.051 | <0.025 | <0.025 | <0.025 | <0.025 | <0.025 | 0.071 | 0.035 | <0.025 | <0.025 | mg/kg | TM30/PM17 |
| Dissolved Barium [#] | 0.055 | 0.068 | 0.010 | <0.003 | <0.003 | <0.003 | <0.003 | 0.011 | 0.021 | <0.003 | <0.003 | mg/l | TM30/PM17 |
| Dissolved Barium (A10) [#] | 0.55 | 0.68 | 0.10 | <0.03 | <0.03 | <0.03 | <0.03 | 0.11 | 0.21 | <0.03 | <0.03 | mg/kg | TM30/PM17 |
| Dissolved Cadmium [#] | <0.0005 | <0.0005 | <0.0005 | <0.0005 | <0.0005 | <0.0005 | <0.0005 | <0.0005 | <0.0005 | <0.0005 | <0.0005 | mg/l | TM30/PM17 |
| Dissolved Cadmium (A10) [#] | <0.005 | <0.005 | <0.005 | <0.005 | <0.005 | <0.005 | <0.005 | <0.005 | <0.005 | <0.005 | <0.005 | mg/kg | TM30/PM17 |
| Dissolved Chromium [#] | <0.0015 | <0.0015 | <0.0015 | <0.0015 | <0.0015 | <0.0015 | <0.0015 | <0.0015 | <0.0015 | <0.0015 | <0.0015 | mg/l | TM30/PM17 |
| Dissolved Chromium (A10) [#] | <0.015 | <0.015 | <0.015 | <0.015 | <0.015 | <0.015 | <0.015 | <0.015 | <0.015 | <0.015 | <0.015 | mg/kg | TM30/PM17 |
| Dissolved Copper [#] | <0.007 | <0.007 | <0.007 | <0.007 | <0.007 | <0.007 | <0.007 | <0.007 | <0.007 | <0.007 | <0.007 | mg/l | TM30/PM17 |
| Dissolved Copper (A10) [#] | <0.07 | <0.07 | <0.07 | <0.07 | <0.07 | <0.07 | <0.07 | <0.07 | <0.07 | <0.07 | <0.07 | mg/kg | TM30/PM17 |
| Dissolved Lead [#] | <0.005 | <0.005 | <0.005 | <0.005 | <0.005 | <0.005 | <0.005 | <0.005 | <0.005 | <0.005 | <0.005 | mg/l | TM30/PM17 |
| Dissolved Lead (A10) [#] | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | mg/kg | TM30/PM17 |
| Dissolved Molybdenum [#] | 0.015 | 0.023 | <0.002 | <0.002 | <0.002 | <0.002 | 0.004 | 0.005 | 0.003 | <0.002 | <0.002 | mg/l | TM30/PM17 |
| Dissolved Molybdenum (A10) [#] | 0.15 | 0.23 | <0.02 | <0.02 | <0.02 | <0.02 | 0.04 | 0.05 | 0.03 | <0.02 | <0.02 | mg/kg | TM30/PM17 |
| Dissolved Nickel [#] | <0.002 | <0.002 | <0.002 | <0.002 | <0.002 | <0.002 | <0.002 | <0.002 | <0.002 | <0.002 | <0.002 | mg/l | TM30/PM17 |
| Dissolved Nickel (A10) [#] | <0.02 | <0.02 | <0.02 | <0.02 | <0.02 | <0.02 | <0.02 | <0.02 | <0.02 | <0.02 | <0.02 | mg/kg | TM30/PM17 |
| Dissolved Selenium [#] | <0.003 | <0.003 | <0.003 | <0.003 | <0.003 | <0.003 | <0.003 | <0.003 | <0.003 | <0.003 | <0.003 | mg/l | TM30/PM17 |
| Dissolved Selenium (A10) [#] | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | mg/kg | TM30/PM17 |
| Dissolved Zinc [#] | <0.003 | 0.005 | <0.003 | <0.003 | <0.003 | <0.003 | <0.003 | <0.003 | <0.003 | <0.003 | <0.003 | mg/l | TM30/PM17 |
| Dissolved Zinc (A10) [#] | <0.03 | 0.05 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | mg/kg | TM30/PM17 |
| Mercury Dissolved by CVAF [#] | <0.00001 | <0.00001 | <0.00001 | <0.00001 | <0.00001 | <0.00001 | <0.00001 | <0.00001 | <0.00001 | <0.00001 | <0.00001 | mg/l | TM61/PM0 |
| Mercury Dissolved by CVAF [#] | <0.0001 | <0.0001 | <0.0001 | <0.0001 | <0.0001 | <0.0001 | <0.0001 | <0.0001 | <0.0001 | <0.0001 | <0.0001 | mg/kg | TM61/PM0 |
| Phenol | <0.01 | <0.01 | <0.01 | <0.01 | <0.01 | <0.01 | <0.01 | <0.01 | <0.01 | <0.01 | <0.01 | mg/l | TM26/PM0 |
| Phenol | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 | mg/kg | TM26/PM0 |
| Fluoride | <0.3 | <0.3 | <0.3 | 0.3 | 0.6 | 0.6 | 0.5 | <0.3 | <0.3 | <0.3 | <0.3 | mg/l | TM173/PM0 |
| Fluoride | <3 | <3 | <3 | <3 | 6 | 6 | 5 | <3 | <3 | <3 | <3 | mg/kg | TM173/PM0 |
| Sulphate as SO4 [#] | 93.2 | 83.4 | 1.1 | 4.4 | 3.4 | 3.1 | 2.4 | 12.6 | <0.5 | 2.5 | <0.5 | mg/l | TM38/PM0 |
| Sulphate as SO4 [#] | 933 | 834 | 11 | 44 | 34 | 31 | 24 | 126 | <5 | 25 | <5 | mg/kg | TM38/PM0 |
| Mass of raw test portion | 0.1038 | 0.0984 | 0.0972 | 0.094 | 0.0974 | 0.0967 | 0.095 | 0.1067 | 0.1052 | 0.0943 | | kg | NONE/PM17 |
| Chloride [#] | 3.1 | 9.2 | 0.4 | 0.6 | 0.6 | 0.7 | 0.5 | 2.0 | 1.8 | 0.5 | <0.3 | mg/l | TM38/PM0 |
| Chloride [#] | 31 | 92 | 4 | 6 | 6 | 7 | 5 | 20 | 18 | 5 | <3 | mg/kg | TM38/PM0 |
| Mass of dried test portion | 0.09 | 0.09 | 0.09 | 0.09 | 0.09 | 0.09 | 0.09 | 0.09 | 0.09 | 0.09 | | kg | NONE/PM17 |
| Dissolved Organic Carbon | 4 | 6 | 5 | <2 | <2 | <2 | <2 | 15 | 8 | <2 | <2 | mg/l | TM60/PM0 |
| Dissolved Organic Carbon | 40 | 60 | 50 | <20 | <20 | <20 | <20 | 150 | 80 | <20 | <20 | mg/kg | TM60/PM0 |
| pH | 8.09 | 8.22 | 8.17 | 8.06 | 7.93 | 8.02 | 8.03 | 8.34 | 8.19 | 8.10 | <0.01 | pH units | TM73/PM0 |

Element Materials Technology

Client Name: Ground Investigations Ireland
Reference: 13614-02-24
Location: Dykes Road Galway
Contact: Mike Sutton
EMT Job No: 24/7795

Report : EN12457_2
Solids: V=60g VOC jar, J=250g glass jar, T=plastic tub

| EMT Sample No. | 1-4 | 5-8 | 9-12 | 13-16 | 17-20 | 21-24 | 25-28 | 29-32 | 33-36 | 37-40 | | | | | | |
|--------------------------------------|----------------------|----------------------|------------|----------------------|------------|------------|----------------------|----------------------|------------|------------|-------|---------------------|-----------|---------|----------|--------------|
| Sample ID | BH01 | BH01 | BH04 | BH05 | BH05 | BH05 | BRC01 | BRC01 | BRC01 | BRC02 | | | | | | |
| Depth | 0.50 | 1.50 | 1.50 | 0.25 | 0.50 | 1.50 | 0.50 | 1.50 | 3.50 | 0.50 | | | | | | |
| COC No / misc | | | | | | | | | | | | | | | | |
| Containers | V J T | V J T | V J T | V J T | V J T | V J T | V J T | V J T | V J T | V J T | | | | | | |
| Sample Date | 06/05/2024 | 06/05/2024 | 06/05/2024 | 06/05/2024 | 06/05/2024 | 06/05/2024 | 06/05/2024 | 06/05/2024 | 06/05/2024 | 06/05/2024 | | | | | | |
| Sample Type | Soil | Soil | Soil | Soil | Soil | Soil | Soil | Soil | Soil | Soil | | | | | | |
| Batch Number | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | Inert | Stable Non-reactive | Hazardous | LOD LOR | Units | Method No. |
| Date of Receipt | 08/05/2024 | 08/05/2024 | 08/05/2024 | 08/05/2024 | 08/05/2024 | 08/05/2024 | 08/05/2024 | 08/05/2024 | 08/05/2024 | 08/05/2024 | | | | | | |
| Solid Waste Analysis | | | | | | | | | | | | | | | | |
| Total Organic Carbon # | 3.64 | 8.97 | 0.47 | 0.62 | 0.28 | 0.63 | 0.65 | 6.69 | 4.84 | 0.37 | 3 | 5 | 6 | <0.02 | % | TM21/PM24 |
| Sum of BTEX | <0.025 ^{SV} | <0.025 ^{SV} | <0.025 | <0.025 ^{SV} | <0.025 | <0.025 | <0.025 ^{SV} | <0.025 ^{SV} | <0.025 | <0.025 | 6 | - | - | <0.025 | mg/kg | TM36/PM12 |
| Sum of 7 PCBs # | <0.035 | <0.350 ^{BA} | <0.035 | <0.350 ^{BA} | <0.035 | <0.035 | <0.035 | <0.035 | <0.035 | <0.035 | 1 | - | - | <0.035 | mg/kg | TM17/PM8 |
| Mineral Oil | 88 | 344 | <30 | 1047 | <30 | <30 | 129 | 52 | <30 | 296 | 500 | - | - | <30 | mg/kg | TM5/PM8/PM16 |
| PAH Sum of 6 # | 3.04 | 19.68 | <0.22 | <0.22 | <0.22 | <0.22 | <0.22 | 0.49 | <0.22 | <0.22 | - | - | - | <0.22 | mg/kg | TM4/PM8 |
| PAH Sum of 17 | 4.74 | 34.54 | <0.64 | <0.64 | <0.64 | <0.64 | <0.64 | 0.88 | <0.64 | <0.64 | 100 | - | - | <0.64 | mg/kg | TM4/PM8 |
| CEN 10:1 Leachate | | | | | | | | | | | | | | | | |
| Arsenic # | 0.026 | 0.051 | <0.025 | <0.025 | <0.025 | <0.025 | <0.025 | 0.071 | 0.035 | <0.025 | 0.5 | 2 | 25 | <0.025 | mg/kg | TM30/PM17 |
| Barium # | 0.55 | 0.68 | 0.10 | <0.03 | <0.03 | <0.03 | <0.03 | 0.11 | 0.21 | <0.03 | 20 | 100 | 300 | <0.03 | mg/kg | TM30/PM17 |
| Cadmium # | <0.005 | <0.005 | <0.005 | <0.005 | <0.005 | <0.005 | <0.005 | <0.005 | <0.005 | <0.005 | 0.04 | 1 | 5 | <0.005 | mg/kg | TM30/PM17 |
| Chromium # | <0.015 | <0.015 | <0.015 | <0.015 | <0.015 | <0.015 | <0.015 | <0.015 | <0.015 | <0.015 | 0.5 | 10 | 70 | <0.015 | mg/kg | TM30/PM17 |
| Copper # | <0.07 | <0.07 | <0.07 | <0.07 | <0.07 | <0.07 | <0.07 | <0.07 | <0.07 | <0.07 | 2 | 50 | 100 | <0.07 | mg/kg | TM30/PM17 |
| Mercury # | <0.0001 | <0.0001 | <0.0001 | <0.0001 | <0.0001 | <0.0001 | <0.0001 | <0.0001 | <0.0001 | <0.0001 | 0.01 | 0.2 | 2 | <0.0001 | mg/kg | TM61/PM0 |
| Molybdenum # | 0.15 | 0.23 | <0.02 | <0.02 | <0.02 | <0.02 | 0.04 | 0.05 | 0.03 | <0.02 | 0.5 | 10 | 30 | <0.02 | mg/kg | TM30/PM17 |
| Nickel # | <0.02 | <0.02 | <0.02 | <0.02 | <0.02 | <0.02 | <0.02 | <0.02 | <0.02 | <0.02 | 0.4 | 10 | 40 | <0.02 | mg/kg | TM30/PM17 |
| Lead # | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | <0.05 | 0.5 | 10 | 50 | <0.05 | mg/kg | TM30/PM17 |
| Antimony # | 0.09 | 0.13 | <0.02 | <0.02 | <0.02 | <0.02 | <0.02 | <0.02 | <0.02 | <0.02 | 0.06 | 0.7 | 5 | <0.02 | mg/kg | TM30/PM17 |
| Selenium # | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | 0.1 | 0.5 | 7 | <0.03 | mg/kg | TM30/PM17 |
| Zinc # | <0.03 | 0.05 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | <0.03 | 4 | 50 | 200 | <0.03 | mg/kg | TM30/PM17 |
| Total Dissolved Solids # | 2441 | 2519 | 540 | <350 | 430 | 450 | <350 | 1959 | 1020 | <350 | 4000 | 60000 | 100000 | <350 | mg/kg | TM20/PM0 |
| Dissolved Organic Carbon | 40 | 60 | 50 | <20 | <20 | <20 | <20 | 150 | 80 | <20 | 500 | 800 | 1000 | <20 | mg/kg | TM60/PM0 |
| Mass of raw test portion | 0.1038 | 0.0984 | 0.0972 | 0.094 | 0.0974 | 0.0967 | 0.095 | 0.1067 | 0.1052 | 0.0943 | - | - | - | | kg | NONE/PM17 |
| Dry Matter Content Ratio | 86.9 | 91.3 | 92.7 | 95.8 | 92.5 | 93.0 | 95.1 | 84.3 | 85.6 | 95.9 | - | - | - | <0.1 | % | NONE/PM4 |
| Leachant Volume | 0.887 | 0.891 | 0.893 | 0.896 | 0.893 | 0.893 | 0.895 | 0.883 | 0.885 | 0.896 | - | - | - | | l | NONE/PM17 |
| Moisture Content 105C (% Dry Weight) | 15.0 | 9.5 | 7.9 | 4.4 | 8.1 | 7.5 | 5.2 | 18.6 | 16.8 | 4.2 | - | - | - | <0.1 | % | PM4/PM0 |
| pH # | 7.66 | 7.71 | 8.78 | 9.09 | 8.67 | 8.51 | 9.03 | 7.83 | 8.17 | 9.11 | - | - | - | <0.01 | pH units | TM73/PM11 |
| Phenol | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 | <0.1 | 1 | - | - | <0.1 | mg/kg | TM26/PM0 |
| Fluoride | <3 | <3 | <3 | <3 | 6 | 6 | 5 | <3 | <3 | <3 | 10 | 150 | 500 | <3 | mg/kg | TM173/PM0 |
| Sulphate as SO4 # | 933 | 834 | 11 | 44 | 34 | 31 | 24 | 126 | <5 | 25 | 1000 | 20000 | 50000 | <5 | mg/kg | TM38/PM0 |
| Chloride # | 31 | 92 | 4 | 6 | 6 | 7 | 5 | 20 | 18 | 5 | 800 | 15000 | 25000 | <3 | mg/kg | TM38/PM0 |

Please see attached notes for all abbreviations and acronyms

NOTES TO ACCOMPANY ALL SCHEDULES AND REPORTS

EMT Job No.: 24/7795

SOILS and ASH

Please note we are only MCERTS accredited (UK soils only) for sand, loam and clay and any other matrix is outside our scope of accreditation.

Where an MCERTS report has been requested, you will be notified within 48 hours of any samples that have been identified as being outside our MCERTS scope. As validation has been performed on clay, sand and loam, only samples that are predominantly these matrices, or combinations of them will be within our MCERTS scope. If samples are not one of a combination of the above matrices they will not be marked as MCERTS accredited.

It is assumed that you have taken representative samples on site and require analysis on a representative subsample. Stones will generally be included unless we are requested to remove them.

All samples will be discarded one month after the date of reporting, unless we are instructed to the contrary. Asbestos samples are retained for 6 months.

If you have not already done so, please send us a purchase order if this is required by your company.

Where appropriate please make sure that our detection limits are suitable for your needs, if they are not, please notify us immediately.

All analysis is reported on a dry weight basis unless stated otherwise. Limits of detection for analyses carried out on as received samples are not moisture content corrected. Results are not surrogate corrected. Samples are dried at 35°C ±5°C unless otherwise stated. Moisture content for CEN Leachate tests are dried at 105°C ±5°C. Ash samples are dried at 35°C ±5°C.

Where Mineral Oil is quoted, this refers to Total Aliphatics C10-C40.

Where a CEN 10:1 ZERO Headspace VOC test has been carried out, a 10:1 ratio of water to wet (as received) soil has been used.

% Asbestos in Asbestos Containing Materials (ACMs) is determined by reference to HSG 264 The Survey Guide - Appendix 2 : ACMs in buildings listed in order of ease of fibre release.

Sufficient amount of sample must be received to carry out the testing specified. Where an insufficient amount of sample has been received the testing may not meet the requirements of our accredited methods, as such accreditation may be removed.

Negative Neutralization Potential (NP) values are obtained when the volume of NaOH (0.1N) titrated (pH 8.3) is greater than the volume of HCl (1N) to reduce the pH of the sample to 2.0 - 2.5. Any negative NP values are corrected to 0.

The calculation of Pyrite content assumes that all oxidisable sulphides present in the sample are pyrite. This may not be the case. The calculation may be an overestimate when other sulphides such as Barite (Barium Sulphate) are present.

WATERS

Please note we are not a UK Drinking Water Inspectorate (DWI) Approved Laboratory .

ISO17025 accreditation applies to surface water and groundwater and usually one other matrix which is analysis specific, any other liquids are outside our scope of accreditation.

As surface waters require different sample preparation to groundwaters the laboratory must be informed of the water type when submitting samples.

Where Mineral Oil is quoted, this refers to Total Aliphatics C10-C40.

STACK EMISSIONS

Where an MCERTS report has been requested, you will be notified within 48 hours of any samples that have been identified as being outside our MCERTS scope. As validation for Dioxins and Furans and Dioxin like PCBs has been performed on XAD-2 Resin, only samples which use this resin will be within our MCERTS scope.

Where appropriate please make sure that our detection limits are suitable for your needs, if they are not, please notify us immediately.

DEVIATING SAMPLES

All samples should be submitted to the laboratory in suitable containers with sufficient ice packs to sustain an appropriate temperature for the requested analysis. The temperature of sample receipt is recorded on the confirmation schedules in order that the client can make an informed decision as to whether testing should still be undertaken.

SURROGATES

Surrogate compounds are added during the preparation process to monitor recovery of analytes. However low recovery in soils is often due to peat, clay or other organic rich matrices. For waters this can be due to oxidants, surfactants, organic rich sediments or remediation fluids. Acceptable limits for most organic methods are 70 - 130% and for VOCs are 50 - 150%. When surrogate recoveries are outside the performance criteria but the associated AQC passes this is assumed to be due to matrix effect. Results are not surrogate corrected.

DILUTIONS

A dilution suffix indicates a dilution has been performed and the reported result takes this into account. No further calculation is required.

BLANKS

Where analytes have been found in the blank, the sample will be treated in accordance with our laboratory procedure for dealing with contaminated blanks.

NOTE

Data is only reported if the laboratory is confident that the data is a true reflection of the samples analysed. Data is only reported as accredited when all the requirements of our Quality System have been met. In certain circumstances where all the requirements of the Quality System have not been met, for instance if the associated AQC has failed, the reason is fully investigated and documented. The sample data is then evaluated alongside the other quality control checks performed during analysis to determine its suitability. Following this evaluation, provided the sample results have not been effected, the data is reported but accreditation is removed. It is a requirement of our Accreditation Body for data not reported as accredited to be considered indicative only, but this does not mean the data is not valid.

Where possible, and if requested, samples will be re-extracted and a revised report issued with accredited results. Please do not hesitate to contact the laboratory if further details are required of the circumstances which have led to the removal of accreditation.

Laboratory records are kept for a period of no less than 6 years.

REPORTS FROM THE SOUTH AFRICA LABORATORY

Any method number not prefixed with SA has been undertaken in our UK laboratory unless reported as subcontracted.

Measurement Uncertainty

Measurement uncertainty defines the range of values that could reasonably be attributed to the measured quantity. This range of values has not been included within the reported results. Uncertainty expressed as a percentage can be provided upon request.

Customer Provided Information

Sample ID and depth is information provided by the customer.

Age of Diesel

The age of release estimation is based on the nC17/pristane ratio only as prescribed by Christensen and Larsen (1993) and Kaplan, Galperin, Alimi et al., (1996).

Age estimation should be treated with caution as it can be influenced by site specific factors of which the laboratory are not aware.

Tentatively Identified Compounds (TICs)

Where Tentatively Identified Compounds (TICs) are reported, up to 10 Tentatively Identified Compounds will be listed where there is found to be a greater than 80% match with the NIST library. The reported concentration is determined semi-quantitatively, with a matrix specific limit of detection.

Note, other compounds may be present but are not reported.

ABBREVIATIONS and ACRONYMS USED

| | |
|---------|--|
| # | ISO17025 (UKAS Ref No. 4225) accredited - UK. |
| SA | ISO17025 (SANAS Ref No.T0729) accredited - South Africa |
| B | Indicates analyte found in associated method blank. |
| DR | Dilution required. |
| M | MCERTS accredited. |
| NA | Not applicable |
| NAD | No Asbestos Detected. |
| ND | None Detected (usually refers to VOC and/SVOC TICs). |
| NDP | No Determination Possible |
| SS | Calibrated against a single substance |
| SV | Surrogate recovery outside performance criteria. This may be due to a matrix effect. |
| W | Results expressed on as received basis. |
| + | AQC failure, accreditation has been removed from this result, if appropriate, see 'Note' on previous page. |
| >> | Results above quantitative calibration range. The result should be considered the minimum value and is indicative only. The actual result could be significantly higher. |
| * | Analysis subcontracted to an Element Materials Technology approved laboratory. |
| CO | Suspected carry over |
| LOD/LOR | Limit of Detection (Limit of Reporting) in line with ISO 17025 and MCERTS |
| ME | Matrix Effect |
| NFD | No Fibres Detected |
| BS | AQC Sample |
| LB | Blank Sample |
| N | Client Sample |
| TB | Trip Blank Sample |
| OC | Outside Calibration Range |
| | |
| AA | x10 Dilution |
| AB | x20 Dilution |
| BA | x10 Dilution |

HWOL ACRONYMS AND OPERATORS USED

| | |
|-------|--|
| HS | Headspace Analysis. |
| EH | Extractable Hydrocarbons - i.e. everything extracted by the solvent. |
| CU | Clean-up - e.g. by florisil, silica gel. |
| 1D | GC - Single coil gas chromatography. |
| Total | Aliphatics & Aromatics. |
| AL | Aliphatics only. |
| AR | Aromatics only. |
| 2D | GC-GC - Double coil gas chromatography. |
| #1 | EH_Total but with humics mathematically subtracted |
| #2 | EU_Total but with fatty acids mathematically subtracted |
| _ | Operator - underscore to separate acronyms (exception for +). |
| + | Operator to indicate cumulative e.g. EH+HS_Total or EH_CU+HS_Total |
| MS | Mass Spectrometry. |

EMT Job No: 24/7795

| Test Method No. | Description | Prep Method No. (if appropriate) | Description | ISO 17025 (UKAS/S ANAS) | MCERTS (UK soils only) | Analysis done on As Received (AR) or Dried (AD) | Reported on dry weight basis |
|-----------------|--|----------------------------------|---|-------------------------|------------------------|---|------------------------------|
| PM4 | Gravimetric measurement of Natural Moisture Content and % Moisture Content at either 35°C or 105°C. Calculation based on ISO 11465:1993(E) and BS1377-2:1990. | PM0 | No preparation is required. | | | AR | |
| TM4 | Modified USEPA 8270D v5:2014 method for the solvent extraction and determination of PAHs by GC-MS. | PM8 | End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required. | | | AR | Yes |
| TM4 | Modified USEPA 8270D v5:2014 method for the solvent extraction and determination of PAHs by GC-MS. | PM8 | End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required. | Yes | | AR | Yes |
| TM5 | Modified 8015B v2:1996 method for the determination of solvent Extractable Petroleum Hydrocarbons (EPH) within the range C8-C40 by GCFID. For waters the solvent extracts dissolved phase plus a sheen if present. | PM16 | Fractionation into aliphatic and aromatic fractions using a Rapid Trace SPE. | | | AR | |
| TM5 | Modified 8015B v2:1996 method for the determination of solvent Extractable Petroleum Hydrocarbons (EPH) within the range C8-C40 by GCFID. For waters the solvent extracts dissolved phase plus a sheen if present. | PM8/PM16 | End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required/Fractionation into aliphatic and aromatic fractions using a Rapid Trace SPE. | | | AR | Yes |
| TM5 | Modified 8015B v2:1996 method for the determination of solvent Extractable Petroleum Hydrocarbons (EPH) within the range C8-C40 by GCFID. For waters the solvent extracts dissolved phase plus a sheen if present. | PM8/PM16 | End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required/Fractionation into aliphatic and aromatic fractions using a Rapid Trace SPE. | Yes | | AR | Yes |
| TM5/TM36 | please refer to TM5 and TM36 for method details | PM8/PM12/PM16 | please refer to PM8/PM16 and PM12 for method details | | | AR | Yes |
| TM17 | Modified US EPA method 8270D v5:2014. Determination of specific Polychlorinated Biphenyl congeners by GC-MS. | PM8 | End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required. | Yes | | AR | Yes |
| TM20 | Modified BS 1377-3:1990/USEPA 160.1/3 (TDS/TS: 1971) Gravimetric determination of Total Dissolved Solids/Total Solids | PM0 | No preparation is required. | Yes | | AR | Yes |
| TM21 | Modified BS 7755-3:1995, ISO10694:1995 Determination of Total Organic Carbon or Total Carbon by combustion in an Eltra TOC furnace/analyser in the presence of oxygen. The CO2 generated is quantified using infra-red detection. Organic Matter (SOM) calculated as per EA MCERTS Chemical Testing of Soil. | PM24 | Preparation of Soil and Marine Sediment Samples for Total Organic Carbon. | Yes | | AD | Yes |

EMT Job No: 24/7795

| Test Method No. | Description | Prep Method No. (if appropriate) | Description | ISO 17025 (UKAS/S ANAS) | MCERTS (UK soils only) | Analysis done on As Received (AR) or Dried (AD) | Reported on dry weight basis |
|-----------------|--|----------------------------------|---|-------------------------|------------------------|---|------------------------------|
| TM26 | Determination of phenols by Reversed Phased High Performance Liquid Chromatography and Electro-Chemical Detection. | PM0 | No preparation is required. | | | AR | Yes |
| TM30 | Determination of Trace Metals by ICP-OES (Inductively Coupled Plasma – Optical Emission Spectrometry): WATERS by Modified USEPA Method 200.7, Rev. 4.4, 1994; Modified EPA Method 6010B, Rev.2, Dec 1996; Modified BS EN ISO 11885:2009: SOILS by Modified USEP 6010B, Rev.2, Dec.1996; Modified EPA Method 3050B, Rev.2, Dec.1996 | PM15 | Acid digestion of dried and ground solid samples using Aqua Regia refluxed at 112.5 °C. Samples containing asbestos are not dried and ground. | | | AD | Yes |
| TM30 | Determination of Trace Metals by ICP-OES (Inductively Coupled Plasma – Optical Emission Spectrometry): WATERS by Modified USEPA Method 200.7, Rev. 4.4, 1994; Modified EPA Method 6010B, Rev.2, Dec 1996; Modified BS EN ISO 11885:2009: SOILS by Modified USEP 6010B, Rev.2, Dec.1996; Modified EPA Method 3050B, Rev.2, Dec.1996 | PM15 | Acid digestion of dried and ground solid samples using Aqua Regia refluxed at 112.5 °C. Samples containing asbestos are not dried and ground. | Yes | | AD | Yes |
| TM30 | Determination of Trace Metals by ICP-OES (Inductively Coupled Plasma – Optical Emission Spectrometry): WATERS by Modified USEPA Method 200.7, Rev. 4.4, 1994; Modified EPA Method 6010B, Rev.2, Dec 1996; Modified BS EN ISO 11885:2009: SOILS by Modified USEP 6010B, Rev.2, Dec.1996; Modified EPA Method 3050B, Rev.2, Dec.1996 | PM17 | Modified method BS EN12457-2:2002 As received solid samples are leached with water in a 10:1 water to soil ratio for 24 hours, the moisture content of the sample is included in the ratio. | Yes | | AR | Yes |
| TM36 | Modified US EPA method 8015B v2:1996. Determination of Gasoline Range Organics (GRO) in the carbon chain range of C4-12 by headspace GC-FID. MTBE by GC/FID co-elutes with 3-methylpentane if present and therefore can give a false positive. Positive MTBE results will be re-run using GC-MS to double check, when requested. | PM12 | Modified US EPA method 5021A v2:2014. Preparation of solid and liquid samples for GC headspace analysis. | | | AR | Yes |
| TM36 | Modified US EPA method 8015B v2:1996. Determination of Gasoline Range Organics (GRO) in the carbon chain range of C4-12 by headspace GC-FID. MTBE by GC/FID co-elutes with 3-methylpentane if present and therefore can give a false positive. Positive MTBE results will be re-run using GC-MS to double check, when requested. | PM12 | Modified US EPA method 5021A v2:2014. Preparation of solid and liquid samples for GC headspace analysis. | Yes | | AR | Yes |
| TM38 | Soluble Ion analysis using Discrete Analyser. Modified US EPA methods: Chloride 325.2 (1978), Sulphate 375.4 (Rev.2 1993), o-Phosphate 365.2 (Rev.2 1993), TON 353.1 (Rev.2 1993), Nitrite 354.1 (1971), Hex Cr 7196A (1992), NH4+ 350.1 (Rev.2 1993) – All anions comparable to BS ISO 15923-1: 2013 | PM0 | No preparation is required. | Yes | | AR | Yes |
| TM38 | Soluble Ion analysis using Discrete Analyser. Modified US EPA methods: Chloride 325.2 (1978), Sulphate 375.4 (Rev.2 1993), o-Phosphate 365.2 (Rev.2 1993), TON 353.1 (Rev.2 1993), Nitrite 354.1 (1971), Hex Cr 7196A (1992), NH4+ 350.1 (Rev.2 1993) – All anions comparable to BS ISO 15923-1: 2013 | PM20 | Extraction of dried and ground or as received samples with deionised water in a 2:1 water to solid ratio using a reciprocal shaker for all analytes except hexavalent chromium. Extraction of as received sample using 10:1 ratio of 0.2M sodium hydroxide to soil for hexavalent chromium using a reciprocal shaker. | Yes | | AR | Yes |
| TM60 | TC/TOC analysis of Waters by High Temperature Combustion followed by NDIR detection. Based on the following modified standard methods: USEPA 9060A (2002), APHA SMEWW 5310B:1999 22nd Edition, ASTM D 7573, and USEPA 415.1. | PM0 | No preparation is required. | | | AR | Yes |
| TM61 | Determination of Mercury by Cold Vapour Atomic Fluorescence - WATERS: Modified USEPA Method 245.7, Rev 2, Feb 2005. SOILS: Modified USEPA Method 7471B, Rev.2, Feb 2007 | PM0 | No preparation is required. | Yes | | AR | Yes |

EMT Job No: 24/7795

| Test Method No. | Description | Prep Method No. (if appropriate) | Description | ISO 17025 (UKAS/S ANAS) | MCERTS (UK soils only) | Analysis done on As Received (AR) or Dried (AD) | Reported on dry weight basis |
|-----------------|---|----------------------------------|---|-------------------------|------------------------|---|------------------------------|
| TM73 | Modified US EPA methods 150.1 (1982) and 9045D Rev. 4 - 2004) and BS1377-3:1990. Determination of pH by Metrohm automated probe analyser. | PM0 | No preparation is required. | | | AR | Yes |
| TM73 | Modified US EPA methods 150.1 (1982) and 9045D Rev. 4 - 2004) and BS1377-3:1990. Determination of pH by Metrohm automated probe analyser. | PM11 | Extraction of as received solid samples using one part solid to 2.5 parts deionised water. | Yes | | AR | No |
| TM173 | Analysis of fluoride by ISE (Ion Selective Electrode) using modified ISE method 9214 - 340.2 (EPA 1998) | PM0 | No preparation is required. | | | AR | Yes |
| NONE | No Method Code | NONE | No Method Code | | | AD | Yes |
| NONE | No Method Code | PM17 | Modified method BS EN12457-2:2002 As received solid samples are leached with water in a 10:1 water to soil ratio for 24 hours, the moisture content of the sample is included in the ratio. | | | | |
| NONE | No Method Code | PM4 | Gravimetric measurement of Natural Moisture Content and % Moisture Content at either 35°C or 105°C. Calculation based on ISO 11465:1993(E) and BS1377-2:1990. | | | AR | |
| Subcontracted | See attached subcontractor report for accreditation status and provider. | | | | | AR | |
| | | | | | | | |
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LABORATORY REPORT



Contract Number: PSL24/3965

Report Date: 28 June 2024
Client's Reference: 13614-02-24
Client Name: Ground Investigations Ireland Ltd
Catherinestown House
Hazelhatch Road
Newcastle
Co Dublin
D22 YD52

For the attention of: Mike Sutton

Contract Title: Dyke Road Galway
Date Received: 5/6/2024
Date Commenced: 5/6/2024
Date Completed: 28/6/2024

Notes: Opinions and Interpretations are outside the UKAS Accreditation

A copy of the Laboratory Schedule of accredited tests as issued by UKAS is attached to this report. This certificate is issued in accordance with the accreditation requirements of the United Kingdom Accreditation Service. The results reported herein relate only to the material supplied to the laboratory. This certificate shall not be reproduced other than in full, without the prior written approval of the laboratory.

Checked and Approved Signatories:

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(Managing Director)

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SUMMARY OF LABORATORY SOIL DESCRIPTIONS

| Hole Number | Sample Number | Sample Type | Top Depth m | Base Depth m | Description of Sample |
|-------------|---------------|-------------|-------------|--------------|---|
| BH01 | | UT | 3.00 | 3.45 | Very soft brown organic CLAY. |
| BH01 | | B | 3.50 | | Brown slightly sandy slightly gravelly organic CLAY. |
| BH01 | | B | 6.70 | | Brown slightly sandy slightly gravelly organic CLAY. |
| BH02 | | UT | 1.00 | 1.27 | Dark brown fibrous PEAT. |
| BH02 | | B | 3.00 | | Grey slightly sandy gravelly CLAY with some organic material. |
| BH02 | | UT | 3.00 | 3.45 | Very soft dark grey organic CLAY. |
| BH03 | | UT | 1.00 | 1.45 | Dark brown organic CLAY. |
| BH03 | | UT | 3.00 | 3.45 | Brown organic CLAY. |
| BH05 | | B | 4.00 | | Grey slightly silty sandy GRAVEL with many cobbles. |
| BH06 | | UT | 3.00 | 3.45 | Brown slightly sandy organic CLAY. |
| BRC01 | | UT | 5.30 | 5.75 | Very soft brown organic CLAY. |
| BRC01 | | B | 5.50 | | Brown slightly sandy slightly gravelly organic CLAY. |
| BRC01 | | B | 7.50 | | Brown slightly sandy slightly gravelly organic CLAY. |
| BRC01 | | UT | 7.50 | 7.95 | Very soft brown organic CLAY. |
| BRC02 | | UT | 1.60 | 1.87 | Dark brown PEAT. |
| BRC02 | | B | 2.00 | | Dark brown slightly sandy gravelly organic CLAY. |
| BRC02 | | UT | 3.00 | 3.45 | Very soft light brown organic CLAY. |
| BRC02 | | B | 3.50 | | Brown slightly sandy organic CLAY. |
| BRC02 | | UT | 5.00 | 5.45 | Very soft brown organic CLAY. |



Dyke Road Galway

Contract No:

PSL24/3965

Client Ref:

13614-02-24

SUMMARY OF LABORATORY SOIL DESCRIPTIONS

| Hole Number | Sample Number | Sample Type | Top Depth m | Base Depth m | Description of Sample |
|-------------|---------------|-------------|-------------|--------------|--|
| BRC02 | | B | 7.00 | | Grey silty sandy GRAVEL with many cobbles. |
| BRC03 | | UT | 3.00 | 3.45 | Very soft brown slightly gravelly organic CLAY. |
| BRC03 | | B | 4.00 | | Brown slightly sandy slightly gravelly CLAY with many cobbles. |
| BRC03 | | B | 7.50 | | Brown slightly sandy slightly gravelly CLAY. |
| BRC03 | | B | 8.50 | | Grey sandy slightly gravelly CLAY. |
| BRC03 | | B | 9.50 | | Grey slightly sandy slightly gravelly CLAY. |
| BRC04 | | B | 4.00 | | Grey slightly silty sandy GRAVEL with many cobbles. |
| BRC05 | | B | 2.60 | | Grey slightly sandy slightly gravelly CLAY with many cobbles. |
| BRC05 | | B | 3.50 | | Grey slightly sandy slightly gravelly CLAY with many cobbles. |
| BRC05 | | B | 4.50 | | Grey slightly sandy gravelly SILT. |
| TP01 | | B | 0.50 | | Grey clayey sandy GRAVEL. |
| TP01 | | B | 1.00 | | Dark brown fibrous PEAT. |
| TP02 | | B | 2.00 | | Brown peaty SILT. |
| TP03 | | B | 2.00 | | Brown peaty SILT. |
| TP05 | | B | 1.00 | | Grey very sandy very silty GRAVEL. |
| | | | | | |
| | | | | | |
| | | | | | |
| | | | | | |



Dyke Road Galway

Contract No:

PSL24/3965

Client Ref:

13614-02-24

SUMMARY OF SOIL CLASSIFICATION TESTS

(BS1377 : PART 2 : 1990)

| Hole Number | Sample Number | Sample Type | Top Depth m | Base Depth m | Moisture Content % Clause 3.2 | Linear Shrinkage % Clause 6.5 | Particle Density Mg/m ³ Clause 8.2 | Liquid Limit % Clause 4.3/4 | Plastic Limit % Clause 5.3 | Plasticity Index % Clause 5.4 | Passing .425mm % | Remarks |
|-------------|---------------|-------------|----------------|-----------------|-------------------------------------|-------------------------------------|---|-----------------------------------|----------------------------------|-------------------------------------|---------------------|------------------------------|
| BH01 | | UT | 3.00 | 3.45 | 136 | | | 150 | 55 | 95 | 100 | Extremely High Plasticity CE |
| BH01 | | B | 3.50 | | 145 | | | 141 | 45 | 96 | 96 | Extremely High Plasticity CE |
| BH01 | | B | 6.70 | | 123 | | | 151 | 55 | 96 | 88 | Extremely High Plasticity CE |
| BH02 | | UT | 1.00 | 1.27 | 80 | | | | NP | | | |
| BH02 | | B | 3.00 | | 27 | | | 56 | 26 | 30 | 55 | High Plasticity CH |
| BH02 | | UT | 3.00 | 3.45 | 133 | | | 161 | 58 | 103 | 100 | Extremely High Plasticity CE |
| BH06 | | UT | 3.00 | 3.45 | 159 | | | 258 | 84 | 174 | 100 | Extremely High Plasticity CE |
| BRC01 | | UT | 5.30 | 5.75 | 141 | | | 237 | 78 | 159 | 100 | Extremely High Plasticity CE |
| BRC01 | | B | 5.50 | | 160 | | | 243 | 80 | 163 | 94 | Extremely High Plasticity CE |
| BRC01 | | B | 7.50 | | 197 | | | 274 | 88 | 186 | 51 | Extremely High Plasticity CE |
| BRC01 | | UT | 7.50 | 7.95 | 192 | | | 233 | 77 | 156 | 100 | Extremely High Plasticity CE |
| BRC02 | | UT | 1.60 | 1.87 | 92 | | | 302 | 135 | 167 | 92 | Extremely High Plasticity ME |
| BRC02 | | B | 2.00 | | 88 | | | 321 | 101 | 220 | 38 | Extremely High Plasticity CE |
| BRC02 | | UT | 3.00 | 3.45 | 115 | | | 135 | 51 | 84 | 100 | Extremely High Plasticity CE |
| BRC02 | | B | 3.50 | | 121 | | | 124 | 48 | 76 | 100 | Extremely High Plasticity CE |
| BRC02 | | B | 7.00 | | 1.2 | | | | NP | | | |
| BRC03 | | UT | 3.00 | 3.45 | 113 | | | 256 | 83 | 173 | 100 | Extremely High Plasticity CE |
| BRC03 | | B | 4.00 | | 8.2 | | | 20 | 12 | 8 | 41 | Low Plasticity CL |
| BRC03 | | B | 7.50 | | 8.2 | | | 24 | 13 | 11 | 65 | Low Plasticity CL |

SYMBOLS : NP : Non Plastic

* : Liquid Limit and Plastic Limit Wet Sieved.



Dyke Road Galway

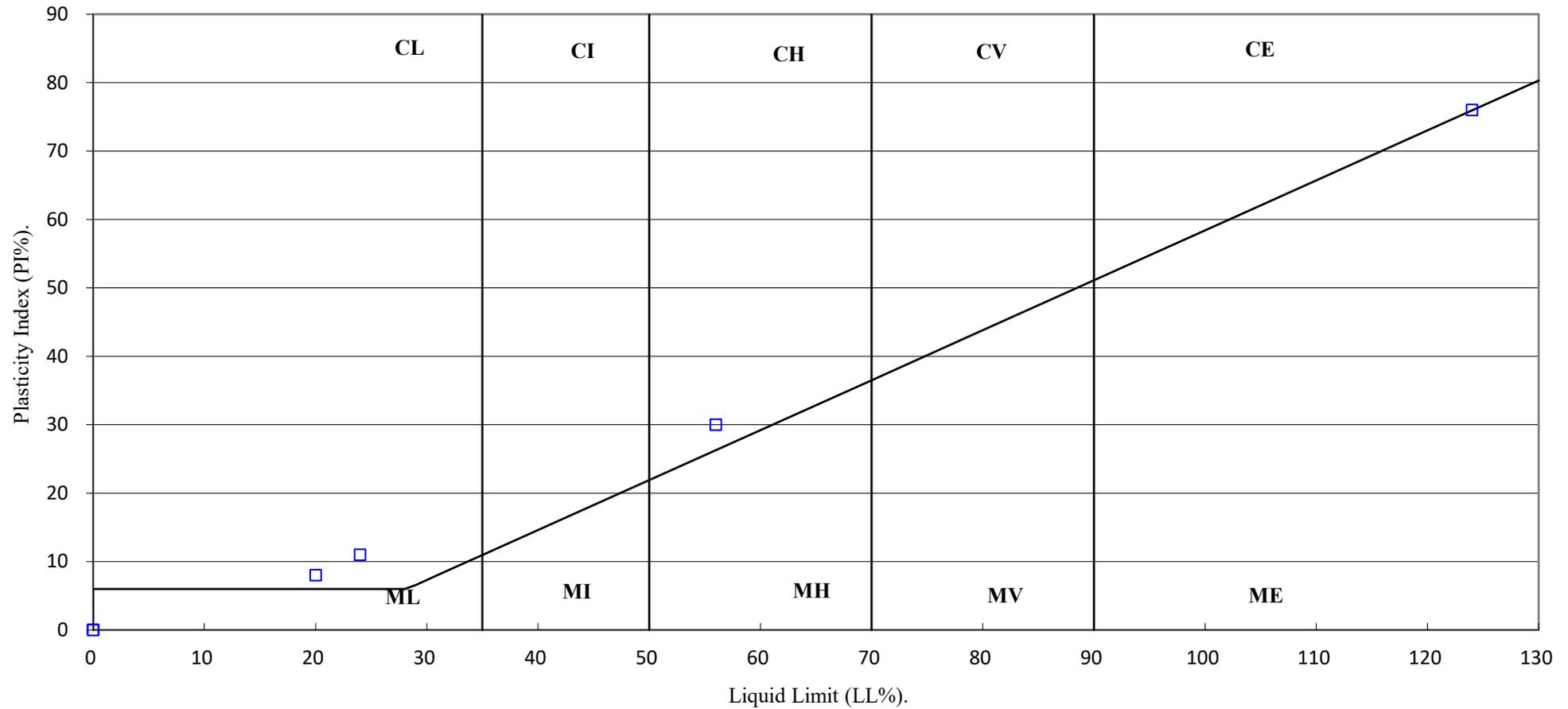
Contract No:

PSL24/3965

Client Ref:

13614-02-24

PLASTICITY CHART FOR CASAGRANDE CLASSIFICATION.



Dyke Road Galway

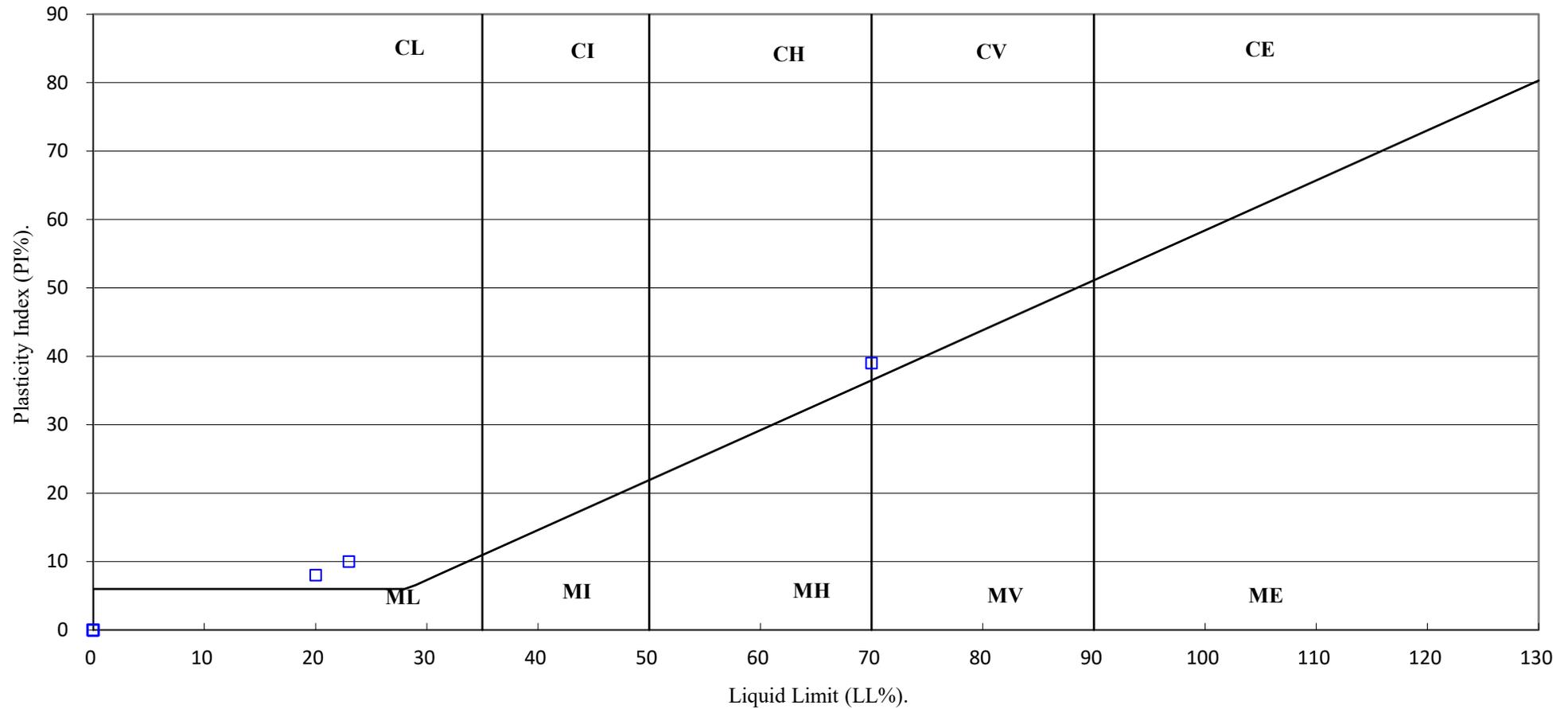
Contract No:

PSL24/3965

Client Ref:

13614-02-24

PLASTICITY CHART FOR CASAGRANDE CLASSIFICATION.



Dyke Road Galway

Contract No:

PSL24/3965

Client Ref:

13614-02-24

PARTICLE SIZE DISTRIBUTION TEST

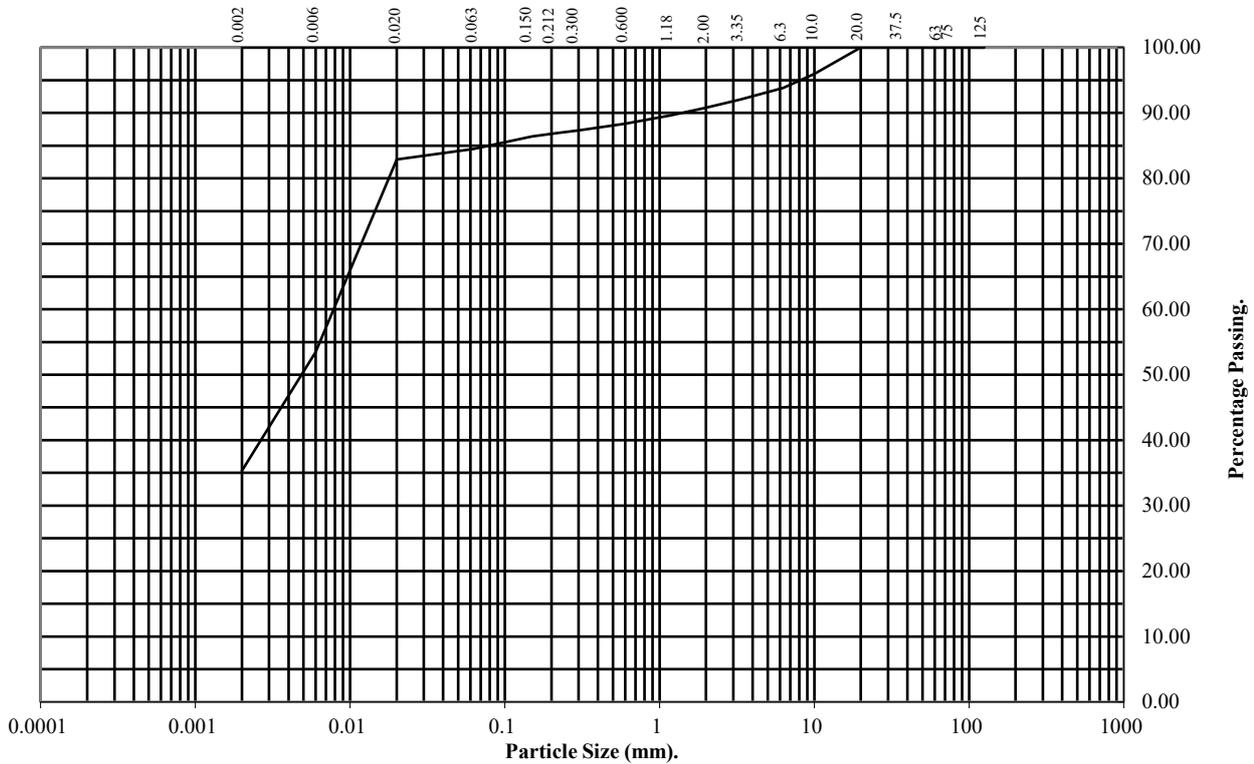
BS1377 : Part 2 : 1990

Wet Sieve & Pipette Analysis, Clause 9.2 & 9.4

Hole Number: **BH01** Top Depth (m): **6.70**

Sample Number: Base Depth(m):

Sample Type: **B**



| BS Test Sieve (mm) | Percentage Passing |
|--------------------|--------------------|
| 125 | 100 |
| 75 | 100 |
| 63 | 100 |
| 37.5 | 100 |
| 20 | 100 |
| 10 | 96 |
| 6.3 | 94 |
| 3.35 | 92 |
| 2 | 91 |
| 1.18 | 90 |
| 0.6 | 88 |
| 0.3 | 87 |
| 0.212 | 87 |
| 0.15 | 86 |
| 0.063 | 85 |

| Particle Diameter | Percentage Passing |
|-------------------|--------------------|
| 0.02 | 83 |
| 0.006 | 53 |
| 0.002 | 35 |

| Soil Fraction | Total Percentage |
|---------------|------------------|
| Cobbles | 0 |
| Gravel | 9 |
| Sand | 6 |
| Silt | 50 |
| Clay | 35 |

Remarks:
See Summary of Soil Descriptions



Dyke Road Galway

Contract No:
PSL24/3965
Client Ref:
13614-02-24

PARTICLE SIZE DISTRIBUTION TEST

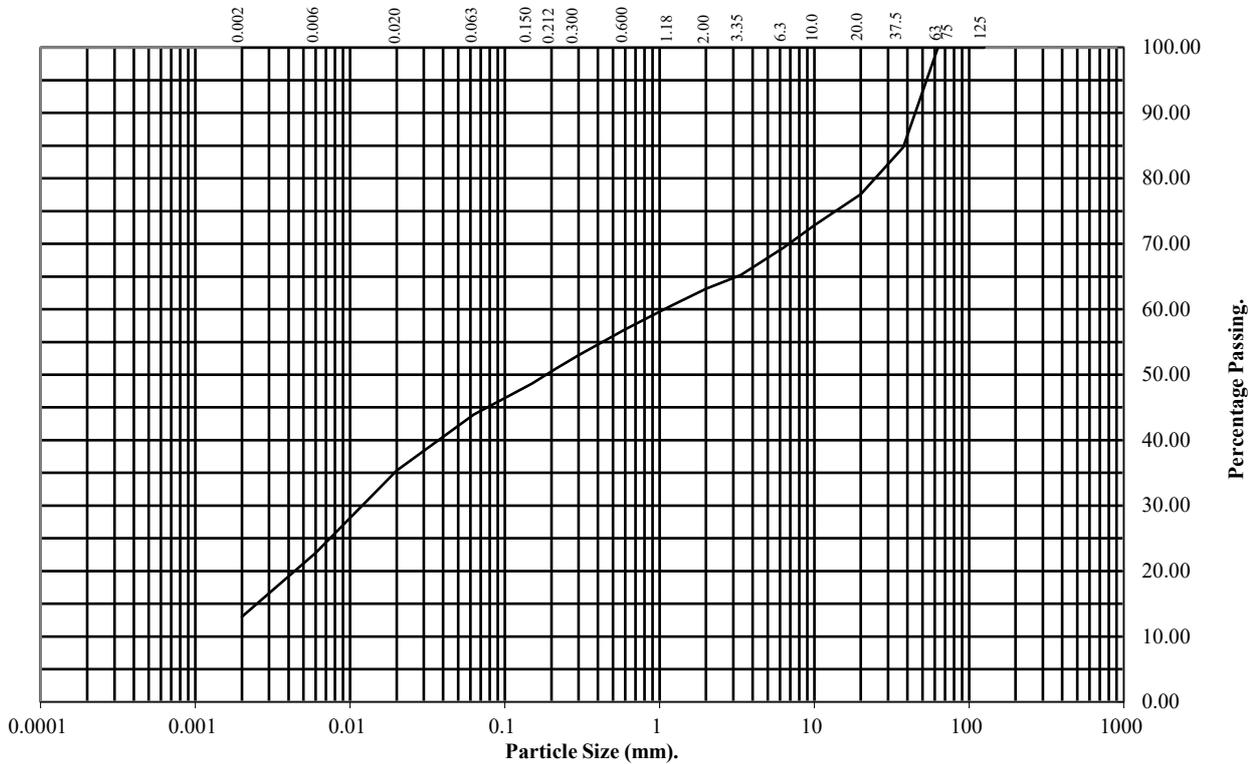
BS1377 : Part 2 : 1990

Wet Sieve & Pipette Analysis, Clause 9.2 & 9.4

Hole Number: **BH02** **Top Depth (m):** **3.00**

Sample Number: **Base Depth(m):**

Sample Type: **B**



| BS Test Sieve (mm) | Percentage Passing |
|--------------------|--------------------|
| 125 | 100 |
| 75 | 100 |
| 63 | 100 |
| 37.5 | 85 |
| 20 | 78 |
| 10 | 73 |
| 6.3 | 69 |
| 3.35 | 65 |
| 2 | 63 |
| 1.18 | 60 |
| 0.6 | 57 |
| 0.3 | 53 |
| 0.212 | 51 |
| 0.15 | 49 |
| 0.063 | 44 |

| Particle Diameter | Percentage Passing |
|-------------------|--------------------|
| 0.02 | 35 |
| 0.006 | 23 |
| 0.002 | 13 |

| Soil Fraction | Total Percentage |
|---------------|------------------|
| Cobbles | 0 |
| Gravel | 37 |
| Sand | 19 |
| Silt | 31 |
| Clay | 13 |

Remarks:
See Summary of Soil Descriptions



Dyke Road Galway

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| Contract No: |
| PSL24/3965 |
| Client Ref: |
| 13614-02-24 |

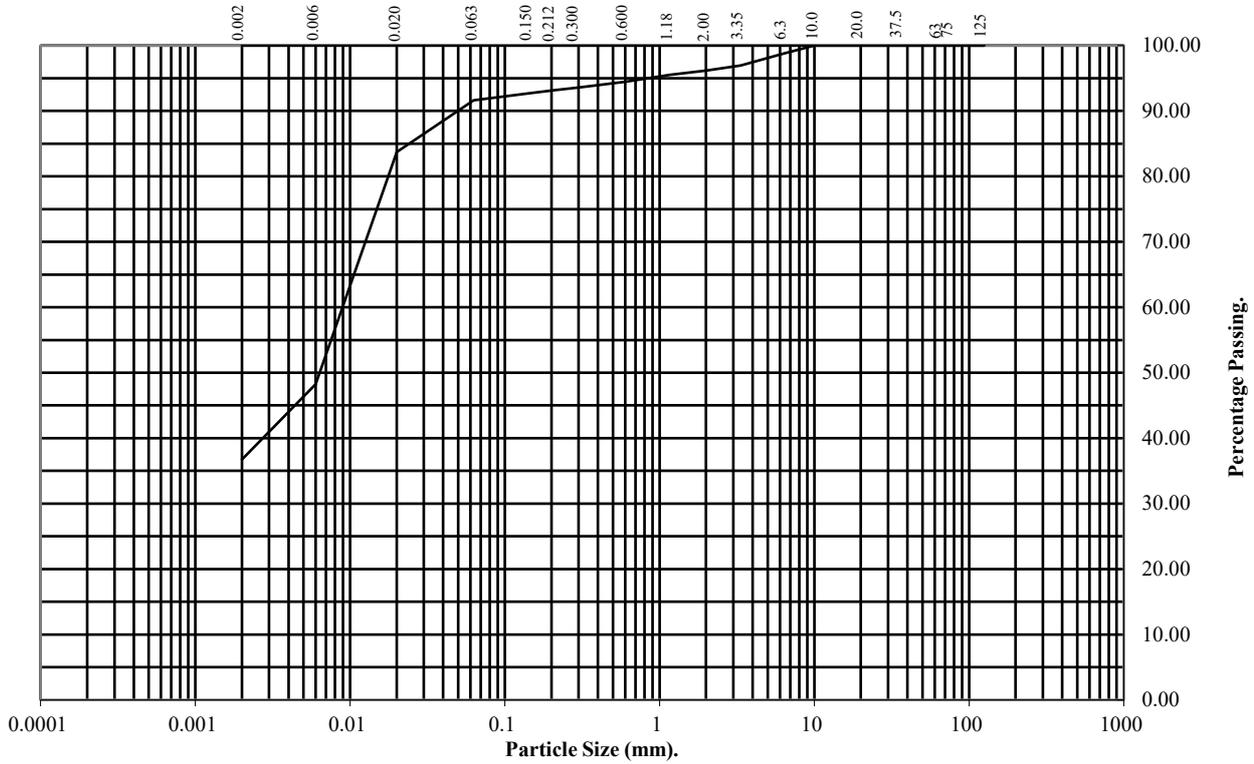
PARTICLE SIZE DISTRIBUTION TEST

BS1377 : Part 2 : 1990
Wet Sieve & Pipette Analysis, Clause 9.2 & 9.4

Hole Number: **BRC01** **Top Depth (m):** **5.50**

Sample Number: **Base Depth(m):**

Sample Type: **B**



| BS Test Sieve (mm) | Percentage Passing |
|--------------------|--------------------|
| 125 | 100 |
| 75 | 100 |
| 63 | 100 |
| 37.5 | 100 |
| 20 | 100 |
| 10 | 100 |
| 6.3 | 99 |
| 3.35 | 97 |
| 2 | 96 |
| 1.18 | 96 |
| 0.6 | 94 |
| 0.3 | 94 |
| 0.212 | 93 |
| 0.15 | 93 |
| 0.063 | 92 |

| Particle Diameter | Percentage Passing |
|-------------------|--------------------|
| 0.02 | 84 |
| 0.006 | 48 |
| 0.002 | 37 |

| Soil Fraction | Total Percentage |
|---------------|------------------|
| Cobbles | 0 |
| Gravel | 4 |
| Sand | 4 |
| Silt | 55 |
| Clay | 37 |

Remarks:
See Summary of Soil Descriptions



Dyke Road Galway

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| Contract No: |
| PSL24/3965 |
| Client Ref: |
| 13614-02-24 |

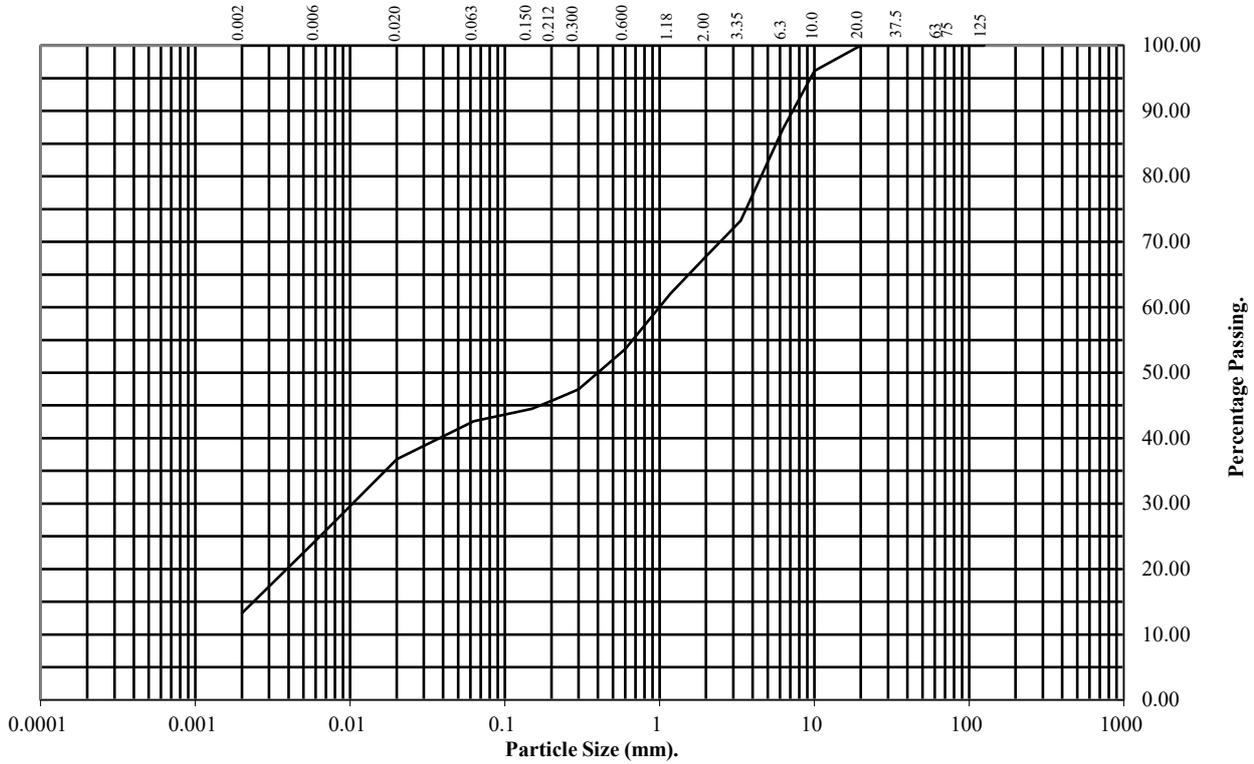
PARTICLE SIZE DISTRIBUTION TEST

BS1377 : Part 2 : 1990
Wet Sieve & Pipette Analysis, Clause 9.2 & 9.4

Hole Number: **BRC01** **Top Depth (m):** **7.50**

Sample Number: **Base Depth(m):**

Sample Type: **B**



| BS Test Sieve (mm) | Percentage Passing |
|--------------------|--------------------|
| 125 | 100 |
| 75 | 100 |
| 63 | 100 |
| 37.5 | 100 |
| 20 | 100 |
| 10 | 96 |
| 6.3 | 87 |
| 3.35 | 73 |
| 2 | 68 |
| 1.18 | 62 |
| 0.6 | 54 |
| 0.3 | 47 |
| 0.212 | 46 |
| 0.15 | 44 |
| 0.063 | 43 |

| Particle Diameter | Percentage Passing |
|-------------------|--------------------|
| 0.02 | 37 |
| 0.006 | 24 |
| 0.002 | 13 |

| Soil Fraction | Total Percentage |
|---------------|------------------|
| Cobbles | 0 |
| Gravel | 32 |
| Sand | 25 |
| Silt | 30 |
| Clay | 13 |

Remarks:
See Summary of Soil Descriptions



Dyke Road Galway

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| Contract No: |
| PSL24/3965 |
| Client Ref: |
| 13614-02-24 |

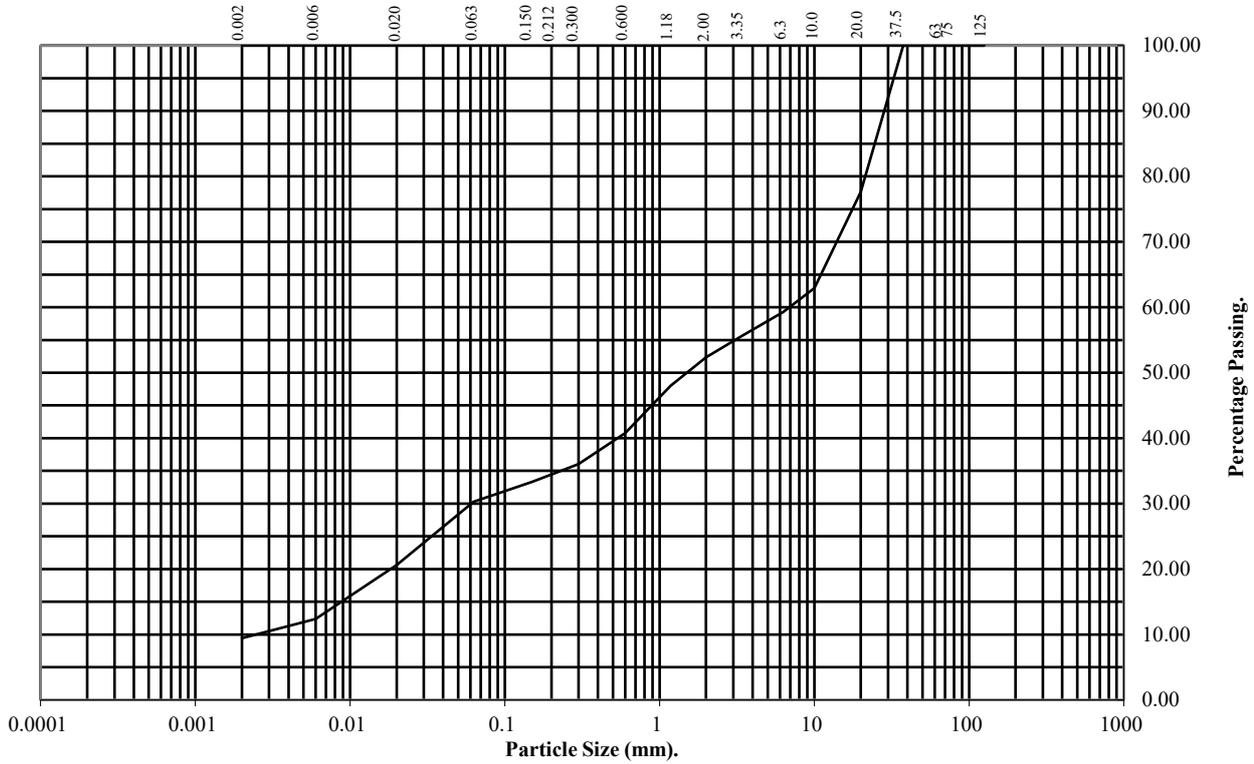
PARTICLE SIZE DISTRIBUTION TEST

BS1377 : Part 2 : 1990
Wet Sieve & Pipette Analysis, Clause 9.2 & 9.4

Hole Number: **BRC02** Top Depth (m): **2.00**

Sample Number: Base Depth(m):

Sample Type: **B**



| BS Test Sieve (mm) | Percentage Passing |
|--------------------|--------------------|
| 125 | 100 |
| 75 | 100 |
| 63 | 100 |
| 37.5 | 100 |
| 20 | 78 |
| 10 | 63 |
| 6.3 | 59 |
| 3.35 | 56 |
| 2 | 52 |
| 1.18 | 48 |
| 0.6 | 41 |
| 0.3 | 36 |
| 0.212 | 35 |
| 0.15 | 33 |
| 0.063 | 30 |

| Particle Diameter | Percentage Passing |
|-------------------|--------------------|
| 0.02 | 21 |
| 0.006 | 12 |
| 0.002 | 9 |

| Soil Fraction | Total Percentage |
|---------------|------------------|
| Cobbles | 0 |
| Gravel | 48 |
| Sand | 22 |
| Silt | 21 |
| Clay | 9 |

Remarks:
See Summary of Soil Descriptions



Dyke Road Galway

Contract No:
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13614-02-24

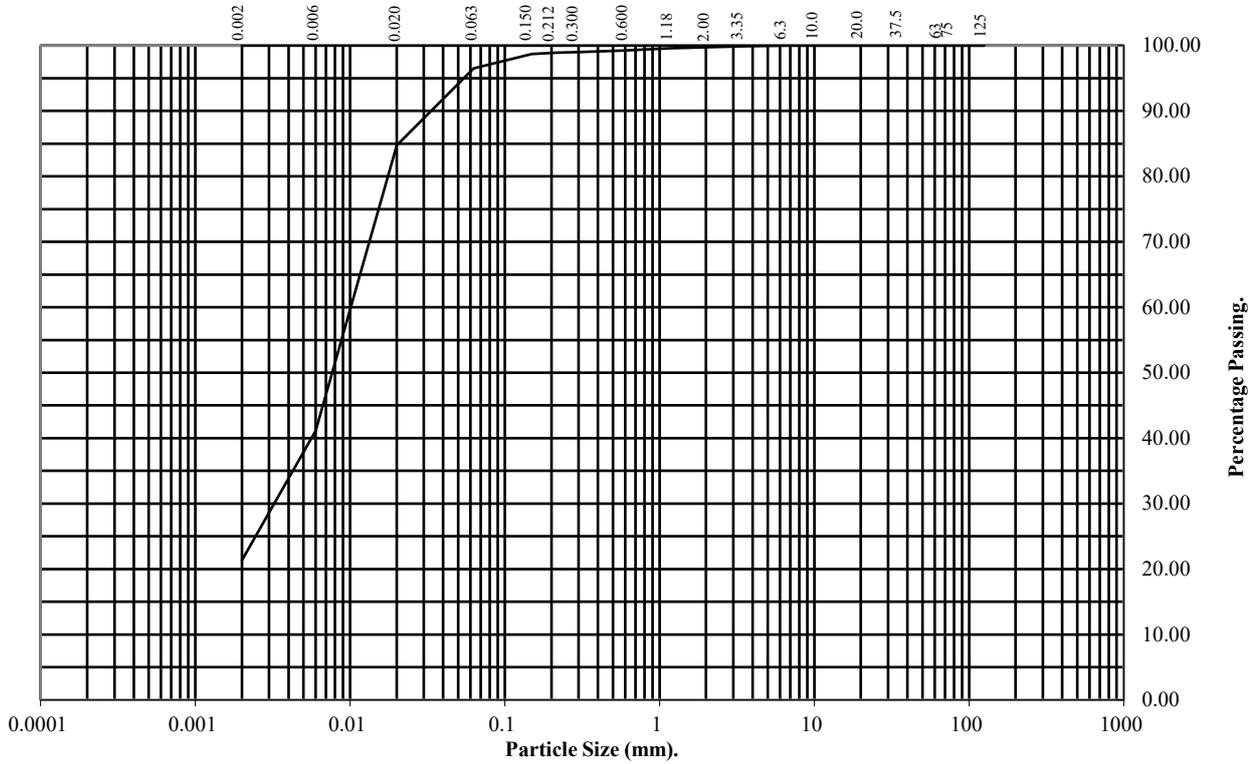
PARTICLE SIZE DISTRIBUTION TEST

BS1377 : Part 2 : 1990
Wet Sieve & Pipette Analysis, Clause 9.2 & 9.4

Hole Number: **BRC02** **Top Depth (m):** **3.50**

Sample Number: **Base Depth(m):**

Sample Type: **B**



| BS Test Sieve (mm) | Percentage Passing |
|--------------------|--------------------|
| 125 | 100 |
| 75 | 100 |
| 63 | 100 |
| 37.5 | 100 |
| 20 | 100 |
| 10 | 100 |
| 6.3 | 100 |
| 3.35 | 100 |
| 2 | 100 |
| 1.18 | 100 |
| 0.6 | 99 |
| 0.3 | 99 |
| 0.212 | 99 |
| 0.15 | 99 |
| 0.063 | 97 |

| Particle Diameter | Percentage Passing |
|-------------------|--------------------|
| 0.02 | 85 |
| 0.006 | 41 |
| 0.002 | 21 |

| Soil Fraction | Total Percentage |
|---------------|------------------|
| Cobbles | 0 |
| Gravel | 0 |
| Sand | 3 |
| Silt | 76 |
| Clay | 21 |

Remarks:
See Summary of Soil Descriptions



Dyke Road Galway

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|---------------------|
| Contract No: |
| PSL24/3965 |
| Client Ref: |
| 13614-02-24 |

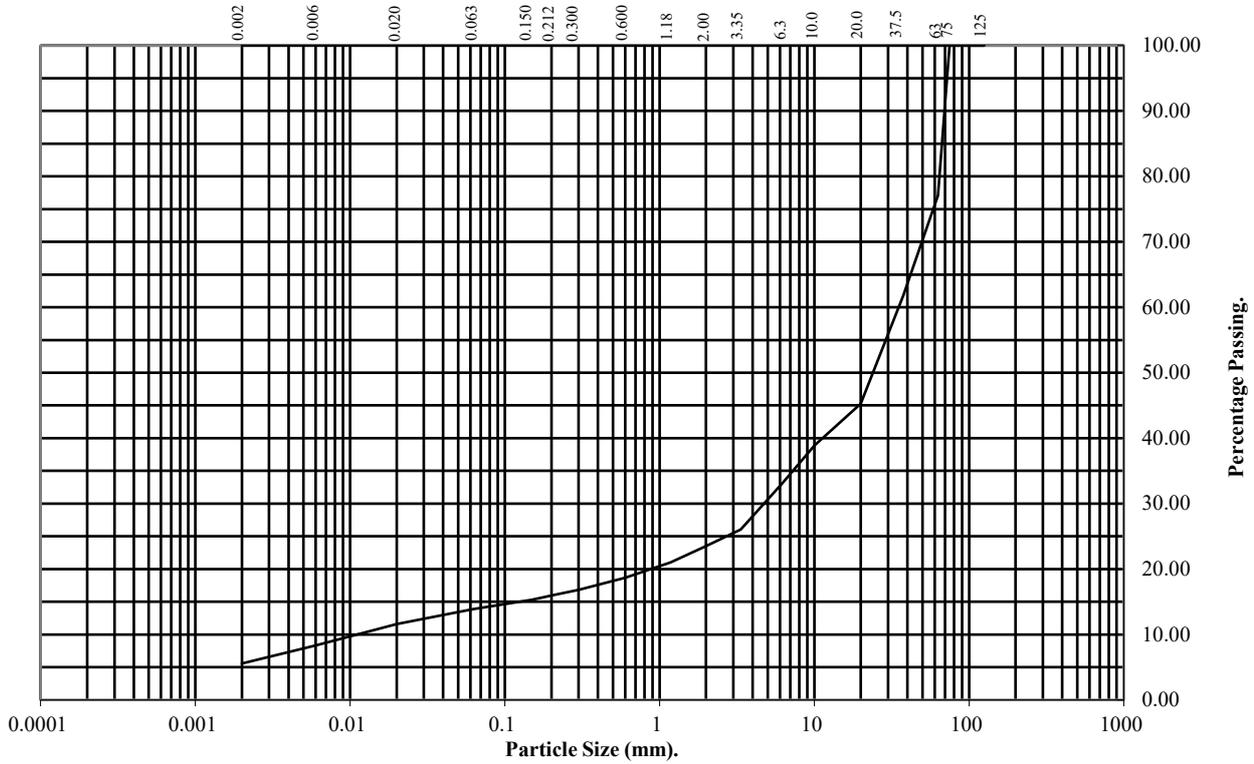
PARTICLE SIZE DISTRIBUTION TEST

BS1377 : Part 2 : 1990
Wet Sieve & Pipette Analysis, Clause 9.2 & 9.4

Hole Number: **BRC02** **Top Depth (m):** **7.00**

Sample Number: **Base Depth(m):**

Sample Type: **B**



| BS Test Sieve (mm) | Percentage Passing |
|--------------------|--------------------|
| 125 | 100 |
| 75 | 100 |
| 63 | 77 |
| 37.5 | 62 |
| 20 | 45 |
| 10 | 39 |
| 6.3 | 33 |
| 3.35 | 26 |
| 2 | 24 |
| 1.18 | 21 |
| 0.6 | 19 |
| 0.3 | 17 |
| 0.212 | 16 |
| 0.15 | 15 |
| 0.063 | 14 |

| Particle Diameter | Percentage Passing |
|-------------------|--------------------|
| 0.02 | 12 |
| 0.006 | 8 |
| 0.002 | 6 |

| Soil Fraction | Total Percentage |
|---------------|------------------|
| Cobbles | 23 |
| Gravel | 53 |
| Sand | 10 |
| Silt | 8 |
| Clay | 6 |

Remarks:
See Summary of Soil Descriptions



Dyke Road Galway

| |
|---------------------|
| Contract No: |
| PSL24/3965 |
| Client Ref: |
| 13614-02-24 |

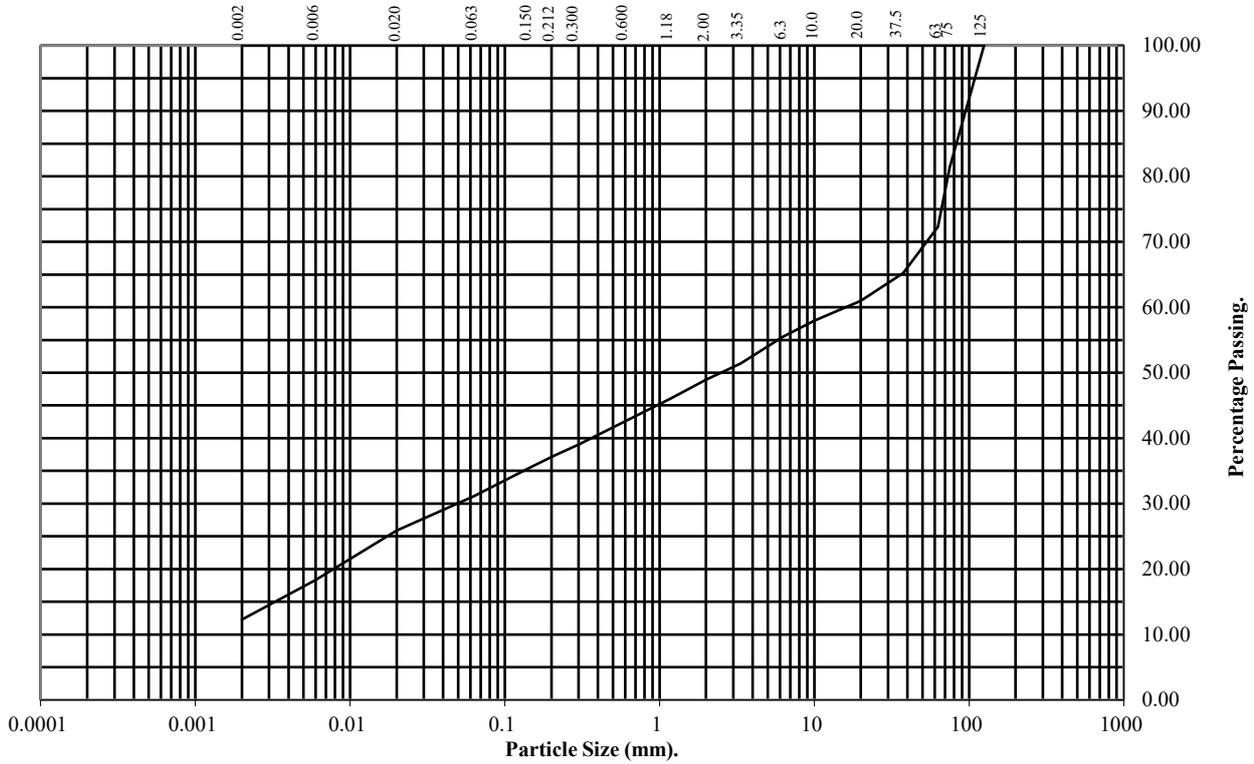
PARTICLE SIZE DISTRIBUTION TEST

BS1377 : Part 2 : 1990
Wet Sieve & Pipette Analysis, Clause 9.2 & 9.4

Hole Number: **BRC03** **Top Depth (m):** **4.00**

Sample Number: **Base Depth(m):**

Sample Type: **B**



| BS Test Sieve (mm) | Percentage Passing |
|--------------------|--------------------|
| 125 | 100 |
| 75 | 81 |
| 63 | 72 |
| 37.5 | 65 |
| 20 | 61 |
| 10 | 58 |
| 6.3 | 56 |
| 3.35 | 51 |
| 2 | 49 |
| 1.18 | 46 |
| 0.6 | 43 |
| 0.3 | 39 |
| 0.212 | 37 |
| 0.15 | 36 |
| 0.063 | 31 |

| Particle Diameter | Percentage Passing |
|-------------------|--------------------|
| 0.02 | 26 |
| 0.006 | 18 |
| 0.002 | 12 |

| Soil Fraction | Total Percentage |
|---------------|------------------|
| Cobbles | 28 |
| Gravel | 23 |
| Sand | 18 |
| Silt | 19 |
| Clay | 12 |

Remarks:
See Summary of Soil Descriptions



Dyke Road Galway

| |
|---------------------|
| Contract No: |
| PSL24/3965 |
| Client Ref: |
| 13614-02-24 |

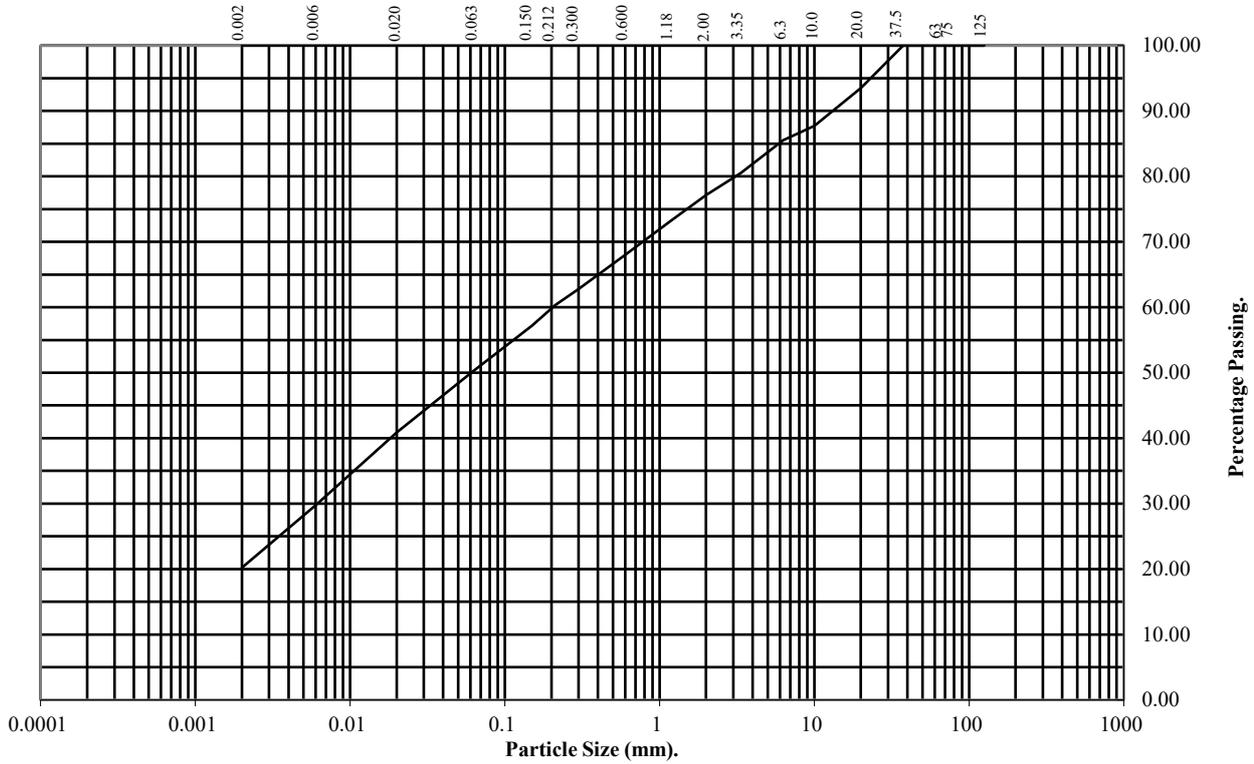
PARTICLE SIZE DISTRIBUTION TEST

BS1377 : Part 2 : 1990
Wet Sieve & Pipette Analysis, Clause 9.2 & 9.4

Hole Number: **BRC03** **Top Depth (m):** **7.50**

Sample Number: **Base Depth(m):**

Sample Type: **B**



| BS Test Sieve (mm) | Percentage Passing |
|--------------------|--------------------|
| 125 | 100 |
| 75 | 100 |
| 63 | 100 |
| 37.5 | 100 |
| 20 | 93 |
| 10 | 88 |
| 6.3 | 86 |
| 3.35 | 80 |
| 2 | 77 |
| 1.18 | 73 |
| 0.6 | 68 |
| 0.3 | 63 |
| 0.212 | 60 |
| 0.15 | 57 |
| 0.063 | 50 |

| Particle Diameter | Percentage Passing |
|-------------------|--------------------|
| 0.02 | 41 |
| 0.006 | 30 |
| 0.002 | 20 |

| Soil Fraction | Total Percentage |
|---------------|------------------|
| Cobbles | 0 |
| Gravel | 23 |
| Sand | 27 |
| Silt | 30 |
| Clay | 20 |

Remarks:
See Summary of Soil Descriptions



Dyke Road Galway

| |
|---------------------|
| Contract No: |
| PSL24/3965 |
| Client Ref: |
| 13614-02-24 |

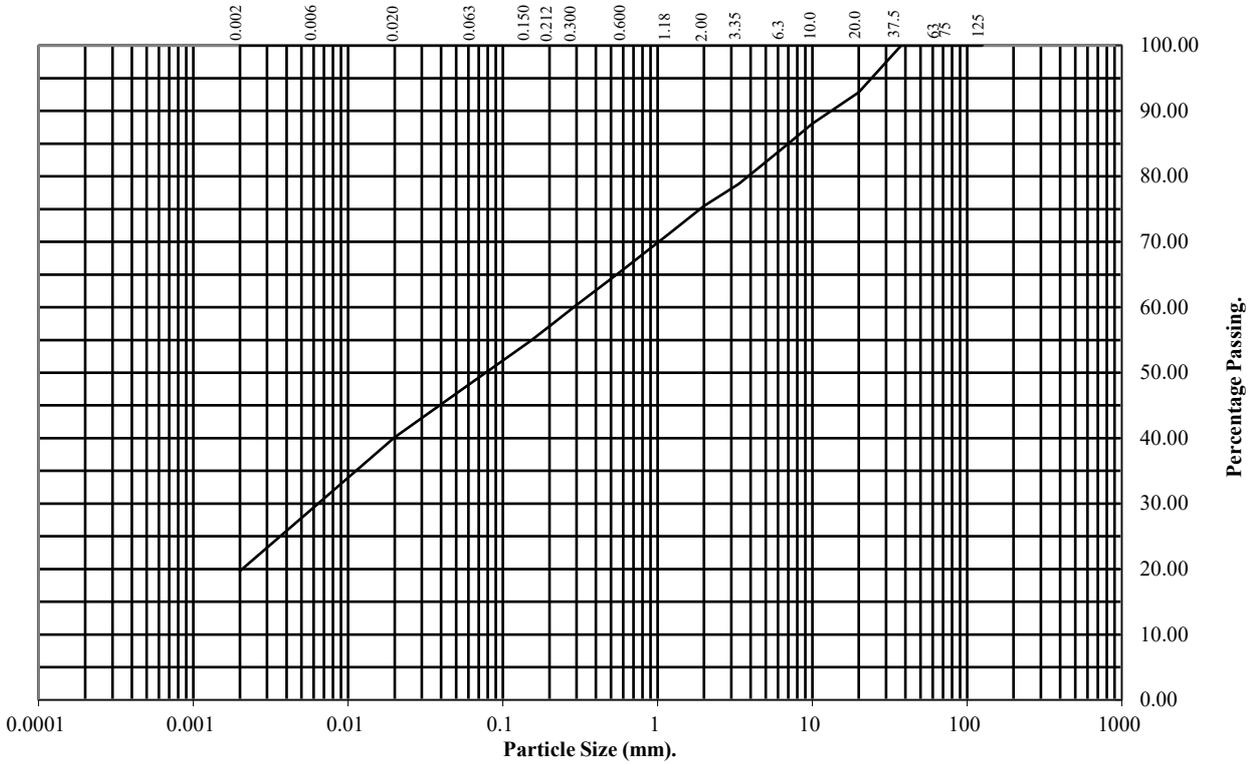
PARTICLE SIZE DISTRIBUTION TEST

BS1377 : Part 2 : 1990
Wet Sieve & Pipette Analysis, Clause 9.2 & 9.4

Hole Number: **BRC03** **Top Depth (m):** **9.50**

Sample Number: **Base Depth(m):**

Sample Type: **B**



| BS Test Sieve (mm) | Percentage Passing |
|--------------------|--------------------|
| 125 | 100 |
| 75 | 100 |
| 63 | 100 |
| 37.5 | 100 |
| 20 | 93 |
| 10 | 88 |
| 6.3 | 84 |
| 3.35 | 79 |
| 2 | 75 |
| 1.18 | 71 |
| 0.6 | 66 |
| 0.3 | 60 |
| 0.212 | 58 |
| 0.15 | 55 |
| 0.063 | 48 |

| Particle Diameter | Percentage Passing |
|-------------------|--------------------|
| 0.02 | 40 |
| 0.006 | 29 |
| 0.002 | 20 |

| Soil Fraction | Total Percentage |
|---------------|------------------|
| Cobbles | 0 |
| Gravel | 25 |
| Sand | 27 |
| Silt | 28 |
| Clay | 20 |

Remarks:
See Summary of Soil Descriptions



Dyke Road Galway

| |
|---------------------|
| Contract No: |
| PSL24/3965 |
| Client Ref: |
| 13614-02-24 |

PARTICLE SIZE DISTRIBUTION TEST

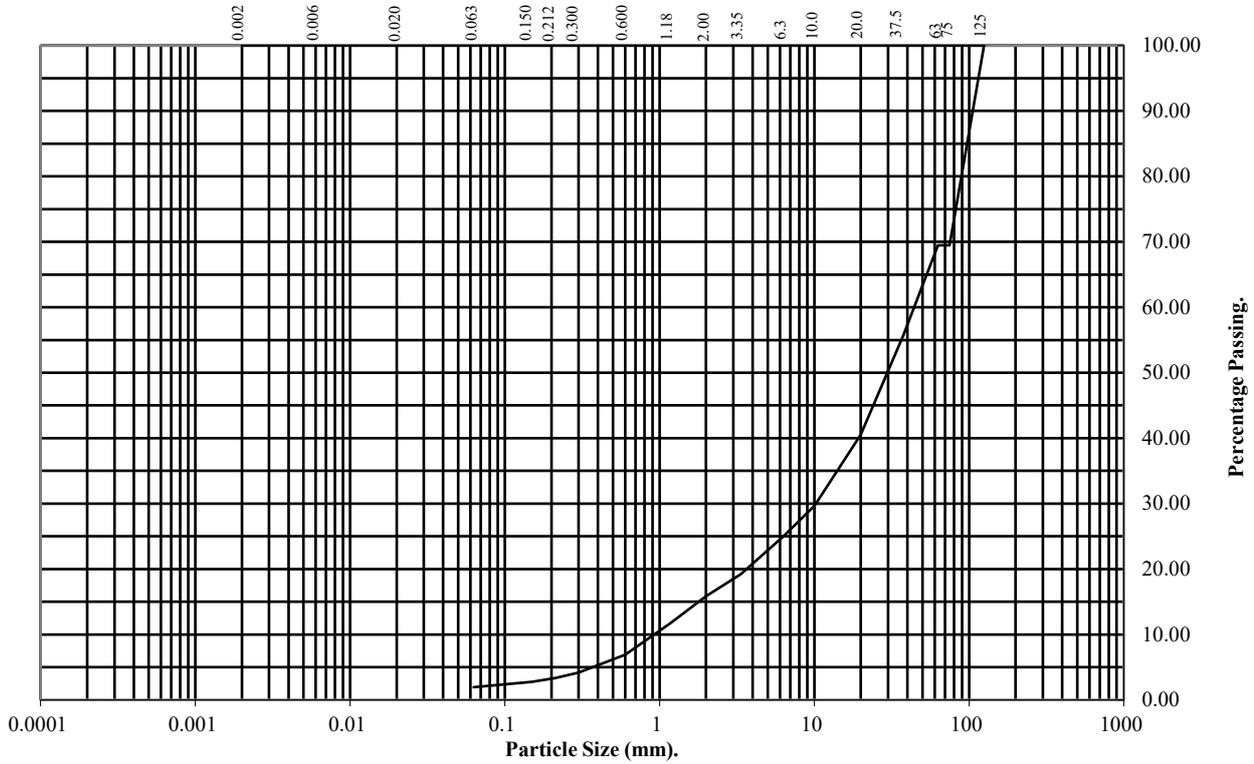
BS1377 : Part 2 : 1990

Wet Sieve, Clause 9.2

Hole Number: **BRC04** Top Depth (m): **4.00**

Sample Number: Base Depth(m):

Sample Type: **B**



| BS Test Sieve (mm) | Percentage Passing |
|--------------------|--------------------|
| 125 | 100 |
| 75 | 69 |
| 63 | 69 |
| 37.5 | 56 |
| 20 | 41 |
| 10 | 30 |
| 6.3 | 25 |
| 3.35 | 19 |
| 2 | 16 |
| 1.18 | 12 |
| 0.6 | 7 |
| 0.3 | 4 |
| 0.212 | 3 |
| 0.15 | 3 |
| 0.063 | 2 |

| Soil Fraction | Total Percentage |
|---------------|------------------|
| Cobbles | 31 |
| Gravel | 53 |
| Sand | 14 |
| Silt/Clay | 2 |

Remarks:
See Summary of Soil Descriptions



Dyke Road Galway

| |
|---------------------|
| Contract No: |
| PSL24/3965 |
| Client Ref: |
| 13614-02-24 |

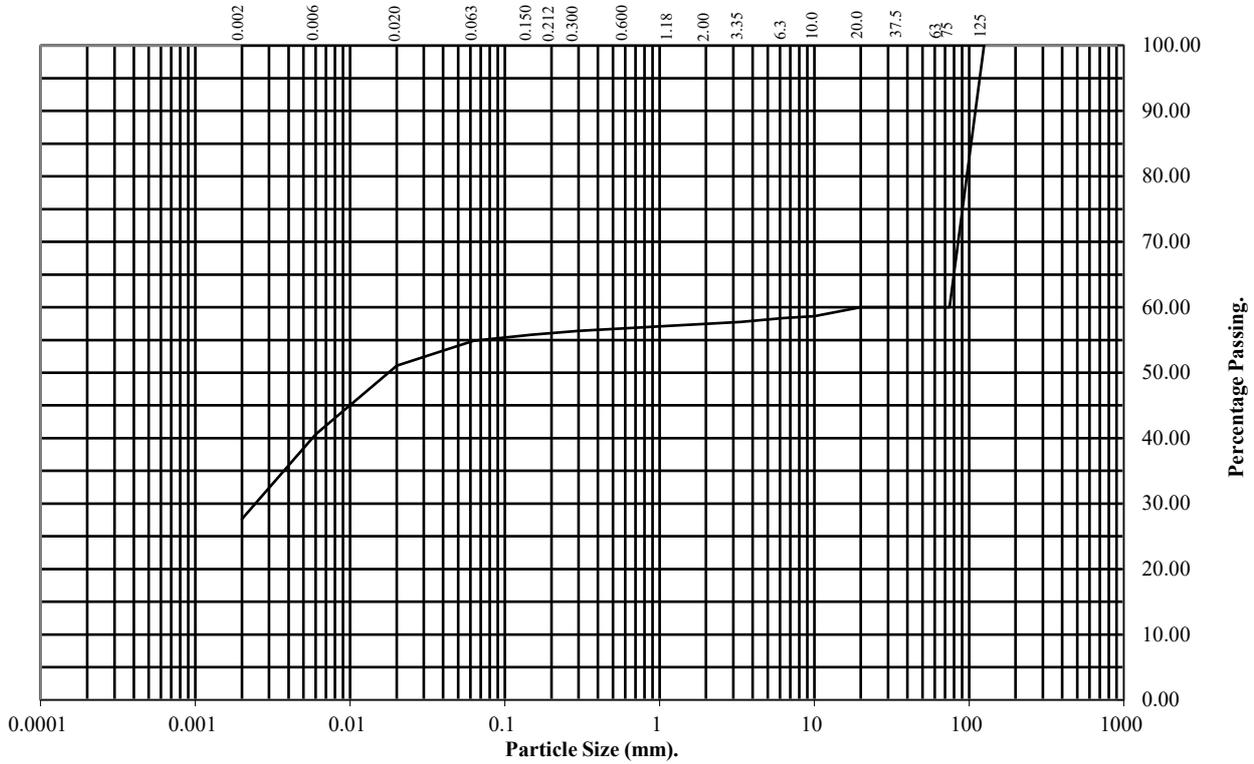
PARTICLE SIZE DISTRIBUTION TEST

BS1377 : Part 2 : 1990
Wet Sieve & Pipette Analysis, Clause 9.2 & 9.4

Hole Number: **BRC05** **Top Depth (m):** **2.60**

Sample Number: **Base Depth(m):**

Sample Type: **B**



| BS Test Sieve (mm) | Percentage Passing |
|--------------------|--------------------|
| 125 | 100 |
| 75 | 60 |
| 63 | 60 |
| 37.5 | 60 |
| 20 | 60 |
| 10 | 59 |
| 6.3 | 58 |
| 3.35 | 58 |
| 2 | 57 |
| 1.18 | 57 |
| 0.6 | 57 |
| 0.3 | 56 |
| 0.212 | 56 |
| 0.15 | 56 |
| 0.063 | 55 |

| Particle Diameter | Percentage Passing |
|-------------------|--------------------|
| 0.02 | 51 |
| 0.006 | 41 |
| 0.002 | 28 |

| Soil Fraction | Total Percentage |
|---------------|------------------|
| Cobbles | 40 |
| Gravel | 3 |
| Sand | 2 |
| Silt | 27 |
| Clay | 28 |

Remarks:
See Summary of Soil Descriptions



Dyke Road Galway

| |
|---------------------|
| Contract No: |
| PSL24/3965 |
| Client Ref: |
| 13614-02-24 |

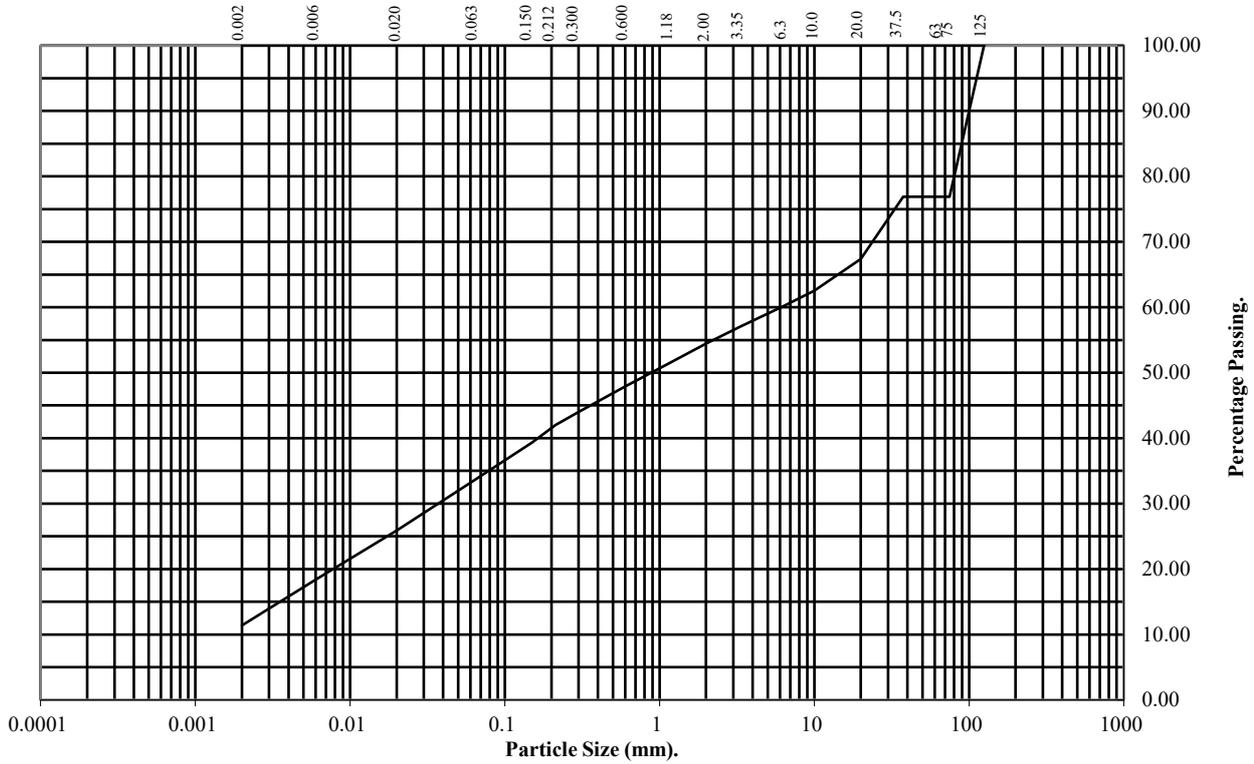
PARTICLE SIZE DISTRIBUTION TEST

BS1377 : Part 2 : 1990
Wet Sieve & Pipette Analysis, Clause 9.2 & 9.4

Hole Number: **BRC05** **Top Depth (m):** **3.50**

Sample Number: **Base Depth(m):**

Sample Type: **B**



| BS Test Sieve (mm) | Percentage Passing |
|--------------------|--------------------|
| 125 | 100 |
| 75 | 77 |
| 63 | 77 |
| 37.5 | 77 |
| 20 | 67 |
| 10 | 63 |
| 6.3 | 60 |
| 3.35 | 57 |
| 2 | 54 |
| 1.18 | 52 |
| 0.6 | 48 |
| 0.3 | 44 |
| 0.212 | 42 |
| 0.15 | 39 |
| 0.063 | 33 |

| Particle Diameter | Percentage Passing |
|-------------------|--------------------|
| 0.02 | 26 |
| 0.006 | 18 |
| 0.002 | 11 |

| Soil Fraction | Total Percentage |
|---------------|------------------|
| Cobbles | 23 |
| Gravel | 23 |
| Sand | 21 |
| Silt | 22 |
| Clay | 11 |

Remarks:
See Summary of Soil Descriptions



Dyke Road Galway

| |
|---------------------|
| Contract No: |
| PSL24/3965 |
| Client Ref: |
| 13614-02-24 |

PARTICLE SIZE DISTRIBUTION TEST

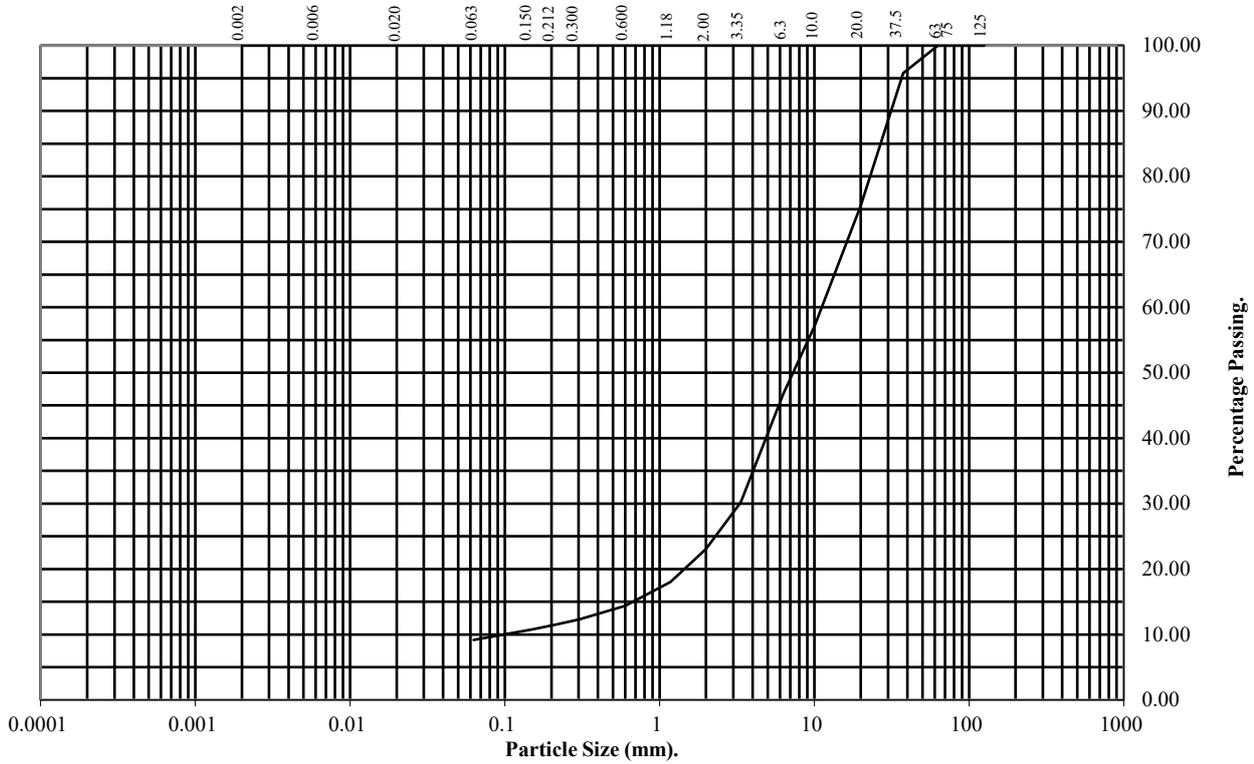
BS1377 : Part 2 : 1990

Wet Sieve, Clause 9.2

Hole Number: **TP01** Top Depth (m): **0.50**

Sample Number: Base Depth(m):

Sample Type: **B**



| BS Test Sieve (mm) | Percentage Passing |
|--------------------|--------------------|
| 125 | 100 |
| 75 | 100 |
| 63 | 100 |
| 37.5 | 96 |
| 20 | 76 |
| 10 | 57 |
| 6.3 | 47 |
| 3.35 | 30 |
| 2 | 23 |
| 1.18 | 18 |
| 0.6 | 14 |
| 0.3 | 12 |
| 0.212 | 12 |
| 0.15 | 11 |
| 0.063 | 9 |

| Soil Fraction | Total Percentage |
|---------------|------------------|
| Cobbles | 0 |
| Gravel | 77 |
| Sand | 14 |
| Silt/Clay | 9 |

Remarks:
See Summary of Soil Descriptions



Dyke Road Galway

| |
|---------------------|
| Contract No: |
| PSL24/3965 |
| Client Ref: |
| 13614-02-24 |

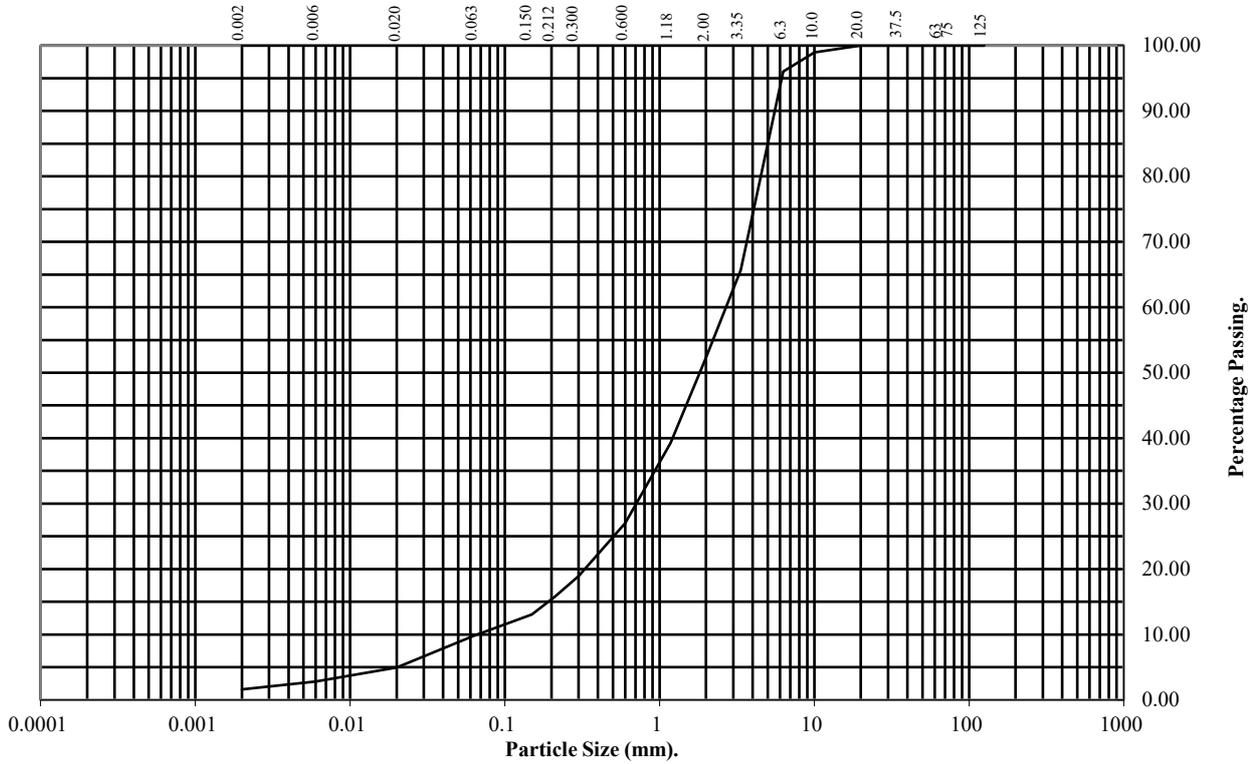
PARTICLE SIZE DISTRIBUTION TEST

BS1377 : Part 2 : 1990
Wet Sieve & Pipette Analysis, Clause 9.2 & 9.4

Hole Number: TP01 **Top Depth (m):** 1.00

Sample Number: **Base Depth(m):**

Sample Type: B



| BS Test Sieve (mm) | Percentage Passing |
|--------------------|--------------------|
| 125 | 100 |
| 75 | 100 |
| 63 | 100 |
| 37.5 | 100 |
| 20 | 100 |
| 10 | 99 |
| 6.3 | 96 |
| 3.35 | 66 |
| 2 | 52 |
| 1.18 | 39 |
| 0.6 | 27 |
| 0.3 | 19 |
| 0.212 | 16 |
| 0.15 | 13 |
| 0.063 | 10 |

| Particle Diameter | Percentage Passing |
|-------------------|--------------------|
| 0.02 | 5 |
| 0.006 | 3 |
| 0.002 | 2 |

| Soil Fraction | Total Percentage |
|---------------|------------------|
| Cobbles | 0 |
| Gravel | 48 |
| Sand | 42 |
| Silt | 8 |
| Clay | 2 |

Remarks:
See Summary of Soil Descriptions



Dyke Road Galway

| |
|---------------------|
| Contract No: |
| PSL24/3965 |
| Client Ref: |
| 13614-02-24 |

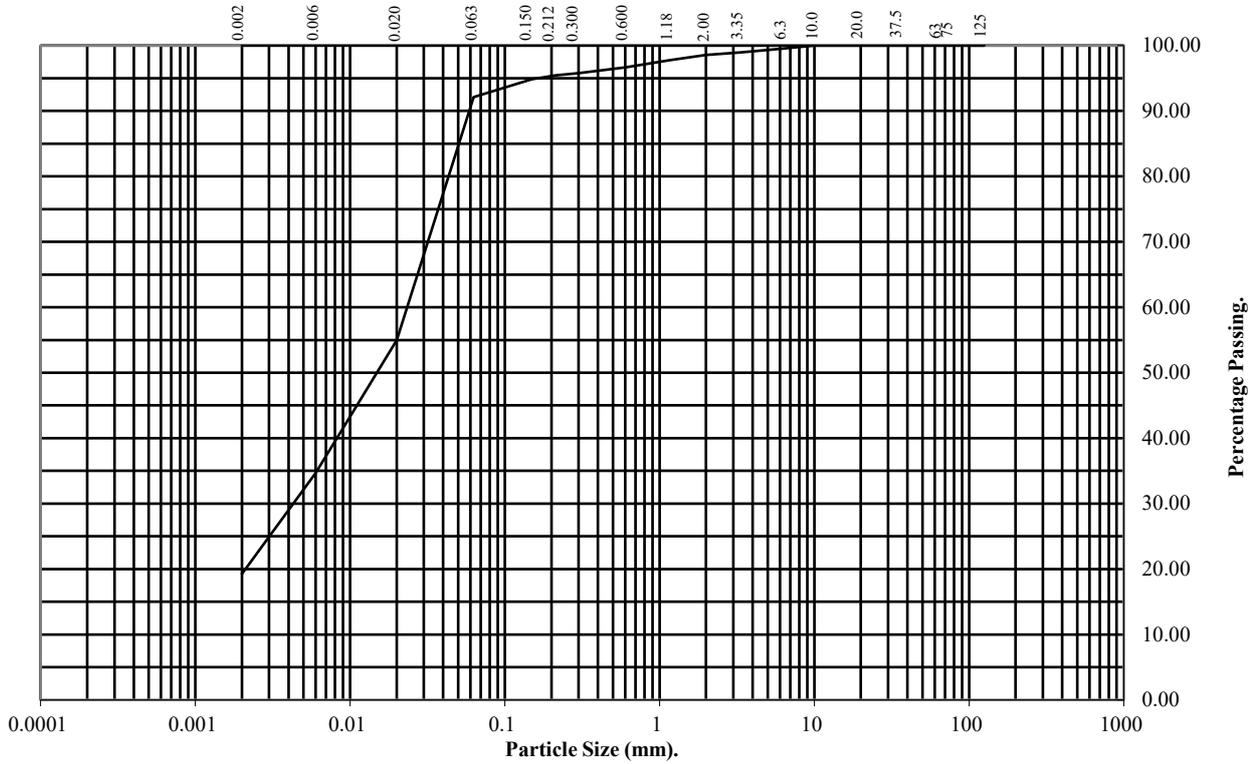
PARTICLE SIZE DISTRIBUTION TEST

BS1377 : Part 2 : 1990
Wet Sieve & Pipette Analysis, Clause 9.2 & 9.4

Hole Number: TP02 **Top Depth (m):** 2.00

Sample Number: **Base Depth(m):**

Sample Type: B



| BS Test Sieve (mm) | Percentage Passing |
|--------------------|--------------------|
| 125 | 100 |
| 75 | 100 |
| 63 | 100 |
| 37.5 | 100 |
| 20 | 100 |
| 10 | 100 |
| 6.3 | 100 |
| 3.35 | 99 |
| 2 | 99 |
| 1.18 | 98 |
| 0.6 | 97 |
| 0.3 | 96 |
| 0.212 | 95 |
| 0.15 | 95 |
| 0.063 | 92 |

| Particle Diameter | Percentage Passing |
|-------------------|--------------------|
| 0.02 | 55 |
| 0.006 | 35 |
| 0.002 | 19 |

| Soil Fraction | Total Percentage |
|---------------|------------------|
| Cobbles | 0 |
| Gravel | 1 |
| Sand | 7 |
| Silt | 73 |
| Clay | 19 |

Remarks:
See Summary of Soil Descriptions



Dyke Road Galway

| |
|---------------------|
| Contract No: |
| PSL24/3965 |
| Client Ref: |
| 13614-02-24 |

PARTICLE SIZE DISTRIBUTION TEST

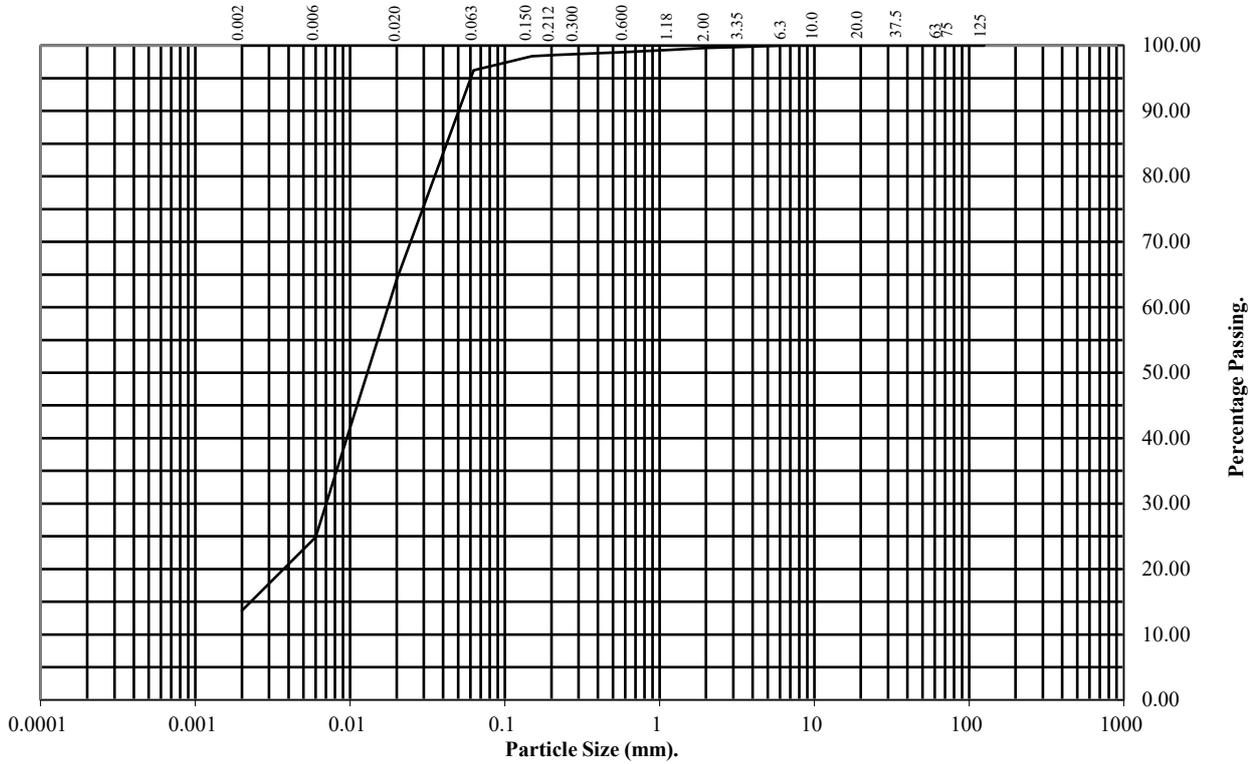
BS1377 : Part 2 : 1990

Wet Sieve & Pipette Analysis, Clause 9.2 & 9.4

Hole Number: **TP03** **Top Depth (m):** **2.00**

Sample Number: **Base Depth(m):**

Sample Type: **B**



| BS Test Sieve (mm) | Percentage Passing |
|--------------------|--------------------|
| 125 | 100 |
| 75 | 100 |
| 63 | 100 |
| 37.5 | 100 |
| 20 | 100 |
| 10 | 100 |
| 6.3 | 100 |
| 3.35 | 100 |
| 2 | 100 |
| 1.18 | 99 |
| 0.6 | 99 |
| 0.3 | 99 |
| 0.212 | 99 |
| 0.15 | 98 |
| 0.063 | 96 |

| Particle Diameter | Percentage Passing |
|-------------------|--------------------|
| 0.02 | 64 |
| 0.006 | 25 |
| 0.002 | 14 |

| Soil Fraction | Total Percentage |
|---------------|------------------|
| Cobbles | 0 |
| Gravel | 0 |
| Sand | 4 |
| Silt | 82 |
| Clay | 14 |

Remarks:
See Summary of Soil Descriptions



Dyke Road Galway

| |
|---------------------|
| Contract No: |
| PSL24/3965 |
| Client Ref: |
| 13614-02-24 |

PARTICLE SIZE DISTRIBUTION TEST

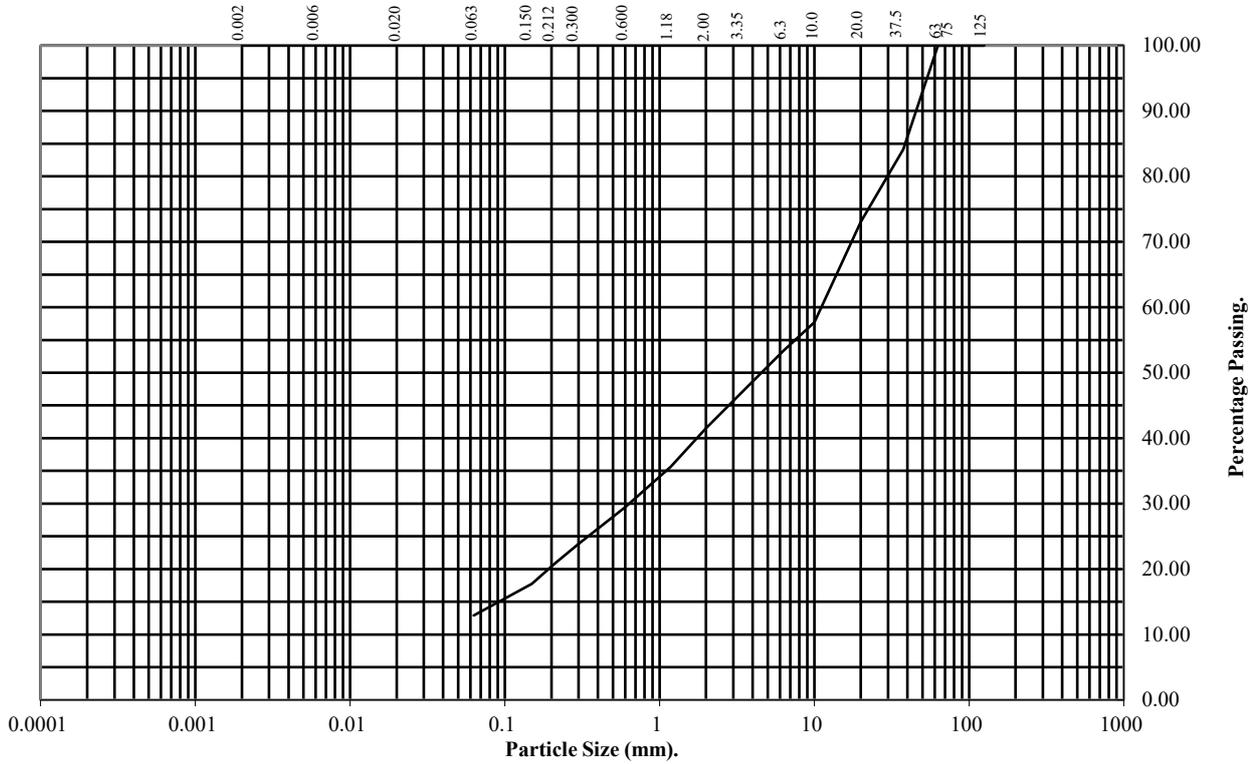
BS1377 : Part 2 : 1990

Wet Sieve, Clause 9.2

Hole Number: **TP05** Top Depth (m): **1.00**

Sample Number: Base Depth(m):

Sample Type: **B**



| BS Test Sieve (mm) | Percentage Passing |
|--------------------|--------------------|
| 125 | 100 |
| 75 | 100 |
| 63 | 100 |
| 37.5 | 84 |
| 20 | 73 |
| 10 | 58 |
| 6.3 | 53 |
| 3.35 | 47 |
| 2 | 42 |
| 1.18 | 36 |
| 0.6 | 29 |
| 0.3 | 24 |
| 0.212 | 21 |
| 0.15 | 18 |
| 0.063 | 13 |

| Soil Fraction | Total Percentage |
|---------------|------------------|
| Cobbles | 0 |
| Gravel | 58 |
| Sand | 29 |
| Silt/Clay | 13 |

Remarks:
See Summary of Soil Descriptions



Dyke Road Galway

| |
|---------------------|
| Contract No: |
| PSL24/3965 |
| Client Ref: |
| 13614-02-24 |

MOISTURE CONDITION VALUE

BS1377 : Part 4 : 1990 Clause 5.4

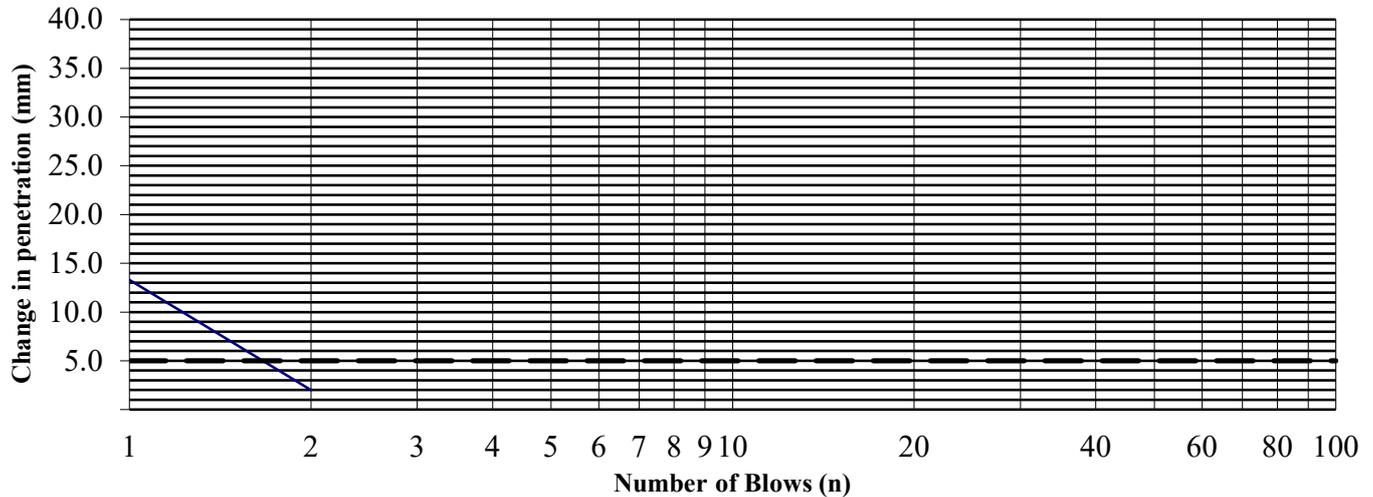
Hole Number: **BRC03** Top Depth (m): **8.50**

Sample Number: Base Depth (m):

Sample Type: **B**

| | |
|--|----|
| Material Retained on the 20mm BS Test Sieve (%): | 12 |
| Interpretation based on steepest straight line intercept with 5mm change in penetration. | |

MCV Determination



| Blows (N) | Penetration (mm) | n to 4n (mm) |
|-----------|------------------|--------------|
| 1 | 44.3 | 13.3 |
| 2 | 32.7 | 2.0 |
| 3 | 31.1 | |
| 4 | 31.0 | |
| 6 | 30.8 | |
| 8 | 30.7 | |
| 12 | | |
| 16 | | |
| 24 | | |
| 32 | | |
| 48 | | |
| 64 | | |
| 96 | | |
| 128 | | |
| 192 | | |
| 256 | | |

Test Results.

| | |
|----------------------|-----|
| Moisture Content (%) | 14 |
| MCV | 2.2 |



Dyke Road Galway

Contract No:
PSL24/3965
Client Ref:
13614-02-24

MOISTURE CONDITION VALUE

BS1377 : Part 4 : 1990 Clause 5.4

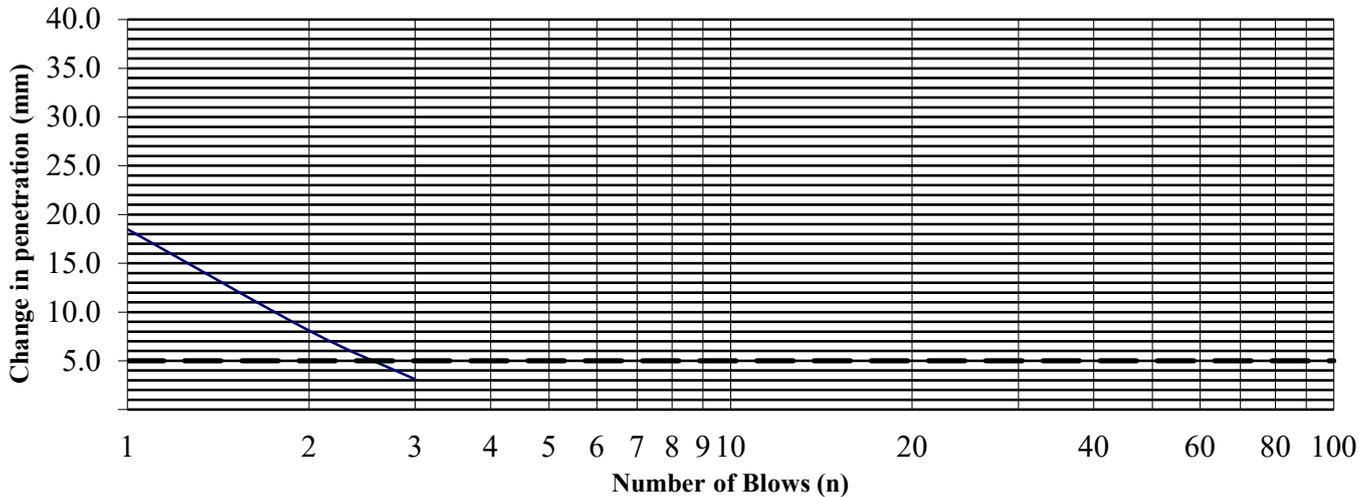
Hole Number: **BRC05** Top Depth (m): **4.50**

Sample Number: Base Depth (m):

Sample Type: **B**

| | |
|--|----|
| Material Retained on the 20mm BS Test Sieve (%): | 49 |
| Interpretation based on steepest straight line intercept with 5mm change in penetration. | |

MCV Determination



| Blows (N) | Penetration (mm) | n to 4n (mm) |
|-----------|------------------|--------------|
| 1 | 50.9 | 18.5 |
| 2 | 39.9 | 8.1 |
| 3 | 34.6 | 3.1 |
| 4 | 32.4 | |
| 6 | 32.0 | |
| 8 | 31.8 | |
| 12 | 31.5 | |
| 16 | | |
| 24 | | |
| 32 | | |
| 48 | | |
| 64 | | |
| 96 | | |
| 128 | | |
| 192 | | |
| 256 | | |

Test Results.

| | |
|----------------------|-----|
| Moisture Content (%) | 10 |
| MCV | 4.1 |



Dyke Road Galway

Contract No:
PSL24/3965
Client Ref:
13614-02-24

MOISTURE CONDITION VALUE

BS1377 : Part 4 : 1990 Clause 5.4

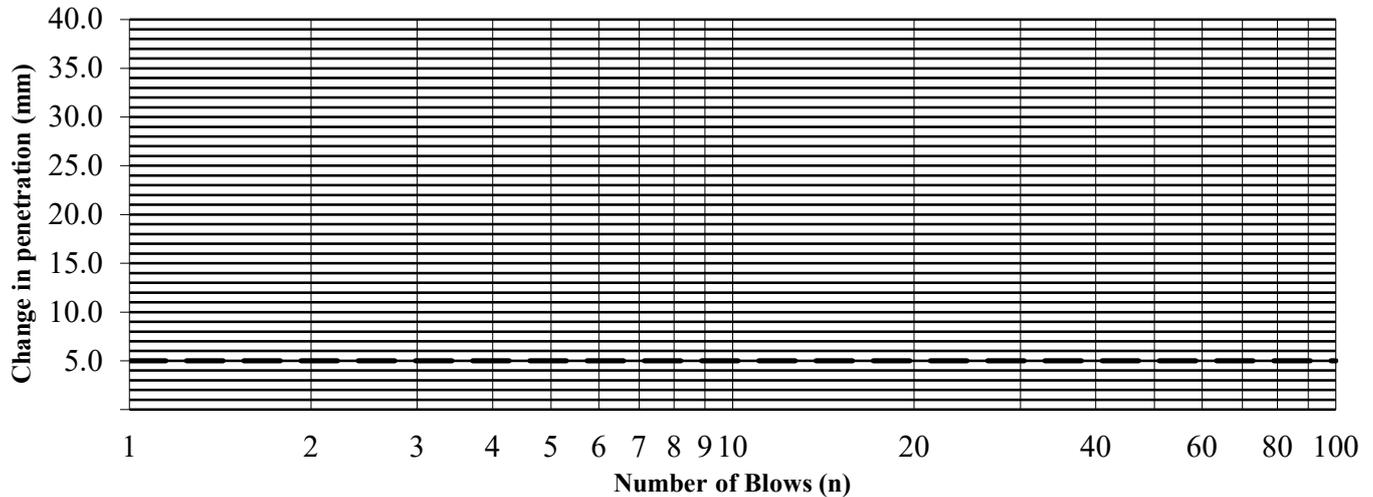
Hole Number: TP01 Top Depth (m): 1.00

Sample Number: Base Depth (m):

Sample Type: B

| | |
|--|---|
| Material Retained on the 20mm BS Test Sieve (%): | 0 |
| Interpretation based on steepest straight line intercept with 5mm change in penetration. | |

MCV Determination



| Blows (N) | Penetration (mm) | n to 4n (mm) |
|-----------|------------------|--------------|
| 1 | 136.8 | 2.6 |
| 2 | 135.1 | |
| 3 | 134.6 | |
| 4 | 134.2 | |
| 6 | | |
| 8 | | |
| 12 | | |
| 16 | | |
| 24 | | |
| 32 | | |
| 48 | | |
| 64 | | |
| 96 | | |
| 128 | | |
| 192 | | |
| 256 | | |

Test Results.

| | |
|----------------------|-----|
| Moisture Content (%) | 614 |
| MCV | <1 |



Dyke Road Galway

Contract No:
PSL24/3965
Client Ref:
13614-02-24

MOISTURE CONDITION VALUE

BS1377 : Part 4 : 1990 Clause 5.4

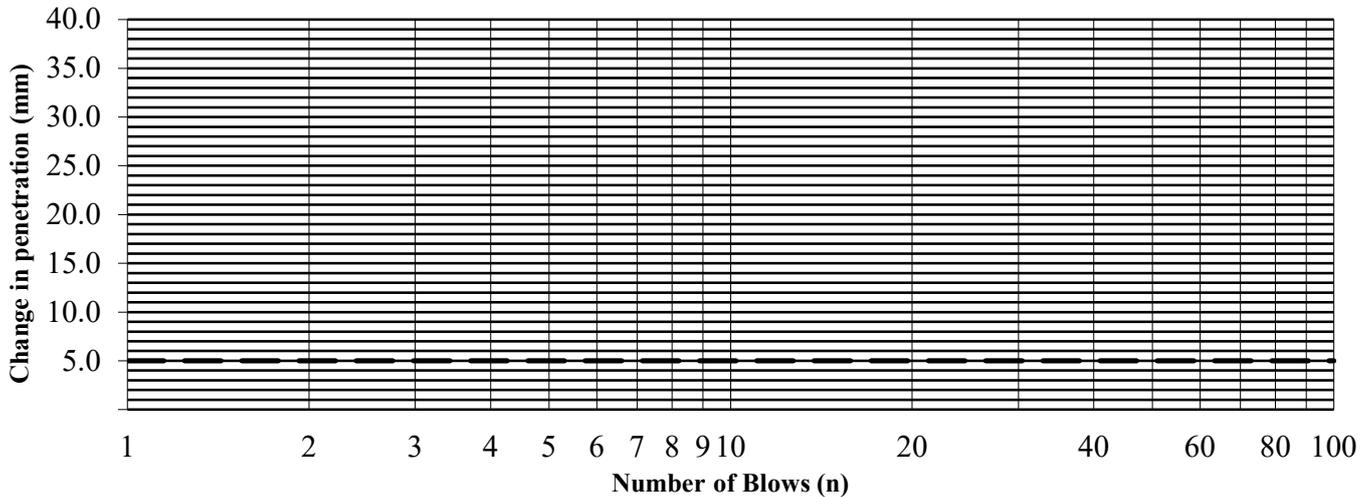
Hole Number: TP02 Top Depth (m): 2.00

Sample Number: Base Depth (m):

Sample Type: B

| | |
|--|---|
| Material Retained on the 20mm BS Test Sieve (%): | 0 |
| Interpretation based on steepest straight line intercept with 5mm change in penetration. | |

MCV Determination



| Blows (N) | Penetration (mm) | n to 4n (mm) |
|-----------|------------------|--------------|
| 1 | 97.3 | 3.2 |
| 2 | 95.0 | |
| 3 | 94.4 | |
| 4 | 94.1 | |
| 6 | | |
| 8 | | |
| 12 | | |
| 16 | | |
| 24 | | |
| 32 | | |
| 48 | | |
| 64 | | |
| 96 | | |
| 128 | | |
| 192 | | |
| 256 | | |

Test Results.

| | |
|----------------------|-----|
| Moisture Content (%) | 131 |
| MCV | <1 |



Dyke Road Galway

Contract No:
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Client Ref:
13614-02-24

MOISTURE CONDITION VALUE

BS1377 : Part 4 : 1990 Clause 5.4

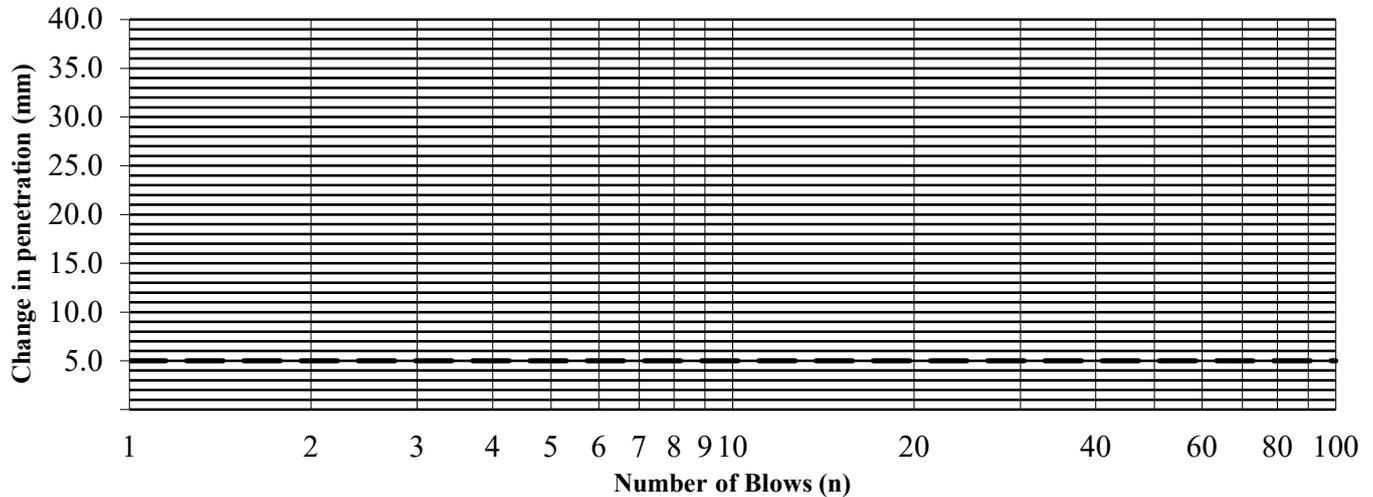
Hole Number: TP03 Top Depth (m): 2.00

Sample Number: Base Depth (m):

Sample Type: B

| | |
|--|---|
| Material Retained on the 20mm BS Test Sieve (%): | 0 |
| Interpretation based on steepest straight line intercept with 5mm change in penetration. | |

MCV Determination



| Blows (N) | Penetration (mm) | n to 4n (mm) |
|-----------|------------------|--------------|
| 1 | 94.1 | 2.2 |
| 2 | 92.9 | |
| 3 | 92.3 | |
| 4 | 91.9 | |
| 6 | | |
| 8 | | |
| 12 | | |
| 16 | | |
| 24 | | |
| 32 | | |
| 48 | | |
| 64 | | |
| 96 | | |
| 128 | | |
| 192 | | |
| 256 | | |

Test Results.

| | |
|----------------------|-----|
| Moisture Content (%) | 145 |
| MCV | <1 |



Dyke Road Galway

Contract No:
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Client Ref:
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MOISTURE CONDITION VALUE

BS1377 : Part 4 : 1990 Clause 5.4

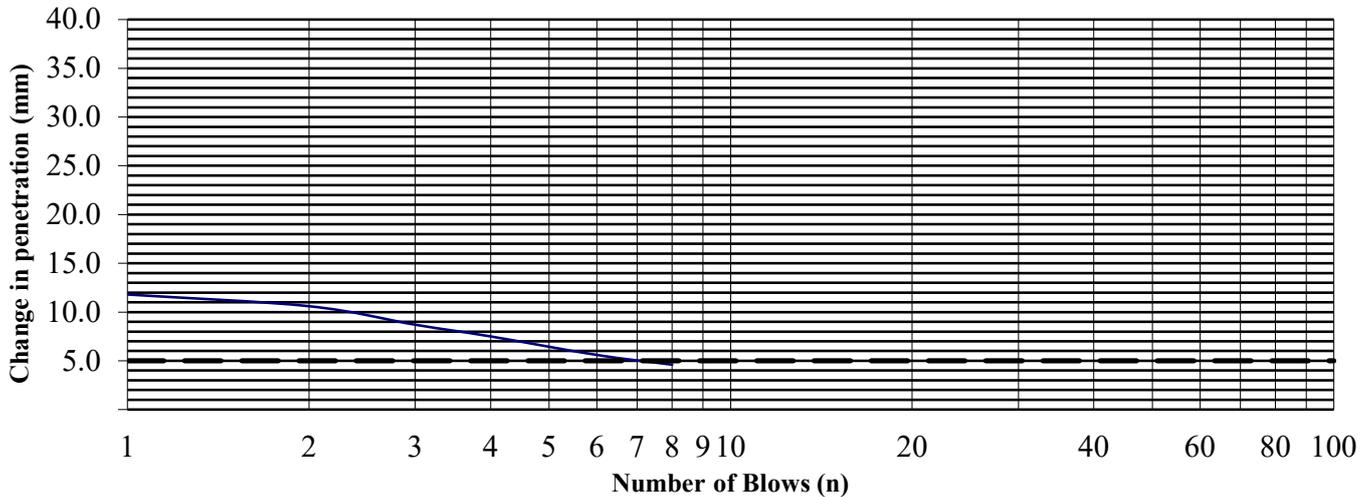
Hole Number: TP05 Top Depth (m): 1.00

Sample Number: Base Depth (m):

Sample Type: B

| | |
|--|----|
| Material Retained on the 20mm BS Test Sieve (%): | 27 |
| Interpretation based on steepest straight line intercept with 5mm change in penetration. | |

MCV Determination



| Blows (N) | Penetration (mm) | n to 4n (mm) |
|-----------|------------------|--------------|
| 1 | 50.0 | 11.8 |
| 2 | 44.8 | 10.6 |
| 3 | 40.5 | 8.7 |
| 4 | 38.2 | 7.5 |
| 6 | 35.6 | 5.6 |
| 8 | 34.2 | 4.6 |
| 12 | 31.8 | |
| 16 | 30.7 | |
| 24 | 30.0 | |
| 32 | 29.6 | |
| 48 | | |
| 64 | | |
| 96 | | |
| 128 | | |
| 192 | | |
| 256 | | |

Test Results.

| | |
|----------------------|-----|
| Moisture Content (%) | 8.1 |
| MCV | 8.5 |



Dyke Road Galway

Contract No:
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Client Ref:
13614-02-24

UNDRAINED SHEAR STRENGTH IN TRIAXIAL COMPRESSION

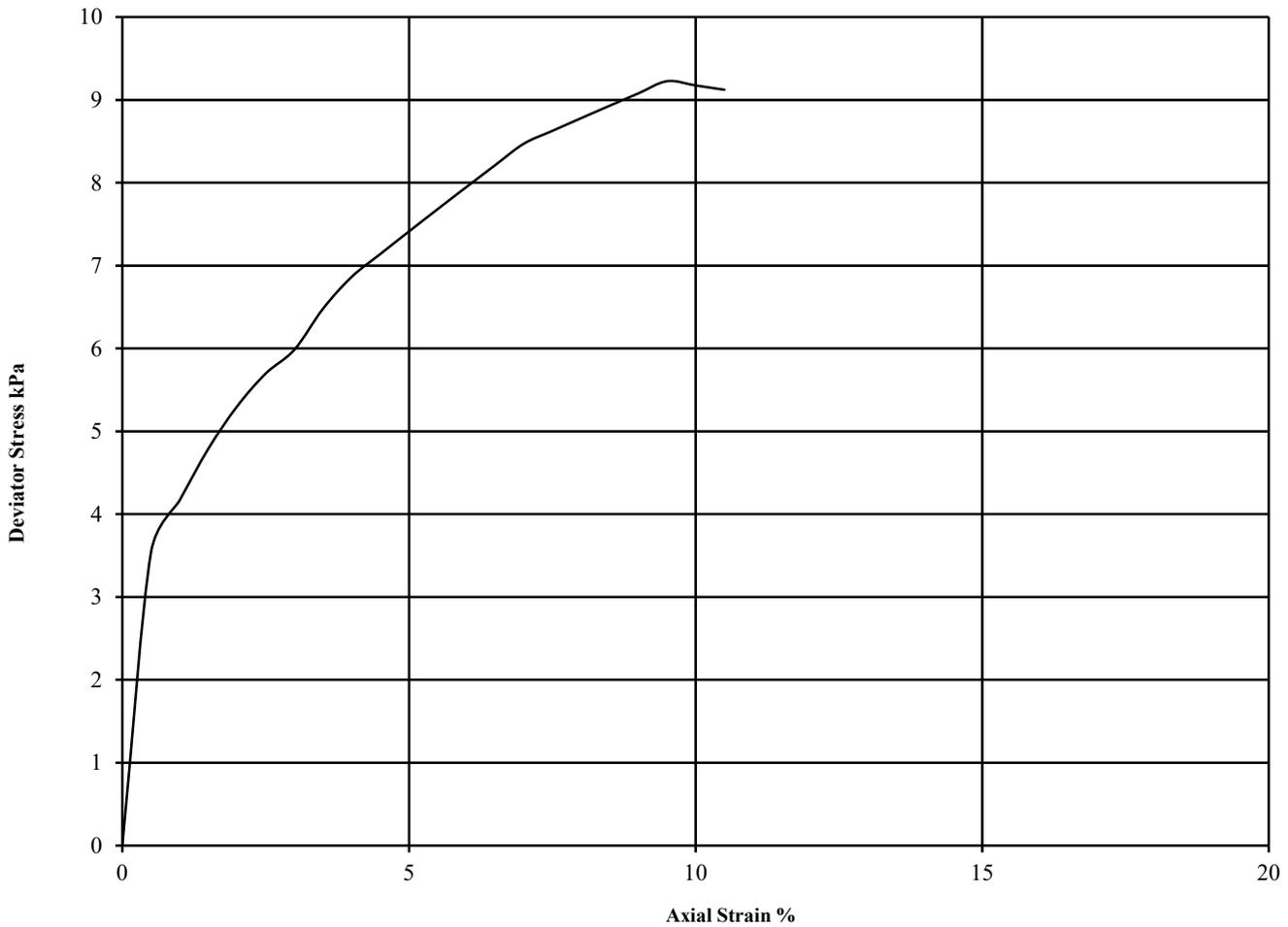
WITHOUT MEASUREMENT OF PORE PRESSURE

BS1377 : Part7 : 1990: Clause 8

Hole Number: **BH01** Top Depth (m): **3.00**

Sample Number: Base Depth (m): **3.45**

Sample Type **UT**



| Diameter (mm): | | 102 | Height (mm): | | 204 | Test: | UU Single Stage | | Remarks: |
|----------------|----------------------|-----------------------------------|----------------------------------|---------------------|----------------------------------|--------------------------------------|--------------------|-----------------|---|
| Specimen | Moisture Content (%) | Bulk Density (Mg/m ³) | Dry Density (Mg/m ³) | Cell Pressure (kPa) | Corr. Max. Deviator Stress (kPa) | Shear Strength Cu (kPa) | Failure Strain (%) | Mode of Failure | Undisturbed Sample Sample taken from top of tube Rate of strain = 2 %/min Latex Membrane used 0.2 mm thick, Correction applied 0.35 |
| | | | | θ_3 | $(\theta_1 - \theta_3)_f$ | $\frac{1}{2}(\theta_1 - \theta_3)_f$ | | | |
| 1 | 79 | 1.33 | 0.74 | 60 | 9 | 5 | 9.5 | Intermediate | See summary of soil descriptions |



Dyke Road Galway

Contract No:
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Client Ref:
13614-02-24

UNDRAINED SHEAR STRENGTH IN TRIAXIAL COMPRESSION

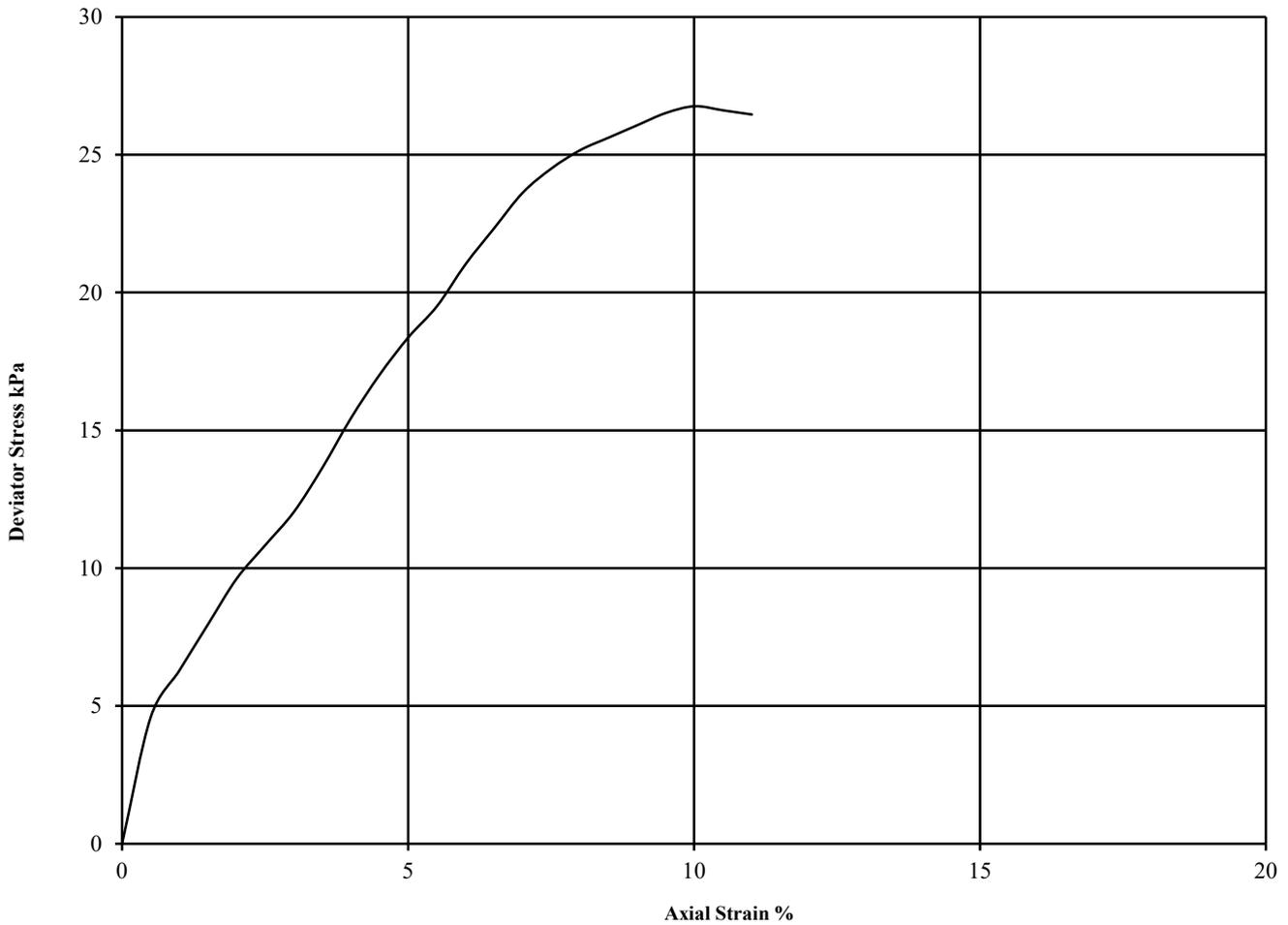
WITHOUT MEASUREMENT OF PORE PRESSURE

BS1377 : Part7 : 1990: Clause 8

Hole Number: **BH02** Top Depth (m): **3.00**

Sample Number: Base Depth (m): **3.45**

Sample Type **UT**



| Diameter (mm): | | 102 | | Height (mm): | | 204 | | Test: | | UU Single Stage | | Remarks: | |
|----------------|----------------------|-----------------------------------|----------------------------------|---------------------|----------------------------------|--------------------------------------|--------------------|-----------------|---|-----------------|--|----------|--|
| Specimen | Moisture Content (%) | Bulk Density (Mg/m ³) | Dry Density (Mg/m ³) | Cell Pressure (kPa) | Corr. Max. Deviator Stress (kPa) | Shear Strength Cu (kPa) | Failure Strain (%) | Mode of Failure | Undisturbed Sample Sample taken from top of tube Rate of strain = 2 %/min Latex Membrane used 0.2 mm thick, Correction applied 0.35 See summary of soil descriptions | | | | |
| | | | | θ_3 | $(\theta_1 - \theta_3)_f$ | $\frac{1}{2}(\theta_1 - \theta_3)_f$ | | | | | | | |
| 1 | 96 | 1.33 | 0.68 | 60 | 27 | 13 | 10.0 | Intermediate | | | | | |



Dyke Road Galway

Contract No:
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Client Ref:
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UNDRAINED SHEAR STRENGTH IN TRIAXIAL COMPRESSION

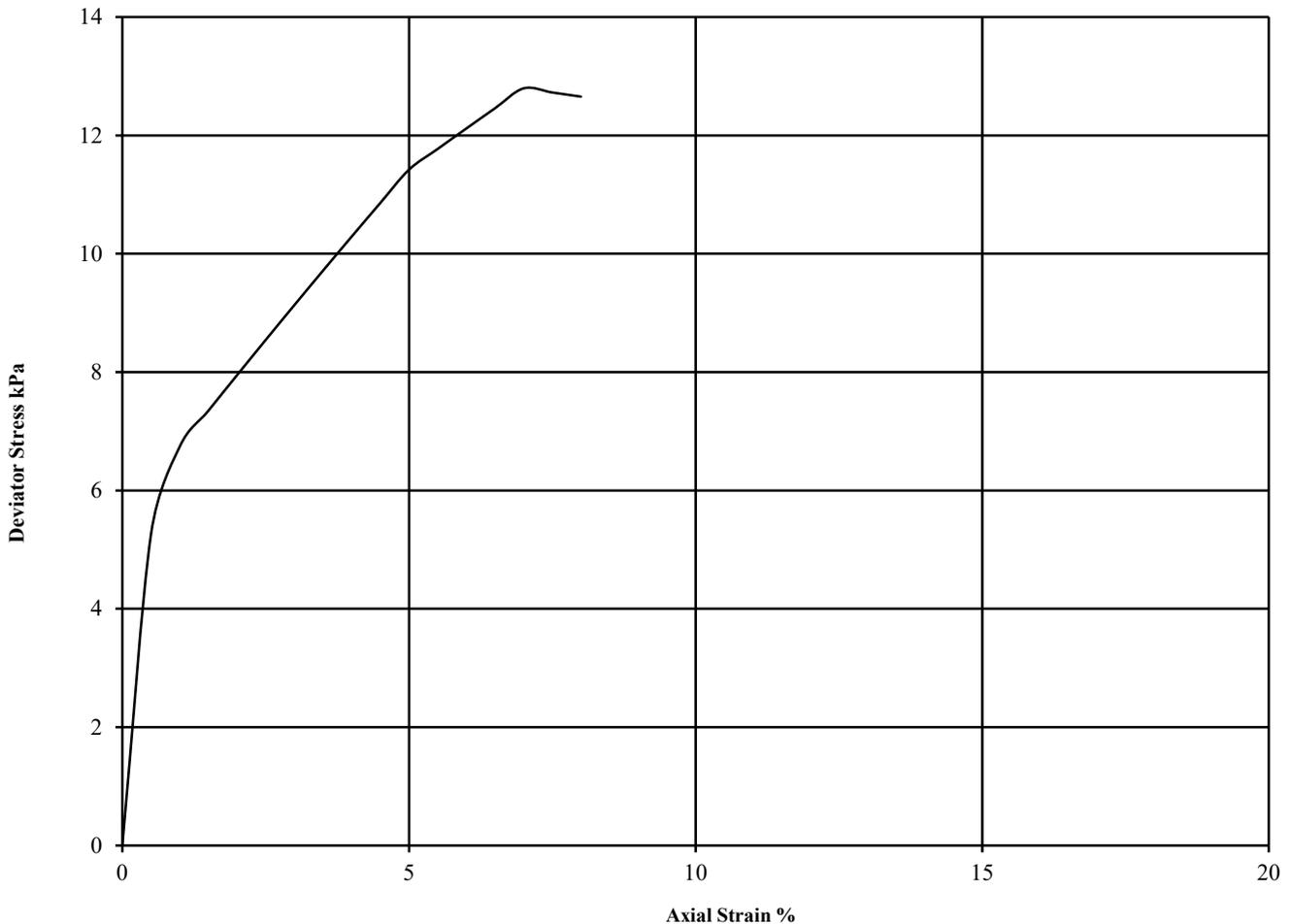
WITHOUT MEASUREMENT OF PORE PRESSURE

BS1377 : Part7 : 1990: Clause 8

Hole Number: **BRC01** Top Depth (m): **5.30**

Sample Number: Base Depth (m): **5.75**

Sample Type **UT**



| Diameter (mm): | | 102 | | Height (mm): | | 204 | | Test: | | UU Single Stage | | Remarks: | |
|----------------|----------------------|-----------------------------------|----------------------------------|---------------------|----------------------------------|--------------------------------------|--------------------|-----------------|--|-----------------|--|----------|-----------------------------------|
| Specimen | Moisture Content (%) | Bulk Density (Mg/m ³) | Dry Density (Mg/m ³) | Cell Pressure (kPa) | Corr. Max. Deviator Stress (kPa) | Shear Strength Cu (kPa) | Failure Strain (%) | Mode of Failure | | | | | Undisturbed Sample |
| | | | | θ_3 | $(\theta_1 - \theta_3)_f$ | $\frac{1}{2}(\theta_1 - \theta_3)_f$ | | | | | | | Sample taken from top of tube |
| | | | | | | | | | | | | | Rate of strain = 2 %/min |
| | | | | | | | | | | | | | Latex Membrane used 0.2 mm thick, |
| | | | | | | | | | | | | | Correction applied 0.36 |
| 1 | 123 | 1.27 | 0.57 | 110 | 13 | 6 | 7.0 | Intermediate | | | | | See summary of soil descriptions |



Dyke Road Galway

Contract No:

PSL24/3965

Client Ref:

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UNDRAINED SHEAR STRENGTH IN TRIAXIAL COMPRESSION

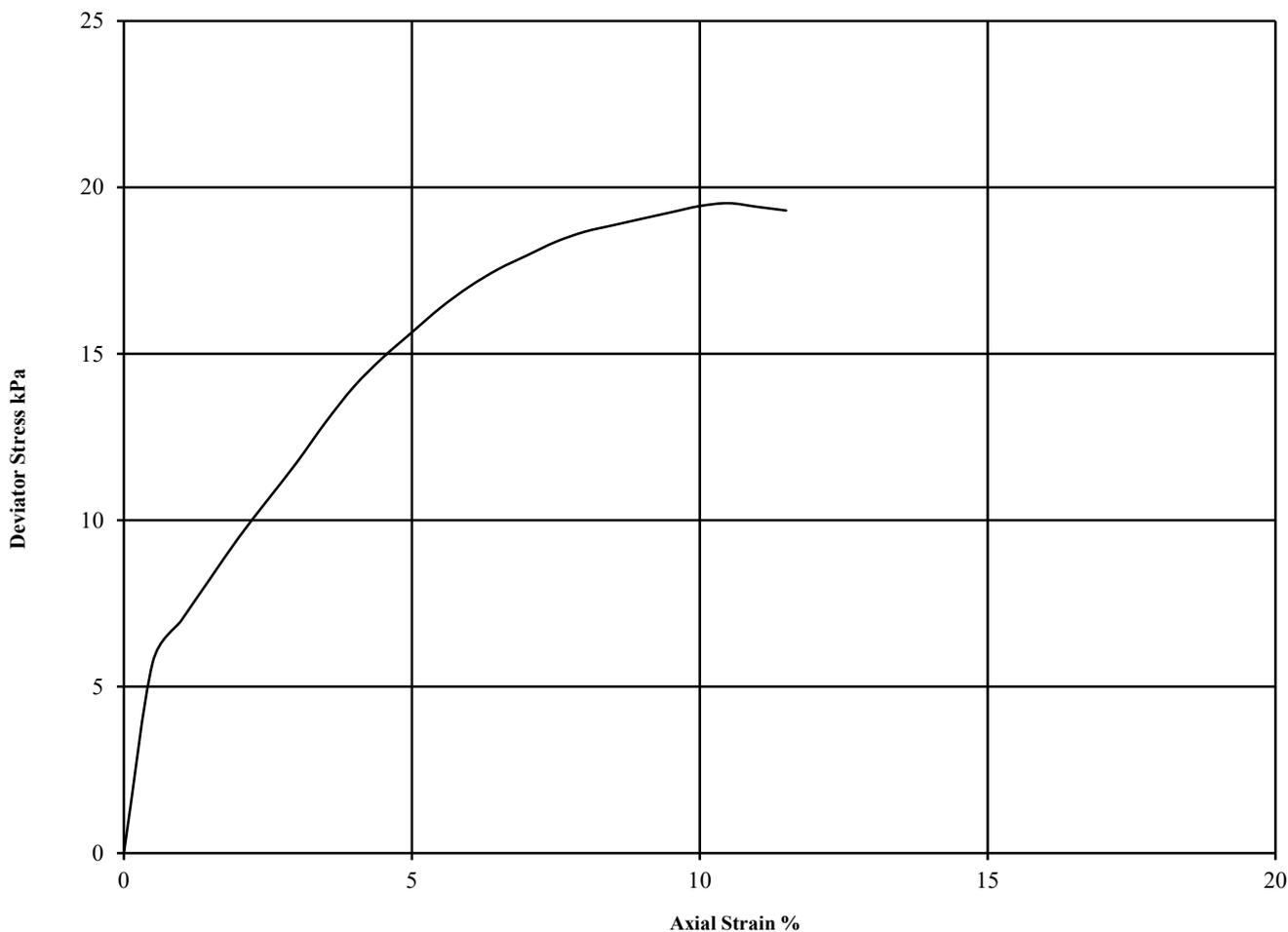
WITHOUT MEASUREMENT OF PORE PRESSURE

BS1377 : Part7 : 1990: Clause 8

Hole Number: **BRC02** Top Depth (m): **3.00**

Sample Number: Base Depth (m): **3.45**

Sample Type **UT**



| Diameter (mm): | | 102 | | Height (mm): | | 204 | | Test: | | UU Single Stage | | Remarks: | |
|----------------|----------------------|----------------------|---------------------|---------------------|----------------------------------|--------------------------------------|--------------------|-----------------|--|-----------------|--|----------|-----------------------------------|
| Specimen | Moisture Content (%) | Bulk Density (Mg/m3) | Dry Density (Mg/m3) | Cell Pressure (kPa) | Corr. Max. Deviator Stress (kPa) | Shear Strength Cu (kPa) | Failure Strain (%) | Mode of Failure | | | | | Undisturbed Sample |
| | | | | θ_3 | $(\theta_1 - \theta_3)_f$ | $\frac{1}{2}(\theta_1 - \theta_3)_f$ | | | | | | | Sample taken from top of tube |
| | | | | | | | | | | | | | Rate of strain = 2 %/min |
| | | | | | | | | | | | | | Latex Membrane used 0.2 mm thick, |
| | | | | | | | | | | | | | Correction applied 0.35 |
| 1 | 86 | 1.30 | 0.70 | 60 | 20 | 10 | 10.5 | Intermediate | | | | | See summary of soil descriptions |



Dyke Road Galway

Contract No:

PSL24/3965

Client Ref:

13614-02-24

UNDRAINED SHEAR STRENGTH IN TRIAXIAL COMPRESSION

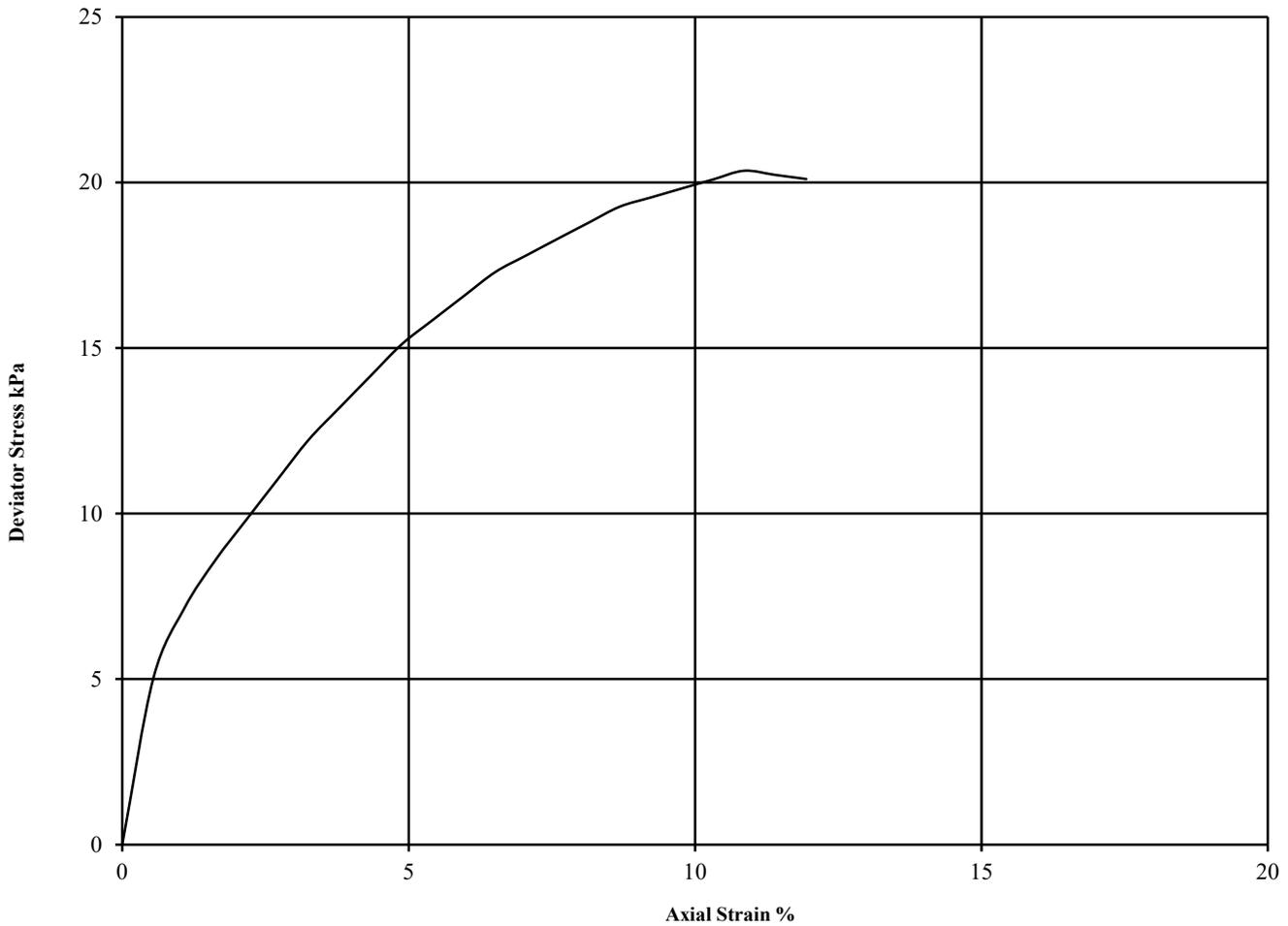
WITHOUT MEASUREMENT OF PORE PRESSURE

BS1377 : Part7 : 1990: Clause 8

Hole Number: **BRC02** Top Depth (m): **5.00**

Sample Number: Base Depth (m): **5.45**

Sample Type **UT**



| Diameter (mm): | | 102 | | Height (mm): | | 188 | | Test: | | UU Single Stage | | Remarks: | |
|----------------|----------------------|-----------------------------------|----------------------------------|---------------------|----------------------------------|--------------------------------------|--------------------|-----------------|--|-----------------|--|----------|-----------------------------------|
| Specimen | Moisture Content (%) | Bulk Density (Mg/m ³) | Dry Density (Mg/m ³) | Cell Pressure (kPa) | Corr. Max. Deviator Stress (kPa) | Shear Strength Cu (kPa) | Failure Strain (%) | Mode of Failure | | | | | Undisturbed Sample |
| | | | | θ_3 | $(\theta_1 - \theta_3)_f$ | $\frac{1}{2}(\theta_1 - \theta_3)_f$ | | | | | | | Sample taken from top of tube |
| | | | | | | | | | | | | | Rate of strain = 2 %/min |
| | | | | | | | | | | | | | Latex Membrane used 0.2 mm thick, |
| | | | | | | | | | | | | | Correction applied 0.35 |
| 1 | 131 | 1.27 | 0.55 | 100 | 20 | 10 | 10.9 | Intermediate | | | | | See summary of soil descriptions |



Dyke Road Galway

Contract No:
PSL24/3965
Client Ref:
13614-02-24

UNDRAINED SHEAR STRENGTH IN TRIAXIAL COMPRESSION

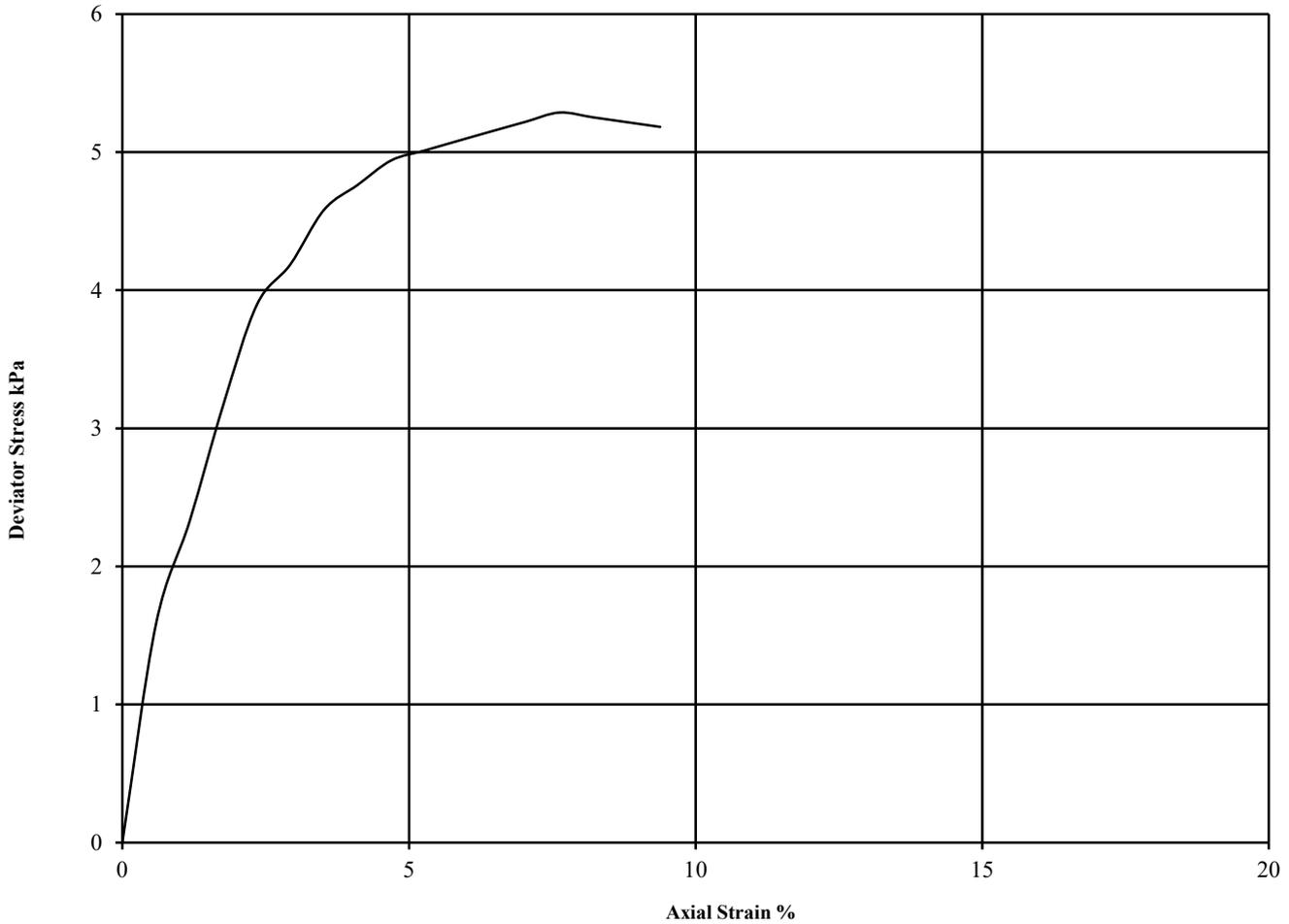
WITHOUT MEASUREMENT OF PORE PRESSURE

BS1377 : Part7 : 1990: Clause 8

Hole Number: **BRC03** Top Depth (m): **3.00**

Sample Number: Base Depth (m): **3.45**

Sample Type **UT**



| Diameter (mm): | | 102 | | Height (mm): | | 174 | | Test: | | UU Single Stage | | Remarks: | |
|----------------|----------------------|-----------------------------------|----------------------------------|---------------------|----------------------------------|-------------------------|--------------------|-----------------|---|-----------------|--|----------|--|
| Specimen | Moisture Content (%) | Bulk Density (Mg/m ³) | Dry Density (Mg/m ³) | Cell Pressure (kPa) | Corr. Max. Deviator Stress (kPa) | Shear Strength Cu (kPa) | Failure Strain (%) | Mode of Failure | Undisturbed Sample Sample taken from top of tube Rate of strain = 2 %/min Latex Membrane used 0.2 mm thick, Correction applied 0.36 | | | | |
| 1 | 87 | 1.44 | 0.77 | 60 | 5 | 3 | 7.6 | Intermediate | See summary of soil descriptions | | | | |



Dyke Road Galway

Contract No:
PSL24/3965
Client Ref:
13614-02-24

ONE DIMENSIONAL CONSOLIDATION TEST

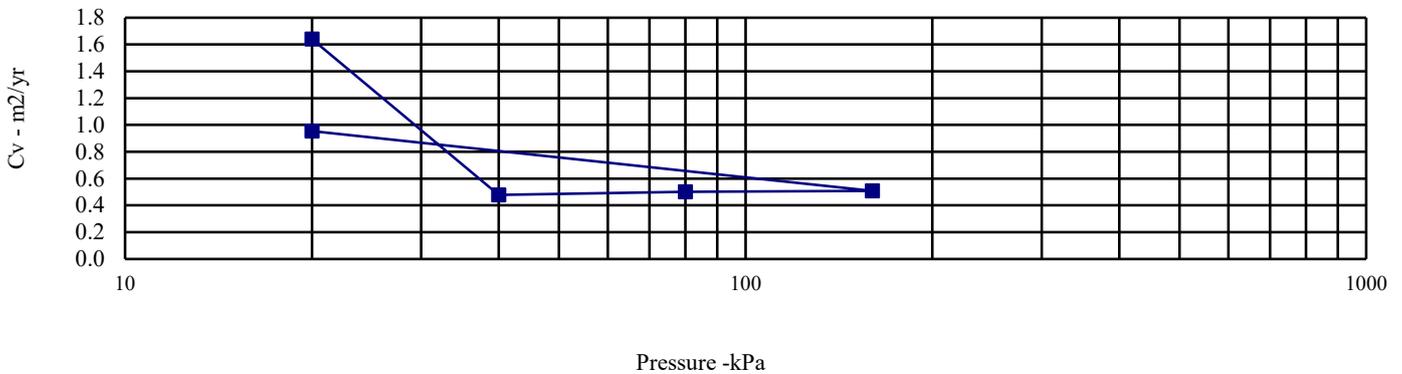
BS 1377: Part 5: 1990: Clause 3

Hole Number: BH02 Top Depth (m): 1.00

Sample Number: Base Depth (m) : 1.27

Sample Type: UT

| Initial Conditions | | Pressure Range | | Mv | Cv | Specimen location |
|--------------------------------------|--------|----------------|-----|-------|-------|--|
| Moisture Content (%): | 80 | kPa | | m2/MN | m2/yr | within tube: Top |
| Bulk Density (Mg/m3): | 1.31 | 0 | 20 | 2.298 | 1.641 | Method used to determine CV: |
| Dry Density (Mg/m3): | 0.73 | 20 | 40 | 2.103 | 0.478 | |
| Voids Ratio: | 2.626 | 40 | 80 | 0.975 | 0.502 | Nominal temperature |
| Degree of saturation: | 80.4 | 80 | 160 | 0.639 | 0.509 | during test ' C: 20 |
| Height (mm): | 20.014 | 160 | 20 | 0.228 | 0.953 | Remarks: See summary of soil descriptions |
| Diameter (mm) | 75.008 | | | | | |
| Particle Density (Mg/m3): Assumed | 2.65 | | | | | |



Dyke Road Galway

Contract No:
PSL24/3965
Client Ref:
13614-02-24

ONE DIMENSIONAL CONSOLIDATION TEST

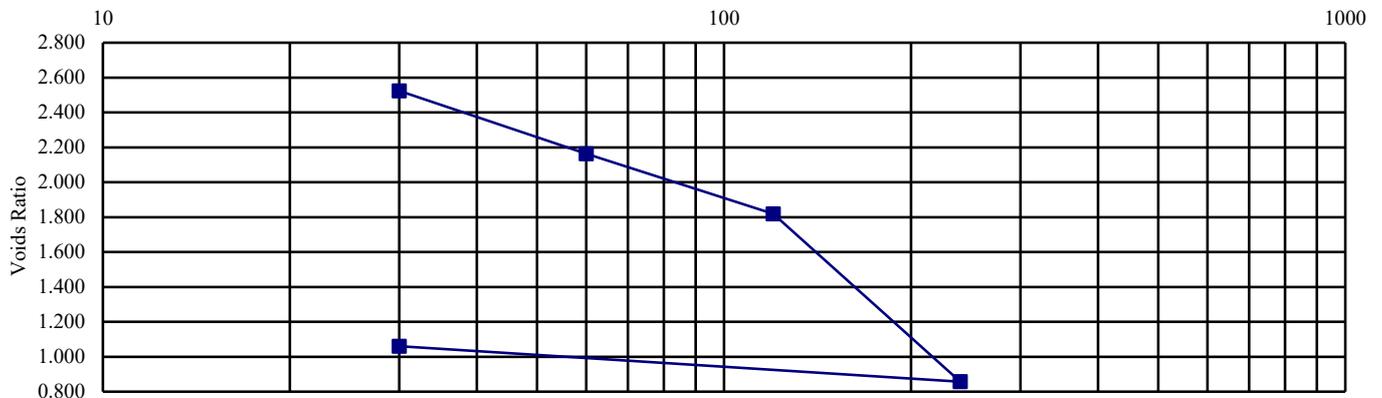
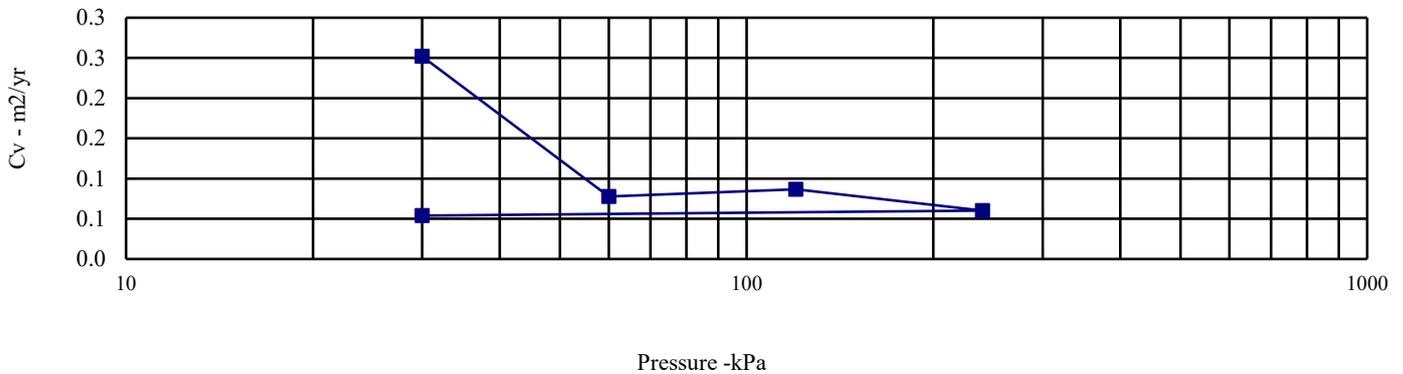
BS 1377: Part 5: 1990: Clause 3

Hole Number: BH02 **Top Depth (m):** 3.00

Sample Number: **Base Depth (m) :** 3.45

Sample Type: UT

| Initial Conditions | | Pressure Range | | Mv | Cv | Specimen location |
|---------------------------|-------|--|-----|-------|-------|---------------------|
| | | kPa | | m2/MN | m2/yr | within tube: |
| Moisture Content (%): | 104 | 0 | 30 | 3.883 | 0.252 | Top |
| Bulk Density (Mg/m3): | 1.35 | 30 | 60 | 3.400 | 0.078 | Method used to |
| Dry Density (Mg/m3): | 0.66 | 60 | 120 | 1.820 | 0.087 | determine CV: |
| Voids Ratio: | 2.989 | 120 | 240 | 2.843 | 0.060 | Nominal temperature |
| Degree of saturation: | 92.2 | 240 | 30 | 0.522 | 0.054 | during test ' C: |
| Height (mm): | 20.01 | | | | | 20 |
| Diameter (mm) | 75.01 | Remarks: See summary of soil descriptions | | | | |
| Particle Density (Mg/m3): | 2.65 | | | | | |
| Assumed | | | | | | |



| | | | |
|--|--|--------------------------------|---------------------|
| | | <p>Dyke Road Galway</p> | Contract No: |
| | | | PSL24/3965 |
| | | | Client Ref: |
| | | | 13614-02-24 |

ONE DIMENSIONAL CONSOLIDATION TEST

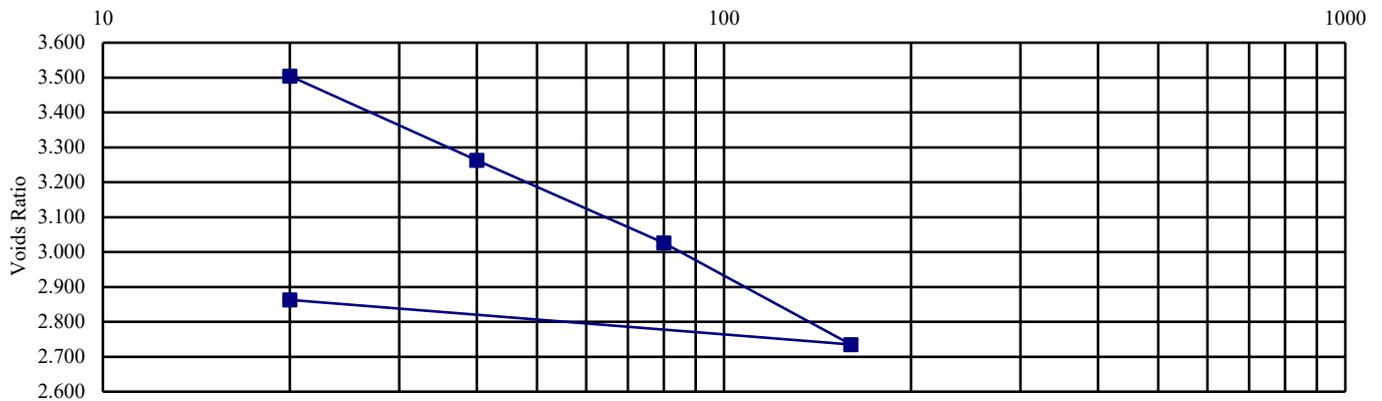
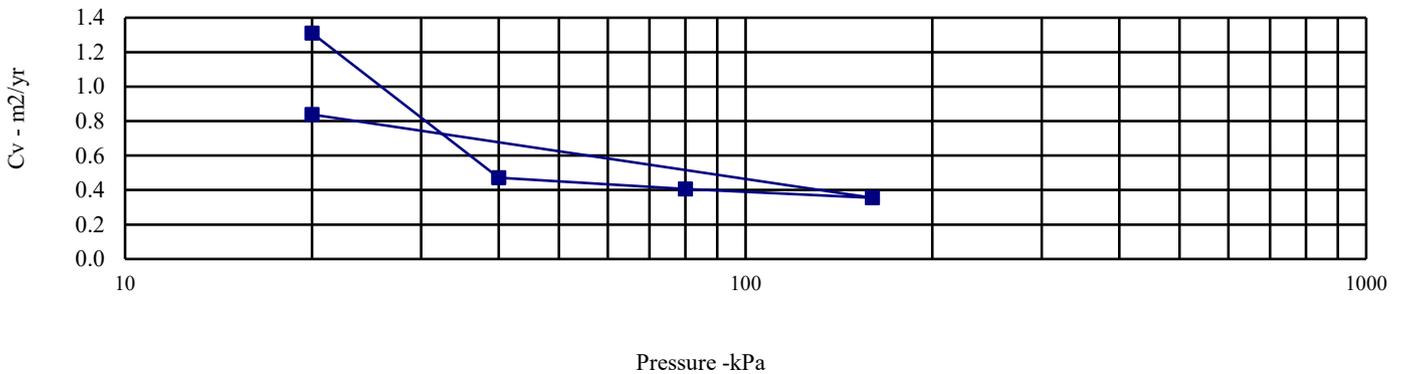
BS 1377: Part 5: 1990: Clause 3

Hole Number: BH03 Top Depth (m): 1.00

Sample Number: Base Depth (m) : 1.45

Sample Type: UT

| Initial Conditions | | Pressure Range | | Mv | Cv | Specimen location | |
|---------------------------|--------|----------------------------------|-----|-------|-------|---------------------|-----|
| Moisture Content (%): | 147 | kPa | | m2/MN | m2/yr | within tube: | Top |
| Bulk Density (Mg/m3): | 1.31 | 0 | 20 | 4.867 | 1.311 | Method used to | |
| Dry Density (Mg/m3): | 0.53 | 20 | 40 | 2.677 | 0.471 | determine CV: | T90 |
| Voids Ratio: | 3.990 | 40 | 80 | 1.388 | 0.406 | Nominal temperature | |
| Degree of saturation: | 97.7 | 80 | 160 | 0.905 | 0.355 | during test ' C: | 20 |
| Height (mm): | 20.032 | 160 | 20 | 0.244 | 0.838 | Remarks: | |
| Diameter (mm) | 74.955 | See summary of soil descriptions | | | | | |
| Particle Density (Mg/m3): | 2.65 | | | | | | |
| Assumed | | | | | | | |



Dyke Road Galway

Contract No:
PSL24/3965
Client Ref:
13614-02-24

ONE DIMENSIONAL CONSOLIDATION TEST

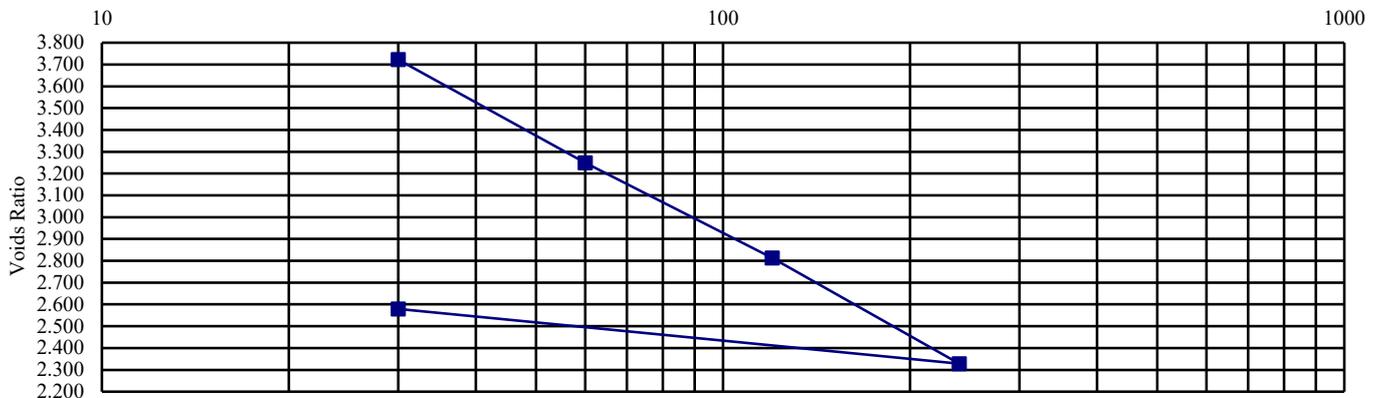
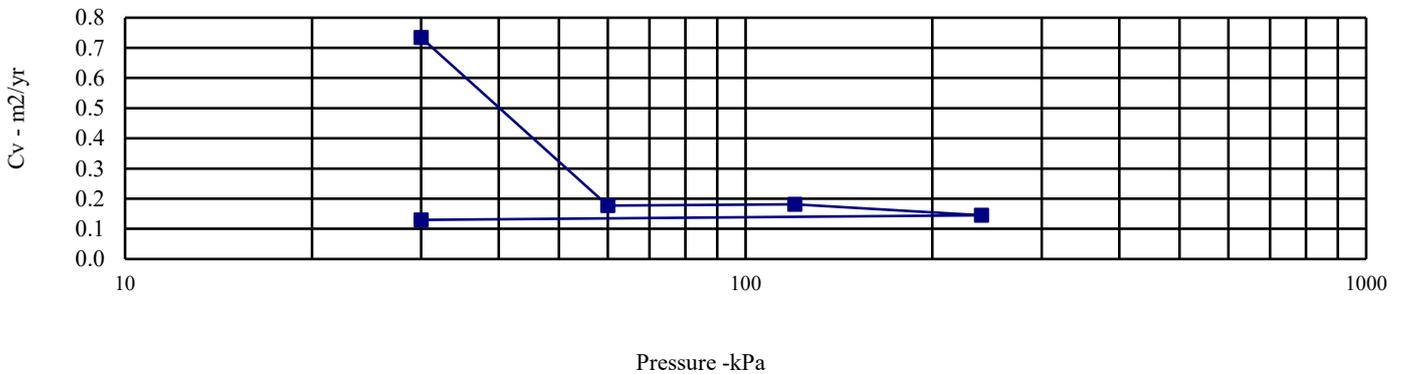
BS 1377: Part 5: 1990: Clause 3

Hole Number: BH03 **Top Depth (m):** 3.00

Sample Number: **Base Depth (m) :** 3.45

Sample Type: UT

| Initial Conditions | | Pressure Range | | Mv | Cv | Specimen location |
|---------------------------|--------|--|-----|-------|-------|--------------------------------------|
| Moisture Content (%): | 151 | kPa | | m2/MN | m2/yr | within tube: |
| Bulk Density (Mg/m3): | 1.17 | 0 | 30 | 5.672 | 0.734 | Top |
| Dry Density (Mg/m3): | 0.47 | 30 | 60 | 3.341 | 0.177 | Method used to determine CV: |
| Voids Ratio: | 4.691 | 60 | 120 | 1.710 | 0.181 | T90 |
| Degree of saturation: | 85.5 | 120 | 240 | 1.059 | 0.145 | Nominal temperature during test ' C: |
| Height (mm): | 20.016 | 240 | 30 | 0.359 | 0.130 | 20 |
| Diameter (mm) | 75.028 | Remarks: See summary of soil descriptions | | | | |
| Particle Density (Mg/m3): | 2.65 | | | | | |
| Assumed | | | | | | |



Dyke Road Galway

| |
|---------------------|
| Contract No: |
| PSL24/3965 |
| Client Ref: |
| 13614-02-24 |

ONE DIMENSIONAL CONSOLIDATION TEST

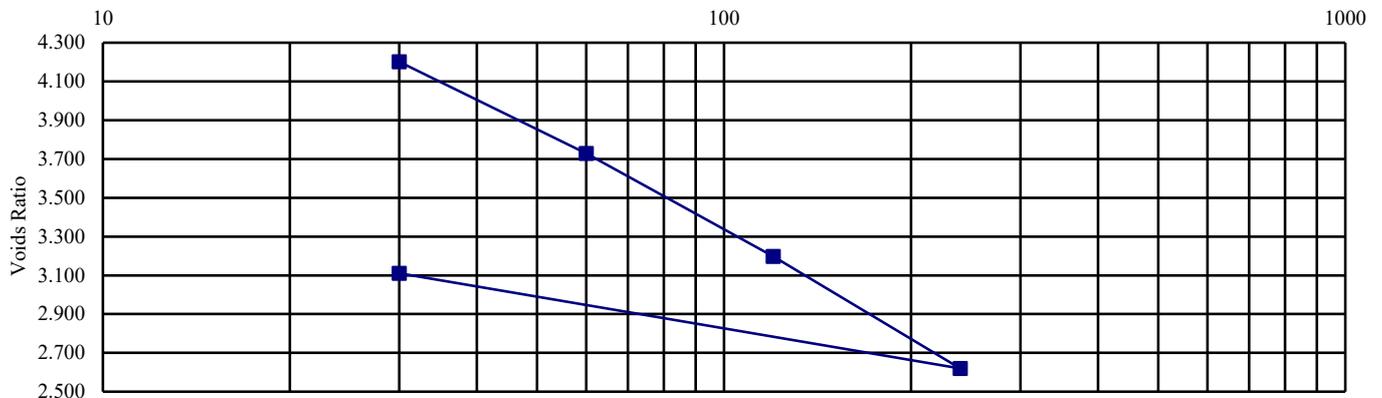
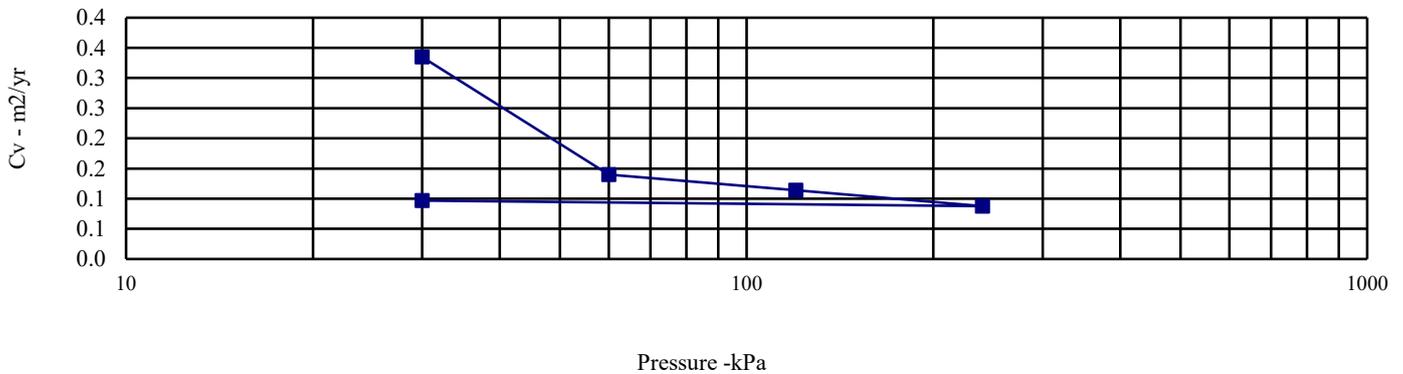
BS 1377: Part 5: 1990: Clause 3

Hole Number: BH06 Top Depth (m): 3.00

Sample Number: Base Depth (m) : 3.45

Sample Type: UT

| Initial Conditions | | Pressure Range | | Mv | Cv | Specimen location | |
|---------------------------|--------|----------------------------------|-----|-------|-------|---------------------|-----|
| Moisture Content (%): | 167 | kPa | | m2/MN | m2/yr | within tube: | Top |
| Bulk Density (Mg/m3): | 1.25 | 0 | 30 | 2.584 | 0.335 | Method used to | |
| Dry Density (Mg/m3): | 0.47 | 30 | 60 | 3.028 | 0.140 | determine CV: | T90 |
| Voids Ratio: | 4.638 | 60 | 120 | 1.871 | 0.114 | Nominal temperature | |
| Degree of saturation: | 95.2 | 120 | 240 | 1.148 | 0.088 | during test ' C: | 20 |
| Height (mm): | 20.024 | 240 | 30 | 0.645 | 0.097 | Remarks: | |
| Diameter (mm) | 75.02 | See summary of soil descriptions | | | | | |
| Particle Density (Mg/m3): | 2.65 | | | | | | |
| Assumed | | | | | | | |



Dyke Road Galway

Contract No:
PSL24/3965
Client Ref:
13614-02-24



7 - 11 Harding Street
Leicester
LE1 4DH

Professional Soils Laboratory

5/7 Hexthorpe Road
Hexthorpe
Doncaster
DN4 0AR

Analytical Test Report: L24/05476/PSL - 24-46441

Your Project Reference: **PSL24/3965 Dyke Road Galway**

Your Order Number: PSL24/3965 Samples Received / Instructed: 18/06/2024 / 18/06/2024

Report Issue Number: 1 Sample Tested: 18/06 to 24/06/2024

Samples Analysed: 10 soil samples Report issued: 24/06/2024

Signed

James Gane
Analytical Services Manager
CTS Group

Notes:

General

Please refer to Methodologies page for details pertaining to the analytical methods undertaken.
 Samples will be retained for 14 days after issue of this report unless otherwise requested.
 Moisture Content was determined in accordance with CTS method statement MS - CL - Sample Prep, oven dried at <30°C.
 Moisture Content is reported as a percentage of the dry mass of soil, this calculation is in accordance with BS1377, Part 2, 1990, Clause 3.2
 Stone Content was determined in accordance with CTS method statement MS - CL - Sample Prep and refers to the percentage of stones retained on a 10mm BS test sieve.
Where specification limits are included these are for guidance only. Where a measured value has been highlighted this is not implying acceptance or failure and certainty of measurement values have not been taken into account.
 Uncertainty of measurement values are available on request.
 Samples were supplied by customer, results apply to the samples as received.

Deviating Samples

On receipt samples are compared against our sample holding and handling protocols, where any deviations have been noted these are reported on our deviating sample page (if present)

Accreditation Key

UKAS = UKAS Accreditation, MCERTS = MCERTS Accreditation, u = Unaccredited, subUKAS - Subcontracted to a laboratory UKAS accredited for this test, subMCERTS - Subcontracted to a laboratory MCERTS accredited for this test
 MCERTS Accreditation only covers the SAND, CLAY and LOAM matrices

Date of Issue: 29.05.2024

Issued by: J. Gane

Issue No: 4

Rev No: 10



7 - 11 Harding Street
Leicester
LE1 4DH

L24/05476/PSL - 24-46441

Project Reference - PSL24/3965 Dyke Road Galway

Analytical Test Results - Solid

| Lab Reference | 372762 | 372763 | 372765 | 372766 | 372767 | 372768 | | |
|------------------------|--------------|----------------------|--------|--------|--------|--------|-------|-------|
| Client Sample ID | - | - | - | - | - | - | | |
| Client Sample Location | BH01 | BH01 | BRC02 | BRC02 | BRC02 | BRC03 | | |
| Client Sample Type | B | B | B | B | B | B | | |
| Client Sample Number | - | - | - | - | - | - | | |
| Depth - Top (m) | 3.50 | 6.70 | 2.00 | 3.50 | 7.00 | 4.00 | | |
| Depth - Bottom (m) | 3.50 | 6.70 | 2.00 | 3.50 | 7.00 | 4.00 | | |
| Date of Sampling | - | - | - | - | - | - | | |
| Time of Sampling | - | - | - | - | - | - | | |
| Sample Matrix | Clay | Loam | Loam | Clay | Other | Clay | | |
| Determinant | Units | Accreditation | | | | | | |
| SOM (via TOC) | (%) | UKAS | 2.9 | 5.1 | 20 | 2.6 | < 0.9 | < 0.9 |



7 - 11 Harding Street
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LE1 4DH

L24/05476/PSL - 24-46441

Project Reference - PSL24/3965 Dyke Road Galway

Analytical Test Results - Solid

| Lab Reference | 372769 | 372770 | 372771 |
|------------------------|--------------|----------------------|--------|
| Client Sample ID | - | - | - |
| Client Sample Location | BRC03 | BRC04 | BRC05 |
| Client Sample Type | B | B | B |
| Client Sample Number | - | - | - |
| Depth - Top (m) | 7.50 | 4.00 | 3.50 |
| Depth - Bottom (m) | 7.50 | 4.00 | 3.50 |
| Date of Sampling | - | - | - |
| Time of Sampling | - | - | - |
| Sample Matrix | Clay | Other | Clay |
| Determinant | Units | Accreditation | |
| SOM (via TOC) | (%) | UKAS | < 0.9 |
| | | | < 0.9 |
| | | | < 0.9 |



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LE1 4DH

L24/05476/PSL - 24-46441

Project Reference - PSL24/3965 Dyke Road Galway

Analytical Test Results - Chemical Analysis

| Lab Reference | | | 372762 | 372763 | 372764 | 372765 | 372766 | 372767 |
|--|--------------|----------------------|--------|--------|--------|--------|--------|--------|
| Client Sample ID | | | - | - | - | - | - | - |
| Client Sample Location | | | BH01 | BH01 | BH05 | BRC02 | BRC02 | BRC02 |
| Client Sample Type | | | B | B | B | B | B | B |
| Client Sample Number | | | - | - | - | - | - | - |
| Depth - Top (m) | | | 3.50 | 6.70 | 4.00 | 2.00 | 3.50 | 7.00 |
| Depth - Bottom (m) | | | 3.50 | 6.70 | 4.00 | 2.00 | 3.50 | 7.00 |
| Date of Sampling | | | - | - | - | - | - | - |
| Time of Sampling | | | - | - | - | - | - | - |
| Sample Matrix | | | Clay | Loam | Other | Loam | Clay | Other |
| Determinant | Units | Accreditation | | | | | | |
| Water soluble sulphate (as SO ₄) | (mg/l) | u | 1400 | 1500 | 1500 | 820 | 74 | 230 |
| pH Value | pH Units | MCERTS | 8.1 | 7.7 | 9.2 | 7.8 | 8.3 | 8.8 |
| Water Soluble Chloride | (mg/l) | u | 27 | 24 | 12 | 44 | 23 | 11 |



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L24/05476/PSL - 24-46441

Project Reference - PSL24/3965 Dyke Road Galway

Analytical Test Results - Chemical Analysis

| Lab Reference | | | 372768 | 372769 | 372770 | 372771 |
|--|--------------|----------------------|--------|--------|--------|--------|
| Client Sample ID | | | - | - | - | - |
| Client Sample Location | | | BRC03 | BRC03 | BRC04 | BRC05 |
| Client Sample Type | | | B | B | B | B |
| Client Sample Number | | | - | - | - | - |
| Depth - Top (m) | | | 4.00 | 7.50 | 4.00 | 3.50 |
| Depth - Bottom (m) | | | 4.00 | 7.50 | 4.00 | 3.50 |
| Date of Sampling | | | - | - | - | - |
| Time of Sampling | | | - | - | - | - |
| Sample Matrix | | | Clay | Clay | Other | Clay |
| Determinant | Units | Accreditation | | | | |
| Water soluble sulphate (as SO ₄) | (mg/l) | u | 100 | 76 | 89 | 71 |
| pH Value | pH Units | MCERTS | 8.7 | 9.0 | 9.3 | 8.7 |
| Water Soluble Chloride | (mg/l) | u | 4.9 | 7.8 | 4.6 | 7.6 |



7 - 11 Harding Street
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L24/05476/PSL - 24-46441

Project Reference - PSL24/3965 Dyke Road Galway

Sample Descriptions

| Lab Reference | Client Sample ID | Client Sample Location | Client Sample Type | Client Sample Number | Description | Moisture Content (%) | Stone Content (%) | Passing 2mm test sieve (%) |
|---------------|------------------|------------------------|--------------------|----------------------|--|----------------------|-------------------|----------------------------|
| 372762 | - | BH01 | B | - | Greyish light brown sandy silty clay | 130 | < 0.1 | 79 |
| 372763 | - | BH01 | B | - | Brownish grey clayey loam with rare rootlets | 110 | < 0.1 | 87 |
| 372764 | - | BH05 | B | - | Grey sandy crushed rock | - | - | 100 |
| 372765 | - | BRC02 | B | - | Dark brown slightly gravelly slightly clayey silty loam with rare rootlets | 75 | 49 | 37 |
| 372766 | - | BRC02 | B | - | Greyish light brown sandy silty clay | 100 | < 0.1 | 100 |
| 372767 | - | BRC02 | B | - | Light grey silty crushed rock | 3.9 | 30 | 21 |
| 372768 | - | BRC03 | B | - | Light grey slightly gravelly slightly sandy silty clay | 9.4 | 9.4 | 56 |
| 372769 | - | BRC03 | B | - | Light grey slightly gravelly slightly sandy silty clay | 10 | 2.7 | 57 |
| 372770 | - | BRC04 | B | - | Greenish grey sandy gravel | 4.4 | 29 | 26 |
| 372771 | - | BRC05 | B | - | Light grey slightly gravelly slightly sandy silty clay | 7.7 | 24 | 66 |



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L24/05476/PSL - 24-46441

Project Reference - PSL24/3965 Dyke Road Galway

Sample Comments

| Lab Reference | Client Sample ID | Client Sample Location | Client Sample Type | Client Sample Number | Comments |
|---------------|------------------|------------------------|--------------------|----------------------|----------|
| 372762 | - | BH01 | B | - | |
| 372763 | - | BH01 | B | - | |
| 372764 | - | BH05 | B | - | |
| 372765 | - | BRC02 | B | - | |
| 372766 | - | BRC02 | B | - | |
| 372767 | - | BRC02 | B | - | |
| 372768 | - | BRC03 | B | - | |
| 372769 | - | BRC03 | B | - | |
| 372770 | - | BRC04 | B | - | |
| 372771 | - | BRC05 | B | - | |



7 - 11 Harding Street
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L24/05476/PSL - 24-46441

Project Reference - PSL24/3965 Dyke Road Galway

Analysis Methodologies

| Test Code | Test Name / Reference | Sample condition for analysis | Sample Preparation | Test Details |
|------------|--|-------------------------------|-------------------------|--|
| ANIONSS | MS - CL - Anions by Aquakem (2:1Extract) | Oven dried | Passing 2mm test sieve | Determination of Anions (inc Sulphate, chloride etc.) in soils by Aquakem. Analysis is based on a 2:1 water to soil extraction ratio |
| PHS | MS - CL - pH in Soils | As received | Passing 10mm test sieve | Determination of pH in soils using a pH probe (using a 1:3 soil to water extraction) |
| TOCS | MS - CL - TOC Eltra | Air Dried | Passing 10mm test sieve | Determination of Total Organic Carbon in soils |
| SAMPLEPREP | MS - CL - Sample Preparation | - | - | Preparation of samples (including determination of moisture content) to allow for subsequent analysis |



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L24/05476/PSL - 24-46441

Project Reference - PSL24/3965 Dyke Road Galway

Sample Deviations

Deviations are listed below against each sample and associated test method, where deviation(s) are noted it means data may not be representative of the sample at the time of sampling and it is possible that results provided may be compromised.

Observations on receipt

A - No date of sampling provided

C - Received in inappropriate container

H - Contains headspace

T - Temperature on receipt exceeds storage temperature

R - Sample(s) received with less than 96 hours for testing to commence/complete, any result formally classed as deviating will be marked with an X against the applicable test (i.e. RX)

Observations whilst in laboratory

X - Exceeds sampling to extraction or analysis timescales

| Lab Reference | Client Sample ID | Client Sample Location | Client Sample Type | Client Sample Number | Test | Deviations |
|---------------|------------------|------------------------|--------------------|----------------------|------|------------|
| 372762 | - | BH01 | B | - | | A |
| 372763 | - | BH01 | B | - | | A |
| 372764 | - | BH05 | B | - | | A |
| 372765 | - | BRC02 | B | - | | A |
| 372766 | - | BRC02 | B | - | | A |
| 372767 | - | BRC02 | B | - | | A |
| 372768 | - | BRC03 | B | - | | A |
| 372769 | - | BRC03 | B | - | | A |
| 372770 | - | BRC04 | B | - | | A |
| 372771 | - | BRC05 | B | - | | A |

Laboratory Test Report Uniaxial Compressive Strength

| | | | |
|--------------------|--|----------------------|-------------|
| Project: | Dyke Road Galway | Job Number | 13614-02-24 |
| Client: | Ground Investigations Ireland Catherinstown House, Hazelhatch Road Newcastle. Co. Dublin | Lab Ref No | ST 28878 |
| Originator: | Mike Sutton | Date Received | 20/06/2024 |
| | | Date Tested | 27/06/2024 |
| | | Date Reported | 01/07/2024 |

| Sample Reference | Moisture Content | Density (Mg/m ³) | Uniaxial Compressive Strength (N/mm ²) |
|-------------------|------------------|------------------------------|--|
| BRC01 14.65-14.88 | 0.2 | 2659 | 116.4 |
| BRC02 18.37-18.60 | 0.7 | 2691 | 140.3 |
| BRC03 17.17-17.45 | 0.2 | 2692 | 100.2 |
| BRC04 10.11-10.30 | 0.8 | 2888 | 88.1 |
| BRC05 9.65-9.88 | 0.4 | 2709 | 112.7 |

Remarks: Core BRC01 14.65-14.88 Visible Cracks

Approved Signature
James Ward, Operations Manager
CMTL Ireland Limited

**Laboratory Test Report
 Point Load Strength Index**

| | |
|---|---------------------------------|
| Project : Dyke Road Galway | Job Number 13614-02-24 |
| Client : Ground Investigations Ireland | Lab Ref No ST 28879 |
| Catherinestown House, Hazelhatch Road | Date Received 20/06/2024 |
| Newcastle, Co. Dublin | Date Tested 26/06/2024 |
| Originator : Mike Sutton | Date Reported 01/07/2024 |

Point Load Strength Index

| Sample No:- | Depth (m) | Description | Type | Orientation | W (mm) | D (mm) | P (kN) | A | De (mm) | I _s | F | I _{s(50)} MN/m ² |
|-------------|-------------|-------------|------|-------------|--------|--------|--------|------|---------|----------------|------|--------------------------------------|
| BRC01 | 12.50-12.60 | 1 | D | ⊥ | 63.0 | 64.0 | 25.00 | 4032 | 64.0 | 6.104 | 1.12 | 6.82 |
| BRC01 | 16.05-16.15 | 1,2 | D | ⊥ | 63.0 | 63.0 | 28.00 | 3969 | 63.0 | 7.055 | 1.11 | 7.83 |
| BRC02 | 15.37-15.50 | 1 | D | ⊥ | 63.0 | 65.0 | 33.00 | 4095 | 65.0 | 7.811 | 1.13 | 8.79 |
| BRC02 | 18.20-18.35 | 1,3 | D | ⊥ | 63.0 | 63.0 | 14.00 | 3969 | 63.0 | 3.527 | 1.11 | 3.91 |
| BRC03 | 16.40-16.50 | 1,2 | D | ⊥ | 63.0 | 63.0 | 29.00 | 3969 | 63.0 | 7.307 | 1.11 | 8.11 |
| BRC03 | 19.95-20.05 | 1,2 | D | ⊥ | 63.0 | 64.0 | 24.00 | 4032 | 64.0 | 5.859 | 1.12 | 6.55 |
| BRC04 | 8.40-8.57 | 1,2 | D | ⊥ | 63.0 | 65.0 | 10.00 | 4095 | 65.0 | 2.367 | 1.13 | 2.66 |
| BRC04 | 9.50-9.60 | 1,2 | D | ⊥ | 63.0 | 63.0 | 25.00 | 3969 | 63.0 | 6.299 | 1.11 | 6.99 |
| BRC05 | 7.71-7.80 | 1,2,3 | D | ⊥ | 63.0 | 64.0 | 22.00 | 4032 | 64.0 | 5.371 | 1.12 | 6.00 |
| BRC05 | 9.25-9.36 | 1,2 | D | ⊥ | 63.0 | 64.0 | 35.00 | 4032 | 64.0 | 8.545 | 1.12 | 9.55 |
| BRC06 | 11.00-11.17 | 1,2 | D | ⊥ | 63.0 | 65.0 | 11.00 | 4095 | 65.0 | 2.604 | 1.13 | 2.93 |
| BRC06 | 13.34-13.44 | 1,2 | D | ⊥ | 63.0 | 63.0 | 14.00 | 3969 | 63.0 | 3.527 | 1.11 | 3.91 |

Description 1 : Blue/ Grey
 Description 2 : White Veins
 Description 2 : Cracks

| I _{s(50)} MN/m ² for | Description 1,2,3 |
|--|-------------------|
| Min | 2.66 |
| Mean | 6.68 |
| Max | 9.55 |

Test
 A = axial
 D = diametrical

Relationship to planes of weakness
 IL = irregular lump ⊥ = perpendicular
 II = parallel

Mean Value

| | I _{s(50)} MN/m ² | U.C.S. MN/m ² |
|--------------------|--------------------------------------|--------------------------|
| Extremely Weak | <0.05 | 0.6-1.0 |
| Very Weak | 0.05-0.20 | 1.0-5.0 |
| Weak | 0.20-0.50 | 5.0-25.0 |
| Medium Strong | 0.50-2.00 | 25-50 |
| Strong | 2.00-4.50 | 50-100 |
| Very Strong | 4.50-9.00 | 100-250 |
| Extremely Strong | 9.00 + | >250 |

Approved Signature
 James Ward, Operations Manager
 CMTL Ireland Limited

APPENDIX 9 – Groundwater Monitoring Records





GROUND INVESTIGATIONS IRELAND
Geotechnical & Environmental

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GROUNDWATER MONITORING

Dyke Road

| BOREHOLE | DATE | TIME | GROUNDWATER (m BGL) | Comments |
|----------|------------|-------|----------------------|----------|
| BRC01 | 26/06/2024 | 11:10 | 0.17 | |
| BRC02 | 26/06/2024 | 10:32 | 0.87 | |
| BRC04 | 26/06/2024 | 09:25 | 2.25 | |
| BRC05 | 26/06/2024 | 09:55 | 1.30 | |
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LDA Dyke Road
Galway

Geophysical Survey

Report Status: Final

MGX Project Number: 6756

MGX File Reference: 6756f-005.doc

1st July 2024

Confidential Report To:

Land Development Agency
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Report submitted by:
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Subsurface Geophysical Investigations

EXECUTIVE SUMMARY

1. Minerex Geophysics Ltd. (MGX) carried out a geophysical survey consisting of 2D-Resistivity (ERT), seismic refraction (p-wave) and MASW (s-wave) surveying for the ground investigation for the proposed LDA development at Dyke Road, County Galway.
2. The main objectives of the survey were to determine the ground conditions under the site, to determine the depth to rock and the overburden thickness, and to detect possible karstified rock in order to help guide the locations of direct ground investigations.
3. The recommended locations were drilled and the results are a zone of Metagabbro (BRC04) within the limestone and a deeper pocket of very stiff to hard clay within the good limestone (BRC03).
4. The clean limestone present is liable to karstification, but it does not have to be karstified.
5. The three different methods allowed for correlations to be identified between them and to improve the interpretation.
6. The urban nature of the site has negative effects on all the surveying methods.
7. The seismic refraction data was interpreted with four layers.
8. Layer 1 is interpreted as road construction material underlain by urban made ground and peat.
9. Layer 2 is described as soft to firm clay and silt or urban made ground or peat and extends to depths of 4 – 8m below ground level (bgl).
10. Layer 3 is only present in the NW of the site and is interpreted as very stiff or very dense overburden but may contain some very weathered rock.
11. Rock is indicated by Layer 4. The depth to the top of this layer ranges from 4 – 9m bgl across most of the site but is 11 to 12m bgl in the NW in BRC01 and BH01.
12. Peat present in layer 1 or 2 would be compressed because of the urban layers.
13. Map 2 indicates three zones reviewed as 'Deep Rock', 'very stiff to hard Clay' and 'Metagabbro/anomalous Rock'. These were interpreted after targeted drilling.
14. The MASW data gives results for 3.6m to 4.5m of soft to firm material across much of the site and to deeper levels in the NW.
15. This report was reviewed and finalised after the complete direct ground investigation data had been received.

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1. INTRODUCTION

1.1 Background

Minerex Geophysics Ltd. (MGX) carried out a geophysical survey for an LDA site on the Dyke Road, Galway. The survey consisted of 2D-Resistivity (ERT), seismic refraction (p-wave) and MASW (s-wave) measurements. The survey was commissioned by the LDA.

This survey utilized various complementary geophysical methods to improve final interpretations. The role of geophysics as a non-destructive fast method is to provide a geological interpretation over a wide area to complement direct ground investigations at specific locations. The direct ground investigation results can be used to improve the initial geophysical results and interpretation.

The survey was aimed at investigating the ground conditions under the site, the depth to rock and identifying possible karstified rock.

During the tender stage, MGX indicated to the client limitations of the survey in this urban setting and improved the methodology and choice of geophysical methods.

The main purpose of a geophysical survey in this setting is to guide the locations for boreholes, rather than creating a ground model with an accuracy like on a greenfield site. The geophysics will rather have an outcome in a relative scale, like the rock is deeper here than there, and there is rather karst here than there. Then such comparisons can be use to target boreholes.

1.2 Objectives

The main objectives of the geophysical survey were:

- To determine the ground conditions under the site
- To determine the depth to rock and the overburden thickness
- To estimate the strength or stiffness or compaction of overburden materials and the rock quality
- To determine the type of overburden and rock
- To detect lateral changes within the geological layers
- To detect possible karstified zones within the rock or karst features
- To determine the s-wave velocity and to calculate the small strain shear modulus G_{\max}

1.3 Site Description

The site is located in a number of car parks along the Dyke Road just east of the Corrib River in Galway City. The site is bordered by roads to the west, south and southeast, a Retail Park to the east and a greenfield site to the north. The Black Box Theatre is in the north of the site. Access was from the Dyke Road in the west.

The whole site is underlain by tarmac. The total survey area covered an area of approx. 18,000m².

1.4 Geology

Online geological maps of Ireland (GSI, 2024) give the following information:

The quaternary sediments are described as urban while there is fen peat noted on the opposite side of the road to the west.

In terms of rock, the survey area is underlain by Viséan Limestones, described as undifferentiated limestones.

Viséan Limestones are karstifiable and there are many karst features noted in the Galway area, however the nearest features noted to this site are over 2km to the east.

There is an unconformity approx. 200m to the west with metamorphic rock to the west of it.

1.5 Report

This report includes the results and interpretation of the geophysical survey. Maps, figures and tables are included to illustrate the results of the survey. More detailed descriptions of geophysical methods and measurements can be found in GSEG (2002), Milsom (1989) and Reynolds (1997).

The description of soil, rock and the use of geotechnical terms (soft, stiff, dense etc) follows Eurocode (2007) and BSI (2020) standards. The terms are defined in the standards and the physical parameters are related from experience. This geophysical survey has been acquired, processed, interpreted and reported in accordance with these guidelines.

The client provided maps of the site and the digital version was used as the background map in this report. Elevations were surveyed on site and are used in the vertical sections.

The interpretative nature and the non-invasive survey methods must be taken into account when considering the results of this survey and Minerex Geophysics Limited, while using appropriate practice to execute, interpret and present the data, give no guarantees in relation to the existing subsurface.

2. GEOPHYSICAL SURVEY

2.1 Methodology

The methodology consisted of using 2D-Resistivity (ERT), Seismic Refraction and MASW (Multi-Channel Analysis of Surface Waves) surveying.

The 2D-Resistivity survey was carried out first, the data was analysed on site and additional surveying was carried out targeting certain features and ensuring a good spread of lines across the site.

The survey locations are indicated on Map 1. The lines and parameters are tabulated in Table 1.

2.2 2D-Resistivity (ERT)

2D-Resistivity lines were surveyed with electrode spacing of 3m, up to 64 electrodes per set-up and a maximum length of 189m per set-up. The readings were taken with a Tigre Resistivity Meter, Imager Cables, stainless steel electrodes and a laptop with ImagerPro acquisition software.

Lines R2 and R3 were surveyed using the roll-along mode to lengths longer than 189m to achieve continuous coverage to a depth of 15m along the ends of the lines.

The electrodes were placed inside small drilled holes (12mm), and saline water was added to make a good electrical connection. The holes were subsequently re-sealed upon completion of the lines.

During 2D-Resistivity surveying, data is acquired in the form of linear arrays using a suite of metal electrodes. A current is induced into the ground via a pair of electrodes whilst a potential difference is measured across a second pair of electrodes. This allows for the recording of the apparent resistivity in a two-dimensional arrangement below the line. The data is inverted after the survey to obtain a model of subsurface resistivities. The generated model resistivity values and their spatial distribution can then be related to typical values for different geological materials.

The penetration depth of a resistivity set-up increases towards the centre where it reaches an approx. depth of 15m below ground level (bgl).

The presence of metal underground services like water pipes may influence the results of the survey. There are large diameter metal water mains along the dyke road and the resistivity lines were kept away from these in as much as possible.

2D-Resistivity has previously proven zones of anomalous or karstified rock with lateral extents of 10m and more.

2.3 Seismic Refraction

Seismic refraction lines were surveyed with geophone spacing of 3m and 24 geophones per set-up resulting in a 69m length per set-up. The recording equipment consisted of a 24 Channel GEOMETRICS ES-3000 engineering seismograph with 4.5Hz vertical geophones. The seismic energy source consisted of a hammer and plate. A zero-delay trigger was used to start the recording. Normally 7 shot points per p-wave set-up were used.

The seismic refraction survey method focuses on propagating p-waves travelling through the subsurface, which are generated by a seismic source. As the wave propagates through the subsurface, its velocity varies as it travels through overburden, rock with different elastic properties, and along geological boundaries. Velocity data is recorded via the surveying equipment, which is then processed, allowing geological layer thicknesses and boundaries to be established.

Seismic Refraction generally determines the depth to horizontal or near horizontal layers where the compaction or strength or rock quality changes with an accuracy of around 20% of the depth to that layer. Where the layers are shallower than the geophone spacing depth deviations of +- 1m to top of layers can occur. Where low velocity layers or shadow zones are present (e.g., below solid ground surface) or where layers dip with more than 20 degree angle the accuracy becomes much less. This lower accuracy is the case here on this project.

A low velocity layer exists for the seismic waves below the solid surface layer. This makes it less certain or impossible to pick first breaks from geophones near the source and therefore no velocity determination for the shallow subsurface is possible. This results in larger deviations in the modelling and borehole results are required for a final calibration of the results.

The seismic refraction set-ups with 69m individual length have a reasonable penetration depth of around 15m. An internationally accepted maximum depth estimate for a seismic refraction set-up is 1/6 of the set-up length including offshots. The depth penetration varies according to the velocity structure of the subsurface. In this report we used a depth of 15m bgl. where the seismic modelling was ended as deeper modelling becomes less meaningful.

2.4 MASW (Multichannel Analysis of Surface Waves)

The seismic shear wave velocity was determined by active MASW surveying. MASW (Multi-Channel Analysis of Surface Waves) determines the bulk seismic shear wave velocity versus depth. The velocities are used to determine the small strain shear modulus.

The MASW method was acquired along with the seismic refraction survey though the shots were done individually with a larger time window. The recording equipment consisted of a 24 Channel GEOMETRICS ES-3000 engineering seismograph with 4.5Hz vertical geophones. The seismic energy source consisted of a

hammer and plate. A zero-delay trigger was used to start the recording. The shot points were located at the ends of the set-ups.

Many constraints exist for the MASW method and the main factors on this site that affect the methods are strong vertical velocity gradients, particularly between the overburden and rock, and changing velocity structure and layer thicknesses along the lines.

2.5 Site Work

The data acquisition was carried out on the nights of the 28th and 29th of March 2024. The weather conditions were fair throughout the acquisition period. Health and safety standards were adhered to at all times. A traffic management system was in place, clearly highlighted by the use of warning signs and cones.

The locations and elevations were surveyed with a Carlson NR3 RTK-GPS to accuracy < 0.05m.

3. RESULTS AND INTERPRETATION

The interpretation of geophysical data was executed utilizing the known response of geophysical measurements, typical physical parameters for subsurface features that may underlay the site, and the experience of the authors.

Ground investigation results were available after the survey and the abbreviated borehole logs are indicated on the sections. The overburden was abbreviated as clay, silt, sand and gravel. The rock was generally divided into weathered limestone (based on RQD value < 50%) and limestone or metagabbro rock (> 50%). This can be done only to a certain extent as the rock can be very variable. RQD values, fracture indices and non-intact zones often change rapidly with depth. The small size of a borehole only represents a very small volume of ground while the geophysical survey on the other end of the scale averages over a large volume of ground.

3.1 2D-Resistivity (ERT)

The 2D-Resistivity data was positioned and inverted with the RES2DINV inversion package. Lines using the roll-along method were concatenated for a joint inversion. The programme uses a smoothness constrained least-squares inversion method to produce a 2D model of the subsurface resistivities from the recorded apparent resistivity values. Three variations of the least squares method are available and for this project the Jacobian Matrix was recalculated for the first three iterations, then a Quasi-Newton approximation was used for subsequent iterations. Each dataset was inverted using seven iterations resulting in a typical RMS error of <3.0%. The resulting models were colour contoured with the same resistivity scale for all lines and they are displayed as cross sections (Figures 1a – 1b).

The data shows interference from urban ground and conditions. High resistivities near the surface are likely due to road construction material while rapid changes along some lines may be due to interference from surrounding metal.

Resistivities are characteristic for certain overburden and rock types. If there is a high content of clay minerals (which are electrically conductive) then the overburden resistivity will be lower than as if there is a high content of clastic grains like sand or gravel. The purer the clay and the lower the sand and gravel content, the lower the resistivity. Water content in overburden layers can influence the resistivities, but generally clay content has a more dominating effect.

Karstified rock is defined in this report as a formerly intact clean limestone rock, liable to karstification, that has been partially dissolved by water over long geological time scales and where the cavities and voids have either remained empty (filled by air) or became filled by overburden sediment (clay, silt, sand), weathering product of the broken rock itself or water. This process would lead to a reduction of the resistivity of the overall rock and therefore karstified rock has a lower resistivity than intact clean limestone rock. This is generally indicated by lower resistivities embedded within high resistivity at depth.

High resistivities near the surface are likely an effect of the material used for the construction of the car parks. The low resistivities underlying this are interpreted as peat or clay and silts. High resistivities at depth are interpreted as clean limestone while lower resistivities within the rock layer are described as possible karst features. Due to the built-up nature of the site, the data may be disturbed by non-geological features and any possible karst features would require direct ground investigations in order to determine the rock quality in these areas. Between 130 – 160m along line R4, the high resistivities near the surface are likely caused by some disturbance.

Some features within the 2D-Resistivity models may indicate possible karst zones, though disturbance from metal can never be ruled out. Three deeper areas with different resistivities are indicated Map 2. A linear feature within relatively shallow rock in the SE of the site stretching SW to NE is indicated on all four resistivity lines and in comparison with the rotary core logs was interpreted as Metagabbro or anomalous rock. In the NW of the site at the black box theatre the results indicate deeper rock and the modelling was only working to a depth of around 10m bgl. A third area in the central site area relates to a low resistivity feature within the rock layer along line R2 which was found in the drilling to be very stiff to hard Clay.

3.2 Seismic Refraction

The seismic refraction data was positioned and processed with the SEISIMAGER software package to give a layered model of the subsurface. The number of layers has been determined by analysing the seismic traces and a total of 4 layers were used in the models. All seismic lines were subject to a standardised processing sequence which consisted of a topographic correction which was based on integrated elevation data, first break picking, tomographic inversion, travel-time computation via ray-tracing and velocity modelling. Residual deviations of typically 0.4 to 1.8 msec RMS have been obtained for each line. Following each processing stage QC procedures were adhered to. The resulting layer boundaries are shown as thick lines overlaid on the 2D-Resistivity cross sections (Figure 1a – 1b). The average seismic velocities obtained within the layers are annotated on the sections as bold black numbers.

The p-wave seismic velocity is closely linked to the density of subsurface materials and to parameters like compaction, stiffness, strength and rock quality. The higher the density of the subsurface materials the higher the seismic velocity. More compacted, stiffer, denser and stronger material will have a higher seismic velocity. For rock, the seismic velocity is higher when the rock is stronger, less weathered and has a higher quality. If the rock is more weathered, broken, fractured, fissured or karstified then the seismic velocity will be reduced compared to that of intact fresh rock.

Because of the above relationship, the seismic refraction method and seismic velocities are suitable to investigate ground where the layers get denser, more compacted and stronger with depth. A disadvantage is that some materials may have the same seismic velocity: Stiff to very stiff highly consolidated overburden and a weathered rock can have the same seismic velocity range (as could be the case in the layer 3 below).

The modelled seismic data has created the following layered ground model:

Layer 1 has seismic velocities of 500m/s. The velocity is a mix of the tarmac and underlying material that represents a 'shadow zone' as described in Ch2.3.

Layer 2 was modelled with a velocity of 1200m/s and is interpreted as overburden material with soft to firm strength or compaction. This depth of the layer extends to elevation 0mOD across much of the site but is deeper to the NW where the deep rock is interpreted.

Layer 3 velocities of 2000m/s indicate a very stiff or very dense overburden. This layer is only present in the NW of the site. This layer may also contain some very weathered limestone.

Good Rock (Layer 4) is indicated by seismic velocities of 3500 - 4000m/s. The depth to the top of the good rock ranges from 4 – 9m below ground level (bgl) across much of the site but falls to 11 to 12m bgl in the NW in RC BRC01 and BH01. There is a pocket of very stiff to hard clay included in this layer, as drilled in BRC03.

3.3 Interpretation of Resistivity and Seismic Refraction

Table 2 summarises the interpretation. The stiffness or compaction of overburden and the rock strength or quality have been estimated from the seismic velocity. The estimation of the excavatability for the bedrock has been made according to the caterpillar chart published in Reynolds (1997). The geotechnical assessment for rippability will have to take factors like rock type and jointing into account and the estimation in this report is solely based on the seismic velocities.

Interpreted cross sections are shown in Figure 2a – 2b as well as areas highlighted as containing possible karstified limestone or deep rock. Possible Karst zones or deep rock are shown on Map 2. The interpretation has been made from all available information. For overburden layers and the top of the rock the seismic refraction data has been used as seismic refraction is the best method to delineate layer boundaries. The resistivity model values have been used in a general sense to determine overburden materials and rock type as well as identifying possible karst zones within the rock.

Resistivity data is better suited to show overburden material, rock types and features within the rock while seismic refraction velocities are indicating the change of compaction, stiffness or rock quality with depth. Along short sections where only one data type is available an interpolation for the interpreted layers was made.

Table 2: Summary of Interpretation

| Layer | General Seismic Velocity Range (m/sec) | General Resistivity Range (Ohmm) | Interpretation | Estimated Excavation Method |
|-------|--|----------------------------------|--|-----------------------------|
| 1 | 500 | Any | Road Construction Material over urban Made Ground or Peat | Diggable |

| | | | | |
|----|-------------|-------|---|---------------------|
| 2 | 1200 | <125 | Soft to firm Clay and Silt or urban Made Ground or Peat | Diggable |
| 3 | 2000 | <250 | Very stiff or very dense Overburden | Diggable |
| 4a | N/A | < 125 | Very stiff to hard Clay | Digging and ripping |
| 4b | 3500 - 4000 | Any | Limestone | Breaking & Blasting |
| 4c | 3500 - 4000 | Any | Metagabbro/anomalous Rock | Breaking & Blasting |

3.4 MASW (Multichannel Analysis of Surface Waves)

The MASW lines were positioned, processed, analysed and modelled with the SEISIMAGER/SW and the SURFSEIS6 software packages. The objective is to obtain a model of shear wave velocity versus depth.

All shot points were analysed in order to extract the best possible dispersion curve for the modelling stage.

Following processing steps are done to achieve this:

1. Edit the shot point geometry and display the shot points for each array
2. Edit traces and/or apply filters to improve the shot record for the next step
3. A dispersion curve (phase velocity versus frequency plot or dispersion image) is computed
4. For each shot the maximum amplitude at each frequency of the dispersion image is selected and the picks for the dispersion curve are truncated (frequency gate), smoothed and brought forward into the modelling process
5. An initial model of shear-wave velocity, V_s , versus depth is computed
6. An inversion is carried out to create the final V_s curve (Shear wave versus depth). The valid useful depth range is noted and the data saved in a file
7. For stable repeatable results the shear wave velocity versus depth is extracted and the depth range covered by the real survey data is then listed in Table 3
8. The results for the two opposed shot points at the end of each array are compared and an average shear wave velocity is computed
9. The small strain shear modulus (also named G_{max}) for each shot point and depth is computed by using a density of 1800 kg/m³ typical for consolidated overburden (Eq. 1)

$$(Eq. 1) \quad G = V_s^2 * \rho * 10^{-6}$$

where G = Shear Modulus (MPa)
 V_s = Seismic Shear Wave Velocity (m/s)
 ρ = Density (kg/m³)

The results are displayed in tabular format in Table 3. The results show the seismic shear wave velocity and the small strain shear modulus vs. depth. S-Wave Velocities across all the lines range from 74 – 292m/s and the small strain shear moduli from 10 – 153MPa.

The relationship between shear wave velocities and material stiffness is summarised below:

Table 4: Shear Wave Velocity to Stiffness Relationship

| Shear Wave Velocity Vs Range in m/s | Material Stiffness |
|--|---------------------------|
| < 150 | Soft |
| 150 to 300 | Firm |
| 300 to 500 | Stiff |
| > 500 m/s | Very Stiff |

Intensive efforts have been made to extract the best dispersion curves by time gating, selecting and test processing various source versus receiver trace distances and trace ranges and by directional selection of traces.

The depth of surveying was generally restricted by the strong vertical velocity gradient in particular between the overburden and rock layers. The MASW data can survey the softer layers below the construction material which is not possible by the seismic refraction method. The data shows soft to firm material across the site. The depth of the survey ranges from 3.6m to 4.5m across much of the site but increases to 7.2 – 9m in the NW which also indicating a change in the depth to rock.

4. CONCLUSIONS

The following conclusions are made:

- The geophysical surveys carried out at the LDA Dyke Road site consisted of 2D-Resistivity (ERT), seismic refraction and MASW surveying.
- The purpose of this survey was to provide some guidance for borehole locations by indicating general geological changes across the site and highlighting possible areas of karstified limestone.
- The recommended locations were drilled and the results are incorporated into this interpretation.
- The drilling found a zone of Metagabbro (BRC04) within the limestone and a deeper pocket of very stiff to hard clay within the good limestone (BRC03).
- The urban environment has interfered with the data, namely the tarmac and low velocity layer for the seismic refraction and underground metal for the resistivity.
- At all locations there was a correlation between all three geophysical survey methods, high resistivities generally match with increases in seismic refraction velocities while the depth of the MASW survey showing soft to firm materials ties in with layers 1 and 2 from the seismic refraction survey and extends to greater depths in the NW where these layers are thicker.
- Some high resistivities at depth indicate that there is clean limestone present that is liable to karstification, but it does not have to be karstified.
- The seismic refraction survey was modelled with a total of four layers.
- Layer 1 is mainly affected by the road construction. High resistivities near the surface indicate road construction material such as gravel and tarmac. This layer would also contain urban made ground and peat.
- Layer 2 is interpreted as soft to firm clay and silt or urban made ground or peat. This layer extends down to an elevation of approx. 0mOD across much of the site but extends deeper in the NW.
- Layer 3 is described as very stiff or very dense overburden. This layer is only present in the NW of the site. It may contain some very weathered rock.
- Layer 4 is interpreted as rock. The depth to the top of this layer is between 4 – 9m bgl across most of the site but 11 to 12m bgl in the NW in RC BRC01 and BH01. Due to the interference the seismic modelling depth was limited here to around 10m.
- Resistivities and rotary coring lead to a subdivision of Layer 4 into 3 zones with 4a as very stiff to hard clay, 4b limestone and 4c metagabbro/anomalous rock. These are indicated in Figures 2a and 2b.

- The resistivities give general indications of overburden types and possible features within the rock but the built-up nature of the site can have a non-geological effect on the data as well and care must be taken when using the interpretation.
- There are generally high resistivities near the surface. This would be typical for road construction materials such as tarmac and gravel fill.
- Peat present in layer 1 or 2 would be compressed because of the urban layers.
- Map 2 indicates three zones reviewed as 'Deep Rock' in the NW, very stiff to hard Clay and Metagabbro/anomalous Rock. These were interpreted after targeted drilling.
- Table 3 gives the results from the MASW survey and shows shear wave velocities of 74 – 292m/s across the site which indicates soft to firm material. The small strain shear moduli range from 10 to 153MPa. The depth of these layers is shown to increase towards the NW.
- The interpretation presented here was reviewed after the geotechnical data became available.

5. REFERENCES

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2. **Eurocode, 2007.** EN 1997-2:2007. Eurocode 7. Part 2 Ground Investigation and Testing 2007
3. **GSEG, 2002.** Geophysics in Engineering Investigations. Geological Society Engineering Geology Special Publication 19, London, 2002
4. **GSI, 2024.** Online Geological Map of Ireland. Geological Survey of Ireland 2024
5. **Milsom, 1989.** Field Geophysics. John Wiley and Sons, 1989
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7. **Weaver, 1975.** Geological Factors significant in the Assessment of Rippability, 1975

Table 1: Geophysical Survey Locations and Acquisition Parameters

| 2D-Resistivity (ERT) Survey | | | | | |
|------------------------------------|-------------|------------------------------|-------------------|--------------------------|-------------------------|
| Site | Line | Electrode Spacing (m) | Length (m) | Display direction | Survey Type/Mode |
| Car Park | R1 | 3 | 189 | NW-SE | Traverse |
| Car Park | R2 | 3 | 333 | N-S | Roll Along |
| Car Park | R3 | 3 | 237 | NW-SE | Roll Along |
| Car Park | R4 | 3 | 189 | NW-SE | Traverse |
| | | SUM | 948 | | |
| Seismic Refraction Survey | | | | | |
| Site | Line | Geophone Spacing (m) | Length (m) | Display direction | Survey Type/Mode |
| Car Park | S1 | 3 | 69 | NW-SE | Single Setup |
| Car Park | S2 | 3 | 69 | N-S | Single Setup |
| Car Park | S3 | 3 | 69 | N-S | Single Setup |
| Car Park | S4 | 3 | 69 | N-S | Single Setup |
| Car Park | S5 | 3 | 69 | N-S | Single Setup |
| Car Park | S6 | 3 | 69 | N-S | Single Setup |
| Car Park | S7 | 3 | 69 | NW-SE | Single Setup |
| Car Park | S8 | 3 | 69 | NW-SE | Single Setup |
| Car Park | S9 | 3 | 69 | NW-SE | Single Setup |
| Car Park | S10 | 3 | 69 | NW-SE | Single Setup |
| | | SUM | 690 | | |
| MASW Survey | | | | | |
| Site | Line | Geophone Spacing (m) | Length (m) | Display direction | Survey Type/Mode |
| Car Park | M1 | 3 | 69 | NW-SE | 1D-MASW |
| Car Park | M2 | 3 | 69 | N-S | 1D-MASW |
| Car Park | M3 | 3 | 69 | N-S | 1D-MASW |
| Car Park | M4 | 3 | 69 | N-S | 1D-MASW |
| Car Park | M5 | 3 | 69 | N-S | 1D-MASW |
| Car Park | M6 | 3 | 69 | N-S | 1D-MASW |
| Car Park | M7 | 3 | 69 | NW-SE | 1D-MASW |
| Car Park | M8 | 3 | 69 | NW-SE | 1D-MASW |
| Car Park | M9 | 3 | 69 | NW-SE | 1D-MASW |
| Car Park | M10 | 3 | 69 | NW-SE | 1D-MASW |
| | | SUM | 690 | | |

Table 3: MASW S-Wave Velocities and Gmax

| Line | Depth (m) | Left S-Wave Velocity (m/s) | Right S-Wave Velocity (m/s) | Average S-Wave Velocity (m/s) | Average Gmax - Shear Modulus (Mpa) |
|-------------|------------------|-----------------------------------|------------------------------------|--------------------------------------|---|
| S1 | | | | | |
| | 0.8 | 160 | 147 | 153 | 42 |
| | 1.6 | 178 | 165 | 171 | 53 |
| | 2.4 | 122 | 117 | 119 | 26 |
| | 3.2 | 107 | 53 | 80 | 11 |
| | 4.0 | 151 | 129 | 140 | 35 |
| | 4.8 | 143 | 192 | 168 | 51 |
| | 5.6 | 146 | 96 | 121 | 26 |
| | 6.4 | 141 | 92 | 117 | 25 |
| | 7.2 | 181 | 92 | 136 | 33 |
| S2 | | | | | |
| | 0.4 | 66 | 82 | 74 | 10 |
| | 0.8 | 131 | 111 | 121 | 27 |
| | 1.2 | 168 | 138 | 153 | 42 |
| | 1.6 | 147 | 114 | 130 | 31 |
| | 2.0 | 103 | 114 | 109 | 21 |
| | 2.4 | 66 | 113 | 90 | 14 |
| | 2.8 | 80 | 116 | 98 | 17 |
| | 3.2 | 120 | 106 | 113 | 23 |
| | 3.6 | 112 | 105 | 108 | 21 |
| S3 | | | | | |
| | 1.0 | 247 | 217 | 232 | 97 |
| | 2.0 | 234 | 278 | 256 | 118 |
| | 3.0 | 269 | 232 | 250 | 113 |
| | 4.0 | 108 | 147 | 128 | 29 |

Table 3: MASW S-Wave Velocities and Gmax

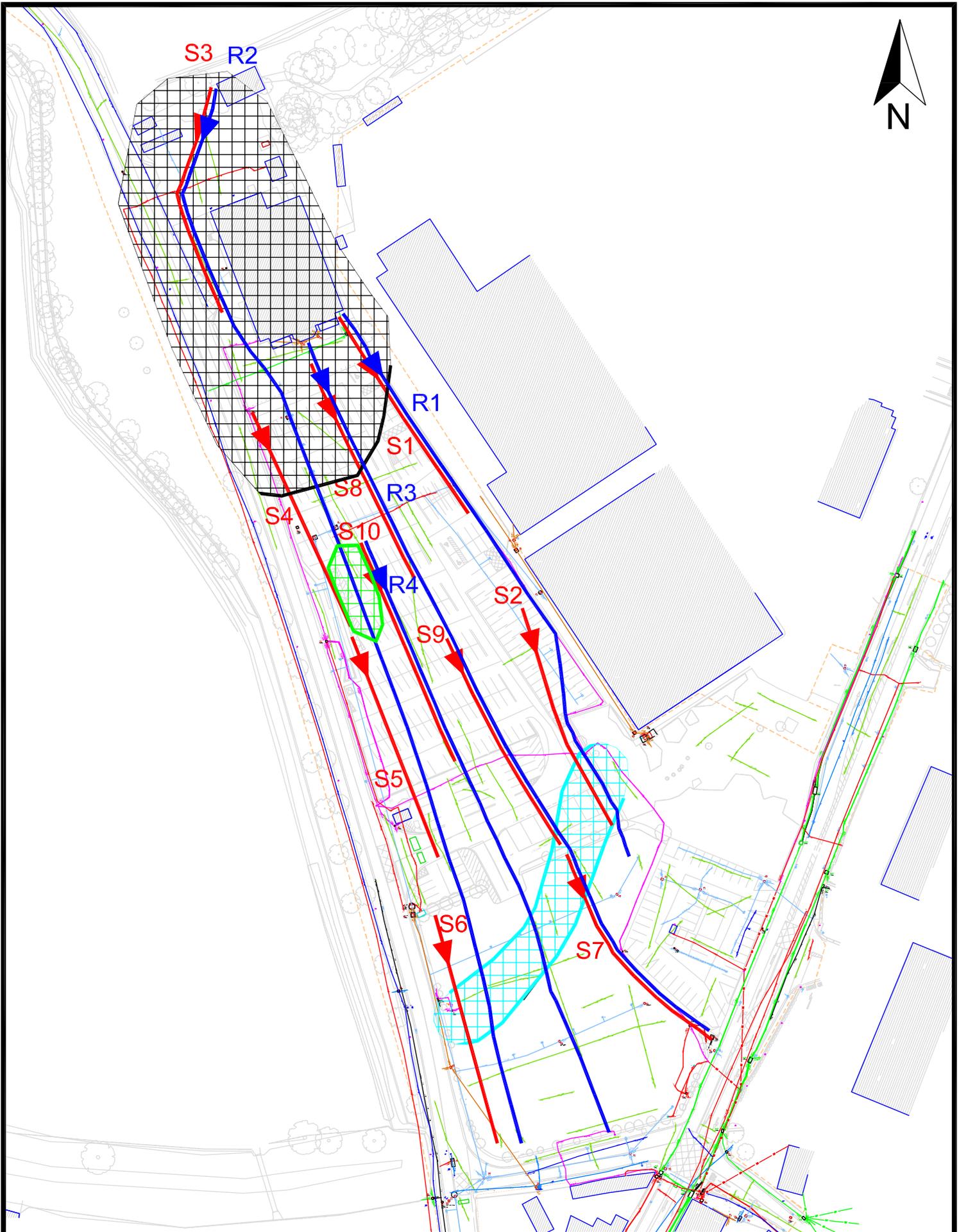
| Line | Depth (m) | Left S-Wave Velocity (m/s) | Right S-Wave Velocity (m/s) | Average S-Wave Velocity (m/s) | Average Gmax - Shear Modulus (Mpa) |
|-------------|------------------|-----------------------------------|------------------------------------|--------------------------------------|---|
| | 5.0 | 79 | 110 | 95 | 16 |
| | 6.0 | 145 | 173 | 159 | 46 |
| | 7.0 | 279 | 247 | 263 | 124 |
| | 8.0 | 293 | 259 | 276 | 137 |
| | 9.0 | 135 | 189 | 162 | 47 |
| S4 | | | | | |
| | 1.0 | 213 | 240 | 226 | 92 |
| | 2.0 | 286 | 248 | 267 | 129 |
| | 3.0 | 263 | 133 | 198 | 71 |
| | 4.0 | 173 | 98 | 135 | 33 |
| | 5.0 | 124 | 170 | 147 | 39 |
| | 6.0 | 180 | 219 | 200 | 72 |
| | 7.0 | 240 | 228 | 234 | 99 |
| | 8.0 | 304 | 208 | 256 | 118 |
| | 9.0 | 300 | 186 | 243 | 106 |
| S5 | | | | | |
| | 0.4 | 101 | 152 | 126 | 29 |
| | 0.8 | 81 | 161 | 121 | 26 |
| | 1.2 | 111 | 166 | 139 | 35 |
| | 1.6 | 157 | 159 | 158 | 45 |
| | 2.0 | 175 | 144 | 159 | 46 |
| | 2.4 | 162 | 128 | 145 | 38 |
| | 2.8 | 138 | 116 | 127 | 29 |
| | 3.2 | 115 | 110 | 113 | 23 |
| | 3.6 | 104 | 107 | 105 | 20 |

Table 3: MASW S-Wave Velocities and Gmax

| Line | Depth (m) | Left S-Wave Velocity (m/s) | Right S-Wave Velocity (m/s) | Average S-Wave Velocity (m/s) | Average Gmax - Shear Modulus (Mpa) |
|-------------|------------------|-----------------------------------|------------------------------------|--------------------------------------|---|
| S6 | | | | | |
| | 0.4 | 210 | 227 | 219 | 86 |
| | 0.8 | 234 | 253 | 244 | 107 |
| | 1.2 | 248 | 288 | 268 | 129 |
| | 1.6 | 231 | 311 | 271 | 132 |
| | 2.0 | 190 | 316 | 253 | 115 |
| | 2.4 | 135 | 300 | 217 | 85 |
| | 2.8 | 108 | 257 | 183 | 60 |
| | 3.2 | 86 | 216 | 151 | 41 |
| | 3.6 | 77 | 120 | 99 | 18 |
| S7 | | | | | |
| | 0.5 | 214 | 242 | 228 | 94 |
| | 1.0 | 281 | 250 | 266 | 127 |
| | 1.5 | 319 | 265 | 292 | 153 |
| | 2.0 | 289 | 256 | 273 | 134 |
| | 2.5 | 215 | 242 | 229 | 94 |
| | 3.0 | 139 | 217 | 178 | 57 |
| | 3.5 | 117 | 204 | 161 | 46 |
| | 4.0 | 142 | 198 | 170 | 52 |
| | 4.5 | 164 | 193 | 178 | 57 |
| S8 | | | | | |
| | 1.0 | 118 | 136 | 127 | 29 |
| | 2.0 | 192 | 171 | 182 | 59 |
| | 3.0 | 157 | 125 | 141 | 36 |
| | 4.0 | 86 | 94 | 90 | 15 |

Table 3: MASW S-Wave Velocities and Gmax

| Line | Depth (m) | Left S-Wave Velocity (m/s) | Right S-Wave Velocity (m/s) | Average S-Wave Velocity (m/s) | Average Gmax - Shear Modulus (Mpa) |
|-------------|------------------|-----------------------------------|------------------------------------|--------------------------------------|---|
| | 5.0 | 61 | 153 | 107 | 21 |
| | 6.0 | 80 | 126 | 103 | 19 |
| | 7.0 | 122 | 124 | 123 | 27 |
| | 8.0 | 149 | 179 | 164 | 49 |
| | 9.0 | 136 | 156 | 146 | 38 |
| S9 | | | | | |
| | 0.4 | 110 | 126 | 118 | 25 |
| | 0.8 | 154 | 187 | 171 | 52 |
| | 1.2 | 192 | 233 | 212 | 81 |
| | 1.6 | 215 | 232 | 223 | 90 |
| | 2.0 | 221 | 184 | 203 | 74 |
| | 2.4 | 220 | 117 | 168 | 51 |
| | 2.8 | 197 | 80 | 138 | 34 |
| | 3.2 | 140 | 65 | 102 | 19 |
| | 3.6 | 92 | 59 | 75 | 10 |
| S10 | | | | | |
| | 0.5 | 80 | 88 | 84 | 13 |
| | 1.0 | 180 | 173 | 176 | 56 |
| | 1.5 | 266 | 235 | 250 | 113 |
| | 2.0 | 235 | 215 | 225 | 91 |
| | 2.5 | 158 | 165 | 161 | 47 |
| | 3.0 | 199 | 154 | 177 | 56 |
| | 3.5 | 245 | 166 | 206 | 76 |
| | 4.0 | 190 | 182 | 186 | 62 |
| | 4.5 | 169 | 169 | 169 | 51 |



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CLIENT Land Development Agency
Accom

PROJECT Dyke Road, Galway City
Geophysical Survey

TITLE Map 2: Geophysical Survey
Interpretation Map

SCALE: 1:1000 @ A3

PROJECT: 6756

DRAWN: JS

DATE: 01/07/2024

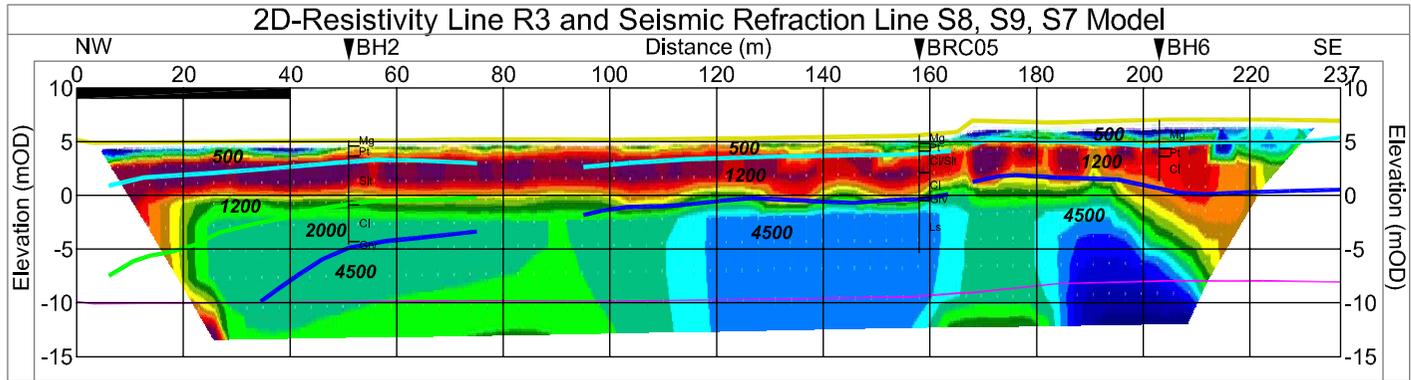
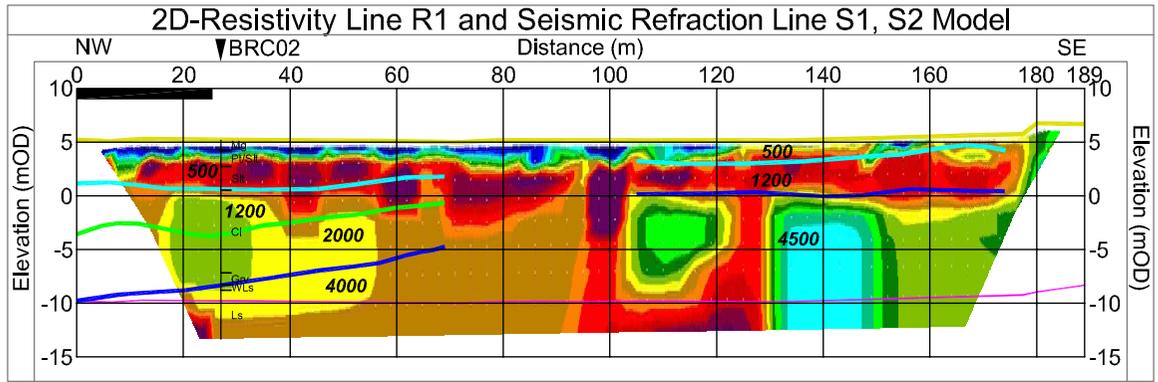
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STATUS: Final

Geophysical Survey Locations:

- R2 2D-Resistivity Line
- S1 Seismic Refraction and MASW Line
-  Deep Rock
-  Very stiff to hard Clay
-  Metagabbro/anomalous Rock

Locations are in Irish Transverse Mercator (ITM). Elevations are in mOD (Malin Head)



Abbreviated GI Logs:

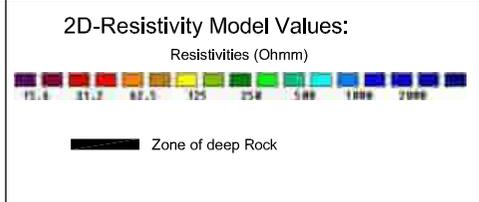
| ▼ BRC06 | | Borehole Name and Location | |
|---------|--------|----------------------------|---------------------|
| Pt | Peat | Mg | Made Ground |
| Cl | Clay | Slt | Silt |
| Grv | Gravel | WLS | Weathered Limestone |
| Sd | Sand | Ls | Limestone |

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| | |
|---------|---|
| CLIENT | Land Development Agency AECOM |
| PROJECT | LDA Dyke Road, Galway Geophysical Survey |
| TITLE | Figure 1a: Models of Geophysical Survey |

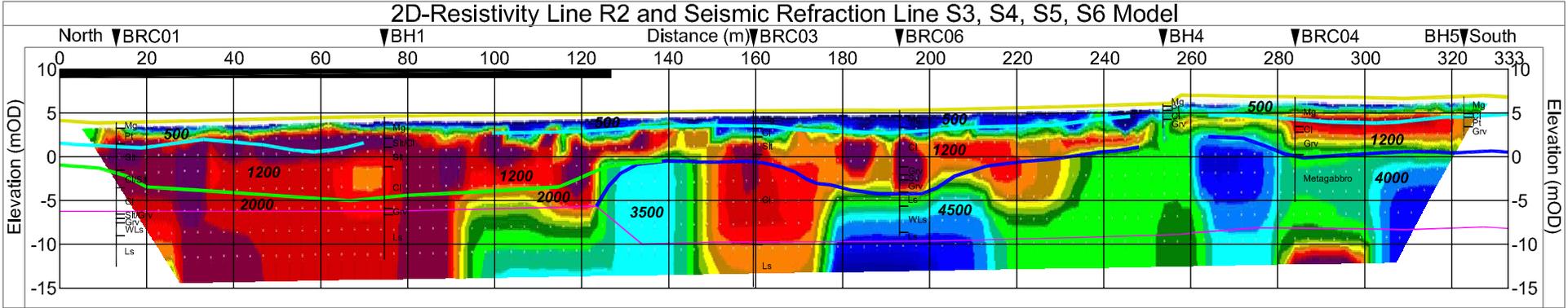
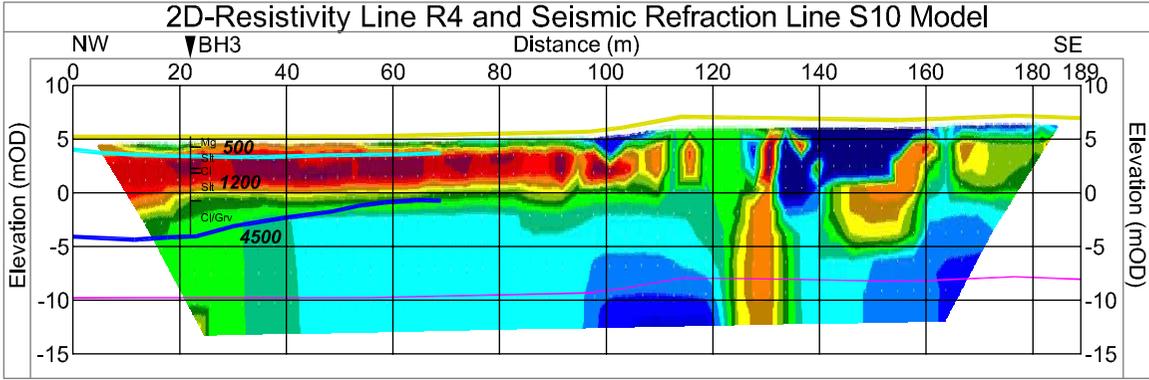
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| PROJECT: | 6756 |
| DRAWN: | JS |
| DATE: | 01/07/2024 |
| MGX FILE: | 6756f_Drawings.dwg |
| STATUS: | Final |



Layers from Seismic Refraction Model:

- Ground Surface/Top of Layer 1 (500 m/s)
- Top of Layer 2 (1200 m/s)
- Top of Layer 3 (2000 m/s)
- Top of Layer 4 (3500 - 4500 m/s)
- Seismic Modelling Depth

1800
Seismic Velocity in m/s



Abbreviated GI Logs:

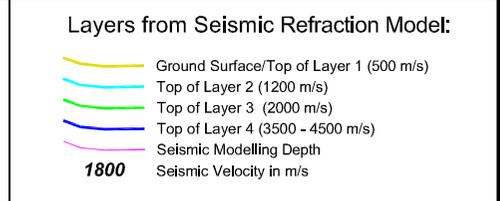
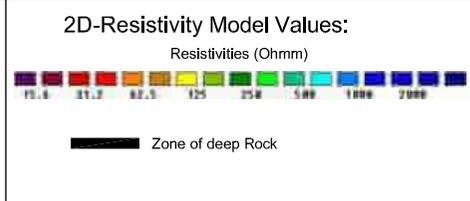
| ▼ BRC06 Borehole Name and Location | |
|------------------------------------|--------------------------------|
| Pt | Peat Mg Made Ground |
| Cl | Clay Silt Silt |
| Grv | Gravel WLS Weathered Limestone |
| Sd | Sand Ls Limestone |

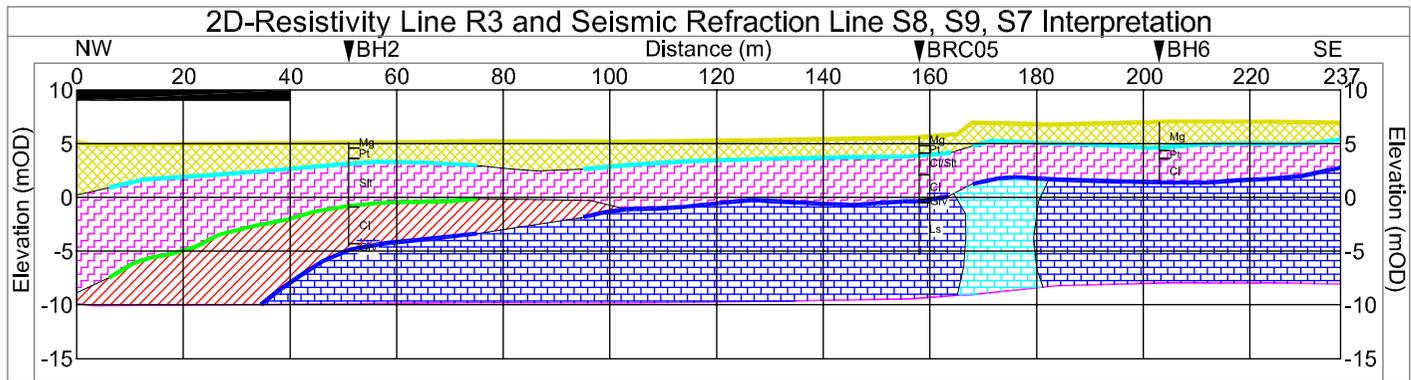
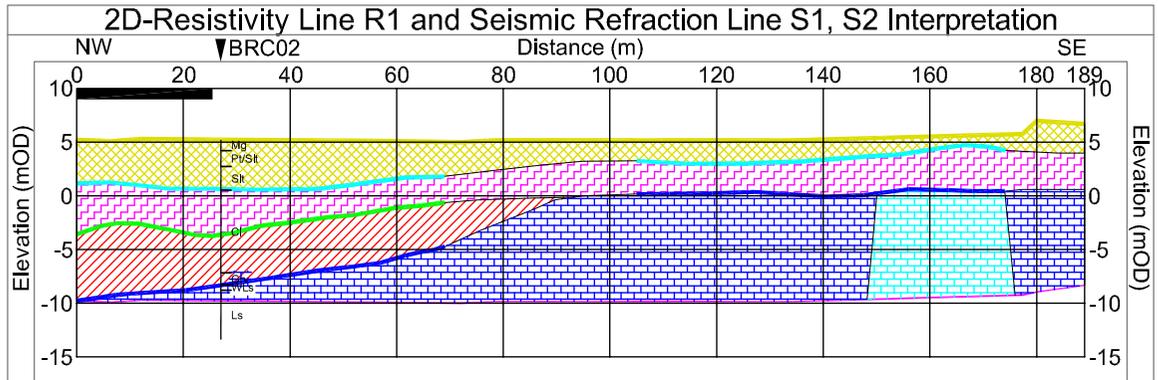
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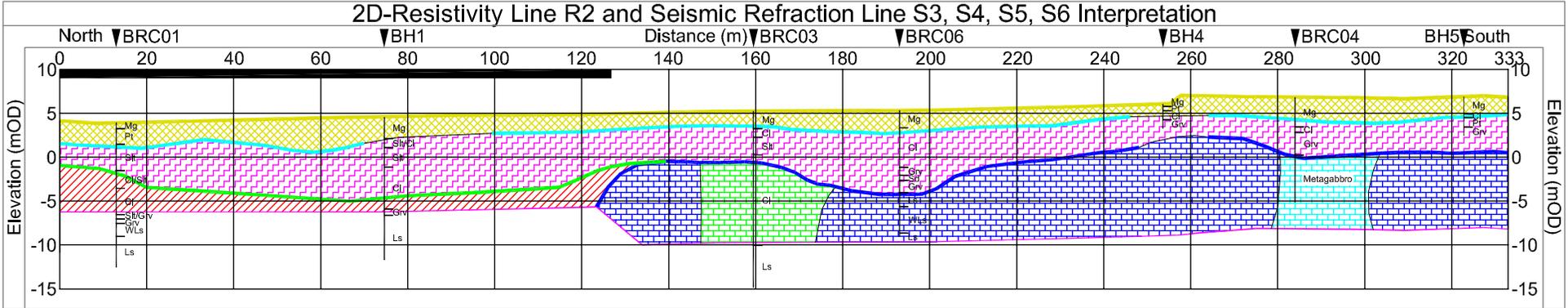
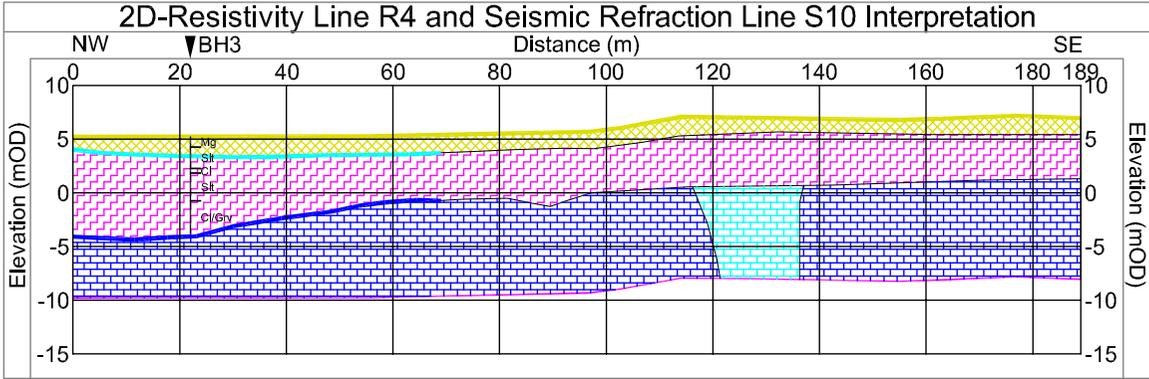
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| | |
|---------|---|
| CLIENT | Land Development Agency AECOM |
| PROJECT | LDA Dyke Road, Galway Geophysical Survey |
| TITLE | Figure 1b: Models of Geophysical Survey |

| | |
|-----------|---------------------|
| SCALE: | 1:1000 @ A3, VE x 2 |
| PROJECT: | 6756 |
| DRAWN: | JS |
| DATE: | 01/07/2024 |
| MGX FILE: | 6756f_Drawings.dwg |
| STATUS: | Final |







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| CLIENT | Land Development Agency AECOM |
| PROJECT | LDA Dyke Road, Galway Geophysical Survey |
| TITLE | Figure 2b: Interpretation of Geophysical Survey |

| | |
|-----------|---------------------|
| SCALE: | 1:1000 @ A3, VE x 2 |
| PROJECT: | 6756 |
| DRAWN: | JS |
| DATE: | 01/07/2024 |
| MGX FILE: | 6756f_Drawings.dwg |
| STATUS: | Final |

Interpretation:

| | |
|--|---|
| | 1 Road Construction Material over Urban Made Ground or Peat |
| | 2 Soft to firm Clay and Silt or urban Made Ground or Peat |
| | 3 Very stiff or very dense Overburden |
| | 4a Very stiff to hard Clay |
| | 4b Limestone |
| | 4c Metagabbro/anomalous Rock |
| | Zone of deep Rock |

Abbreviated GI Logs:

| | | | |
|--------|----------------------------|------|---------------------|
| ▼BRC06 | Borehole Name and Location | | |
| Pt | Peat | Mg | Made Ground |
| Cl | Clay | Silt | Silt |
| Grv | Gravel | WLS | Weathered Limestone |
| Sd | Sand | Ls | Limestone |

